	<b>BREVIATIONS</b> L ABBREVIATIONS USED)	STRUCTURAL DESIGN C	RITERIA - WASH BAY		
#XX A.F.F.	X NUMBER BUILDING CODE: 2021 ARKANSAS FIRE PREVENTION CODE (BASED ON 2021 IBC)		DE (BASED ON 2021 IBC)	A	. <b>C</b> 1.
ADD'L	FLOOR	RISK CATEGORY (2021 IBC TABLE 1604.5): II			
ADJ	ADJACENT	GRAVITY LOADS (REFERENCE: 2021 IBC & ASCE 7-16):			2.
ARCH. B.F.F.	ARCHITECTURAL BELOW FINISHED	DEAD LOADS:	DOF		
BLDG.	FLOOR BUILDING		PSF PSF (ASSUMED)		3.
BOT BTWN	BOTTOM BETWEEN	COLLATERAL 7 PS		B	. SI
С	STANDARD CHANNEL	FLOOR LIVE LOADS:	205		1.
CFS	COLD-FORMED STEEL		PSF 00 LBS FRONT AXLE LOAD		2.
CJ	CONTROL/ CONSTRUCTION/	400	00 LBS REAR DUAL AXLE LOAD		3.
	CONTRACTION JOINT	ROOF LIVE LOADS:	20 PSF (NON-REDUCIBLE)		4.
CJP	COMPLETE JOINT PENETRATION	RAIN LOADS:			5.
CL CLR.	CENTER LINE CLEAR	15 MINUTE DURATION / 100 YR RETURN PERIOD 60 MINUTE DURATION / 100 YR RETURN PERIOD			6.
CMU	CONCRETE				7.
COL.	MASONRY UNIT COLUMN	SNOW LOADS: GROUND SNOW LOAD	Pg = 10 PSF	С	. S
CONC. CONN.		FLAT ROOF SNOW LOAD SLOPED ROOF SNOW LOAD	Pf = 7.0 PSF Ps = 7.0 PSF		1.
CONT. D.B.	CONTINUOUS DECK BEARING	SNOW LOAD IMPORTANCE FACTOR SNOW EXPOSURE FACTOR	ls = 1.0 Ce = 1.0		2.
DBA	DEFORMED BAR	THERMAL FACTOR	Ct = 1.0		3.
DIA	ANCHOR DIAMETER	LATERAL LOADS (REFERENCE: 2021 IBC & ASCE 7-16):			4.
DTL E.F.	DETAIL EACH FACE	WIND:			5.
EA.	EACH	ULTIMATE WIND SPEED	Vult = 105 MPH	D	. G
ELEV. EW	ELEVATION EACH WAY	NOMINAL WIND SPEED TERRAIN EXPOSURE	Vasd = 85 MPH C		1.
	EXISTING EXPANSION	INTERNAL PRESSURE COEFFICIENTS	+/- 0.55		2. 3.
F	FINISHED FLOOR				4.
FLR FS	FLOOR FAR SIDE	SEISMIC IMPORTANCE FACTOR MAPPED SPECTRAL RESPONSE ACCELERATION	le = 1.00 IS Ss = 0.446		5. 6.
TG G.C.	FOOTING GENERAL	SITE CLASS	S1 = 0.161		7.
	CONTRACTOR	DESIGN SPECTRAL RESPONSE ACCELERATION	S SDS = 0.429		
GA. GALV.	GAUGE GALVANIZED	SEISMIC DESIGN CATEGORY	SD1 = 0.245 D		8.
I.S.	HIGH STRENGTH HORIZONTAL	SEISMIC FORCE RESISTING SYSTEM DESIGN BASE SHEAR	STEEL ORDINARY MOMENT FRAMES V = 0.123W		
IORIZ. ISS	ROUND, SQUARE, OR	SEISMIC RESPONSE COEFFICIENT	Cs = 0.123	E	. SI
	RECTANGULAR STRUCTURAL	RESPONSE MODIFICATION COEFFICIENT ANALYSIS PROCEDURE	R = 3.5 EQUIVALENT LATERAL FORCE		1.
D	TUBING INSIDE DIAMETER	SYSTEMS AND COMPONENTS REQUIRING SPECIAL INSP	ECTION: SEE SPECIFICATION SECTION 014533		
IT.	JOINT				
Cork CJ	KIP (1,000 LBS) KEYED CONTROL				
(SI	JOINT KIPS PER SQUARE	STRUCTURAL DESIGN AP			2. 3.
	INCH	STRUCTURAL DESIGN AP			0.
- BS	ANGLE POUNDS	THE WASH BAY BUILDING IS A SINGLE STORY METAL BU			
_F _LH	LINEAL FOOT LONG LEG	STANDING SEAM METAL ROOFING SYSTEM SUPPORTED FRAMES. LATERAL STABILITY IS PROVIDED BY THE RIGI			4.
_LV	HORIZONTAL LONG LEG VERTICAL	STRUCTURE IS SUPPORTED BY A SHALLOW FOUNDATIO	N SYSTEM.		5.
_SL	LONG SLOTTED			]_	_
MANUF.	HOLES MANUFACTURER			F	1.
1ATL. 1AX.	MATERIAL MAXIMUM				2.
1777. 1C	MISCELLANEOUS				
/IECH.	CHANNEL MECHANICAL				3.
MIN. MISC	MINIMUM MISCELLANEOUS				4.
I.T.S.	NOT TO SCALE				5.
IS Ø	NEAR SIDE DIAMETER				Э.
).C. )D	ON CENTER OUTSIDE DIAMETER				
PP	OPPOSITE				6.
P.J. PAF	PANEL JOINT POWDER ACTUATED				7.
ր	FASTENER PLATE				8.
PLBG	PLUMBING				9.
PSF	POUNDS PER SQ FOOT				10
PSI Reinf.	POUNDS PER SQ INCH REINFORCEMENT			G	i <b>. C</b> ( 1.
REQ'D.	REQUIRED				2.
SC SECT.	SLIP CRITICAL SECTION				
GHT. GIM.	SHEET SIMILAR				
SJ SPA.	SAWN JOINT SPACE				3.
SQ	SQUARE				4.
SSL	SHORT SLOTTED HOLES				5. 6.
STD. T&B	STANDARD TOP AND BOTTOM				
T.O.F.	TOP OF FOOTING				
T.O.S.	TOP OF STEEL or TOP OF SLAB				
T.O.W. TC	TOP OF WALL TENSION CONTROL				7.
THRU	THROUGH				8.
fyp. J.n.o.	TYPICAL UNLESS NOTED				
/ERT.	OTHERWISE				
DR V /SC	VERTICAL VERTICAL SLIDING				
	CLIP				9. 10
N N.W.R.	WIDE FLANGE WELDED WIRE REINF.				10 11
w/ NP	WITH WORK POINT				12
NP NT	TEE SHAPE MADE				13
x/S-YYY	FROM W SHAPE SECTION/DETAIL "X"				14
XX#	ON SHEET "S-YYY" POUNDS				15 16
ZRC	ZINC BASE PAINT				
					17
					18
					19 20
					21 22
					-
		1		1	

# STRUCTURAL GENERAL NOTES

#### ONTRACTOR DELEGATED DESIGN COMPONENTS:

THE FOLLOWING ITEMS ARE NOTED AS A DELEGATED DESIGN COMPONENT AND SHALL BE DESIGNED BY THE CONTRACTOR. THE CONTRACTOR SHALL EMPLOY A SPECIALTY STRUCTURAL ENGINEER LICENSED IN THE STATE OF ARKANSAS TO DESIGN THE FOLLOWING ITEMS: a. METAL BUILDING SYSTEMS

THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR EACH DELEGATED DESIGN COMPONENT. ALL STRUCTURAL DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY THE SPECIALTY STRUCTURAL ENGINEER. THE DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR AND THE DESIGN ENGINEER PRIOR TO SUBMITTAL. INCOMPLETE SHOP DRAWINGS AND SHOP DRAWINGS THAT HAVE NOT BEEN REVIEWED BY THE CONTRACTOR AND THE SPECIALTY STRUCTURAL ENGINEER WILL BE RETURNED WITHOUT REVIEW BY THE ARCHITECT/ENGINEER. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS OF ALL DELEGATED DESIGN COMPONENTS AND THEIR ACCESSORIES WITH OTHER TRADES TO AVOID CONFLICTS, e.g., JOIST BRIDGING AND FIRE SUPPRESSION SYSTEMS.

PECIAL INSPECTIONS: QUALIFIED INSPECTORS SHALL CONDUCT SPECIAL INSPECTIONS AND TESTS AND FURNISH REPORTS AS SPECIFIED IN SECTION 014533 AND IN ACCORDANCE WITH CHAPTER 17, INTERNATIONAL BUILDING CODE.

THE CONTRACTOR SHALL COORDINATE THE SPECIAL INSPECTIONS AND TESTING SERVICES WITH THE PROGRESS OF THE WORK, PROVIDE THE APPROPRIATE DOCUMENTATION AND PERFORM OTHER TASKS AS SPECIFIED IN SECTION 014533.

CONSTRUCTION THAT REQUIRES CONTINUOUS INSPECTION PER SECTION 014533 CAN NOT PROGRESS WITHOUT INSPECTORS PRESENT.

THE CONTRACTOR IS RESPONSIBLE FOR ALL OTHER INSPECTIONS OR TESTS IN THE SPECIFICATIONS, NOT LISTED IN THE SCHEDULE OF SPECIAL INSPECTION SERVICES IN SECTION 014533. THE CONTRACTOR IS RESPONSIBLE FOR THE COST OF REPAIR, REINSPECTION AND RETESTING FOR ITEMS THAT DO NOT PASS THE INSPECTIONS OR TESTS.

SPECIAL INSPECTION SERVICES DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR COMPLIANCE WITH OTHER CONSTRUCTION DOCUMENT REQUIREMENTS OR REGULATORY REQUIREMENTS.

THE CONTRACTOR IS RESPONSIBLE FOR THE COST OF DEMOLITION, RECONSTRUCTION, INSPECTION AND TESTING OF ANY WORK COMPLETED WITHOUT INSPECTION AND TESTING AS SPECIFIED IN SECTION 014533. **TABILITY DURING CONSTRUCTION, SHORING, & TEMPORARY STRUCTURES:** 

PERMANENT STABILITY OF THE BUILDING AND COMPONENTS IS NOT PROVIDED UNTIL ALL THE STRUCTURAL ELEMENTS ARE INSTALLED AS SHOWN ON THE CONTRACT DRAWINGS. PROVIDE STABILITY TO ALL NON-SELF SUPPORTING ELEMENTS UNTIL PERMANENT STRUCTURAL SUPPORTS ARE INSTALLED. PROVIDE BRACING, SHORING, AND/OR

TEMPORARY STRUCTURES AS REQUIRED IN ORDER TO SATISFY THE CONTRACT REQUIREMENTS. PROVIDE ALL BRACING NECESSARY TO STABILIZE THE BUILDING DURING THE ERECTION PROCESS. BRACING SHALL BE DESIGNED AND INSTALLED SUCH THAT IT DOES NOT TWIST OR DISTORT MEMBERS.

ANCHOR RODS FOR STEEL COLUMNS ARE NOT DESIGNED TO STABILIZE STRUCTURE BY PROVIDING FIXITY OF THE COLUMN BASE. PROVIDE TEMPORARY BRACING FOR STABILITY DURING THE ERECTION PHASE UNTIL ALL LATERAL LOAD RESISTING ELEMENTS ARE IN PLACE AND WELDING AND/OR BOLTING INSPECTIONS ARE COMPLETE. COMPLY WITH ALL APPLICABLE OSHA SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION. ENERAL REQUIREMENTS:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH DRAWINGS RELATING TO OTHER TRADES. CHECK AND COORDINATE DIMENSIONS, CLEARANCES, OPENINGS, PIPE SLEEVES, CURBS, ETC. WITH THE WORK OF OTHER TRADES.

WORK NOT INDICATED ON A PART OF THE DRAWING BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE REPEATED. DETAILS DESIGNATED AS "TYPICAL" APPLY TO ALL AREAS WHERE THE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED IN THE DETAIL. THE PLANS AND DETAILS IN THE CONTRACT DRAWINGS SHALL NOT BE REVISED WITHOUT PRIOR APPROVAL BY THE ARCHITECT/ENGINEER.

ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS.

SPLICING OF STRUCTURAL MEMBERS WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR APPROVAL OF ARCHITECT/ENGINEER. IF APPROVED, ADDITIONAL TESTING AND INSPECTION SHALL BE AS SPECIFIED BY THE ARCHITECT/ENGINEER AND PAID FOR BY THE CONTRACTOR. NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE: HOLES, SLOTS, CUTS, ETC., ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS

THEY ARE DETAILED ON THE APPROVED SHOP DRAWINGS. ENSURE THAT ALL CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS AND THAT THESE LOADS ARE NOT PUT ON THE STRUCTURAL MEMBERS PRIOR TO THE TIME THAT THE CONCRETE REACHES THE FULL DESIGN STRENGTH AND ALL FRAMING MEMBERS AND THEIR CONNECTIONS ARE IN PLACE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THE ADEQUACY OF SLABS ON GRADE FOR SUPPORTING ALL CONSTRUCTION EQUIPMENT, INCLUDING AREAL LIFTS.

HOP DRAWINGS:

SUBMIT SHOP DRAWINGS FOR REVIEW BY THE ARCHITECT/ENGINEER FOR THE FOLLOWING ITEMS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL

REQUIREMENTS: a. CONCRETE REINFORCING STEEL

1. INDICATE ALL REINFORCING STEEL IN FOUNDATIONS AND SLABS ON GRADE

2. INDICATE ALL HORIZONTAL, VERTICAL, AND TIE REINFORCING

3. INDICATE TYPE AND LOCATION OF ALL REINFORCING STEEL SPLICESb. METAL BUILDING SYSTEMS

SUBMIT OTHER SHOP DRAWINGS FOR REVIEW BY ARCHITECT/ENGINEER AS REQUIRED BY PROJECT SPECIFICATIONS.

DETAILS FOR SOME SPECIAL CONDITIONS WILL NEED TO BE DEVELOPED BY THE DETAILER DURING THE DETAILING PROCESS. FINAL REVIEW OF THE DETAILS WILL BE AT THE DISCRETION OF THE ENGINEER OF RECORD. NO ADDITIONAL CHARGES FOR MAKING CORRECTIONS, CHANGES, OR ADDITIONS TO THE SHOP DRAWINGS ("RE-DETAILING COST") WILL BE ALLOWED. CONTRACTOR SHALL MAKE PROVISIONS FOR DETAILING CORRECTIONS AND MISCELLANEOUS MATERIAL IN THE BID PRICE. ADJUSTMENTS TO THE CONTRACT WILL ONLY BE MADE FOR CHANGE ORDERS APPROVED PRIOR TO THE COMMENCEMENT OF ANY ACTION ON THE CHANGES. ALL SHOP DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR / CONSTRUCTION MANAGER PRIOR TO SUBMITTAL. INCOMPLETE SHOP DRAWINGS AND SHOP DRAWINGS THAT HAVE NOT BEEN REVIEWED BY THE CONTRACTOR WILL BE RETURNED WITHOUT REVIEW BY THE ARCHITECT/ENGINEER. VERIFY AND COORDINATE ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS. IN CASE OF CONFLICTS, THE ARCHITECT/ENGINEER IS TO BE NOTIFIED AND WILL PROVIDE THE CORRECT ELEVATIONS AND DIMENSIONS FOR WHICH SHALL BE INCORPORATED INTO THE SHOP DRAWINGS AT NO EXTRA COST.

FOUNDATION DESIGN IS BASED ON SOIL INVESTIGATION AND REPORT BY GTS, INC. (PROJECT NO.: 24-55017). FOUNDATION DESIGN IS BASED ON THE FOLLOWING MINIMUM NET ALLOWABLE BEARING PRESSURE:

a. CONTINUOUS FOOTINGS: 2.0 KSF

b. INDIVIDUAL PAD FOOTINGS: 2.5 KSF

ALL FOUNDATION BEARING CONDITIONS SHALL BE VERIFIED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION. BOTTOM OF FOUNDATION ELEVATIONS ARE GIVEN FOR BIDDING PURPOSES ONLY. ALL FOUNDATIONS SHALL BE FOUNDED A MINIMUM OF 2 FEET BELOW FINAL GRADE IN PROPERLY COMPACTED STRUCTURAL FILL.

THE SITE SHALL BE STRIPPED APPROXIMATELY 4'-0" (EXTENDING 5 FEET BEYOND THE BUILDING PERIMETER), PROOF ROLLED, COMPACTED FILL PLACED, AND

EXCAVATED AS REQUIRED FOR FOUNDATIONS UNDER THE BUILDING. SEE SPECIFICATION DIVISION 31 FOR EARTHWORK REQUIREMENTS. DURING CONSTRUCTION, GRADES SHOULD BE DEVELOPED TO DIRECT SURFACE WATER FLOW AWAY FROM OR AROUND THE SITE. EXPOSED SUBGRADES SHOULD BE SLOPED TO PROVIDE POSITIVE DRAINAGE SO THAT SATURATION OF THE SUBGRADE IS AVOIDED. SURFACE WATER SHOULD NOT BE PERMITTED TO ACCUMULATE ON THE SITE TO REDUCE THE POTENTIAL FOR STRENGTH LOSS OF THE SUBGRADE SOILS. THE FAT CLAY SOILS ENCOUNTERED AT THE SITE MAY CAUSE PERCHED WATER CONDITIONS TO DEVELOP DURING THE WET SEASON. TEMPORARY DEWATERING MAY BE NECESSARY IF EXCAVATION IS INITIATED DURING THE WET SEASON. TAKE ADEQUATE MEASURES TO ALLOW FOR WORKING SURFACE DURING CONSTRUCTION OF FOUNDATIONS AND SLAB-ON-GRADE, SUCH AS GRAVEL BED OF ADEQUATE

DEPTH, ETC. PROVIDE EARTH RETENTION SYSTEMS AND TEMPORARY BRACING OR SHORING AS REQUIRED TO SUPPORT EXCAVATIONS DURING CONSTRUCTION. TRENCHING AND EXCAVATIONS SHALL MEET ALL OSHA REQUIREMENTS.

WATER ACCUMULATION IS ANTICIPATED IN FOOTING EXCAVATIONS; PROVIDE DRAINAGE OF EXCAVATIONS FROM SURFACE WATER AND SEEPAGE. EXCAVATIONS SHALL BE DRAINED OR PUMPED DRY BEFORE POURING CONCRETE.

PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING. I. IN NO CASE SHALL BULLDOZERS OR OTHER HEAVY EQUIPMENT BE PERMITTED CLOSER THAN 8'-0" FROM ANY FOUNDATION ELEMENT.

ONCRETE AND REINFORCING STEEL: THE DESIGN OF THE CONCRETE STRUCTURE IS BASED ON ACI318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

CAST IN PLACE CONCRETE SHALL HAVE THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS (fc):

COMPONENT COMPRESSIVE STRENGTH

	FUUTINGS	3500 PSI	
	EXTERIOR SLABS	4500 PSI	
	INTERIOR SLABS ON GRADE	3500 PSI	
-			N ALL

SEE SPECIFICATION SECTION 033000 FOR ADDITIONAL MIX DESIGN REQUIREMENTS. ALL DEFORMED REINFORCING STEEL SHALL BE A615 GRADE 60 STEEL, U.N.O.

ALL WELDED WIRE REINFORCING STEEL SHALL BE A1064. ALL WELDED WIRE REINFORCEMENT SHALL BE PROVIDED IN SHEETS.

ALL CONCRETE WORK SHALL CONFORM TO THE LATEST ACI CODE AND ACI DETAILING MANUAL.

MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE: CONCRETE CAST AGAINST EARTH: 3"

CONCRETE EXPOSED TO EARTH OR WEATHER:

CONCRETE EXPOSED TO EAR
#5 BARS AND SMALLER:

#6 BARS AND LARGER:

SLABS, WALLS, AND JOISTS BEAMS AND COLUMNS:

ALL CONCRETE CONSTRUCTION AND MATERIALS SHALL BE PLACED ACCORDING TO ACI 117 TOLERANCES

ALL CONCRETE REINFORCING STEEL SHALL BE SPLICED USING TENSION SPLICES:

a. UNLESS NOTED OTHERWISE, LAP SPLICE ALL CONCRETE REINFORCING STEEL

BARS #6 AND SMALLER:48 BAR DIAMETERSBARS #7 AND LARGER:60 BAR DIAMETERS

WELDED WIRE REINFORCING: ONE MESH PLUS 2"

 b. ONLY APPROVED MECHANICAL SPLICE SYSTEMS SHALL BE USED TO PROVIDE TENSION SPLICES. MECHANICAL SPLICES SHALL DEVELOP 125% OF THE YIELD STRENGTH OF THE BAR.
 ALL CONCRETE REINFORCING SHALL BE SPLICED WHERE DETAILED ON THE DRAWINGS. UNLESS NOTED OTHERWISE STAGGER ALL TENSION LAP SPLICE LOCATIONS.

TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS END WITH STANDARD HOOKS.
 PROVIDE CORNER BARS IN ALL CONCRETE MEMBERS AT INTERSECTIONS. MATCH SIZE AND SPACING OF HORIZONTAL BARS IN THOSE MEMBERS.
 ALL REINFORCING STEEL SHALL BE SECURELY HELD IN PLACE WHILE PLACING CONCRETE. ADDITIONAL BARS OR STIRRUPS SHALL BE PROVIDED AS REQUIRED TO FURNISH SUPPORT FOR ALL REINFORCING STEEL.

 PROVIDE SUPPORT FOR ALL CONCRETE REINFORCING (INCLUDING SLABS ON GRADE) AS REQUIRED TO MAINTAIN CLEAR COVER DIMENSIONS. SPACING SHALL NOT EXCEED 3'-0".
 SUBMIT DRAWINGS SHOWING INTENDED POURING SEQUENCE AND LOCATION OF CONSTRUCTION JOINTS TO THE ARCHITECT/ENGINEER FOR APPROVAL.

5. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED.

 PIPES OR CONDUITS PLACED IN FOUNDATIONS AND SLABS SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTERS. PIPES AND CONDUITS PLACED IN SLAB SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN 1/3 OF SLAB THICKNESS. ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE. NO CONDUIT SHALL BE PLACED WITHIN 24" OF COLUMN FACE.
 LOCATION OF SLOTTED INSERTS, WELD PLATES AND ALL OTHER ITEMS TO BE EMBEDDED IN CONCRETE SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

B. REINFORCING BARS SHALL NOT BE WELDED.

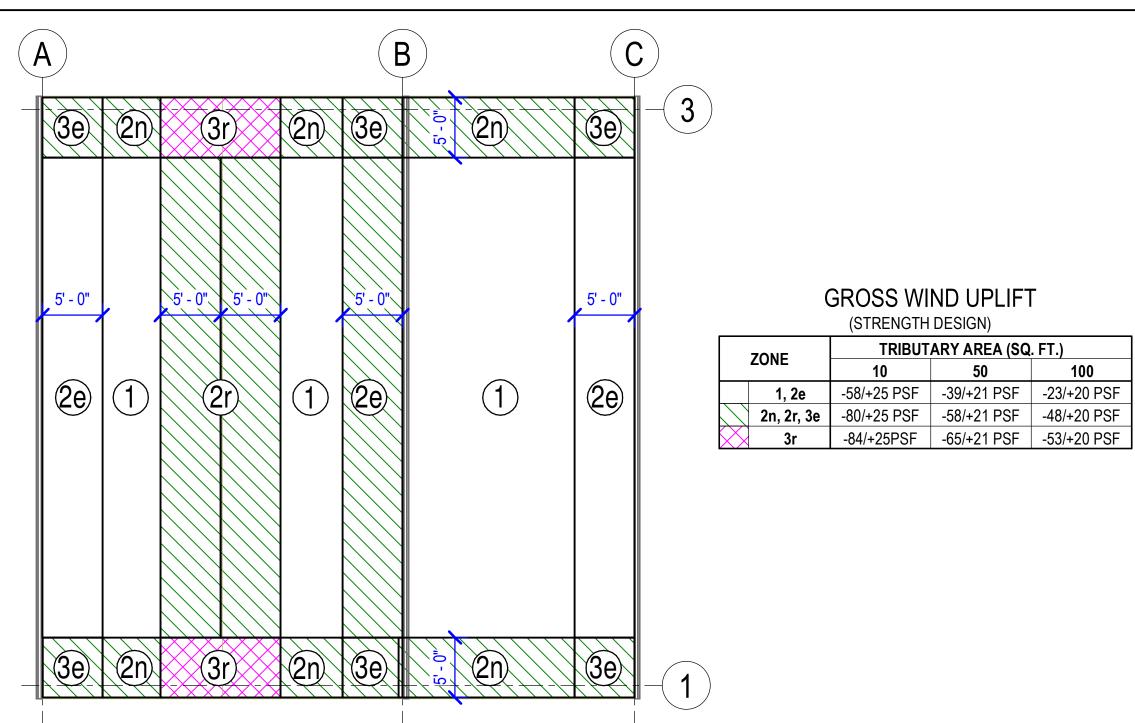
 VERIFY DIMENSIONS AND LOCATIONS OF ALL OPENINGS, PIPE SLEEVE CURBS, ETC., AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.
 AGGREGATE FOR CONCRETE SHALL NOT CONTAIN LIGNITE, STEEL, OR OTHER MATERIALS THAT MAY BE DETRIMENTAL TO THE CONCRETE. ALKALI-SILICA REACTIVE (ASR) AGGREGATES ARE NOT ALLOWED.

MAXIMUM TOLERANCE FOR SLAB EDGES IS 1/2" +/- EXCEPT WHERE TIGHTER TOLERANCE IS REQUIRED FOR ARCHITECTURAL REASONS.
 CONCRETE SHALL BE PLACED AND CURED IN ACCORDANCE WITH THE SPECIFICATIONS. WHEN THE AIR TEMPERATURE IS OVER 85 DEGREES FOLLOW THE RECOMMENDATIONS OF ACI 305R. WHEN THE AIR TEMPERATURE IS BELOW 40 DEGREES FOLLOW THE RECOMMENDATIONS OF ACI 306R.

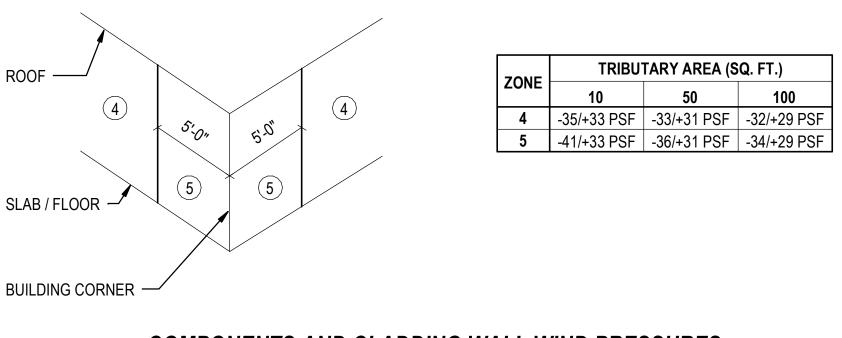
#### H. METAL BUILDING SYSTEMS:

- 1. THE METAL BUILDING SYSTEM MANUFACTURER SHALL BE IAS AC472 ACCREDITED AND A MEMBER OF MBMA
- THE METAL BUILDING SYSTEM MANUFACTURER SHALL:
  a. DESIGN THE METAL BUILDING SYSTEM FOR THE LOADS AND DESIGN CRITERIA SHOWN ON THE PLANS AND IN SPECIFICATIONS. ALL STEEL FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE. ALL STRUCTURAL STEEL DETAILING SHALL BE IN ACCORDANCE WITH THE LATEST AISC SPECIFICATION.
- b. DESIGN THE BUILDING FOR A MAXIMUM DRIFT OF H/100 UNDER THE NOMINAL WIND SPEED INDICATED UNDER THE STRUCTURAL DESIGN CRITERIA. SEISMIC DRIFT SHALL BE LIMITED BASED ON ASCE 7 ASSUMING ACCOMMODATIONS FOR STORY DRIFTS HAVE NOT BEEN INCORPORATED INTO THE DESIGN. COLUMN BASES SHALL BE ASSUMED TO BE PINNED CONDITION.
- CHECK THE FOUNDATION DESIGN LOADS SHOWN ON THE DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER IF ANY OF THE LOADS FROM THE BUILDING WILL EXCEED THE LOADS SHOWN ON THE DRAWINGS.
   WIDE FLANGE STEEL SECTIONS AND CHANNELS SHALL BE A992, 50 KSI.
- 4. GALVANIZING OF ALL STEEL MEMBERS SHALL CONFORM TO ASTM A123. ALL GALVANIZED STEEL REQUIRED TO BE PAINTED SHALL BE CLEANED AND PREPPED
- ACCORDING TO ASTM D6386. 5. DO NOT CONSTRUCT FOUNDATIONS UNTIL THE ARCHITECT/ENGINEER HAS APPROVED THE METAL BUILDING SYSTEM SUBMITTAL AND MADE ANY NECESSARY CHANGES
- TO THE FOUNDATION DRAWINGS. COLUMN ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36. ANCHOR RODS SHALL HAVE A PLATE WASHER PER AISC TABLE 14-1 AND ONE HEAVY HEX NUT AT THE TOP AND ONE HEAVY HEX NUT AT THE BOTTOM TACK WELDED TO THE ROD, UNLESS NOTED OTHERWISE.
- THE TOP AND ONE HEAVY HEX NOT AT THE BOTTOM TACK WELDED TO THE ROD, UNLESS NOTED OTHERWISE.
  PLACE AND SECURE ANCHOR RODS IN FOOTING EXCAVATION PRIOR TO POURING CONCRETE FOR FOOTING. DO NOT PLACE ANCHOR RODS IN WET CONCRETE
  ALL WELDING SHALL BE DEDEODMED BY WELDERS CERTIFIED BY AWS TO DEDEODM THE WELDING IN ACCORDANCE WITH AWS
- ALL WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED BY AWS TO PERFORM THE WELDING IN ACCORDANCE WITH AWS.
  FINAL BOLTING OR WELDING SHALL NOT BE PERFORMED UNTIL THE STRUCTURE HAS BEEN PROPERLY ALIGNED.
- THE METAL BUILDING SYSTEM MANUFACTURER'S DESIGN ENGINEER OR QUALIFIED REPRESENTATIVE SHALL INSPECT THE ERECTED STRUCTURE FOR CONFORMANCE WITH THE ERECTION DRAWINGS AND MANUFACTURER'S INSTRUCTIONS; PROVIDE A REPORT TO THE ARCHITECT/ENGINEER.
   POST-INSTALLED ANCHORS IN CONCRETE:
- 1. POST-INSTALLED ANCHORS (MECHANICAL OR ADHESIVE) SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER–OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR DOWELS. POST-INSTALLED ANCHORS SHALL BE BUILDING CODE COMPLIANT, INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND INSPECTED PER THE APPLICABLE ICC-ES OR IAPMO UES EVALUATION REPORT. SEE SPECIFICATIONS SECTION(S) 033000 FOR ADDITIONAL INFORMATION.

# **COMPONENTS AND CLADDING WIND PRESSURES - WASH BAY**



## **COMPONENTS AND CLADDING ROOF WIND PRESSURES**



### **COMPONENTS AND CLADDING WALL WIND PRESSURES**

- 1. WIND PRESSURES ARE BASED ON ASCE 7-16 STRENGTH DESIGN (ULTIMATE).
- 2. POSITIVE / NEGATIVE VALUES INDICATE FORCES ARE ACTING TOWARDS / AWAY FROM ELEMENT, RESPECTIVELY.
- 3. COMPONENTS SUBJECTED TO PARAPET WIND FORCE ON BOTH SIDES (e.g. WALL PANELS)
- SHALL BE DESIGNED FOR CUMULATIVE FORCES.4. SERVICE LEVEL LOADS MAY BE CALCULATED BY MULTIPLYING THE NUMBERS ABOVE BY 0.6.

