Schedule of Special Inspections & Construction Materials Testing

Item	Inspection/Test/Certification	C or P	Extent/Comments	Agent
1000.00	General Conditions			
1000.01	Review of Structural Documents and Shop Drawings to determine differences not approved by Architect or Engineer of Record	Continuous	Structural Documents should take precedence over any shop drawings. Special Inspector should use the Architectural and Structural Documents as the primary documents for review of construction. Shop drawing should be used as secondary document to review details not shown on the Architectural and Structural Documents. Any discrepancy between the two documents should be resolved by the Architect or Engineer of Record before proceeding with construction.	1
1000.02	The Special Inspector duties for missing details, conflicting details or coordination issues.	Continuous	Reasonable attempts have been made on the part of the design team to properly coordinate drawings. However in the event that a question arises on the project the Special Inspector shall obtain clarification from the Architect on all items. No changes shall be made to the drawings or construction without written conformation.	1
1400.00	Fabricators			
1400.01	Steel Fabricator shall either 1) be AISC certified or 2) includE the cost of Special Inspections for shop fabrication of the project specific pieces in their bid. Documentation of either item shall be submitted	Periodic		1
2300.00	Soils and Deep Foundations			
2300.01	Verify bearing capacities of soils beneath footings.	Periodic	As recommended in approved soils report and specified in earthwork specifications.	2
2300.02	Verify assumed bearing capacities and determine settlements of soils beneath footings and building pad.	Periodic	As noted on the drawings, recommended by the geotechnical engineer, and specified in earthwork specifications.	2
2300.03	Verify site preparation prior to beginning fill placement. Verify fill material type, placement method, lift thickness, and compaction of fill material. Verify in-place density of compacted fill.	Periodic	As recommended in approved soils report and specified in earthwork specifications.	2
2300.04	Inspect installation of helical pile foundations.	Continuous	Recored installation equipment, pile dimensions, tip elevations, final depth, final installationl torque.	2
3300.00	Concrete Construction			
3300.00.3	Nonstructural slabs supported directly on the ground are excepted from list below			1
3300.01	Inspect concrete formwork except as noted above for proper dimensions. Verify that construction joints are properly keyed. Verify that slab recesses, if any, have been installed.	Periodic	Prior to each pour.	1
3300.02	Inspect reinforcing steel except as noted above for installation including size, spacing and bar clearances. Verify that lap splices and embedment lengths are per the construction documents. Verify that dowels for work above are properly aligned and spaced to match other work.	Periodic	Prior to each pour.	1
3300.08	Inspect bolts	Periodic		1
3300.09	Verify each proposed concrete mix for the project.	Periodic	For each proposed mix	1
3300.10	Sample all concrete for strength tests and test concrete for slump, air content, temperature, and other tests.	Continuous	During placement operations. Reference concrete specifications for specific tests and frequencies.	2
3300.11	Inspect concrete placement except as noted above.	Continuous		1
3300.12	Inspect all concrete curing operations as noted in the extents column.	Periodic	Monitor during hot, cold and windy conditions. Reference concrete specifications.	1
3300.16	Confirm in-situ concrete strength prior to stressing of tendons in post tensioned concrete.	Periodic	Prior to tensioning operations.	1
4810.00	Masonry Construction			
4810.01	Inspect proportions of site prepared mortar and grout. Inspect placement of masonry units and construction of mortar joints. Inspect reinforcement for correct size and spacing. Inspect work for correct size and location of structural elements.	Periodic	At beginning of masonry construction and every 250 square feet of masonry thereafter.	1
4810.02	Inspect masonry cells and cleanouts prior to placement of grout. Inspect placement of all grout.	Continuous	During grouting.	1
4810.03	Inspect type size and location of anchors, including details of anchorage of masonry to structural members, frames or other construction.	Continuous	During installation of anchors.	1
4810.04	Inspect protection of masonry during cold weather and hot weather.	Periodic	During periods with temperatures below 40 degrees or above 90 degrees.	1
4810.05	Inspect preparation of grout specimens, mortar specimens and / or prisms.	Continuous	During preparation of all specimens.	1
4810.06	Verify compliance with all required inspection provisions of the construction documents and approved submittals.	Periodic	As required for duration of project.	1
5120.00	Steel Construction			
5120.01	Tasks Prior to Welding			
5120.01.1	Welding precedure specifications (WPS) available	Perform		1
5120.01.2	Manufacturer certifications for welding consumables available	Perform		1
5120.01.3	Material identification (type/grade)	Observe		1

Schedule of Special Inspections & Construction Materials Testing

Item	Inspection/Test/Certification	C or P	Extent/Comments	Agent
5120.01.4	Welder identification system	Observe	The fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be the lowe-stress type.	1
5120.01.7	Fit-up of fillet welds; dimensions (alignment, gaps at root), cleanliness (condition of steel surfaces), tacking (tack weld quality and location)	Observe		1
5120.01.8	Check welding equipment	As needed		1
5120.02	Tasks During Welding			
5120.02.1	Use of qualified welders	Observe		1
5120.02.2	Control and handling of welding consumables; packing, exposure control	Observe		1
5120.02.3	No welding over cracked tack welds	Observe		1
5120.02.4	Environmental conditions; wind speed limits, precipitation and temperature	Observe		1
5120.02.5	WPS followed; settigns on welding equipment, travel speed, selected welding materials, shiedlding gas type/flow rate, preheat applied, interpass temperature maintained (min/max), proper position (F,V,H,OH)	Observe		1
5120.02.6	Welding techniques; interpass and final cleaning, each pass within profile limitations, each pass meets quality requirements	Observe		1
5120.03	Tasks After Welding			
5120.03.1	Welds cleaned	Observe		1
5120.03.2	Size, length and location of welds	Perform		1
5120.03.3	Welds meet visual accecptance criteria; crack prohibition, weld/base-metal fusion, crater cross section, weld profiles, weld size, undercut, porosity	Perform and Document		1
5120.03.7	Repair activities	Perform and Document		1
5120.03.8	Document acceptance or rejection of welded joint or member	Perform and Document		1
5120.07	Tasks After Bolting			
5120.07.1	Inspect that all parts of the assembly have full contact and the bolts are tensioned to a minumum snug-tight connection. Hand tightening using a spud wrench..	Perform	Snug-tight connections only.	1
5120.07.2	Document acceptance or rejection of bolted connections	Perform		1
5120.08.1	Inspect fabricated steel to verify compliance with details shown on the construction documents and shown on the shop drawings.	Perform and Document	Erected steel frame, member locations, braces, stiffeners, and proper application of joint details at each connection	1
5120.08.2	Inspect anchor rods.	Observe	Diameter, grade, type and length of anchor rods or embedded items, and the extent or depth of embedment into the concrete, shall be verified prior to placement of concrete.	1
5120.09.4	Document acceptance or rejection of steel elements.	Perform and Document		
5310.00	Metal Deck; Verify depth and gauge of all deck elements, v.erify adequate bearing of ends of decking, verify deck support angles are provided for all opening greater than 100 square inches.	Periodic		
5400.00	Cold-Formed Steel Construction and Cold-Formed Steel Floor and Roof Decks			
5400.02	Inspect framing; size, gauge, spacing, bridging, blocking, bracing, fastenening of cold-formed members to other cold-formed members, fastening of cold-formed members to surrounding structure .	Periodic	Applies to non-load-bearing exterior walls.	1
7000.00	Architectural / MEP Components			
7000.04	Verify application of sprayed fire-resistant materials.		Inspect surface preparation of structural members, thickness of applied spray insulation	1

SCHEDULE OF SPECIAL INSPECTION AGENTS

Key Name	Inspection Agent
1	Special Inspector
2	Geotechnical Consultant
3	Structural Engineer of Record
4	The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Inspection Agent(s) may be subject to the approval of the Building Official.

GENERAL NOTES

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1. Contractor shall compare structural drawings and architectural drawings. Any omissions or discrepancies between plans, details, and specifications shall be brought to the attention of the Architect or Engineer before bidding. In all cases, more stringent requirement governs. Architectural dimensions and elevations will control.
2. Structural drawings or parts of the structural drawings may not be used as shop drawings without prior written approval.
3. All or parts of these drawings were produced with computer aided drafting. Drawings are available from the Engineer in DWG format on request.
4. Contractor proposed changes to details must be clearly noted on the first sheet of all shop drawings.
5. Construction shown is stable after the building is complete including interior and exterior finishes. The Contractor is responsible for temporary bracing of the structure during construction.
6. Review of submittal information shall be for general compliance with the contract documents and shall not include checking of detailed dimensions or detailed quantities.
7. Shop drawing detailer shall check all architectural and mechanical drawings for attachments, clips, openings or duct work and shall include these items in the shop drawings.
8. Site visits by Engineer of Record are not considered inspections or special inspections, rather are observations for general compliance with contract documents.
9. Submit electronic shop drawings only.

DESIGN LOADS

1. Reference code for loading 2021 International Building Code, ASCE/SEI 7-16
a. Risk Category III
b. Wind Load
1) V ulimate 115 mph
2) V asd 90 mph
3) Wind Exposure B
4) Internal Pressure Coefficient +/- 0.18
5) Velocity Pressure (qz) 20.0 psf
6) Components & Cladding Pressures see table and diagram
c. Snow Load
1) Ground Snow Load (Pg) 15 psf
2) Snow Exposure (Ce) 1.0
3) Importance Factor 1.1
4) Thermal Factor (Ct) 1.0
5) Flat Roof Snow Load (Pg) 16.5 psf
d. Seismic Load
1) Importance Factor 1.25
2) Mapped Spectral Response Accelerations
Ss 0.154
S1 0.09
3) Site Class B
4) Spectral Response Coefficients
Sds 0.093
Sd1 0.048
5) Seismic Design Category A
6) Base Seismic-Force-Resisting System(s) and Response Modification Factor
Steel Systems Not Specifically Detailed For Seismic 3
7) Design Base Shear 30 kips
8) Seismic Response Coefficient (Cs) 0.01
9) Analysis Procedure = Equivalent Lateral Force
e. Live Load
1) Roof Load 20 psf
2) Slabs on grade 100 psf
3) Fixed Seating 60 psf
4) 2nd Floor
Offices/Classrooms 50 psf
Corridors 80 psf
Stairs/Exitways 100 psf

FOUNDATIONS

1. Foundation design for this project was based on MTA Engineers report dated March 20, 2024.
2. Allowable bearing value of soil: 2000 psf when founded in "Stratum II" per Geotech Report.
3. All footings are to bear on undisturbed soil or engineered fill per Geotech Report.
4. Provide 8'-0" long top steel reinforcing, same size as bottom steel, at transitions between engineered fill and undisturbed soil locations.
5. Install corner bars at all footing intersections and corners (Provide 24 inch lap length e.w.)
6. Step all perimeter footings where necessary to provide a minimum of 12" soil cover to top of footing. Top of all interior footings shall be 0'-8" below finish floor, unless noted.
7. All footing elevations are given to the top of the footings.
8. Footing steps shown on the plans are furnished as a guide for estimating quantities. Final elevations are to be set in the field. Bearing elevations must be approved by a Soils Engineer before any concrete is placed.
9. Coordinate foundation elevations with plumbing requirements. Step footings as required to clear plumbing lines.

EPOXY AND MECHANICAL ANCHORS

1. All anchors shall be installed per manufacturer's instructions.
2. Contractor must get pre-approval from engineer-of-record before using post-installed epoxy or mechanical anchors not detailed in these drawings.
3. All epoxy to be RedHead EPCON C6 Fast Curing Epoxy or approved equal. Typical Embedment shall be 12 x Dia
4. All mechanical anchors to be Simpson TITEN HD Screw Anchors or approved equal. Typical Embedment shall be 8 x Dia.
5. All Powder Actuated Fasteners (PAF) to be 0.157 Hilti X-U with1 ¼" embedment. PAF shall completely penetrate steel.

STEEL JOIST AND JOIST GIRDERS

1. All steel joists shall conform to the standard specifications for the joist noted, as adopted by the Steel Joist Institute.
2. Refer to roof framing plan for uplift loading areas and diagrams.
4. Extend bottom chord of joists to 6"x6" plates on column per OSHA standards. For non-moment joists and joist girders, do not weld bottom chords to plates. For moment frame joists and joist girders, weld bottom chord to plate only after all roof dead load is installed.
5. K Series joists shall be welded to bearing plates or steel members with two 1/8" fillet welds 2" long.
6. LH and DLH Series joists shall be welded to bearing plates or steel members with two 1/4" fillet welds 2" long.
7. Weights of mechanical units are not included in the joist loading designation shown. Design joist for loading shown plus the weight of mechanical shown. General contractor is to verify all weights of mechanical units with Mechanical Subcontractor before submitting shop drawings.
8. Provide double joist under all mechanical units unless otherwise noted.
9. Weights of mechanical units are not included in the joist loading designation shown. Design locations and weights were not available at time of bid documents. Contractor is to coordinate the locations and weights of the units with the joist supplier.

CONCRETE SCHEDULE

Concrete Use	Design Strength	Slump Limits	Entrained Air Range	Weight	Notes
1. Footings	3000 psi	3" to 5"	3% to 5%	NW	-
2. Walls & Column Piers	4000 psi	6" to 8"	-	NW	Use HRWR
3. Slabs on Grade	4000 psi	6" to 8"	-	NW	Use HRWR
4. Slabs on Metal Deck	4000 psi	6" to 8"	-	NW	Use HRWR

STRIP FOOTING SCHEDULE

Type	Width	Thickness	Reinforcing
W24	2' - 0"	1' - 0"	(3)#4 bars cont w/ #4 bars @ 24" o.c. short
W30	2' - 6"	1' - 0"	(3)#4 bars cont w/ #4 bars @ 24" o.c. short
W36	3' - 0"	1' - 0"	(5)#4 bars cont w/ #4 bars @ 12" o.c. short
W60	5' - 0"	2' - 0"	(7) #4 bars cont w/ #4 bars @ 12" o.c. Short

SPREAD FOOTING SCHEDULE

Mark	Width	Length	Thickness	Reinforcing
F42	3' - 6"	3' - 6"	1' - 0"	(5) #4 bars e.w.
F48	4' - 0"	4' - 0"	1' - 0"	(6) #4 bars e.w.
F60	5' - 0"	5' - 0"	1' - 0"	(5) #5 bars e.w.
F66	5' - 6"	5' - 6"	1' - 1"	(6) #5 bars e.w.
F72	6' - 0"	6' - 0"	1' - 2"	(7) #5 bars e.w.
F78	6' - 6"	6' - 6"	1' - 4"	(8) #5 bars e.w.
F84	7' - 0"	7' - 0"	1' - 5"	(6) #6 bars e.w.
F90	7' - 6"	7' - 6"	1' - 6"	(7) #6 bars e.w.
F96	8' - 0"	8' - 0"	1' - 7"	(6) #7 bars e.w.
F102	8' - 6"	8' - 6"	1' - 8"	(7) #7 bars e.w.
F108	9' - 0"	9' - 0"	1' - 9"	(8) #7 bars e.w.
F114	9' - 6"	9' - 6"	1' - 10"	(9) #7 bars e.w.
F120	10' - 0"	10' - 0"	2' - 0"	(9) #7 bars e.w.
F126	10' - 6"	10' - 6"	2' - 1"	(10) #7 bars e.w.

REINFORCING STEEL AND CONCRETE

1. All concrete work is to be in accordance with the "Building Code Requirements for Reinforced Concrete" (ACI 318-11).
2. All detailing is to be in accordance with "ACI Detailing Manual" SP-66
3. Use of Calcium Chloride, Chloride Ions, or other salts in concrete are prohibited.
4. Concrete Properties: See Schedule
a. All concrete must obtain 7 day strength of 70% of design strength.
b. Concrete mixes may replace cement with other cementitious materials, submit for approval.
c. Combined weight of all replacement cementitious materials may not exceed 25% of the total cementitious weight.
d. Concrete mixes may use water reducers, accelerators or retarders with prior approval.
e. Do not provide air entrainment in concrete mixes for interior slabs.
5. All steel reinforcement shall be of deformed bars of billet steel conforming to ASTM A615, Grade 60 in all concrete.
6. Welded wire fabric shall be ASTM 185 and shall lap 2 cross wires or 6" whichever is greater on all sides. All laps shall be wired together.
7. Provide (2) #4 bars x 4'-0" at re-entrant corner locations Typical. Locate 3" away from corner and space 1'-0" apart.
8. All slabs on grade are 4", unless noted. Slabs are to be placed on 10 Mil, PVC vapor barrier over 4" of porous fill. Reinforce slabs with 6x6 W1.4 x W1.4 WWF placed 1" from top of slab. Unless otherwise noted slabs shall have joints placed at a maximum of 12'-0" on center. The aspect ratio of the joint layout should not exceed 1.5. Joints may be control joints or construction joints. See Architectural Plans for floor slopes and recesses for hard tile.
9. Minimum concrete cover for reinforcement:
a) Column pedastals 1 1/2" outside of ties
b) Footings 3"
c) Slabs on Grade 3/4"
d) Slabs on Deck 3/4"
f) Cast-In-Place Walls
Surfaces exposed to weather or soil 2" - #6 and greater, 1-1/2" - #5 and smaller
Other surfaces 3/4"
10. No openings shall be allowed to penetrate any concrete work, unless it is shown on the structural framing plans without prior written approval. Contractor shall submit for review locations of proposed openings not shown 30 days prior to pouring any concrete.
11. Do not run conduit or pipe in slabs or beams unless noted on the plans or specific prior approval from the engineer.
12. All construction joints must be keyed. Contractor shall submit location of proposed construction joints 3 days prior to pouring concrete.
13. Use 3/4" chamfer for all exposed corners unless noted.
14. For concrete work, contractor shall include an allowance of 1.0 tons of reinforcing steel in place in addition to the steel shown on the contract documents in the base bid. This steel is to be placed at no additional cost to the Owner in sizes and at locations as directed by the Architect or Engineer. Unused steel will be credited to the Owner.

STRUCTURAL STEEL

1. Steel Fabricator shall either 1) be AISC certified or 2) include the cost of Special Inspections for shop fabrication of the project specific pieces in their bid. Documentation of either item shall be submitted.
2. All detailing, fabricating, and erection of structural steel shall be in accordance with the AISC 360-10 "Specifications for Structural Steel Buildings". All reactions shown are ASD loads.
3. All connections are to be detailed as Type 2 "simple frame connections".
4. All structural steel W shapes shall be ASTM A992.
5. All structural steel Tube sections shall be ASTM A500 Grade C.
6. All structural steel Pipe sections shall be ASTM A501.
7. All structural steel channels, angles and other sections shall be ASTM A36, unless noted.
8. Headed Studs shall be Type B Shear Connectors.
9. Shop and field connections shall be welded with E-70XX electrodes or bolted with 3/4" dia. A-325N or A-325F bolts, unless noted.
10. Use 3/4" cap and bearing plates, unless noted.
11. Use 3 ASTM 1554 Grade 36 anchor bolts, unless noted.
12. Grout under baseplates with ASTM C 1107 cementitious 6000 psi Non-Shrink Grout.
13. Provide design calculations for connections other than standard frame or seat connections.
14. All steel exposed to weather shall be hot dipped galvanized per ASTM A123.
16. All steel exposed to earth shall receive bitumen coating.
17. Structural steel shall be shop primed per SSPC-SP2 "Hand Tool Cleaning" paint system. Primer shall be SSPC paint with a minimum thickness of 2.0 MILS. Omit Paint at surfaces to be fireproofed.
17. If steel sizes do not meet Specified U.L. listing (See Arch). Thickness of Fire Protections shall be increased as required.
18. Provide L 3"x3"x1/4" frames around all roof opening through metal decking.
19. Provide L 3"x3"x1/4" continuous perimeter deck angle, unless noted otherwise.
20. Where floor or roof decking changes direction, on top of support framing provide L 2-1/2"x2-1/2"x3/16" continuous for 2-1/2" seats or C5x6.7 continuous for 5" seats.
21. Contractor shall include an allowance of 1,000 lineal feet of L 3"x3"x1/4" angle in place in addition to the steel shown on the contract documents in the base bid.
22. Contractor shall include an allowance of 5.0 tons of structural steel in place in addition to the steel shown on the contract documents in the base bid. This steel is to be placed at no additional cost to the Owner in sizes and at locations as directed by the Architect or Engineer. Unused steel will be credited to the Owner.

CONCRETE MASONRY UNITS (CMU)

1. All masonry work to be in accordance with "Building Code Requirements for Concrete Masonry Structures" ACI 530-11 and "Specifications for Masonry Structures" ACI 530.1-13
2. Fill all concrete masonry units with concrete or grout from the top of the footing to the finish floor or to 8" above finish grade whichever is higher.
3. Use ladder type joint reinforcement (Dur-O-Wall SW DA3100 or better) at 16" on center in all cavity walls where brick is used for one or more of the wythes.
4. Use truss type joint reinforcement (Dur-O-Wall SW DA3100 or better) at 16" o/c. in all other masonry walls.
5. Place all CMU in running bond.
6. Use Type "M" or Type "S" mortar in accordance with IBC Table 2103.7(1).
7. Minimum compressive strength of concrete masonry f'm = 2000 psi. Submit for review test data on strength of units before starting any masonry work.
8. Minimum compressive strength of grout f'm = 2000 psi. Use 3/8" max size aggregate. See Special Inspection Schedule for any testing requirements. Grout slump shall be 8" to 11".
9. Use "Fine" grout for all reinforced piers and reinforced wall in accordance with ASTM C 476.
10. Each grout lift shall not exceed 5'-0" unless cleanouts are provided in the bottom course.
11. Fill cells under all lintels with grout.
12. Provide lintels over all openings through wall. See lintel details for reinforcement.
13. Unless otherwise noted provide control joints in all walls 4'-0" from wall intersections or corners and at 20'-0" o.c.
14. Provide continuous bond beam at 8'-0" o.c. vertical spacing TYP
15. Extend all horizontal steel and bond beams thru control joints.
16. Unless noted, all bars are to be located at the center of cell. Where bars are specified at each face, provide minimum 3/4" clear space between reinforcement and CMU face shell.
17. Anchor bolt into grouted cell locations only, unless noted otherwise.
18. For masonry work, contractor shall include an allowance of 0.5 tons of reinforcing steel in place in addition to the steel shown on the contract documents in the base bid. This steel is to be placed at no additional cost to the Owner in sizes and at locations as directed by the Architect or Engineer. Unused steel will be credited to the Owner.

CONSTRUCTION MATERIALS TESTING & SPECIAL INSPECTIONS

1. The owner will provide testing and special inspection under a separate contract. See this sheet or project specifications for schedule of special inspections.

AVL (AUDIO-VISUAL-LIGHTING) SUPPORTS

1. Contractor to provide allowance for steel angle, Unistrut, and threaded rod assemblies for hanging support of AVL equipment. Layout and arrangement to be coordinated once AVL shop drawings with equipment weights/locations are provided.

DELEGATED DESIGN – EXTERIOR COLD-FORMED METAL (CFS) WALL FRAMING

1. Reference Design Loads for Wind, Snow, and Seismic loads on CFS wall framing.
2. Provide shop drawings showing: product data, cut sheets, structural properties, plan views, section views, and detail views.
3. Provide calculations for all CFS members, connections of CFS to CFS, and connections of CFS to primary structure. Calculations to be signed and sealed by a Professional Engineer licensed in the State of the Project.

DELEGATED DESIGN – STAIRS, LADDERS, HANDRAILS, AND GUARDRAILS

1. Design Loads for design of stairs, ladders, handrails, and guardrails.
   a. Stairs
      1) Dead: self weight of materials
      2) Live: 100 psf
   b. Handrails/Guardrails
      1) Live: 200 lb force applied in any direction at any point on handrail or top rail
      2) Live: 50 plf force applied in any direction at any point on handrail or top rail
      3) Live: 50 psf force on intermediate rails, openings, and space between rails
   c. Ladders
      1) Live: 300 lb force applied at any point in any direction on rungs, min one 300 lb load every 10 ft of ladder height
      2) Live: 100 lb force in any direction at any height at each side rail extension
      3) Live: Ships ladders with treads designed for same loads as Stairs
   d. Grab Bars
      1) Live: 250 lb force applied in any direction at any point on grab bar
2. Provide shop drawings showing: product data, cut sheets, structural properties, plan views, section views, and detail views.
3. Provide calculations for all members, connections of members to members, and connections of members to primary structure. Calculations to be signed and sealed by a Professional Engineer licensed in the State of the Project.

DELEGATED DESIGN – ALUMINUM AWNINGS/CANOPIES

1. Reference Architectural drawings for layout, location, size, configuration, etc for all awnings and canopies
2. Reference structural typical details for blocking details at frame and tie rod connection points. Contractor to field locate blocking based on connection points shown in awning shop drawing submittal.
3. Provide shop drawings showing: product data, cut sheets, structural properties, plan views, section views, and detail views.
4. Provide calculations for all members and connections. Calculations to be signed and sealed by a Professional Engineer licensed in the State of the Project.

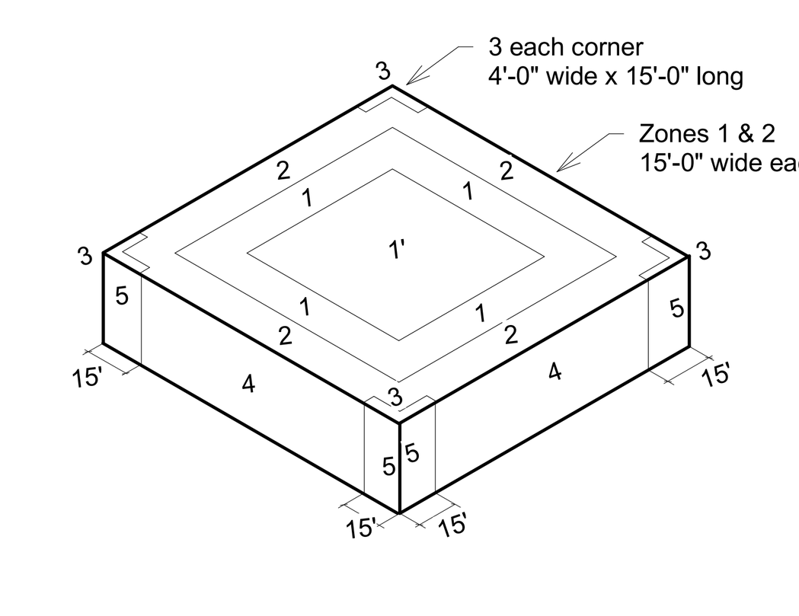
ULTIMATE COMPONENTS & CLADDING PRESSURES (PSF) a = varies, see diagrams

Area	+ Zone 1,1',2,3	- Zone 1'	- Zone 1	- Zone 2	- Zone 3	- Zone 2 OH	- Zone 3 OH	+ Zone 4,5	- Zone 4	- Zone 5
10 SF	+16.0	-21.8	-37.9	-50.0	-96.9	-50.0	-68.2	+21.8	-23.6	-29.0
100 SF	+16.0	-21.8	-29.6	-39.3	-46.8	-25.8	-43.2	+18.6	-20.4	-22.6
500 SF	+16.0	-16.0	-23.8	-31.9	-31.9	-25.8	-25.8	+16.3	-18.1	-18.1

Parapet Wind Pressures by Case and Zone: evaluated at h=36' q h-p = 21.4 psf

Area	Case A (4/-2)	Case A (4 or 5/-3)	Case B (-4/4 or 5)	Case B (-5/4 or 5)
10 SF	+68.0	+87.1	-40.1	-45.9
100 SF	+53.3	+61.2	-33.3	-35.7
500 SF	+43.1	+43.1	-28.7	-28.7

Notes:
1. Joist designer assume 10 psf Dead when computing Net Uplift



See S3.4 for Uplift Zone Roof Plan

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CALVARY CHURCH
CALVARY CHURCH FACILITY ADDITION
BENTONVILLE, AR

Revisions

No.	Description	Date

Project Number
21010

Date
08-12-2025

Notes & Schedules

S0.01