

REQUIRED SPECIAL INSPECTIONS - BY TESTING AGENCY		
In addition to the regular inspections required by Section 110 of the IBC, the following items require Special Inspection in accordance with Section 1704 and 1705 of the 2021 IBC.		
SECTION 1704.2.5 SPECIAL INSPECTION OF FABRICATED ITEMS		
Where fabrication of structural, load-bearing, or lateral load-resisting members or assemblies is being conducted on the premises of a fabricator's shop, special inspections of the fabricated items shall be performed during fabrication, except where the fabricator has been approved to perform work without special inspections in accordance with section 1704.2.5.1.		
SECTION 1704.2.5.1 FABRICATOR APPROVAL		
Special inspections during fabrication are not required where the work is done on the premises of a fabricator approved to perform such work without special inspection. Approval shall be based on review of the fabricator's written fabrication procedures and quality control manuals that provide a basis for control of materials and workmanship, with periodic auditing of fabrication and quality control practices by an approved agency or the building official. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the owner or the owner's authorized agent for submittal to the building official as specified in Section 1704.5 stating that the work was performed in accordance with the approved construction documents.		
SECTION 1705.2.1 STRUCTURAL STEEL		
Special inspections and nondestructive testing of structural steel elements in buildings, structures, and portions thereof shall be in accordance with the quality assurance inspection requirements of AISC 360.		
TABLE 1705.2.2 COLD-FORMED STEEL DECK		
Special inspections and qualification of welding special inspectors for cold-formed steel floor and roof deck shall be in accordance with the quality assurance inspection requirements of SDI QA/QC.		
TABLE 1705.2.3 REQUIRED SPECIAL INSPECTIONS OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS		
VERIFICATION AND INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P)	REQUIRED
Installation of open-web steel joists and joist girders:		
a. End connections - welding or bolted.	P	Y
b. Bridging - horizontal or diagonal.	P	Y
1. Standard bridging.	P	Y
2. Bridging that differs from the SJI specifications listed in section 2207.1.	P	Y
TABLE 1705.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION		
VERIFICATION AND INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P)	REQUIRED
1. Inspect reinforcement, including prestressing tendons, and verify placement.	P	N
2. Reinforcing bar welding:		
a. Verify weldability of reinforcing bars other than ASTM A706.	P	N
b. Inspect single-pass fillet welds, maximum 5/16".	C	N
c. Inspect all other welds.	C	N
3. Inspect anchors cast in concrete.	P	N
4. Inspect anchors post-installed in hardened concrete members:		
a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads.	C	Y
b. Mechanical anchors and other adhesive anchors.	C	Y
5. Verify use of required design mix.	P	Y
6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	C	Y
7. Inspect concrete and shotcrete placement for proper application techniques.	C	N
8. Verify maintenance of specified curing temperature and techniques.	P	N
9. Inspection of prestressed concrete for:		
a. Application of prestressing forces.	C	N
b. Grouting of bonded prestressing tendons.	C	N
10. Inspect erection of precast concrete members.	P	N
11. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category C, D, E, or F, inspect such connections and reinforcement in the field for:		
a. Installation of the embedded parts.	C	N
b. Completion of the continuity of reinforcement across joints.	C	N
c. Completion of connections in the field.	C	N
12. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	P	N
13. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	P	N
14. Inspect formwork for shape, location, and dimensions of the concrete member being formed.	P	N
TABLE 1705.6 REQUIRED VERIFICATION AND INSPECTION OF SOLS		
VERIFICATION AND INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P)	REQUIRED
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	P	Y
2. Verify excavations are extended to proper depth and have reached proper material.	P	Y
3. Perform classification and testing of compacted fill materials.	P	Y
4. During fill placement, verify use of proper materials and procedures in accordance with the provisions of the approved geotechnical report. Verify densities and lift thicknesses during placement and compaction of compacted fill.	C	Y
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	P	Y

CONCRETE MATERIAL

- ALL CONCRETE SHALL BE NORMAL WEIGHT (DENSITY=145 PCF) AND SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH IN ACCORDANCE WITH THE FOLLOWING, U.N.O.:

ALL FOUNDATIONS	3000 PSI
FOUNDATION WALLS	3000 PSI
INTERIOR SLABS	SEE NOTES BELOW
TILT-UP WALL PANELS	4000 PSI
EXTERIOR STRUCTURAL SLABS	3500 PSI
ALL OTHER CONCRETE (U.N.O.)	3000 PSI
CURBS & SIDEWALKS	SEE CIVIL
- WATER-TO-CEMENT PLUS POZZOLANIC MATERIALS RATIO SHALL BE IN ACCORDANCE WITH THE FOLLOWINGS:

F _{cr} (psi)	NON-AIR ENTRAINED	AIR ENTRAINED
4,000	0.67	0.49
3,000	0.68	0.59
- ALL FOUNDATION CONCRETE SHALL BE 4-6% AIR ENTRAINED. SLAB CONCRETE SHALL NOT HAVE ENTRAINED AIR, U.N.O.
- THE SLUMP OF ALL CONCRETE SHALL NOT EXCEED 4" UNLESS A HIGH RANGE WATER-REDUCING ADMIXTURE IS USED. THE SLUMP OF CONCRETE PRIOR TO ADDITION OF A HIGH-RANGE WATER-REDUCING ADMIXTURE SHALL NOT EXCEED 4". THE SLUMP OF CONCRETE CONTAINING A HIGH RANGE WATER-REDUCING ADMIXTURE SHALL NOT EXCEED 8".
- CONCRETE COARSE AGGREGATE SIZE SHALL BE ASTM C33 #57.
- THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR REVIEW A MINIMUM OF ONE WEEK PRIOR TO PLACEMENT OF ANY CONCRETE. THE CONCRETE MIX DESIGNS SHALL INCLUDE ALL STRENGTH DATA NECESSARY TO SHOW COMPLIANCE WITH THE PROJECT SPECIFICATIONS FOR EITHER THE TRIAL BATCH OR FIELD EXPERIENCE METHOD.
- FLY ASH TO BE LIMITED TO 25% OF TOTAL CEMENTITIOUS MATERIAL BY WEIGHT.
- SLAB ON GRADE CONCRETE AS PER CLIENT SPECS.

A. MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE 3,500 PSI.	B. MINIMUM CEMENT CONTENT TO BE 520 POUNDS PER CUBIC YARD.
C. MAXIMUM WATER-TO-CEMENT RATIO TO BE 0.53.	MAXIMUM WATER CONTENT SHALL NOT EXCEED 300 POUNDS PER CUBIC YARD.
D. SLUMP AT POINT OF PLACEMENT SHALL BE INSTALLED AT 5".	E. NOMINAL MAXIMUM COARSE AGGREGATE SIZE OF 12" OR 1". COARSENESS FACTOR OF 45 TO 75%.
F. ADJUSTED WORKABILITY FACTOR TO BE 36 TO 42%.	G. NO HIGH RANGE WATER REDUCERS.
H. NO FLY ASH.	

CONCRETE REINFORCING STEEL

- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. ALL WELDED REINFORCING BARS SHALL CONFORM TO ASTM A706. DEFORMED BAR ANCHORS (DBA) SHALL CONFORM TO ASTM A1064.
- ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064. WIRE FABRIC SHALL BE SUPPLIED IN SHEETS. ROLLED FABRIC WILL NOT BE ACCEPTED. WIRE FABRIC SHALL BE PLACED AT THE MID-DEPTH OF THE SLAB. WIRE FABRIC SHALL BE SUPPORTED ON CONTINUOUS HIGH CHAIRS SPACED NOT MORE THAN 4 FEET O.C.
- ALL REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE DETAILING MANUAL. ALL DOWELS ARE TO BE TIED IN PLACE. IF ANY DOWELS ARE "STABBED" AFTER THE CONCRETE HAS BEEN PLACED, THE CONCRETE SHALL BE REMOVED AND REPLACED.
- ALL REINFORCING SHALL BE SUPPORTED IN FORMS, SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRE-TIED TOGETHER, IN ACCORDANCE WITH THE LATEST EDITION OF THE CRSI "MANUAL OF STANDARD PRACTICE".
- MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:

UNFORMED SURFACE IN CONTACT WITH THE GROUND	3 IN.
FORMED SURFACES EXPOSED TO EARTH OR WEATHER:	
#5 BARS AND LARGER	2 IN.
#5 BARS AND SMALLER	1 1/2 IN.
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:	
BEAMS, GIRDERS AND COLUMNS	1 1/2 IN.
SLABS, WALLS AND JOISTS:	
#11 BARS AND SMALLER	3/4 IN.
#14 AND #18 BARS	1 1/2 IN.
- ALL BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., WHICH ARE BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" OF CONCRETE.
- PROVIDE CORNERS AND INTERSECTIONS OF CONCRETE WALLS, CONCRETE BEAMS, CONTINUOUS FOOTINGS, THICKENED SLABS AND TURNDOWNS. CORNER BAR SIZE SHALL MATCH HORIZONTAL BAR SIZE. CORNER BARS ARE TO BE LAPPED 40 BAR DIAMETERS.
- ALL LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. ALL SPLICES ARE CLASS "B" SPLICES.

SPLICE LENGTHS (IN.)										
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11	
TOP BARS*	21	28	35	42	61	69	78	88	98	
OTHER BARS	16	21	27	32	47	53	60	68	75	

* TOP BARS REFER TO ANY HORIZONTAL REINFORCEMENT WITH MORE THAN 12 INCHES OF FRESH CONCRETE PLACED BELOW.

FOUNDATION, SLAB-ON-GRADE - GENERAL

- THE FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE BEARING PRESSURE OF 7,000 PSF.
- ALL BEARING MATERIAL SHALL BE INSPECTED BY THE INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
- FOOTINGS MAY BE POURED INTO AN EXISTING FORMED TRENCH IF SOIL CONDITIONS PERMIT.
- FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACK FILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE OR UNTIL THE CONCRETE HAS ATTAINED ITS FULL COMPRESSIVE STRENGTH FOR CANTILEVER WALLS.
- WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.
- VERIFY THE USE AND EXTENT OF PERIMETER INSULATION WITH ARCHITECTURAL DRAWINGS PRIOR TO THE INSTALLATION OF FOUNDATIONS. INSTALL PERIMETER INSULATION AS REQUIRED.
- UNDER-SLAB DRAINAGE FILL TO BE A MINIMUM 4-INCH COMPACTED LAYER OF WASHED ASTM C33 No. 57 STONE. THE DRAINAGE FILL MAY BE CAPTURED WITH 2" COMPACTED CLASS 3 BASE TO PREVENT RUTTING.
- NO BUILDING FOUNDATIONS, INCLUDING GRADE BEAMS, ARE TO BE PENETRATED WITH CONDUITS, PIPES, ETC. UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL PLANS OR EXPRESS CONSENT IS GIVEN BY THE E.O.R.

POST-INSTALLED ANCHORS

- ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
- INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING THE HILTI PROFISYST.
- THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
- ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS.
- EXPANSION ANCHORS FOR USE IN CONCRETE SHALL BE ONE OF THE FOLLOWING:

A. HILTI KWIK BOLT KB-T22 EXPANSION ANCHORS, PER ICC ESR-4266.	B. SIMPSON STRONG BOLT 2, PER ICC ESR-3037.
C. DEWALT POWER-STD® S02, PER ICC ESR-2502.	
- EXPANSION ANCHORS FOR MASONRY SHALL BE ONE OF THE FOLLOWING:

A. HILTI KWIK BOLT KB-T22 EXPANSION ANCHORS, PER ICC ESR-4561.	B. SIMPSON STRONG BOLT 2, PER IAPMO ER-240.
C. DEWALT POWER-STD® S01, PER ICC ESR-2618.	
- SCREW ANCHORS FOR USE IN CONCRETE SHALL BE ONE OF THE FOLLOWING:

A. HILTI HIT-RE 500V, PER ICC ESR-3814.	B. SIMPSON SET-3G, PER ICC ESR-4057.
C. DEWALT PURE-220®, PER ICC ESR-5144.	
- POST-INSTALLED EPOXY FOR CONCRETE SHALL BE ONE OF THE FOLLOWING:

A. HILTI HIT-RE 500V, PER ICC ESR-3814.	B. SIMPSON SET-3G, PER ICC ESR-4057.
C. DEWALT PURE-220®, PER ICC ESR-5144.	
- POST-INSTALLED EPOXY FOR MASONRY SHALL BE ONE OF THE FOLLOWING:

A. HILTI HIT-HY 270, PER ICC ESR-4143.	B. SIMPSON SET-3G, PER ICC ESR-4844.
C. DEWALT AC108®-GOLD, PER ICC ESR-3200.	

STRUCTURAL STEEL

- ALL STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

ALL CHANNELS, ANGLES, ETC. (UNLESS NOTED OTHERWISE)	A36
PLATES	A572, GRADE 50 (F _y =60 KSI)
ALL WIDE FLANGES (UNLESS NOTED OTHERWISE)	A992 (F _y =50 KSI)
HOLLOW STRUCTURAL SECTION	
RECTANGULAR	A500, GRADE C (F _y =50 KSI)
ROUND	A500, GRADE C (F _y =50 KSI)
ANCHOR RODS	A53 (F _y =36 KSI)
BOLTS	F1554, GRADE 36
WELD ELECTRODES	A325-N (UNLESS NOTED OTHERWISE)
E70XX	
- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE (LATEST EDITION), EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
- UNLESS DESIGNED AND DETAILED ON THE STRUCTURAL DRAWINGS, ALL STRUCTURAL STEEL STAIRS SHALL BE DESIGNED AND PROVIDED BY THE STEEL FABRICATOR. COORDINATE STAIR LAYOUT WITH THE ARCHITECTURAL DRAWINGS.
- THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE ROOF DECK, FLOOR SLABS AND ATTACHMENT TO THE WALL SYSTEM FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRING TO AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT.
- WHERE A325 BOLTS OF ANY DIAMETER OR A490 EQUAL TO OR LESS THAN 1 INCH IN DIAMETER ARE TO BE INSTALLED AND TIGHTENED IN AN OVERSIZE OR SHORT SLOTTED HOLE IN AN OUTER PLY, A HARDENED WASHER CONFORMING TO ASTM F438 SHALL BE USED.
- ALL STRUCTURAL STEEL SHALL BE PAINTED WITH RED OR GRAY PRIMER. DO NOT PAINT TOP OF BEAMS WHERE HEADED STUD ANCHORS ARE TO BE ATTACHED.
- AFTER ANCHOR RODS HAVE BEEN SET, AND BEFORE CONCRETE IS PLACED, IT IS STRONGLY SUGGESTED THAT THE CONTRACTOR ENGAGE A SURVEYOR TO VERIFY THE PROPER LOCATION AND ELEVATION OF THE ANCHOR RODS.
- PER THE AISC MANUAL OF STEEL CONSTRUCTION, ANCHOR ROD HOLES IN BASE PLATES AND WASHERS SHALL BE THE FOLLOWING:

ANCHOR ROD Ø	MAX. HOLE SIZE IN BASE PL.	WASHER SIZE	MIN. WASHER THICK.	MAX. HOLE SIZE IN WASHER
1/2"	1-5/16"	2"	1/4"	1-9/16"
3/4"	1-9/16"	2 1/4"	5/16"	1-11/16"
1"	1-7/8"	3"	3/4"	1-13/8"
1 1/4"	2-1/8"	3 1/4"	1 1/2"	1-5/8"
1 1/2"	2-3/8"	4"	1 3/4"	1-7/8"
1 3/4"	2-7/8"	4 1/2"	1 7/8"	2-1/8"
2"	3-1/4"	5"	2"	2-1/8"
2 1/4"	3-3/4"	5 1/4"	7/8"	2-5/8"

STEEL JOISTS AND JOIST GIRDERS

- ALL STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SJI STANDARD SPECIFICATIONS FOR STEEL JOISTS AND JOIST GIRDERS (LATEST EDITION).
- ALL STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE JOIST MANUFACTURER. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ALL STEEL JOISTS AND JOIST GIRDERS. JOIST MANUFACTURER SHALL PROVIDE A LETTER, SIGNED AND SEALED, STATING THAT ALL JOIST AND JOIST GIRDERS HAVE BEEN DESIGNED BY OR UNDER THE DIRECT SUPERVISION OF AN ENGINEER LICENSED IN THE STATE WHERE THE STRUCTURE IS LOCATED.
- EXCEPT WHERE ADDITIONAL AND/OR SPECIFIC DESIGN LOADS ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, STEEL JOISTS SHALL BE DESIGNED AS SIMPLY SUPPORTED, UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING. THE UNIFORM DESIGN LOAD SHALL BE THE TOTAL SAFE UNIFORMLY DISTRIBUTED LOAD AS SHOWN IN THE SJI STANDARD LOAD TABLE, OR INDICATED ON THE DRAWINGS. THE JOIST DESIGN SHALL ALSO INCLUDE A MAXIMUM OF 2-20# POINT LOADS ON THE TOP OR BOTTOM CHORD AT ANY LOCATION WITHOUT ADDITIONAL WEB REINFORCING.
- EXCEPT WHERE ADDITIONAL AND/OR SPECIFIC DESIGN LOADS ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, STEEL JOIST GIRDERS SHALL BE DESIGNED AS SIMPLY SUPPORTED PRIMARY MEMBERS, WITH ALL LOADS EQUAL IN MAGNITUDE AND EVENLY SPACED ALONG THE JOIST GIRDER TOP CHORD (UNLESS NOTED OTHERWISE).
- ALL FLOOR JOISTS AND JOIST GIRDERS, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED TO SUPPORT THE LIVE DESIGN LOAD WITHOUT EXCEEDING A DEFLECTION OF L/360. STANDARD SJI CAMBER MAY BE USED TO COMPENSATE FOR DEAD LOAD DEFLECTION.
- ALL ROOF JOISTS AND JOIST GIRDERS, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED TO SUPPORT THE LIVE (OR SNOW) DESIGN LOAD WITHOUT EXCEEDING A DEFLECTION OF L/240. STANDARD SJI CAMBER MAY BE USED TO COMPENSATE FOR DEAD LOAD DEFLECTION.
- STEEL JOIST BRIDGING SHOWN ON THE DRAWINGS IS FOR ILLUSTRATIVE PURPOSES ONLY. ALL STEEL JOIST BRIDGING SHALL BE PROVIDED IN ACCORDANCE WITH THE SJI SPECIFICATION, AND SHALL BE SPECIFIED BY THE JOIST MANUFACTURER. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE INSTALLED, AND STEEL JOIST ENDS FIXED, PRIOR TO THE APPLICATION OF ANY LOADS. BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, OR CONCRETE WALLS SHALL BE ATTACHED THERETO. THE JOIST MANUFACTURER MUST COORDINATE BRIDGING LOCATIONS TO AVOID INTERFERENCE WITH ALL MECHANICAL, ELECTRICAL, AND FIRE PROTECTION EQUIPMENT.
- THE JOIST MANUFACTURER SHALL DESIGN ALL ROOF JOISTS AND JOIST GIRDERS, AND SHALL DESIGN AND SPECIFY ALL REQUIRED ADDITIONAL BRIDGING AND/OR BRACING, FOR MINIMUM NET SERVICE UPLIFT FORCES OF 20 PSF, U.N.O. DO NOT DESIGN JOISTS FOR COMPONENT AND CLADDING LOADINGS.
- ALL JOIST GIRDERS SHALL BE PROPORTIONED SUCH THAT THEY CAN BE ERECTED WITHOUT BRIDGING.
- UNLESS NOTED OTHERWISE, K-SERIES JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL MEMBERS, OR STEEL BEARING PLATES, WITH (2)-2" LONG 3/16" FILLET WELDS (ONE EACH SIDE).
- UNLESS NOTED OTHERWISE, LH-SERIES JOISTS AND JOIST GIRDERS SHALL BE ATTACHED TO SUPPORTING STEEL MEMBERS, OR STEEL BEARING PLATES, WITH (2)-2" LONG 1/4" FILLET WELDS (ONE EACH SIDE).
- STEEL JOIST AT COLUMN CENTERLINES SHALL BE BOLTED TO THE SUPPORTING STEEL MEMBER WITH TWO ERECTION BOLTS, 3/4" FOR K-SERIES JOISTS & 1/2" FOR LH-SERIES JOISTS. FOR JOIST CENTER LINES, USE BOLTED CONNECTIONS FOR THE STEEL JOIST CLOSEST TO THE CENTER LINE, WHERE THE DRAWINGS INDICATE THAT THE JOIST SEAT IS TO BE WELDED TO THE SUPPORTING STEEL. THE BOLTS PROVIDED ARE FOR ERECTION ONLY AND MAY BE REMOVED AFTER THE WELDS ARE COMPLETED.
- STEEL JOISTS AT COLUMN CENTER LINES SHALL BE PROVIDED WITH (5)-5/8" x 2" KNIFE PLATE AT THE BOTTOM CHORD, WELDED TO THE COLUMN, FOR STABILIZATION. DO NOT WELD THE JOIST CHORD TO THE PLATE.
- STEEL JOIST GIRDERS AT COLUMN CENTERLINES SHALL BE BOLTED TO THE STRUCTURAL STEEL COLUMN WITH (2)-X" BOLTS.
- HOLES IN STEEL JOIST BEAMS SHALL NOT BE PERMITTED EXCEPT FOR BOLTED CONNECTIONS AT THE BEARING END OF THE STEEL JOIST, OR WHERE SPECIFIED ON THE DRAWINGS AND SPECIFICALLY DESIGNED FOR BY THE JOIST MANUFACTURER.
- ALL ITEMS SUCH AS MECHANICAL EQUIPMENT, DUCT WORK, PIPES, CEILING SUPPORTS, FIXTURES, DISPLAYS, ETC., WHICH ARE TO BE SUPPORTED BY, OR HUNG FROM, STEEL JOIST GIRDERS SHALL BE FRAMED WITH AUXILIARY FRAMING TO THE PANEL POINTS OF THE STEEL JOIST OR GIRDER WHEN THE CONCENTRATED LOAD EXCEEDS 50 LBS. METHODS OF FRAMING THAT INDUCE BENDING IN THE STEEL JOIST OR GIRDER CHORDS OR WEB MEMBERS WILL NOT BE PERMITTED.
- ADDITIONAL DESIGN LOADS FROM ARCHITECTURAL FEATURES, ROOF TOP EQUIPMENT, OR ANY OTHER CONCENTRATED LOADS SHOWN ON THE DRAWINGS, SHALL BE CONSIDERED AS COLLATERAL LOADS. THESE LOADS SHALL BE CONSIDERED IN THE DESIGN OF THE JOISTS AND JOIST GIRDERS. IN ADDITION TO THE SPECIFIED UNIFORM AND PANEL LOADS, COORDINATE WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND WEIGHTS OF ALL EQUIPMENT, WHERE SUCH LOADS DO NOT OCCUR AT THE PANEL POINTS OF THE JOISTS OR JOIST GIRDERS. AUXILIARY FRAMING SHALL BE ADDED, OR THE TOP CHORD SHALL BE DESIGNED FOR THE EFFECTS OF THE LOAD.
- NO LOADS SHALL BE PLACED ON ANY JOIST GIRDER UNTIL THE STEEL JOISTS BEARING ON THE GIRDER ARE IN PLACE, AND FASTENED TO THE GIRDER AS SPECIFIED.
- ALL DAMAGED JOISTS AND JOIST GIRDERS SHALL BE REPAIRED OR REPLACED. THE PROFESSIONAL-OF-RECORD SHALL BE THE SOLE JUDGE AS TO WHETHER A JOIST, OR JOIST GIRDER, CAN BE REPAIRED OR MUST BE REPLACED. ALL REPAIRS TO JOISTS SHALL BE DESIGNED AND SPECIFIED BY THE JOIST SUPPLIER'S ENGINEER.
- JOIST SEATS TO HAVE A MINIMUM PLATE THICKNESS OF 3/16".
- ALL NECESSARY JOIST TOP CHORD EXTENSIONS SHALL BE "K" TYPE, MEETING OR EXCEEDING THE LOAD CAPACITY OF THE JOIST, U.N.O.
- ALL JOIST & JOIST GIRDERS SHALL BE PAINTED WITH RED OR GRAY PRIMER.

METAL DECK

- METAL DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE "SPECIFICATIONS AND COMMENTARY FOR STEEL DECK" (LATEST EDITION).
- ALL METAL DECK SHALL BE OF CONFIGURATION, DEPTH, AND MINIMUM GA., AS SPECIFIED ON THE DRAWINGS. ATTACHMENT OF METAL DECK TO THE SUPPORTING STRUCTURAL MEMBERS SHALL BE, AT A MINIMUM, AS SPECIFIED ON THE DRAWINGS. SEE THE PLAN NOTES.
- ALL METAL DECK SHALL BE MINIMUM F_y = 50 KSI.
- ACCEPTABLE DECK FASTENERS:

A. PROVIDE SIMPSON XL014T1224 SCREWS(UES ER-326) FOR BASE MEMBER THICKNESSES LESS THAN 3/8".	B. PROVIDE HILTI S-R15+ M9 SCREWS(ESR-3693) FOR BASE MEMBER THICKNESSES LESS THAN 3/8".
C. PROVIDE HILTI X-HSN 24 PAF(ESR-3692) FOR BASE MEMBER THICKNESSES BETWEEN 1/8" TO 3/8".	D. PROVIDE HILTI X-ENP 19 PAF(ESR-2776) FOR BASE MEMBER THICKNESSES GREATER THAN 3/8".
- FUDDLE WELDS ARE NOT ACCEPTABLE FOR DECK FASTENING.
- DO NOT HANG OR SUPPORT ANY LOADS FROM METAL DECK.
- ALL METAL DECK SHEETS SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
- ALL NON COMPOSITE METAL DECK SHEET ENDS SHALL BE LAPPED A MINIMUM OF 2". BUTTED ENDS ARE ACCEPTABLE WHEN THE THICKNESS OF THE DECK EXCEEDS 20GA.
- ALL METAL ROOF DECK SHALL BE G60 GALVANIZED UNLESS NOTED OTHERWISE.
- ALL METAL FORM AND COMPOSITE DECK SHALL BE G60 GALVANIZED UNLESS NOTED OTHERWISE.

TILT PANELS

- CONCRETE WORK SHALL CONFORM TO THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318 CURRENT EDITION) AND SHALL COMPLY WITH ALL LOCAL LAWS AND ORDINANCES. WHERE CONFLICTING REQUIREMENTS OCCUR, THE MORE STRINGENT REQUIREMENT SHALL APPLY.
- DESIGN OF TILT-UP WALL IS BASED ON A MINIMUM COMPRESSIVE STRENGTH, OF 4000 PSI AT 28 DAYS AND A MINIMUM COMPRESSIVE STRENGTH BEFORE LIFTING OF 2800 PSI.
- REINFORCING STEEL SHALL BE GRADE 60, DEFORMED BARS, CONFORMING TO ASTM A615.
- ALL REINFORCING BARS SHALL BE SUPPORTED ON REBAR CHAIRS WITH PLASTIC TIPS.
- PANELS SHALL BE CAST WITH OUTSIDE FACE OF WALL PANEL ORIENTED DOWNWARD.
- CASTING BED FOR PANELS SHALL BE COVERED WITH AN APPROVED BOND BREAKING COMPOUND IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- ALL PANELS SHALL BE PROTECTED DURING CURING.
- WALL PANEL CONSTRUCTION HAS BEEN DESIGNED FOR THE DESIGN LOADS INDICATED ON THE DRAWINGS. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN REQUIREMENTS FOR LIFTING OF PANEL IN PLACE AND TEMPORARY SHORING. TEMPORARY SHORING SHALL REMAIN IN PLACE UNTIL ROOF DECK AND WALL PANELS ARE PROPERLY CONNECTED TO THE BUILDING FRAME, AND THE WALL PANEL HAS DEVELOPED A MINIMUM STRENGTH OF 4000 PSI.
- REFER TO THE PANEL BOOK FOR PANEL DIMENSIONS, EMBEDS, AND REVEALS. THE PANEL BOOK INCLUDES THE PANEL REINFORCEMENT DRAWINGS.
- REFER TO ARCHITECTURAL DRAWINGS FOR THE FINISH OF THE PANELS.
- THE PANEL DIMENSIONS SHOWN UTILIZE A 3/4" JOINT BETWEEN PANELS.
- PROVIDE A 3/4" CHAMFER ON ALL EXTERIOR EDGES OF PANELS AND ON ALL EDGES OF OPENINGS IN THE PANELS.
- THE PANELS SHALL BE SUPPORTED ON EVENLY SPACED SHIMS. THE PANEL BASE SHALL BE FULLY GROUTED AS SOON AS POSSIBLE FOLLOWING ERECTION.

GENERAL NOTES

- THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF ANY SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S RESPONSIBILITY THROUGHOUT THE CONSTRUCTION.
- IT IS SOLELY THE CONTRACTORS RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- ALL DIMENSIONS ON THE STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS. THE ENGINEER OF RECORD SHALL BE NOTIFIED OF ANY RELEVANT DIMENSIONAL DISCREPANCIES.
- GOVERNING CODE: 2021 ARKANSAS FIRE PREVENTION CODE.
- ALL FRAMING SHALL BE COORDINATED WITH THE MECHANICAL DRAWINGS TO ENSURE ADEQUATE CLEARANCES FOR CHASES, DUCT WORK, PIPING, ETC.
- DESIGN LOADS:

ROOF DEAD LOAD:	10 PSF
ROOF LIVE LOAD:	20 PSF
SNOW DESIGN CRITERIA:	
GROUND SNOW LOAD:	10 PSF
SNOW IMPORTANCE FACTOR:	1.0
MINIMUM ROOF SNOW LOAD:	10 PSF
- WIND DESIGN CRITERIA:

RISK CATEGORY:	II
BASIC WIND SPEED:	107 mph
EXPOSURE CATEGORY:	B
COMPONENT AND CLADDING LOADS PER ASCE 7-16 CHAPTER 30	
- SEISMIC DESIGN CRITERIA:

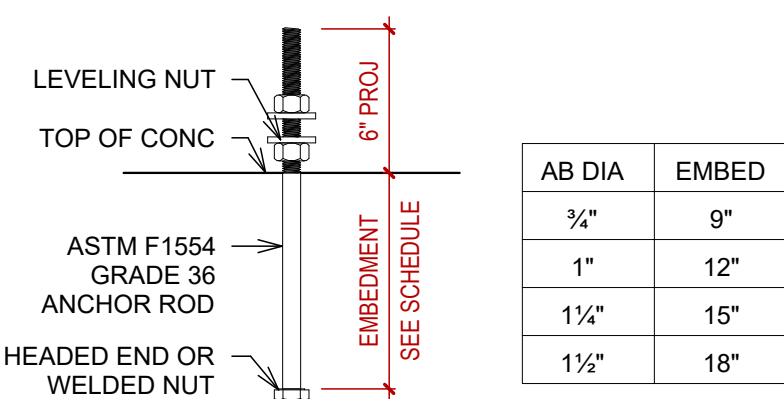
SITE CLASS:	D
SEISMIC RISK CATEGORY:	II
SEISMIC IMPORTANCE FACTOR:	1.0
SEISMIC DESIGN CATEGORY:	C
S _s = 0.53 F _a = 1.376 S _{MS} = 0.729 S _{MS} = 0.486 S _W = 0.28	
- BASIC SEISMIC-FORCE-RESISTING SYSTEM: INTERMEDIATE PRECAST SHEAR WALLS
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL-FORCE PROCEDURE

MISCELLANEOUS

- THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- THE STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING REQUIREMENTS FROM SUCH DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK.
- ANY DETAIL TITLED AS A TYPICAL DETAIL IS APPLICABLE THROUGHOUT THE DESIGN DRAWINGS. THESE DETAILS ARE DEFINED AS GENERAL STANDARDS THAT ARE USUALLY IDENTIFIED BY SPECIFIC REFERENCE WITHIN THE DRAWINGS.
- NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL-OF-RECORD.
- NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT WRITTEN APPROVAL OF THE PROFESSIONAL-OF-RECORD.
- NO OPENINGS IN WALLS AND DECK, WHICH ARE 1'-4" AND LESS ON A SIDE, ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTORS CONSTRUCTION METHODS AND/OR SEQUENCES.
- DO NOT SCALE THESE DRAWINGS. USE SPECIFIED DIMENSIONS.
- CONTRACTORS CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
- THE CONTRACTOR SHALL INFORM THE PROFESSIONAL-OF-RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL-OF-RECORD'S REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL-OF-RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE PROFESSIONAL-OF-RECORD HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.

SUBMITTAL PROCEDURES

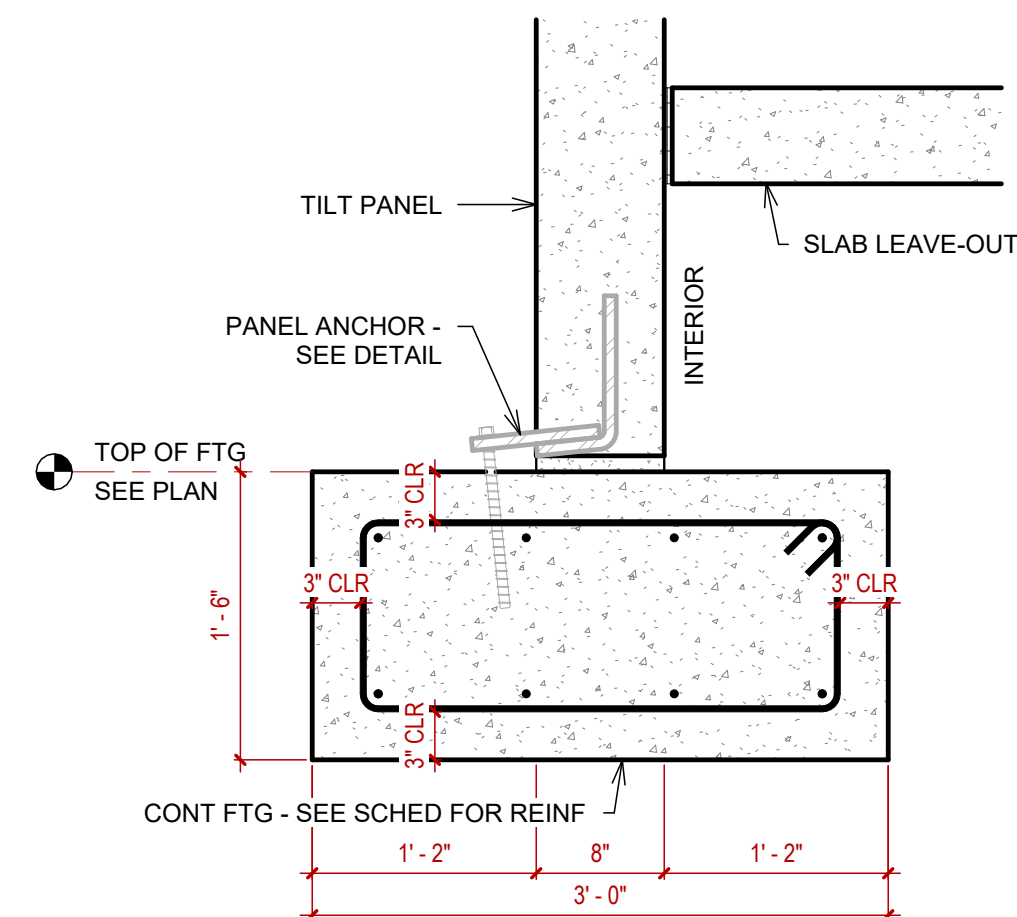
- SHOP DRAWINGS SHALL BE PRODUCED FROM SCRATCH. THE SHOP DRAWINGS SHALL NOT BE PRODUCED FROM DIGITAL COPIES OR SCANS OF THE E.O.R. DRAWINGS. IF THE E.O.R. DRAWINGS ARE DIGITALLY REPRODUCED AND USED IN SUBMITTED SHOP DRAWINGS, THE SHOP DRAWINGS SHALL BE REJECTED IN WHOLE.
- TRANSMIT SUBMITTALS SUFFICIENTLY IN ADVANCE OF RELATED CONSTRUCTION ACTIVITIES TO AVOID UNNECESSARY DELAY. THE STRUCTURAL ENGINEER FOR THIS PROJECT MAY WITHHOLD ACTION ON A SUBMITTAL REQUIRING COORDINATION WITH OTHER SUBMITTALS UNTIL ALL RELATED SUBMITTALS ARE RECEIVED.
- SHOP DRAWINGS SHALL BE SUBMITTED IN AN UNLOCKED PDF ELECTRONIC FORMAT. LOCKED PDF FILES WILL NOT BE ACCEPTED. THE SHOP DRAWINGS WILL BE REVIEWED, MARKED UP, AND RETURNED IN PDF ELECTRONIC FORMAT.
- EACH SUBMITTAL WILL BE REVIEWED A MAXIMUM OF TWO TIMES. AFTER AN INCOMPLETE/INCORRECT SUBMITTAL HAS BEEN REJECTED A SECOND TIME THE CONTRACTOR WILL BE BILLED FOR FURT



- NOTES:
1. THE FABRICATION OF THE ANCHOR ROD AND HEADNUT SHALL CONFORM TO ASTM F1554.
 2. ANCHOR BOLT EMBEDMENT TO BE 12 ANCHOR BOLT DIAMETERS FOR SIZES NOT SHOWN ON THE SCHEDULE.
 3. 'J' BOLTS WILL NOT BE ACCEPTED.

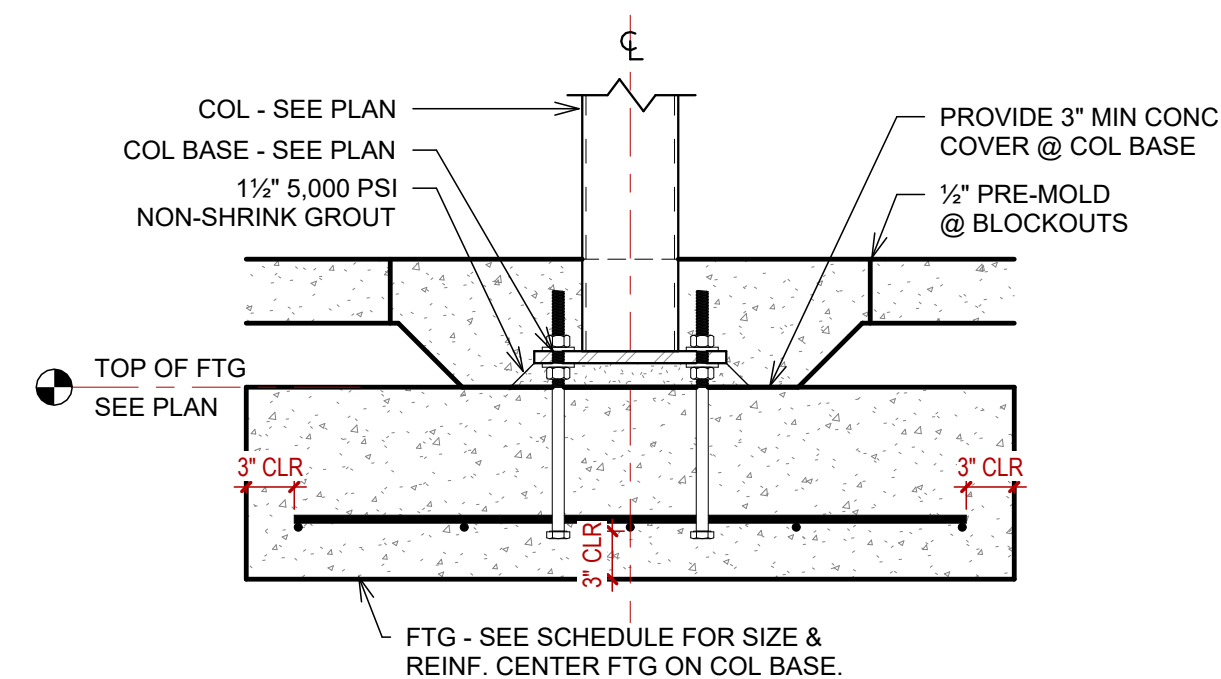
2 ANCHOR BOLT

1 1/2" = 1'-0"



3 CONT FTG @ SLAB LEAVE-OUT

1" = 1'-0"



4 HSS COL FTG

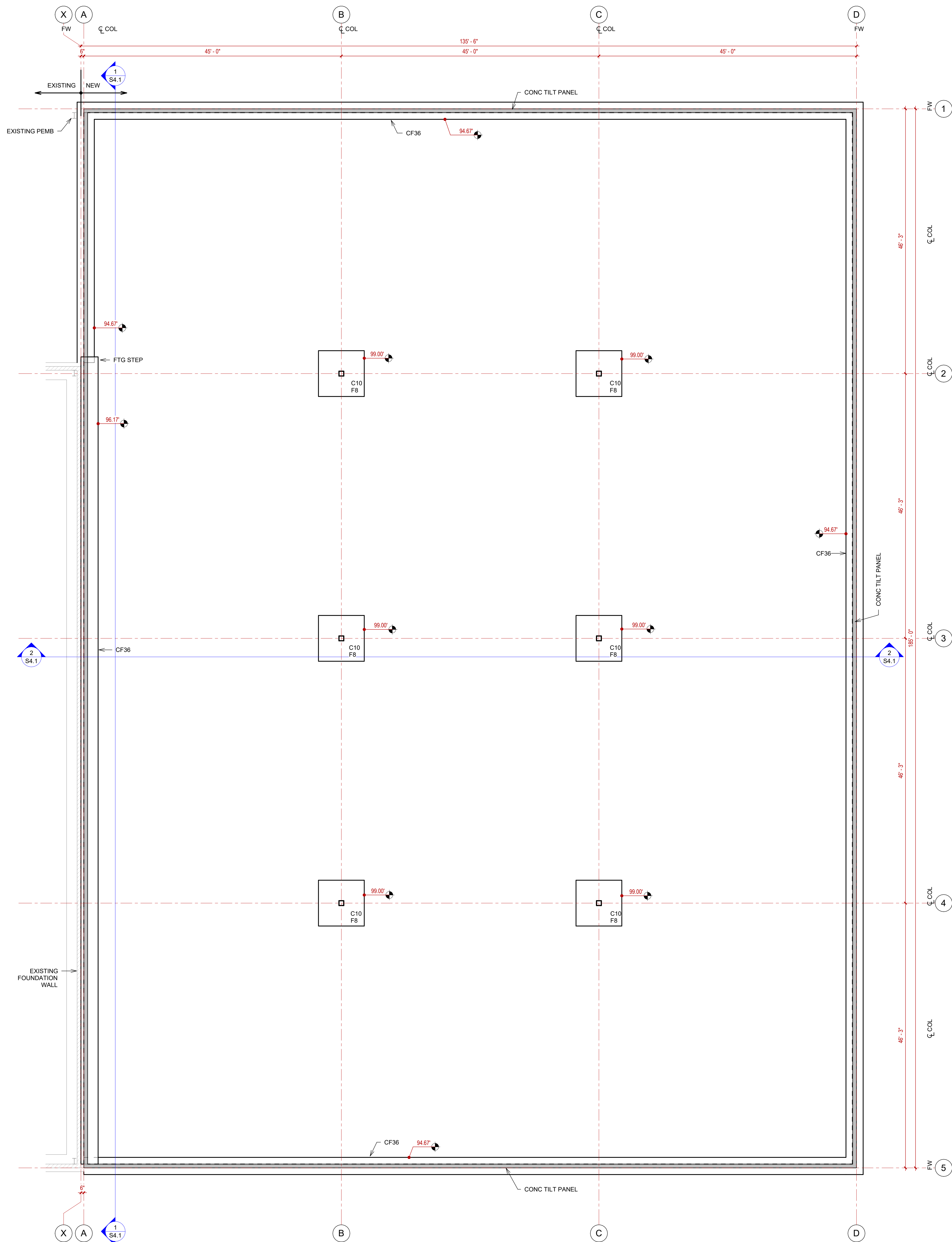
1" = 1'-0"

- PLAN NOTES:
1. FINISH FLOOR ELEVATION (FFE) = 100.0' (ASSUMED)
 2. TOP OF FOOTING (TF) ELEVATION VARIES, SEE PLAN.
 3. TOP OF PEDISTAL (TP) ELEVATION = 99.00'
 4. ALL COLUMNS BEAR AT TF OR TP.

STRUCTURAL STEEL COLUMN SCHEDULE	
COL MARK	SIZE
C10	HSS10X10X1/4

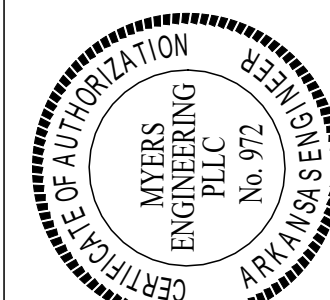
CONTINUOUS FOOTING SCHEDULE			
FTG MARK	WIDTH	THICKNESS	REINFORCEMENT
CF36	3' - 0"	1' - 6"	(4) #4's CONT TOP & BOTTOM w/ #3 STIRRUPS @ 12" O.C.

SPREAD FOOTING SCHEDULE				
FTG MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F8	8' - 0"	8' - 0"	2' - 0"	(9) #5's EACH WAY TOP & BOTTOM



1 FOUNDATION PLAN

1/8" = 1'-0"



SIEMENS EXPANSION

99 BOLTON SULLIVAN DRIVE
HEBER SPRINGS, ARKANSAS

FOUNDATION PLAN

SHEET

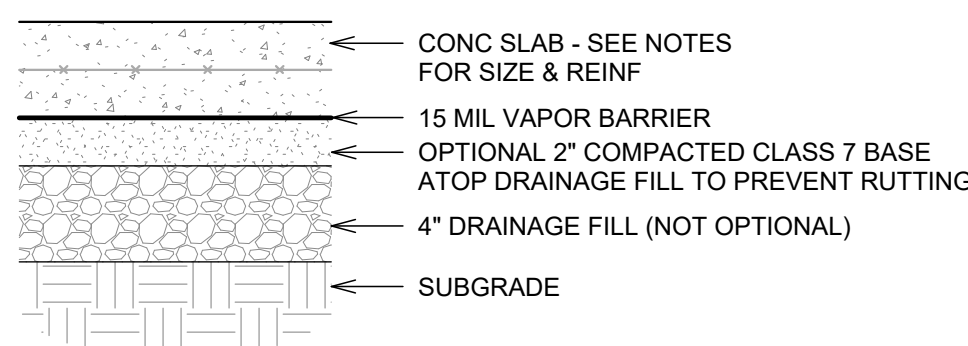
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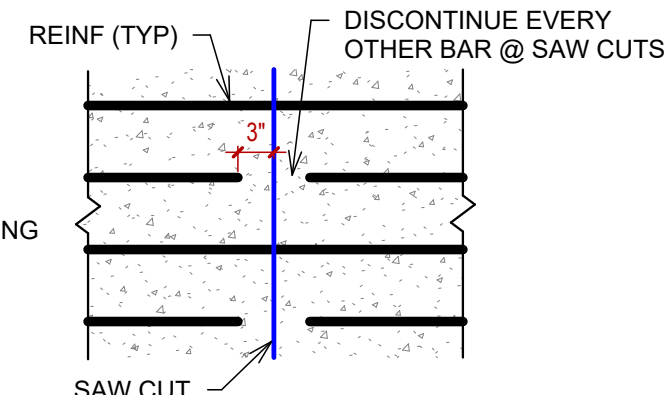
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PROJECT No: ME26063
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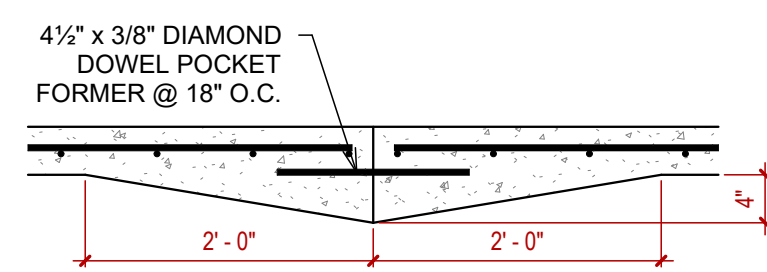
S1.1



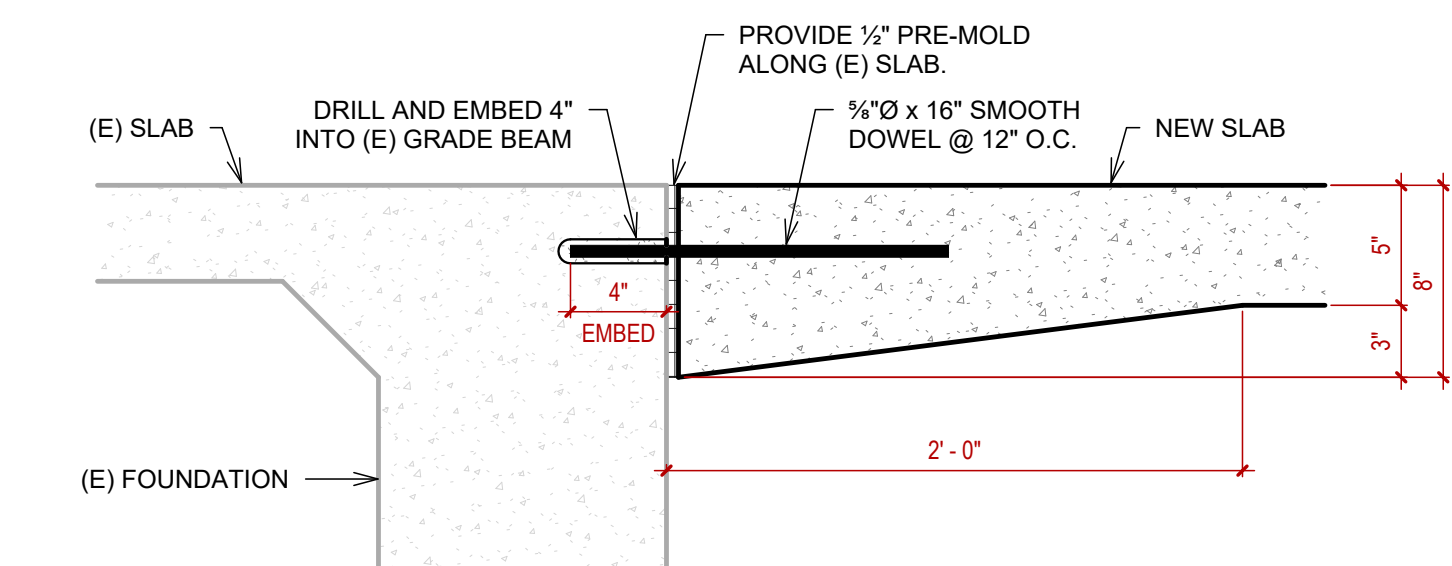
TYP SLAB SECTION



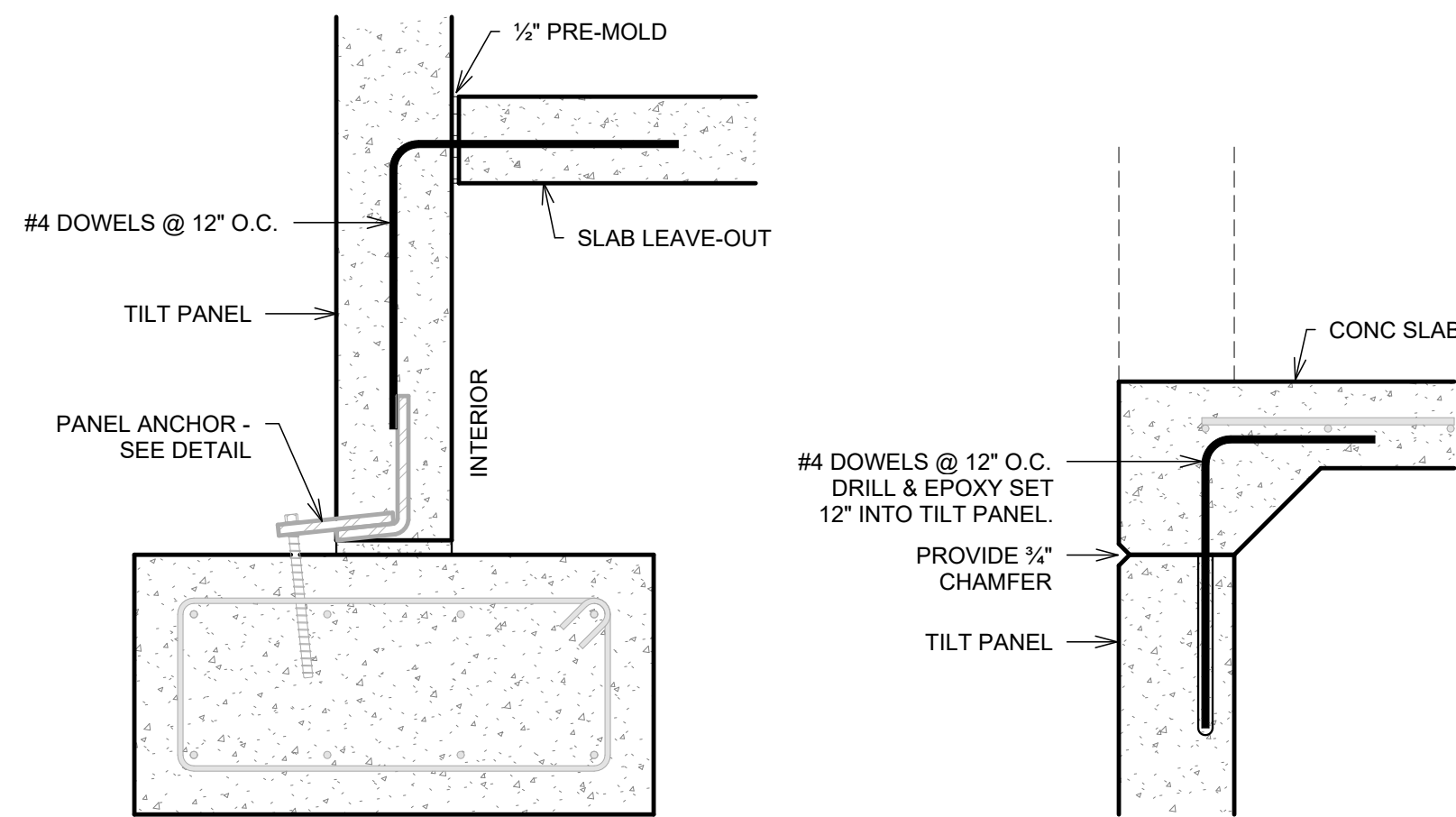
SAW CUT (SC) @ REBAR



CONSTRUCTION JOINT (CJ)



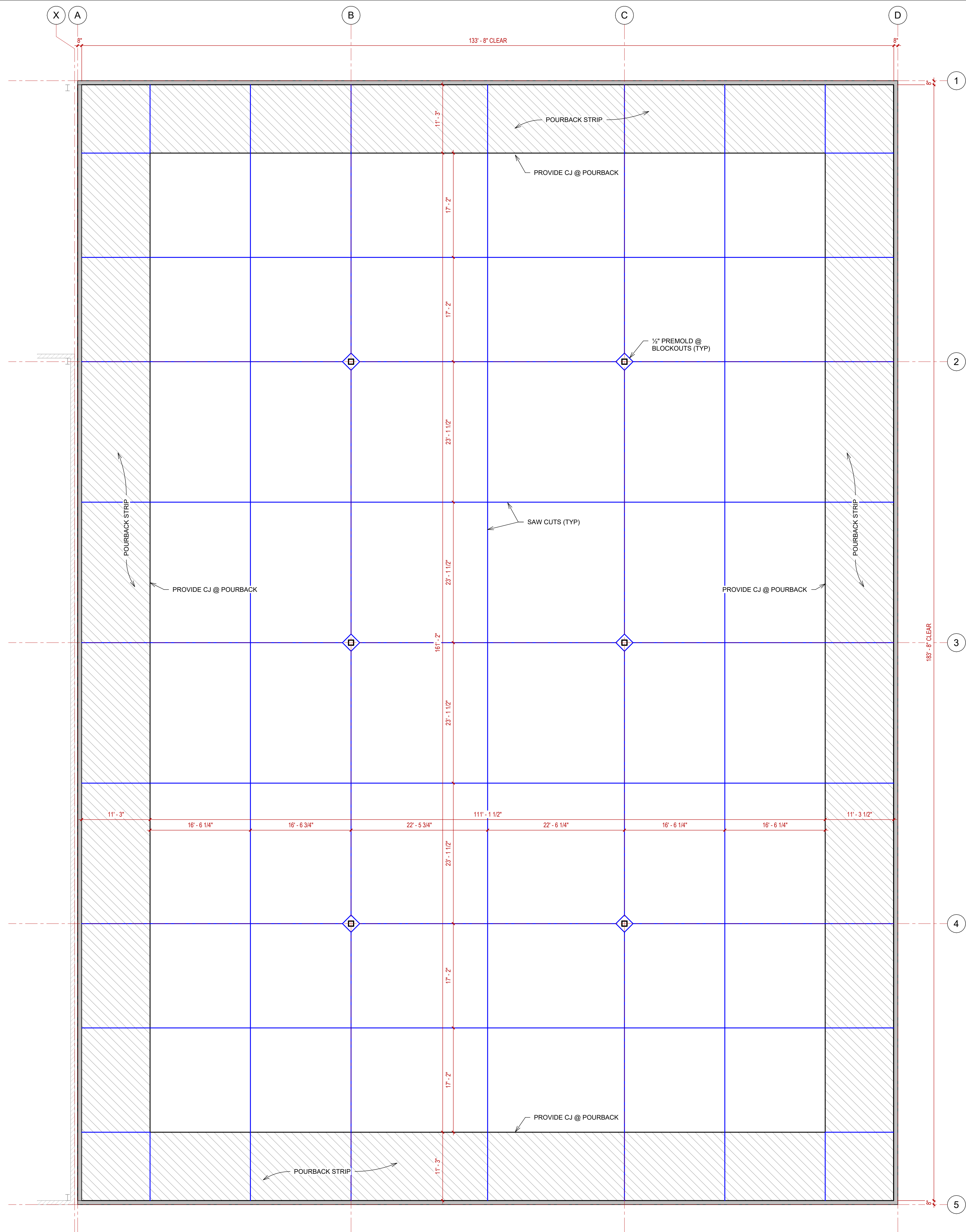
3 SLAB TRANSITION TO (E)
1 1/2" = 1'-0"



SLAB POUR-BACK

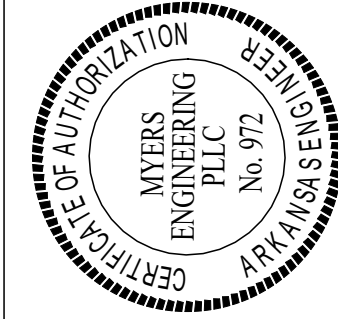
SLAB @ OPENINGS

4 PANEL SLAB EDGE CONDITIONS
1" = 1'-0"



SLAB PLAN NOTES:
1. SLAB TO BE 8" CONCRETE w/ #4's @ 12" O.C. EACH WAY.
2. DISCONTINUE EVERY OTHER BAR AT SAW CUTS. SEE SLAB JOINT DETAILS.

1 SLAB PLAN
1/8" = 1'-0"



SIEMENS EXPANSION

99 BOLTON SULLIVAN DRIVE
HEBER SPRINGS, ARKANSAS

SLAB PLAN

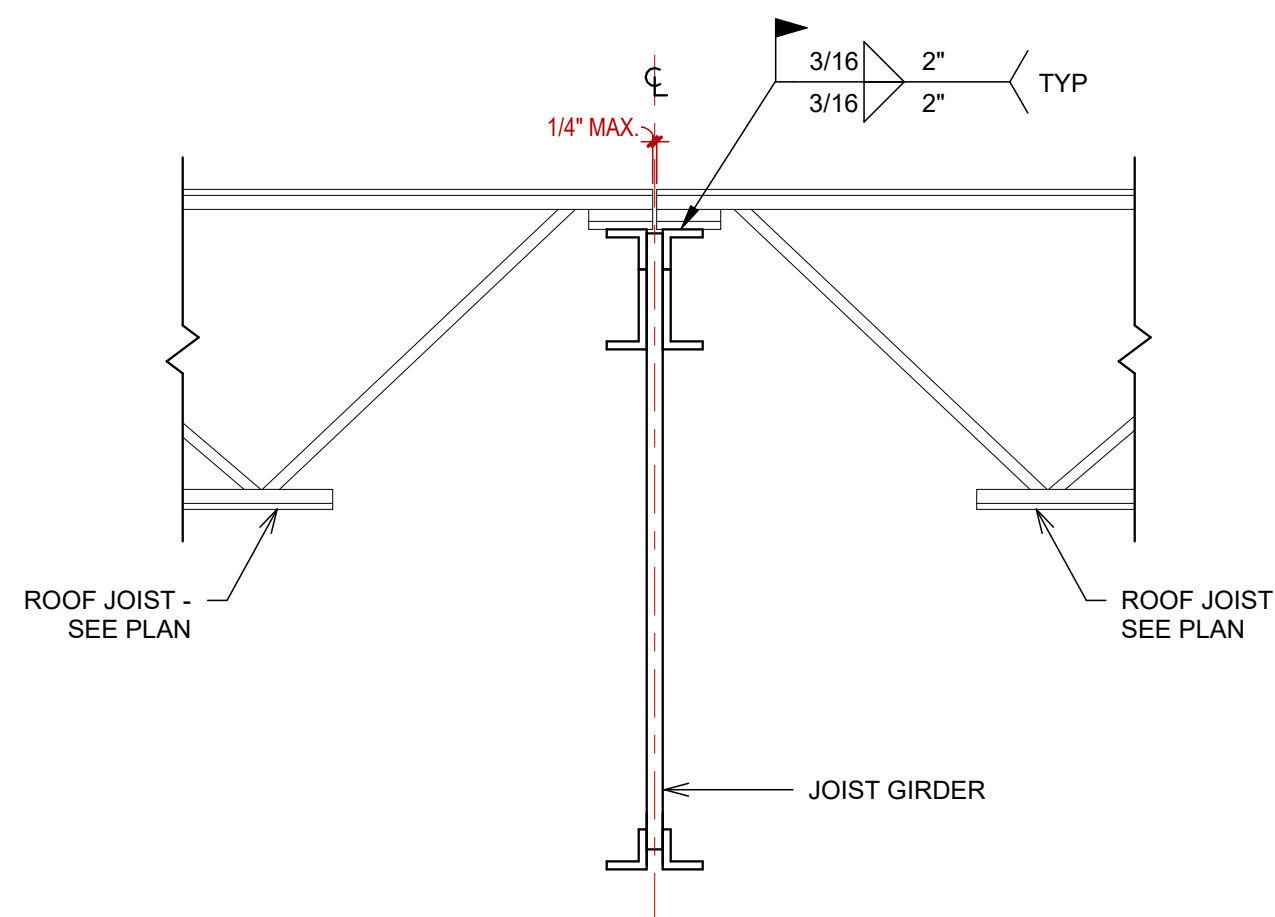
SHEET

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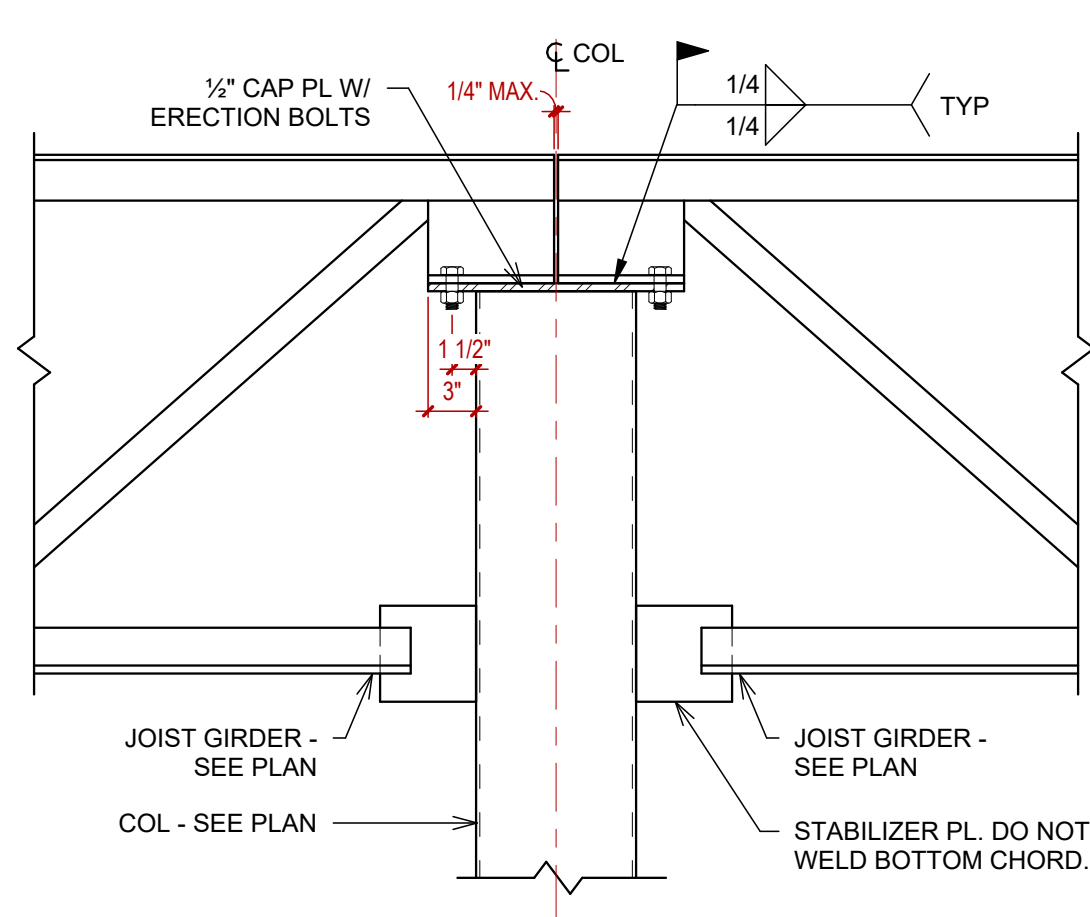
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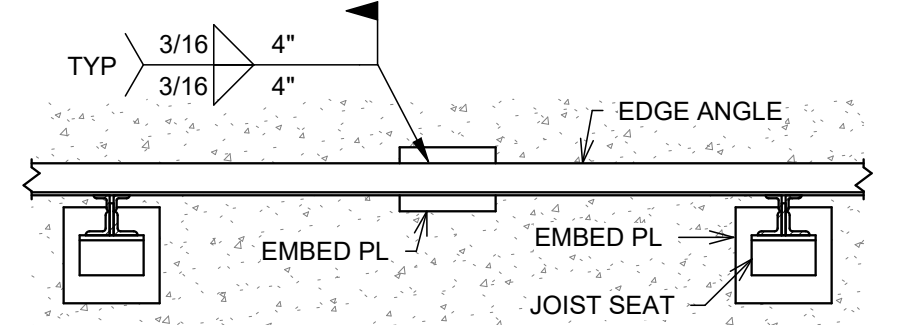
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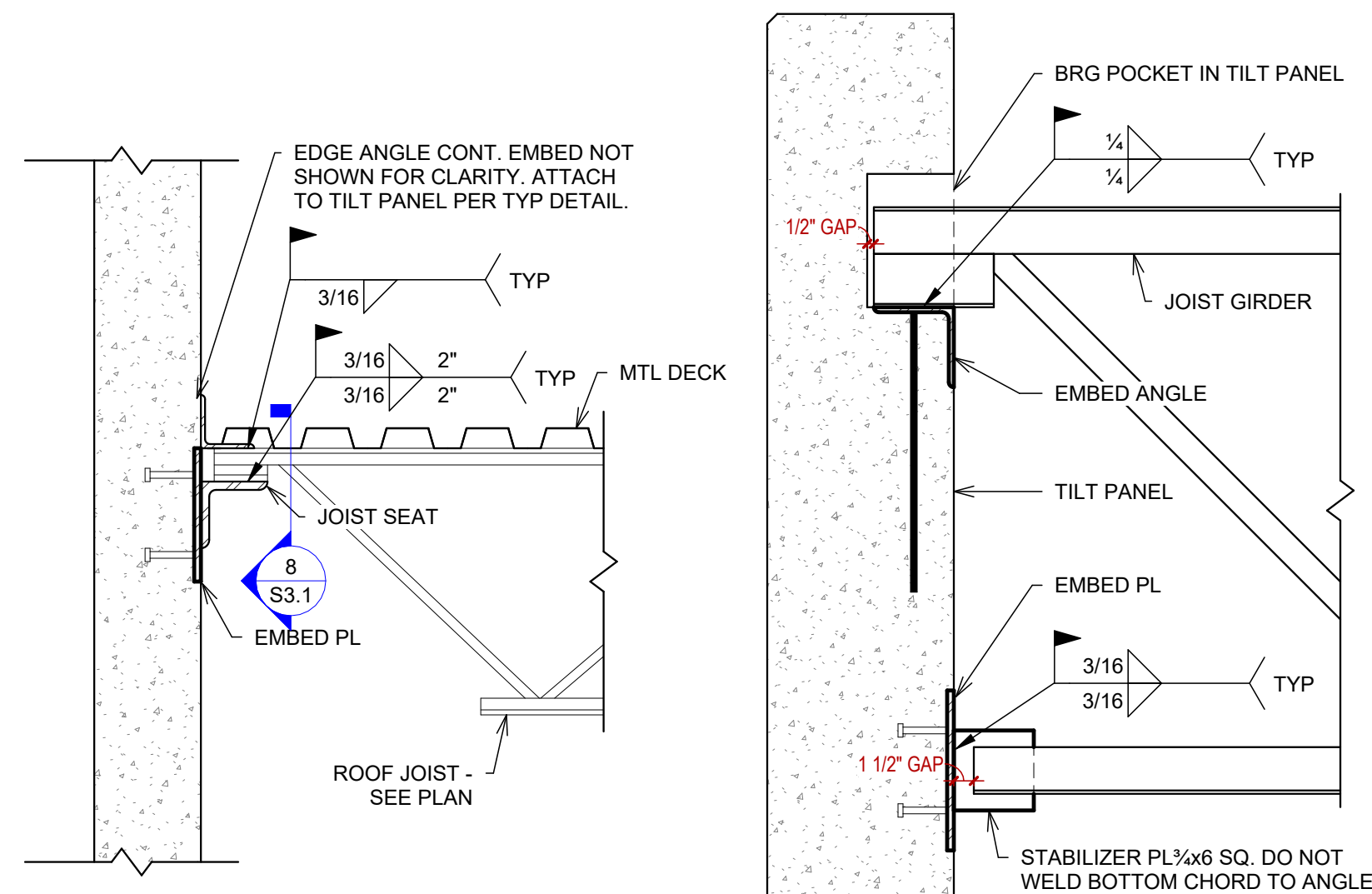
3 JOIST TO JOIST GIRDER
1" = 1'-0"



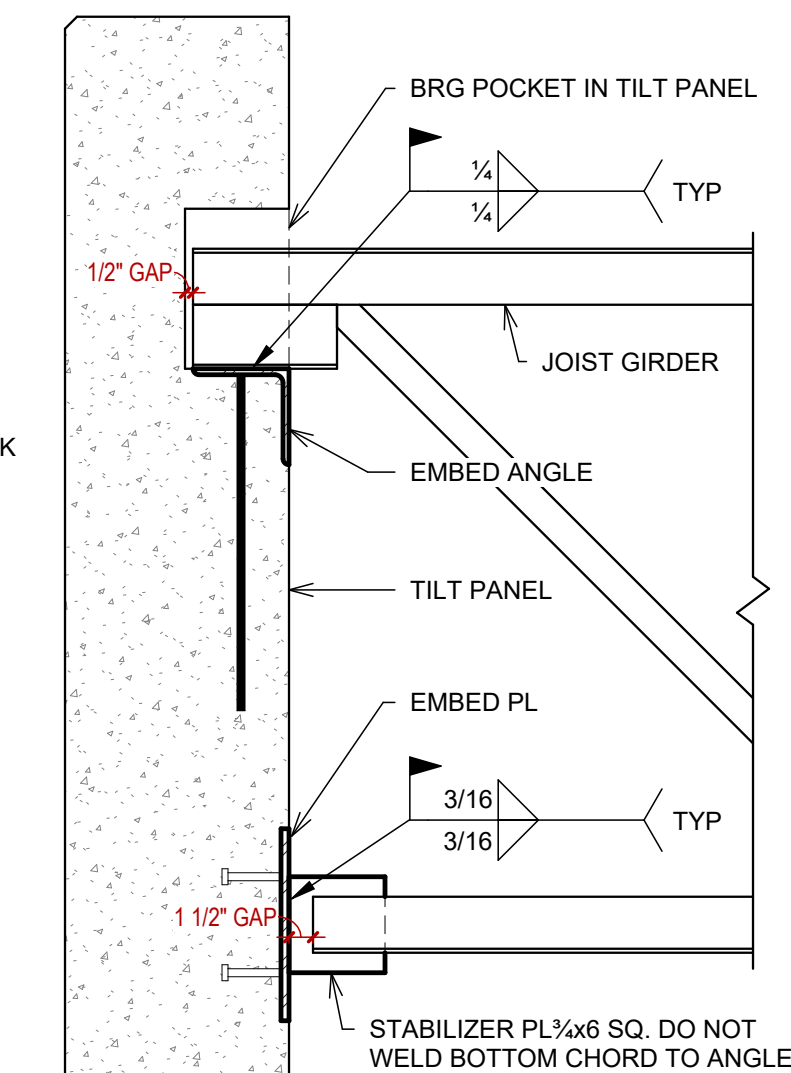
2 JOIST GIRDER TO HSS COL
1" = 1'-0"



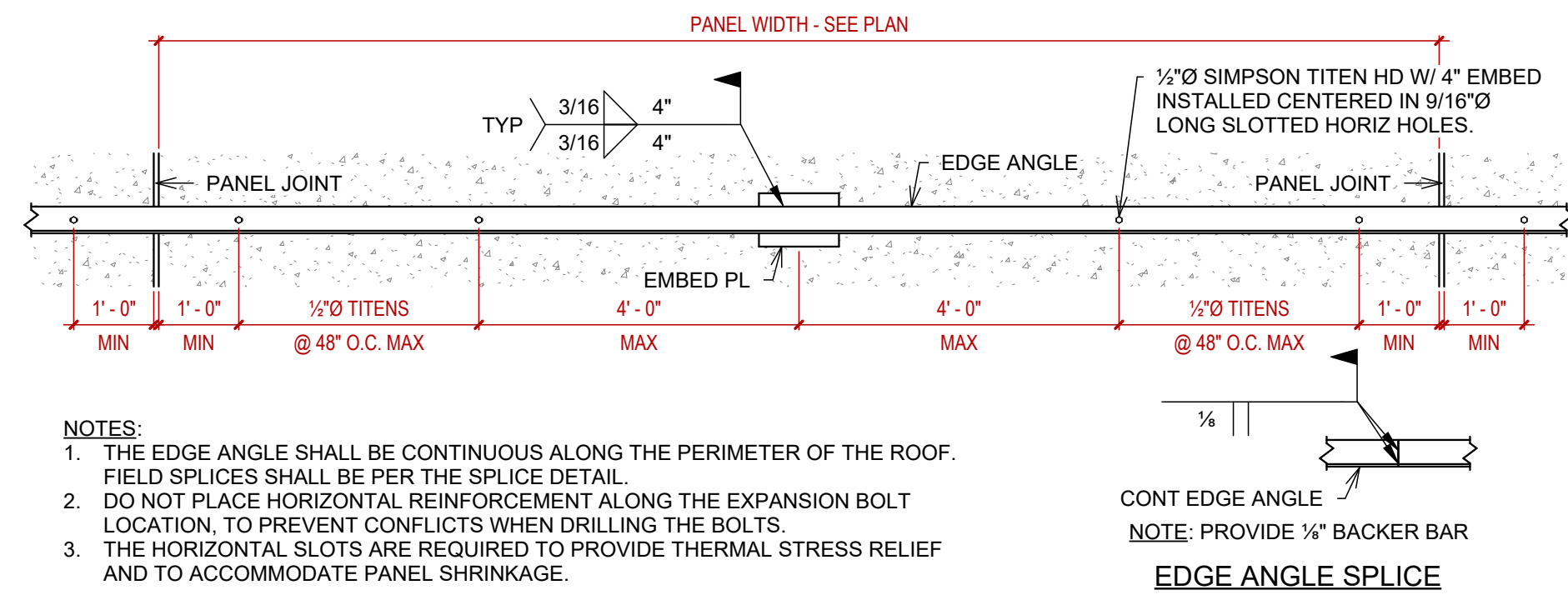
6 EDGE ANGLE TO TILT PANEL
1/2" = 1'-0"



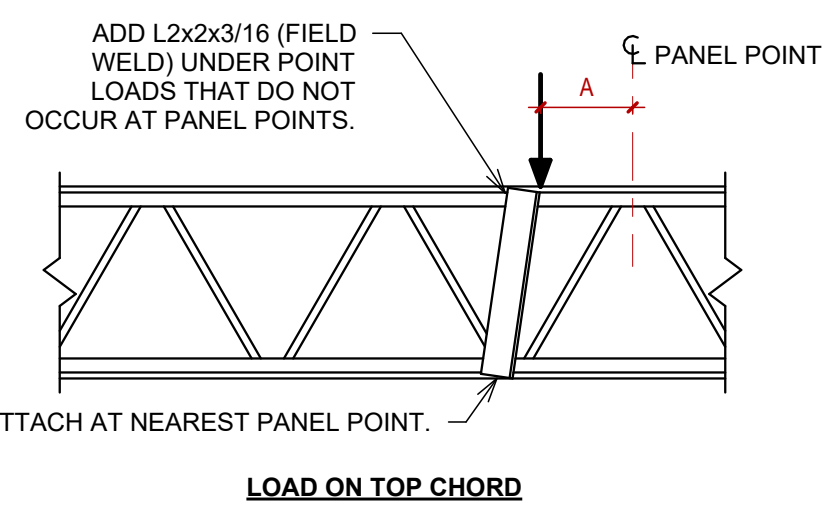
5 ROOF JOIST TO PC PANEL
1" = 1'-0"



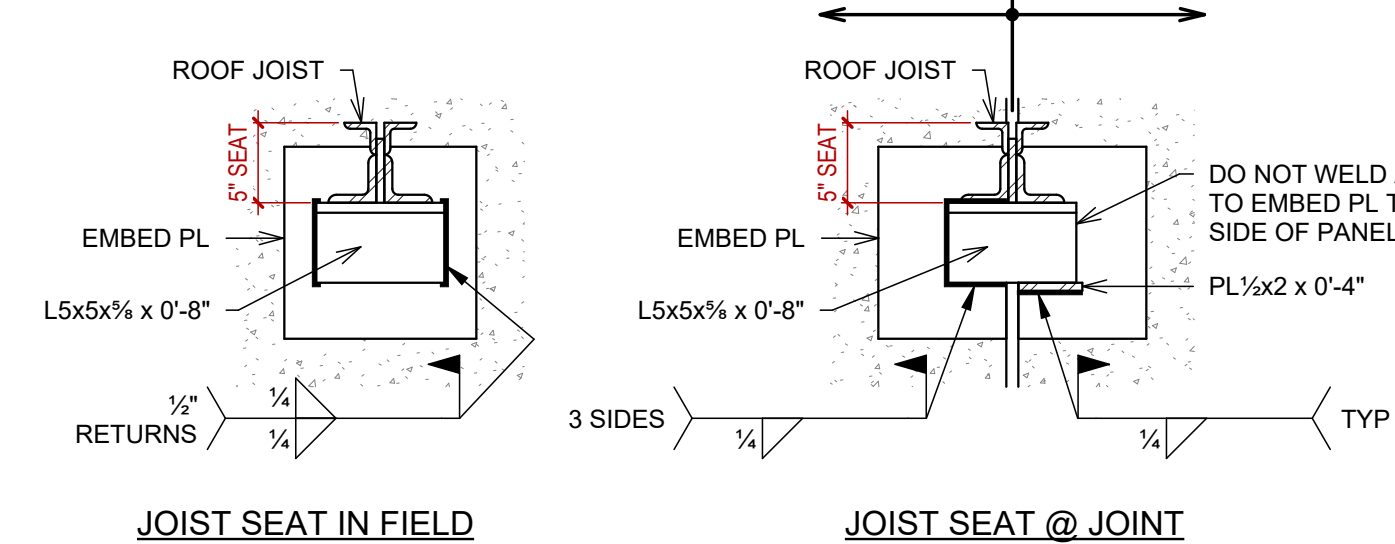
4 JOIST GIRDER BRG @ TILT PANEL
1" = 1'-0"



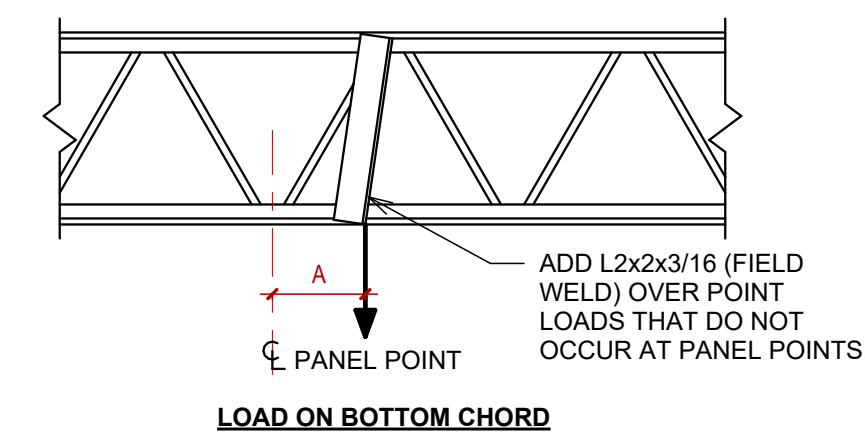
7 EDGE ANGLE TO TILT PANEL
1/2" = 1'-0"



9 PANEL POINT BRACE
1" = 1'-0"



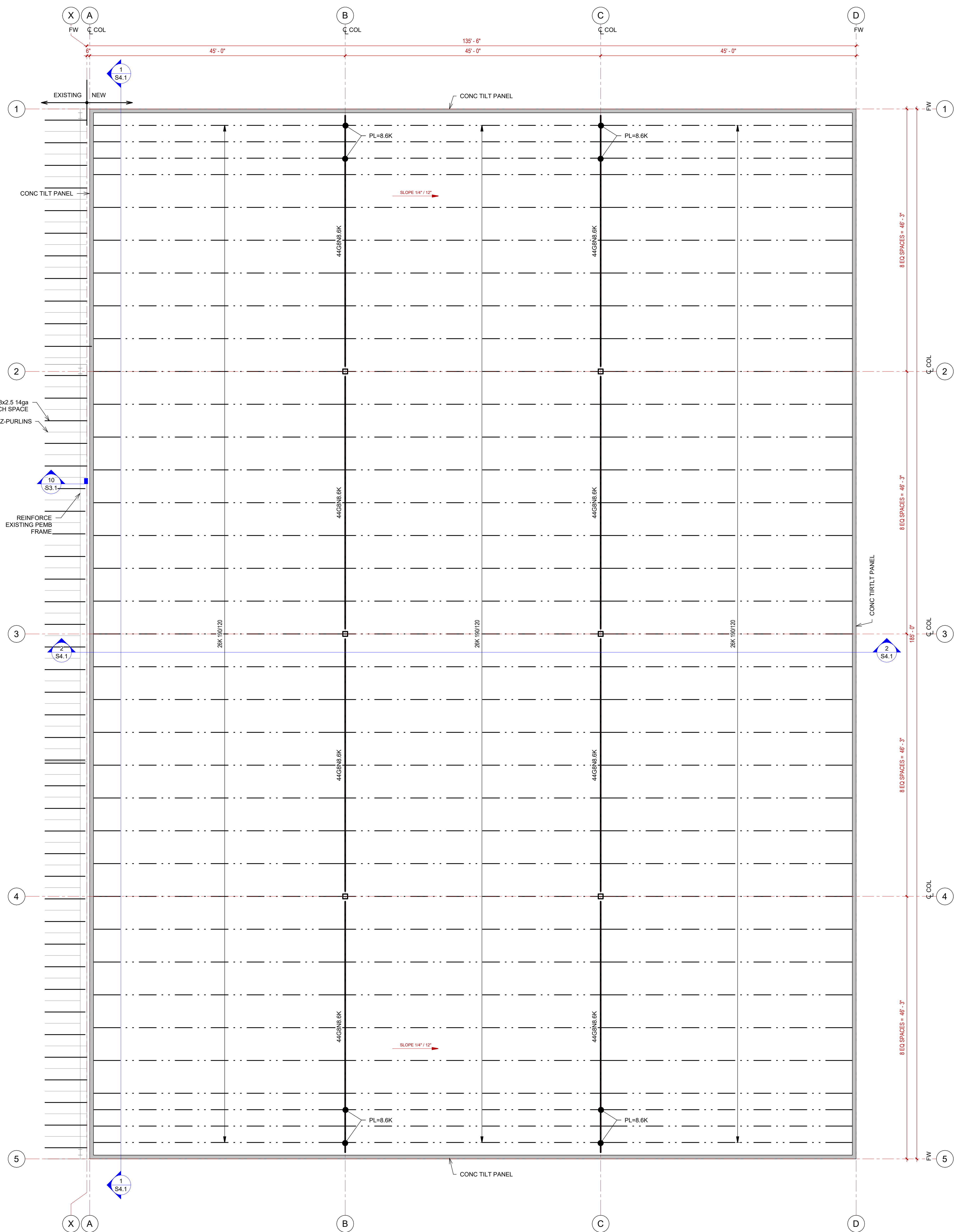
8 JOIST SEAT TO TILT PANEL
1" = 1'-0"



10 PEMB FRAME REINF
1 1/2" = 1'-0"

- PLAN NOTES:
- ALL ROOF JOISTS SHALL HAVE 5" DEEP SEATS.
 - ALL JOIST GIRDERS SHALL HAVE 7/8" DEEP SEATS.
 - SELF-WEIGHT HAS BEEN INCLUDED IN THE JOIST DESIGNATIONS.
 - METAL ROOF DECK TO BE 1.5B 22ga (GALVANIZED) (G60).
 - ATTACH DECK TO SUPPORTS ON A 30/4 PATTERN AND 6" O.C. AT DECK PERIMETERS.
 - SEE METAL DECK NOTES ON S0.1 FOR ACCEPTABLE ROOF DECK FASTENERS.
 - PROVIDE (4) #10 SIDELAP FASTENERS PER SPAN.
 - ROOF EDGE ANGLES TO BE L4x4x1/4.

1 ROOF FRAMING PLAN
1/8" = 1'-0"



MYERS
ENGINEERING
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Van Buren, Arkansas 72956
(479) 474-4412

NABHOLZ
CONSTRUCTION SERVICES

SIEMENS EXPANSION
99 BOLTON SULLIVAN DRIVE
HEBER SPRINGS, ARKANSAS
ROOF FRAMING PLAN

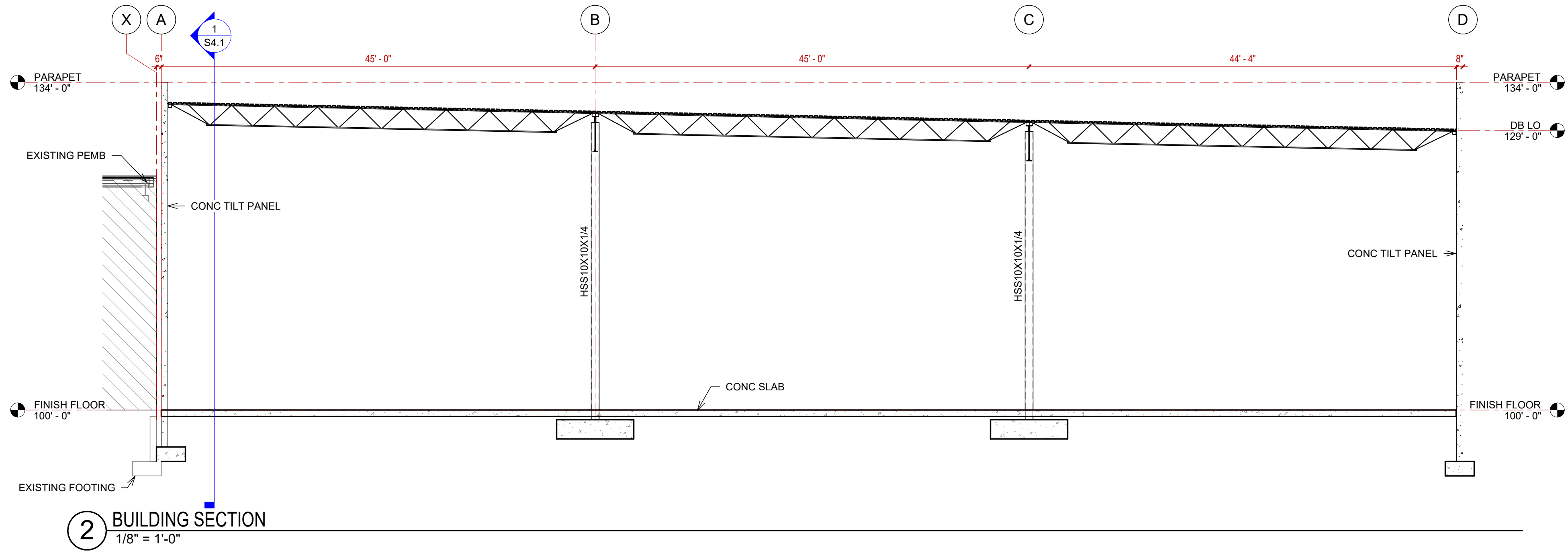
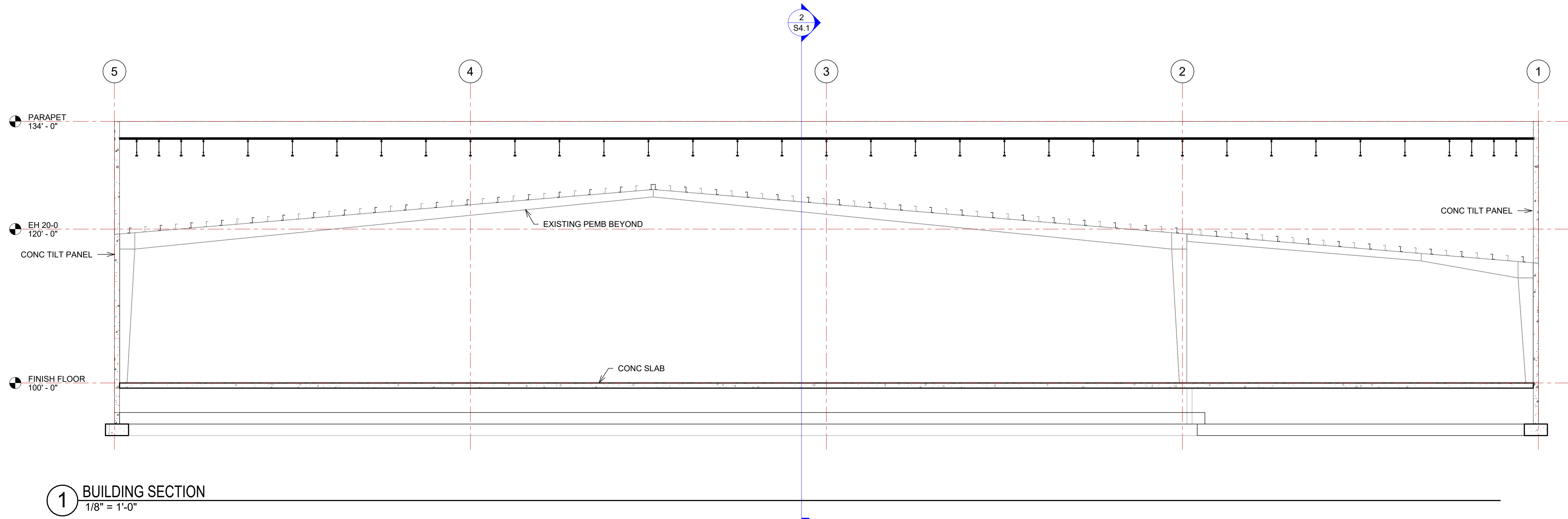
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S3.1



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S4.1

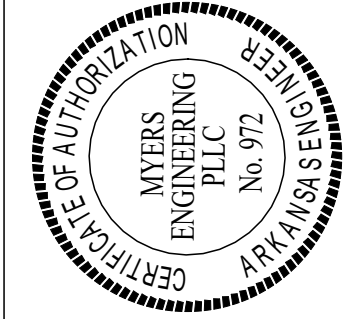
SIEMENS EXPANSION

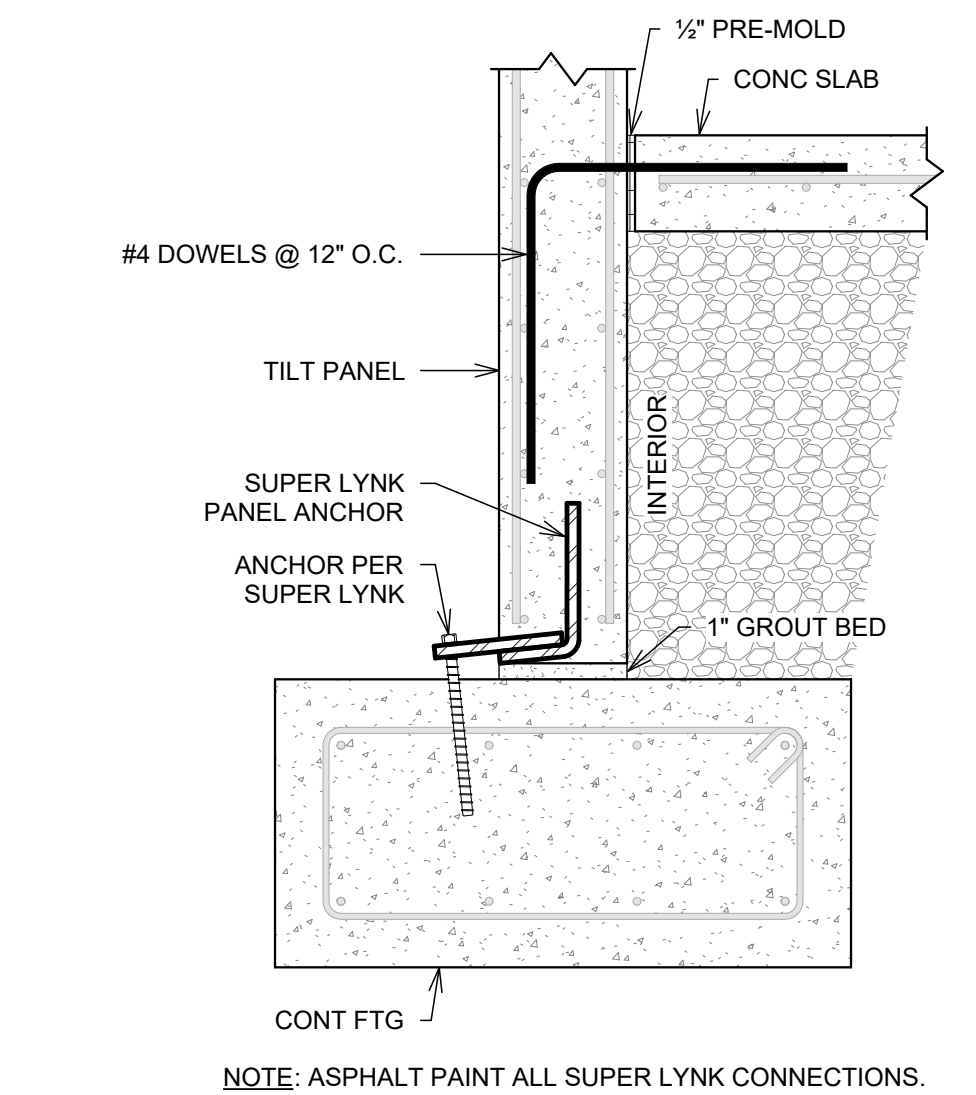
99 BOLTON SULLIVAN DRIVE
HEBER SPRINGS, ARKANSAS

BUILDING SECTIONS

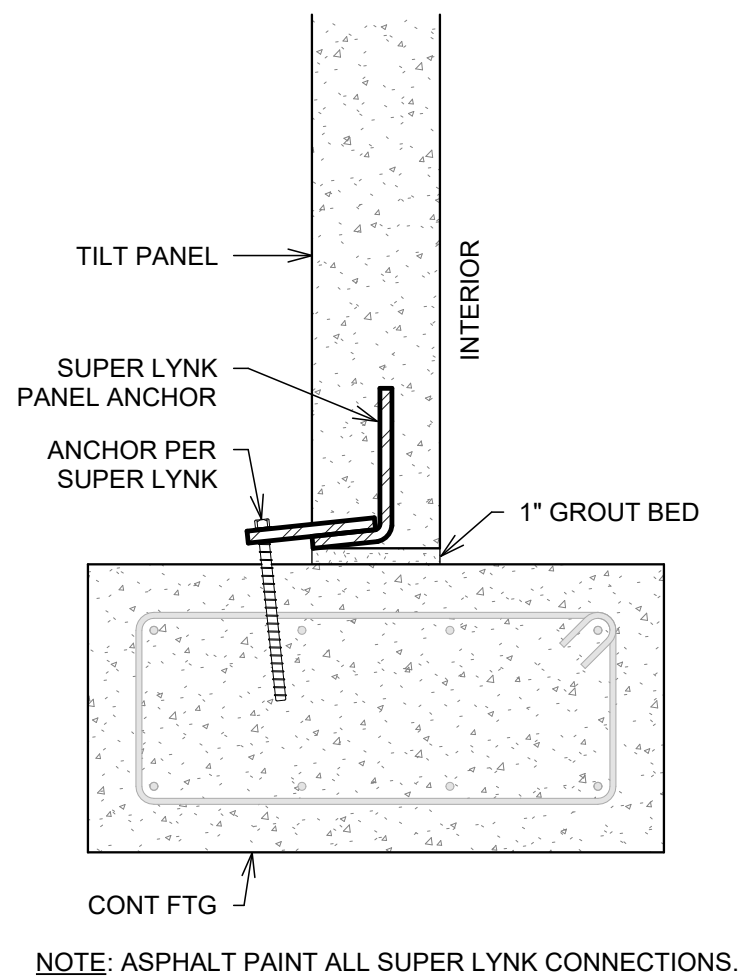
SHEET

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REV #	DATE

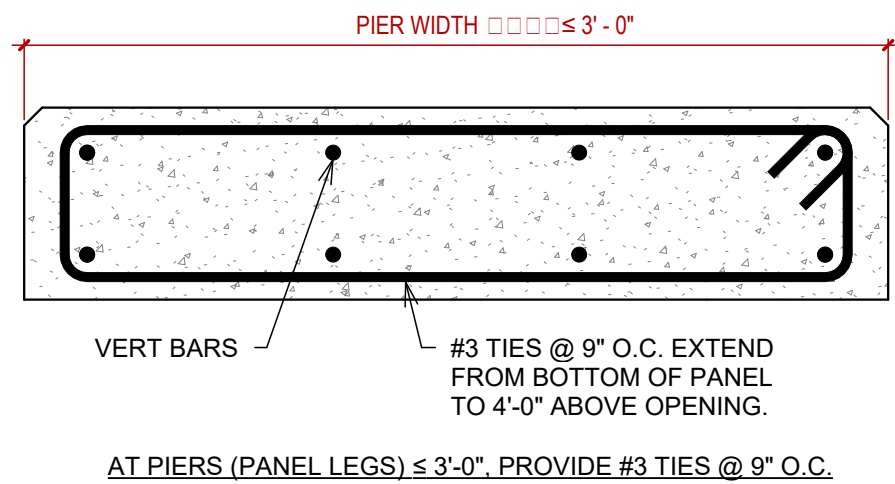




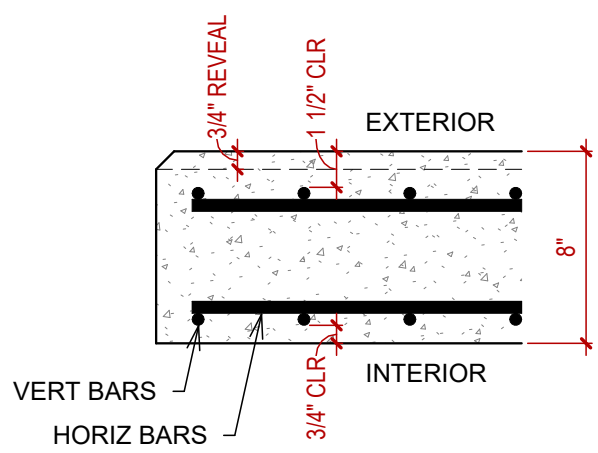
6 TILT PANEL BASE CONX @ DOCK
1" = 1'-0"



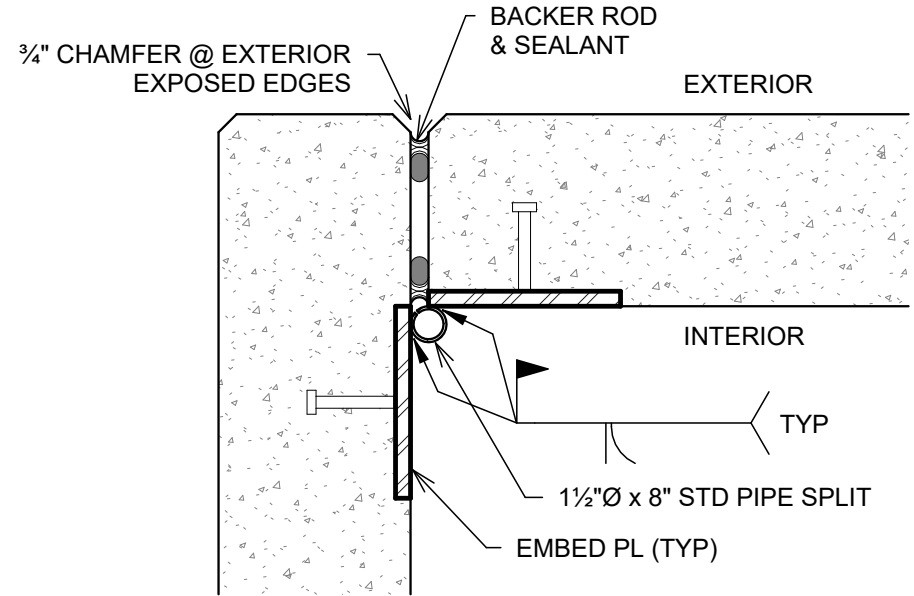
5 TILT PANEL BASE CONX
1" = 1'-0"



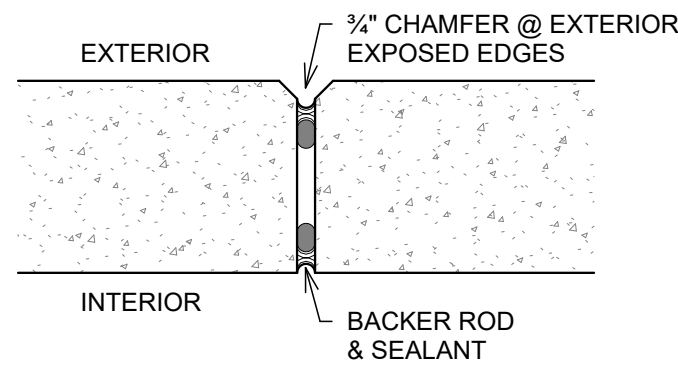
4 TILT PANEL LEG REINF
1 1/2" = 1'-0"



3 TILT PANEL REINF PLACING
1 1/2" = 1'-0"



2 TILT PANEL OUTSIDE CORNER CONX
1 1/2" = 1'-0"

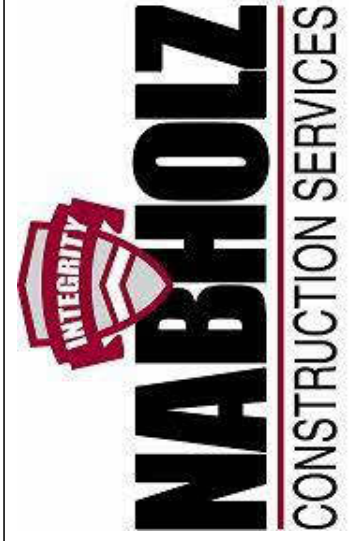


1 TILT PANEL JOINT
1 1/2" = 1'-0"

TILT PANEL NOTES:
1. TYPICAL TILT PANELS ARE 8" CONCRETE.
2. TYPICAL REINFORCEMENT IS #4 @ 12" O.C EACH FACE.
3. PROVIDE #5 CORNER BARS ON EACH FACE OF EACH CORNER.

PRELIMINARY PRICING - NFC

SIEMENS EXPANSION
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HEBER SPRINGS, ARKANSAS



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TILT PANEL DETAILS