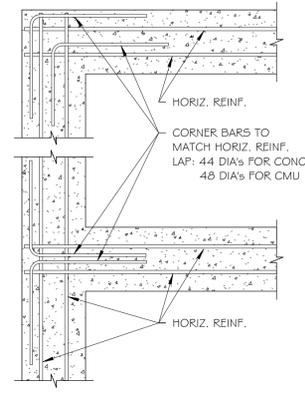
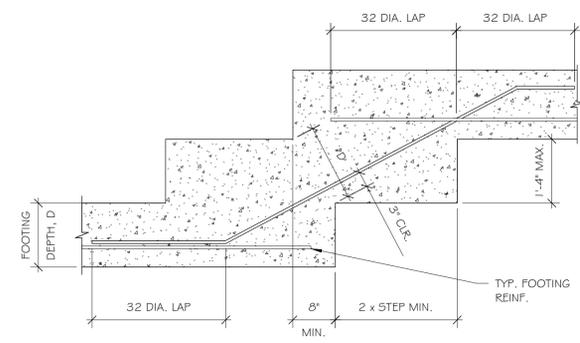


1 TYP. RE-BAR BEND DETAIL
3/4" = 1'-0"



2 TYP. CORNER BARS FOR WALLS AND FTGS.
3/4" = 1'-0"



3 TYPICAL STEPPED FOOTING
3/4" = 1'-0"

- ### STRUCTURAL NOTES
- ALL ELEVATIONS ARE GIVEN WITH REFERENCE TO FINISH FLOOR DATUM 1'00'-0" (MATCH EXISTING).
 - ALL STRUCTURAL SELECT FILL WITHIN THE BUILDING FOOTPRINT, SHALL BE APPROVED MATERIAL PLACED IN 9" LOOSE LIFTS AND COMPACTED TO 98% OF THE MAXIMUM DRY DENSITY PER STANDARD PROCTOR TEST (ASTM D-698). BASE COURSE MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY PER MODIFIED PROCTOR (ASTM D-1557).
 - FOOTINGS ARE TO BEAR IN NATIVE, RELATIVELY UNDISTURBED MEDIUM DENSE TO VERY DENSE STRATUM III GRAVELS AND CHERT. ALL EXCAVATIONS SHALL BE CLEAN CUT AND SQUARE.
 - FOOTING ELEVATIONS NOTED ARE FOR BIDDING PURPOSES AND SHALL BE LOWERED AS NECESSARY TO REACH THE SPECIFIED BEARING STRATA AND COVER.
 - THE FOUNDATION IS DESIGNED FOR A BEARING CAPACITY OF 4000 PSF FOR CONTINUOUS FOOTINGS AND 5200 PSF FOR ISOLATED COLUMN FOOTINGS BASED ON GEOTECHNICAL REPORT DATED AUGUST 26, 2016 BY GTS, INC. PROVIDE 18" MIN. FROM FINISH GRADE OR PAVING TO BOTTOM OF FOOTING AT EXTERIOR CONDITIONS. FOOTING EXCAVATIONS SHALL BE APPROVED BY OWNER'S GEOTECHNICAL CONSULTANT PRIOR TO PLACING RE-BAR.
 - CONCRETE SHALL HAVE A 28 DAY STRENGTH. MAXIMUM SLUMP AND MAXIMUM AGGREGATE SIZE AS FOLLOWS:
FOOTINGS AND STEMWALLS: 3000 PSI - 6" SLUMP - 1 1/2" AGG.
INTERIOR SLAB: 3000 PSI - 4" SLUMP - 1" AGG.
EXTERIOR SLAB: 3500 PSI - 4" SLUMP - 1" AGG.

ALL CONCRETE EXPOSED TO FREEZE/THAW SHALL HAVE APPROX. 5% AIR ENTRAINMENT. REINF. STEEL SHALL CONFORM TO GRADE 60 ASTM A-615. LAP ALL CONCRETE REINFORCING 44 BAR DIAMETERS, 1/2" MINIMUM, UNLESS NOTED OTHERWISE. LAP ALL UNDER-SLAB VAPOR BARRIER SHEETS A MINIMUM OF 6" AT ALL SPLICES.

VERIFY ALL DIMENSIONS, SLOPES, DEPRESSIONS, EMBEDMENTS ETC. BEFORE PLACING CONCRETE.

PROVIDE SLAB CONTROL JOINTS (CJ) WHERE INDICATED ON PLAN. C.J.'S SHALL BE APPROX. 1/4 OF THE SLAB DEPTH AND SHALL BE ONE OF THE FOLLOWING:
A. SAWCUT AS SOON AS POSSIBLE (WITHIN 12 HOURS OF POUR).
B. ZIP CAP TYPE JOINT FORMER.
C. TOOLED JOINTS FOR EXTERIOR SLAB.

CONST. JOINTS W/ 3/8"x4 1/2"x4 1/2" FNA DIAMOND DOWELS AT 18" O.C. MAY BE SUBSTITUTED FOR ANY CONTROL JOINT AND SHALL BE USED WHERE INDICATED ON THE PLAN.

STRUCTURAL STEEL SHALL MEET THE FOLLOWING CRITERIA:
WIDE FLANGE SHAPES: ASTM A-992, 50 KSI
SHAPES, BARS AND PLATES: ASTM A-36, 36 KSI
STRUCTURAL PIPE (HSS): ASTM A-500, 42 KSI
STRUCTURAL TUBING (HSS): ASTM A-500, 46 KSI

ALL BOLTED STEEL CONNECTIONS SHALL BE MADE USING ASTM A-325H H.S. BOLTS UNLESS NOTED OTHERWISE. CONCRETE ANCHOR BOLTS SHALL BE F1554 (GR. 36) UNLESS NOTED OTHERWISE. WELDING SHALL MEET LATEST AWS CODE. E70 ELECTRODES. GROUT BENEATH COLUMN BASE PLATES SHALL BE NON-METALLIC MEETING ASTM C-827, 5000 PSI AT 28 DAYS.

OPEN WEB JOISTS: DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS SHALL CONFORM WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE SPECIFICATION. USE "SPECIAL" JOISTS AT ROOF TOP UNITS AND OTHER LOCATIONS WHERE ADDITIONAL LOADS ARE SHOWN ON THE FRAMING PLAN. REFER TO FRAMING PLAN FOR WEIGHTS AND LOCATIONS. ROOF TOP UNIT WEIGHTS AND WEIGHTS NOTED ON FRAMING PLAN MUST BE ADDED TO THE TOTAL LOAD ON THE JOISTS. COORDINATE WITH MECHANICAL AND ARCH. DRAWINGS. WELD EACH JOIST TO BEAM, JOIST GIRDER OR WELD PLATE WITH A 1/8" FILLET WELD EACH SIDE OF JOIST. WELD LENGTH SHALL BE A MINIMUM OF 2" UNLESS NOTED OTHERWISE. USE BRIDGING AS INDICATED ON THE DRAWINGS OR AS REQUIRED BY THE STEEL JOIST INSTITUTE. CONTINUE ALL BRIDGING TO ROLLED STEEL SHAPES AND/OR WALLS WHICH ARE PARALLEL TO THE JOISTS AND ANCHOR IN ACCORDANCE WITH STEEL JOIST INSTITUTE SPECIFICATIONS. ALL BRIDGING SHALL BE SECURED TO TOP AND BOTTOM OF ALL JOISTS AND BEAMS AND SHALL BE IN ACCORDANCE WITH STEEL JOIST INSTITUTE SPECIFICATIONS. JOISTS SHALL BEAR 4" MINIMUM ON MASONRY AND 2-1/2" MINIMUM ON STEEL. PROVIDE ANCHORAGE AT MASONRY BEARING IN ACCORDANCE WITH STEEL JOIST INSTITUTE SPECIFICATIONS. JOIST MANUFACTURER MUST CHECK ROOF JOISTS AND JOIST GIRDERS FOR A 20 PSF NET UPLIFT PRESSURE (30 PSF AT 14 FOOT WIDE ZONE AT ROOF PERIMETER), AND PROVIDE BRIDGING AND BOTTOM CHORD EXTENSIONS AS REQUIRED TO ADEQUATELY BRACE THE BOTTOM CHORDS AGAINST LATERAL MOVEMENT.

CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS (DIMENSIONS, ELEVATIONS, FTG. LOCATION AND SIZE, ETC.). FIELD VERIFICATION OF ALL EXISTING CONDITIONS SHALL OCCUR PRIOR TO FABRICATION OF ANY NEW STRUCTURAL BUILDING MATERIALS. SEE PLAN AND DETAILS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.

- ### STRUCTURAL NOTES CONT.
- ALL COLD-FORMED STEEL FRAMING SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE WITH THE LATEST EDITION OF "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" BY THE AMERICAN IRON AND STEEL INSTITUTE. STEEL FOR 1 1/2" x 2" GAGE STUDS SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI. STEEL FOR ALL 18 AND 20 GAGE STUDS, JOISTS, AND FOR ALL GAGE TRACK, ACCESSORIES AND BRIDGING SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI. STEEL SHALL BE GALVANIZED WITH A MINIMUM G-60 COATING.
 - ALL STUDS SHALL BE SECURELY SEATED FOR FULL END BEARING ON TOP AND BOTTOM TRACK, UNLESS NOTED OTHERWISE. PROVIDE DOUBLE STUDS AT ALL JAMBS, CORNERS AND INTERSECTIONS. BRIDGING SHALL BE INSTALLED PER RECOMMENDATIONS WITH THE FOLLOWING MINIMUM REQUIREMENTS: PROVIDE BRIDGING AT MID-HEIGHT FOR WALLS LESS THAN OR EQUAL TO 10'-0" HIGH, AND 4'-0" O.C. MAXIMUM FOR WALLS GREATER THAN 10'-0" HIGH. IN ADDITION, BRIDGING SHALL BE PROVIDED AT ROOF LINES AND ELSEWHERE AS NOTED ON THE DRAWINGS. SOLID BLOCKING SHALL BE INSTALLED IN LIEU OF BRIDGING WHERE NOTED ON THE DRAWINGS.
 - STEEL DECKING: ROOF DECK (U.N.O.) SHALL BE 1 1/2" x 22" GAGE TYPE B PAINTED. CONNECT TO ALL SUPPORTS W/ 5/8" DIA. PUDDLE WELDS AT 6" O.C. (6' O.C. AT ALL BOUNDARY ANGLES) (36/7 PATTERN). CONNECT SIDELAPS W/ (4)-#10 TKS PER SPAN.
 - PRE-ENGINEERED LT. GA. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
TOP CHORD: DEAD LOAD = 10 PSF
LIVE LOAD/SNOW LOAD = 20 PSF
SNOW DRIFT = AS REQUIRED
BOTTOM CHORD: DEAD LOAD = 10 PSF

DEAD LOADS INCLUDE TRUSS SELF WEIGHT, CAMBER TRUSSES FOR 1 1/2" x 22" DEAD LOAD DEFLECTION. SUBMIT TRUSS DESIGN DOCUMENTS, INCLUDING LAYOUT, STAMPED BY AN ARKANSAS P.E. TO ARCHITECT FOR REVIEW PRIOR TO FABRICATION. INSTALL TRUSSES ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS AND RECOMMENDATIONS INCLUDING BRIDGING.

SIMPSON STRONG-TIE PRODUCTS AND HILTI PRODUCTS (OR EQUIVALENT EQUALS) SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

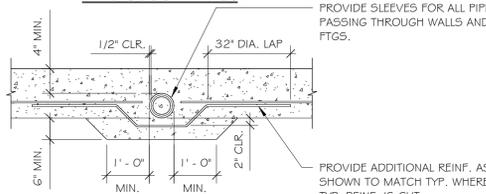
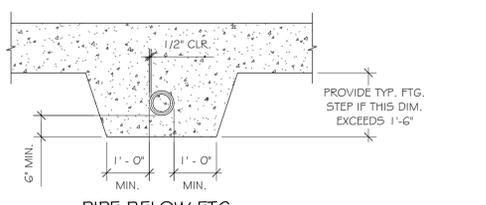
DESIGN CRITERIA
CODE: 2021 ARKANSAS FIRE PREVENTION CODE
ROOF LIVE LOAD: 21 PSF
ROOF DEAD LOAD: 20 PSF

SNOW LOAD
GROUND SNOW LOAD, P_g: 20 PSF
FLAT ROOF SNOW LOAD, P_f: 14 PSF
SNOW EXPOSURE FACTOR, C_e: 1.0
SNOW LOAD IMPORTANCE FACTOR, I_s: 1.0
THERMAL FACTOR, C_t: 1.0
DESIGN SNOW LOAD: 21 PSF

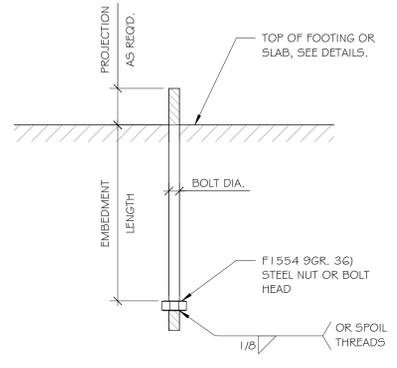
WIND LOAD
DESIGN WIND SPEED, V_{ult}: 107 MPH
DESIGN WIND SPEED, V_{asd}: 90 MPH
RISK CATEGORY: II
WIND EXPOSURE CATEGORY: C
INTERNAL PRESSURE COEFF.: +0.18, -0.18
COMPONENTS & CLADDING (ASD): 20.4 PSF

SEISMIC LOAD
RISK CATEGORY: II
IMPORTANCE FACTOR, I_e: 1.00
MAPPED SPECTRAL RESPONSE COEFF.: S_s=0.155, S₁=0.090
DESIGN SPECTRAL RESPONSE COEFF.: S_{ds}=0.134, S_{d1}=0.090
SITE CLASS: C
DESIGN CATEGORY: B
BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC
DESIGN BASE SHEAR: V=0.0448W
SEISMIC RESPONSE COEFF., C: 0.0448
RESPONSE MODIFICATION COEFF., R: 3.0
ANALYSIS PROCEDURE: EQUIV. LATERAL FORCE

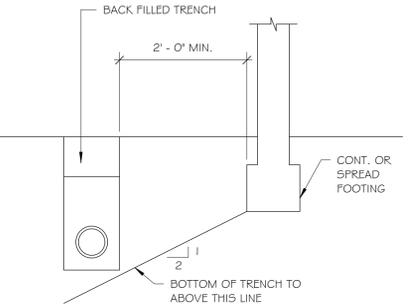
THIS DESIGN COMPLIES WITH THE ARKANSAS SEISMIC STANDARDS.



4 PIPE PERPENDICULAR TO FTG.
1/2" = 1'-0"



5 TYPICAL ANCHOR BOLT
3/4" = 1'-0"



6 TRENCHES PARALLEL TO FTG.
3/4" = 1'-0"

SPECIAL INSPECTIONS

STRUCTURAL SPECIAL INSPECTIONS ARE REQUIRED PER SECTION 1705 OF THE 2021 ARKANSAS FIRE PREVENTION CODE. REFER TO SHEETS S7 AND S8 FOR THE FOLLOWING INFORMATION REGARDING THE REQUIREMENTS OF SPECIAL INSPECTIONS:

- THE MATERIALS, SYSTEMS, COMPONENTS AND WORK REQUIRED TO HAVE SPECIAL INSPECTION OR TESTING BY THE BUILDING OFFICIAL OR BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR EACH PORTION OF THE WORK.
- THE TYPE AND EXTENT OF EACH SPECIAL INSPECTION.
- THE TYPE AND EXTENT OF EACH TEST.
- ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION OR TESTING FOR WIND OR SEISMIC RESISTANCE AS SPECIFIED IN SECTIONS 1705.10, 1705.11 AND 1705.12. (WHEN APPLICABLE)
- FOR EACH TYPE OF SPECIAL INSPECTION, IDENTIFICATION AS TO WHETHER IS WILL BE CONTINUOUS SPECIAL INSPECTION OR PERIODIC SPECIAL INSPECTION.

JLA ENGINEERS INC. DOES NOT PERFORM SPECIAL INSPECTIONS.

SPECIAL INSPECTIONS

1/8" = 1'-0"

EXTERIOR HEADER SCHEDULE

MARK	HEADER SIZE	JAMB	"BOX" MEMBERS	HEADER/JAMB CONN.	SILL MEMBER	NOTES
H1	(2)-800S162-68 [1 1/4 GA.]	(2)-600S162-54 [1 1/4 GA.]	600T150-54 [1 1/4 GA.] T.4B.	SHHG/54 (C2)	600T150-54 [1 1/4 GA.]	1,2,3,4
H2	(2)-800S162-54 [1 1/4 GA.]	(2)-600S162-54 [1 1/4 GA.]	600T150-54 [1 1/4 GA.] T.4B.	SHHG/54 (C2)	N/A	1,2,3
H3	H5510x6x3/8	N/A	N/A	N/A	N/A	5,6
H4	(2)-800S162-54 [1 1/4 GA.]	(1)-600S162-54 [1 1/4 GA.]	600T150-54 [1 1/4 GA.] T.4B.	SHHG/54 (C1)	600T150-54 [1 1/4 GA.]	1,2,3,4
H5	(2)-1200S162-97 [1 1/2 GA.]	(3)-600S162-68 [1 1/4 GA.]	600T200-68 [1 1/2 GA.] T.4B.	SHHG/54 (D2)	600T200-97 [1 1/2 GA.]	1,2,3,7

NOTES:
1. SHHG/54 CONNECTION EA. SIDE OF HEADER AND AT EA. JAMB
2. PROVIDE L5x5x5/16 BRICK SUPPORT WHERE APPLICABLE. CONN. TO LT. GA. HEADER W/ (2)-#12 SCREWS AT 4" O.C. AND EXTEND ANGLE 8" EA. END TO BEAR ON VENEER.
3. ALL "BOX" MEMBERS SHALL BE 50 KSI
4. PROVIDE SIMPSON L550 CLIP (ONE EA. END OF SILL MEMBER) FOR SILL CONN. TO JAMB.
5. CONNECT H55 HEADER TO W/COL. AND H55 COL. PER DETAIL 1/56.
6. PROVIDE L5x5x5/16 BRICK SUPPORT. WELD ANGLE TO H55 HEADER. SEE DETAIL 2/56.
7. PROVIDE SIMPSON L550 CLIP (TWO EA. END OF SILL MEMBER) FOR SILL CONN. TO JAMB.

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
F2.5	2'-6"x5Q.x14" T.	(4)-#4 EA. WAY BOTTOM
F3.5	3'-6"x5Q.x14" T.	(4)-#5 EA. WAY BOTTOM
F5	4'-0"x5'-0"x12" T.	(4)-#5 LONGIT. BOTT.; (5)-#5 TRANS. BOTT.
F6.5	8'-6"x6'-6"x20" T.	(7)-#6 LONGIT. T.4B.; (9)-#6 TRANS. T.4B.
F8.5	8'-6"x6'-0"x20" T.	(6)-#6 LONGIT. T.4B.; (9)-#6 TRANS. T.4B.
WF2	2'-0"xCONT.x14" T.	(3)-#5 CONT. BOTT.; #4 AT 48" O.C. TRANS. BOTT.
WF6	6'-0"xCONT.x22" T.	(7)-#6 LONGIT. T.4B.; #5 AT 24" O.C. T.4B. TRANS.

COLUMN SCHEDULE

MARK	SIZE	BASE PLATE	BASE PLATE TYPE (SEE DTL. 9/54 U.N.O.)	ANCHOR BOLTS
C4	H554x4x1/4	10x3/4"x0'-10"	TYPE 1	(4)-3/4" DIA. x 9" EMBED
C4A	H554x4x1/4	10x3/4"x0'-10"	TYPE 2	(4)-3/4" DIA. x 8" EMBED
C5	H555x5x1/4	11'x3/4"x0'-11"	TYPE 1	(4)-3/4" DIA. x 9" EMBED
C5A	H555x5x1/4	11'x3/4"x0'-11"	TYPE 2	(4)-3/4" DIA. x 8" EMBED
C5B	H555x5x5/16	11'x3/4"x0'-11"	TYPE 1	(4)-3/4" DIA. x 9" EMBED
C6	H556x6x5/16	12'x3/4"x1'-0"	TYPE 1	(4)-3/4" DIA. x 9" EMBED
MC10	W10x39	18"x1 1/2"x1'-6"	TYPE 3	(6)-1" DIA. x 15" EMBED (W/ ANCHOR PLATES)
MC10A	W10x54	22"x1 5/8"x1'-10"	TYPE 4	(8)-1" DIA. x 18" EMBED (W/ ANCHOR PLATES)

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JLA JOB #3245

CERTIFICATE OF AUTHORIZATION
JLA ENGINEERS INCORPORATED
No. 1594
ARKANSAS
2-27-2016



KEY ARCHITECTURE INC.
P.O. BOX 748 FAYETTEVILLE, ARKANSAS 72702
PH: 479.444.6066 FAX: 479.444.1445

Revision Schedule	Description
Rev. #	Date

NWA PEDIATRICS ADDITION
3730 S PINNACLE HILLS PKWY #3
ROGERS, AR 72768

MANA
3363 N MANA CT. SUITE 201
FAYETTEVILLE, AR 72703

DATE	DRAWN BY
02/23/26	LCG
PROJECT #	CHECKED BY
2438	LCG

SHEET
50
STRUCT. NOTES AND DTLS.

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