ABBREVIATION LEGEND

A.R.	ANCHOR RODS
ADD'L	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
ARCH	ARCHITECTURAL
B PL	BASE PLATE
BF	BOTTOM OF FOOTING
BFF	BELOW FINISHED FLOOR
BLDG	BUILDING
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BP	BOTTOM OF PIER
BRG	BEARING
BTWN	BETWEEN
C	CHANNEL SHAPE (i.e. C8x11.5)
C	COLD FORMED C SHAPE
C.G.	CENTER OF GRAVITY
CJ	(KEYED) CONTROL JOINT
CL	CENTERLINE
CLG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
DBL	DOUBLE
DEG	DEGREES
DIA	DIAMETER
DIM	DIMENSION
DTL	DETAIL
DWLS	DOWELS
EA	EACH
EJ	EXPANSION JOINT
EL	ELEVATION
ELEV	ELEVATION
EMBED	EMBEDMENT LENGTH
EOS	EDGE OF SLAB
ERECT	ERECTION
EWEF	EACH WAY, EACH FACE
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FD	FLOOR DRAIN
FF	FINISHED FLOOR
FV	FIELD VERIFY
FFE	FINISHED FLOOR ELEVATION
FIN FLR EL	FINISHED FLOOR ELEVATION
FS	FAR SIDE
FTG	FOOTING
Fy	STEEL YIELD STRENGTH
GA	GAUGE
GB	GRADE BEAM
HORIZ	HORIZONTAL
HS	HEADED STUD
HSS	HOLLOW STRUCTURAL SECTION (STEEL)
I/S	INSIDE
INFO	INFORMATION
INT	INTERIOR
JBE	JOIST BEARING ELEVATION
JT	JOINT
K	KIPS (KILO-POUNDS)
k/ft	KIPS PER FOOT
KD	KILN-DRIED
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	ANGLE (i.e. L3x3x1/4)
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LW	LONG WAY
M.B.	METAL BUILDING
MC	MOMENT CONNECTION
MAT'L	MATERIAL
MAX	MAXIMUM
MC	MISC. CHANNEL SHAPE (i.e. MC12x10.6)
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC.	MISCELLANEOUS
MPH	MILES PER HOUR
MTL	METAL
MISD.	NUMBER
N.S.	NON-SHRINK
NS	NEAR SIDE
NTS	NOT TO SCALE
o.c.	ON CENTER
O/S	OUTSIDE
OD	OUTSIDE DIAMETER
OPNG	OPENING
OPP.	OPPOSITE
OSB	ORIENTED STRAND BOARD
PF###	PAD FOOTING (###-SIZE IN FEET)
PL	PLATE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
R	RADIUS
RE:	REFERENCE
REINF	REINFORCING
REQ'D	REQUIRED
RTU	ROOF TOP UNIT
S	STANDARD STEEL SHAPE (i.e. S10x35)
SCHED.	SCHEDULE
SECT	SECTION
SIM	SIMILAR
SJ	(SAWN) CONTROL JOINT
SQ	SQUARE
STIFF	STIFFENER
SW	SHORT WAY
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TEMP	TEMPERATURE
TF	TOP OF FOOTING
TOC	TOP OF COLUMN
TOC	TOP OF CONCRETE
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VER	VERIFY
VERT	VERTICAL
W	WIDE FLANGE SHAPE (i.e. W8x10)
WP	WORK POINT
w/	WITH
w/o	WITHOUT
WWR	WELDED WIRE REINFORCING (i.e. WIRE MESH)

STRUCTURAL NOTES

GENERAL NOTES

1. THE CONTRACTOR SHALL THOROUGHLY REVIEW ALL CONTRACT DOCUMENTS AND INFORM THE ARCHITECT OF CONFLICTS OR DISCREPANCIES PRIOR TO BIDDING, FABRICATION, AND CONSTRUCTION.
2. IN CASES OF DISCREPANCIES IN DIMENSIONS AND ELEVATIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS, CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION.
3. THE CONTRACTOR SHALL COORDINATE THE FIELD VERIFICATION OF ALL EXISTING SITE CONDITIONS SUCH AS EXISTING FLOOR ELEVATIONS, EXISTING FOOTING ELEVATIONS, EXISTING UTILITIES, ETC, WHETHER NOTED OR NOT IN THE CONTRACT DOCUMENTS AND SHALL NOTIFY THE ARCHITECT OF ANY CONFLICTS, DISCREPANCIES OR UNKNOWN CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTION.
4. REPRODUCTION OF CONTRACT DRAWINGS, IN ANY FORM, WILL NOT BE ACCEPTED AS SHOP DRAWINGS.
5. REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER-OF-RECORD DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL FOR REVIEW. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR ALSO SHALL BE RESPONSIBLE FOR ALL MEANS, METHODS, TECHNIQUES, AND PROCEDURES OF CONSTRUCTION.
6. CONTRACTOR SHALL PROVIDE TEMPORARY GUYS AND BRACING AS REQUIRED DURING CONSTRUCTION. STRUCTURE IS NOT STABLE UNTIL ALL STRUCTURAL MEMBERS, CONNECTIONS, AND DECKING IS IN PLACE.
7. ACI, AISC, AITC AND AWS SPECIFICATIONS SHALL GOVERN ALL PHASES OF FABRICATION AND CONSTRUCTION.

CONCRETE NOTES

CONCRETE REINFORCEMENT

1. CONCRETE REINFORCEMENT SUPPLIER SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW PRIOR TO CONSTRUCTION.
2. ALL REINFORCING STEEL SHALL BE ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
3. PROVIDE THE FOLLOWING PROTECTIVE COVERING FOR ALL REINFORCING BARS UNLESS DETAILED OR NOTED OTHERWISE:

SLAB-ON-GRADE BARS (BOTTOM)	3" CLEAR
BELOW GRADE (CAST AGAINST EARTH)	3" CLEAR
BELOW GRADE (FORMED EDGE)	2" CLEAR
WALLS	2" CLEAR
COLUMNS	1.5" CLEAR TO TIES
ELEVATED BEAMS	1.5" CLEAR TO STIRRUPS
ELEVATED SLABS & JOISTS	0.75" CLEAR

4. DO NOT CUT TIES OR CONTINUOUS BARS TO PROVIDE CLEARANCE FOR EMBEDDED ITEMS OR OTHER OBSTRUCTIONS. INDIVIDUAL BARS AND TIES MAY BE MOVED VERTICALLY UP TO 1.5" AS REQUIRED TO PROVIDE CLEARANCE FOR EMBEDS, HOOKS, ETC. DO NOT HEAT REINFORCING TO BEND IT.
5. IF DOWELS OR VERTICAL REINFORCING ARE CUT OR SEVERELY BENT, CONTRACTOR MAY BE REQUIRED TO REMOVE THE CONCRETE BACK TO THE PREVIOUS POUR JOINT AND REPLACE THE DAMAGED BARS AND CONCRETE AT THE CONTRACTOR'S EXPENSE.
6. REINFORCEMENT SHALL BE SPLICED ONLY AS SHOWN OR NOTED IN THE STRUCTURAL CONTRACT DOCUMENTS. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER-OF-RECORD PRIOR TO FABRICATION.
7. REINFORCING BARS MARKED AS CONTINUOUS SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICES ONLY.
8. ALL TENSION LAP SPLICES SHALL BE CLASS "B" UNLESS NOTED OTHERWISE.
9. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185. LAP REINFORCEMENT 8 INCHES ON SIDES AND ENDS. MAINTAIN WIRE 1 TO 2 INCHES BELOW TOP SURFACE OF SLAB-ON-GRADE, UNLESS NOTED OTHERWISE. WELDED WIRE REINFORCEMENT MUST BE PLACED ON CHAIRS OR BOLSTERS AS REQUIRED TO MAINTAIN POSITION IN THE SLAB.
10. ONCE SHOP DRAWINGS HAVE BEEN REVIEWED, DO NOT ADD REINFORCING OR INFORMATION TO PREVIOUSLY SUBMITTED SHEETS FOR SUBSEQUENT SUBMITTALS UNLESS SHOP DRAWINGS ARE BEING RESUBMITTED AFTER BEING RETURNED "NOT REVIEWED".
11. WHERE ANCHOR RODS ARE CAST INTO CONCRETE, PROVIDE SUPPLEMENTAL REINFORCING EACH WAY, TIED NEAR THE TOP AND BOTTOM OF ALL ANCHOR RODS TO THE ADJACENT REBAR TO SECURE RODS DURING CONCRETE PLACEMENT. (MINIMUM SIZE #4)

CAST-IN-PLACE CONCRETE (03 30 00)

1. CONCRETE SUPPLIER SHALL SUBMIT CONCRETE MIX DESIGN DATA TO THE ARCHITECT FOR REVIEW PRIOR TO CONSTRUCTION.
2. CONCRETE SHALL HAVE AT LEAST THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:

A. FOOTINGS & GRADE BEAMS	4000 PSI
B. REINFORCED CMU & BOND BEAM FILL	(SEE MASONRY NOTES)
C. SLABS-ON-GRADE, WALLS, PILASTERS & PEDESTALS	4000 PSI
D. ELEVATED SLABS, BEAMS & JOISTS	4000 PSI

3. SEE CONCRETE MIX DESIGN TABLE
4. PROPORTIONS OF CONCRETE MIX DESIGNS SHALL BE DETERMINED BY THE PROCEDURES ESTABLISHED IN SECTION 5.3 OF ACI 318-11.
5. MIX DESIGN MAY INCLUDE (TYPE C) FLYASH AS A REPLACEMENT FOR PORTLAND CEMENT UP TO A MAXIMUM OF 25% OF THE TOTAL CEMENTIOUS MATERIAL. DO NOT USE A FLYASH CONTAINING CONCRETE MIX WHEN THE TEMPERATURE DURING PLACEMENT OR CURING IS PROJECTED TO FALL BELOW 60 DEGREES FAHRENHEIT.
6. MIX DESIGN MAY INCLUDE WATER REDUCING ADMIXTURES CONFORMING TO ASTM C494, TYPE A, TO PROVIDE WORKABILITY AND SPECIFIED SLUMP WITHOUT EXCEEDING SPECIFIED WATER/CEMENT RATIOS. WATER SHALL NOT BE ADDED ON SITE WITHOUT PRIOR APPROVAL. ANY APPROVED WATER AMOUNTS ADDED ON SITE MUST BE RECORDED & REPORTED BY THE TESTING AGENCY.
7. ALL CONCRETE EXPOSED TO WEATHER SHALL CONTAIN 5.5% AIR ENTRAINMENT (± 1.5%). DO NOT EXCEED 3% AIR CONTENT IN CONCRETE RECEIVING A STEEL TROWEL FINISH.

ARCHITECTURAL PRECAST CONCRETE (03 45 00)

1. PRECAST MANUFACTURER SHALL SUBMIT CALCULATIONS AND SHOP DRAWINGS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARKANSAS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
2. ARCHITECTURAL PRECAST CONCRETE SHALL BE DESIGNED AND MANUFACTURED BY A FIRM EXPERIENCED IN THE FIELD. MANUFACTURER SHALL ACCEPT FULL RESPONSIBILITY FOR THE DESIGN OF ALL PANELS AND CONNECTIONS TO THE STRUCTURE.
3. PRECAST MANUFACTURER SHALL ADD REINFORCING REQUIRED FOR CONSTRUCTING, TRANSPORTING, LIFTING AND ERECTING ALL MEMBERS.
4. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS (AND COORDINATE WITH THE VARIOUS CONTRACTORS) FOR THE EXACT SIZE AND LOCATION OF OPENINGS IN THE MEMBERS AND FOR ADDITIONAL EMBEDS REQUIRED BY ALL TRADES.
5. COORDINATE THE LOCATION AND ELEVATION OF ALL MEMBER JOINTS WITH THE ARCHITECTURAL DRAWINGS.
6. ALL STEEL CONNECTIONS EXPOSED TO WEATHER (BOLTS, INSERTS, EMBEDS, ETC.) USED TO CONNECT EXTERIOR PRECAST MEMBERS TO THE STRUCTURE SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION. ALL GALVANIZED COATINGS DAMAGED DURING SHIPPING, ERECTING, FIELD WELDING, ETC. SHALL BE REPAIRED WITH TWO (2) COATS OF COLD SPRAY GALVANIZING OR EQUAL REPAIR.
7. ALL REBAR WITH ONE INCH (1") OR LESS CONCRETE COVER TO THE SURFACE OF THE PRECAST MEMBERS SHALL BE HOT DIPPED GALVANIZED OR OTHERWISE PROTECTED FROM CORROSION.
8. CONCRETE FOR PRECAST MEMBERS SHALL HAVE A COMPRESSIVE STRENGTH OF AT LEAST 5000 PSI AT 28 DAYS.
9. SEE ARCHITECT FOR ALL COLOR, AGGREGATE AND FINISH REQUIREMENTS FOR ARCHITECTURAL PRECAST.

CAST-IN-PLACE CONCRETE MIX DESIGN TABLE

MIX DESIGN SHALL INCLUDE AT LEAST THE FOLLOWING AMOUNTS OF PORTLAND CEMENT MEETING ASTM C150 OR D595 PER CUBIC YARD OF CONCRETE

	NON-AIR ENTRAINED		AIR ENTRAINED		
28 DAY MIN. COMPRESSIVE STRENGTH	MIN. CEMENT CONTENT (LBS/YARD³)	MAXIMUM PERMISSIBLE W/C RATIO	MIN. CEMENT CONTENT (LBS/YARD³)	MAXIMUM PERMISSIBLE W/C RATIO	MAX. SLUMP w/ WRA
4000	564	0.44	611	0.40	6"

METALS NOTES (05 00 00)

STRUCTURAL STEEL (05 12 00)

1. STRUCTURAL STEEL SUPPLIER SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
2. ALL STRUCTURAL STEEL SHAPES SHALL BE AS FOLLOWS:
   A. ALL WIDE FLANGE STRUCTURAL STEEL SHAPES (W) SHALL BE ASTM A992.
   B. SQUARE OR RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL BE ASTM A500, GRADE C, Fy = 50 KSI
   C. ROUND HOLLOW STRUCTURAL SECTIONS (HSS) SHALL BE ASTM A500, GRADE C, Fy = 46 KSI
   D. ROUND STEEL PIPES (P, PX, PXX) SHALL BE ASTM A53, GRADE B, FY = 35 KSI.
   E. ALL OTHER STRUCTURAL STEEL (CHANNELS (C), ANGLES (L), PLATES (PL), ETC.) SHALL BE ASTM A36, UNLESS NOTED OTHERWISE.
4. ALL ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 (OR GRADE 55 WITH SUPPLEMENT S1 – WELDABILITY) UNLESS NOTED OTHERWISE.
5. STRUCTURAL BOLTS SHALL BE ASTM F3125 GRADE A325-N, (OR GRADE A490-N) UNLESS OTHERWISE NOTED.
6. BOLTS THRU WOOD BLOCKING SHALL BE ASTM A307. ALL BOLTS IN CONTACT WITH TREATED WOOD SHALL BE STAINLESS STEEL (TYPE 316L), OR HOT DIPPED GALVANIZED WITH A MINIMUM COATING THICKNESS OF 0.2 OUNCES PER SQUARE FOOT (ASTM A153). USE STAINLESS BOLTS WITH STAINLESS STEEL CONNECTORS AND GALVANIZED BOLTS WITH GALVANIZED CONNECTORS IF ONLY ONE IS SPECIFIED.
7. POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE SHALL BE STANDARD ASTM A36 THREADED RODS (OR APPROVED EQUAL) WITH A MINIMUM STEEL YIELD STRENGTH OF Fy=36ksi OR ASTM F593 STAINLESS STEEL ANCHORS WITH A MINIMUM STEEL YIELD STRENGTH OF fy=45ksi, UNLESS SHOWN OTHERWISE ON THE DRAWINGS. ADHESIVE SHALL BE HILTI "HIT-RE 500-SD" SYSTEM (REF: ICC-ES ESR-2322), SIMPSON STRONG-TIE "SET-3G" SYSTEM (REF: ICC-ES AC308), DEWALT "PURE 110+" SYSTEM (REF: ISS-ES ESR 3298), (OR APPROVED EQUAL). (SEE PRODUCT MANUALS FOR HOLE CLEANING, INSTALLATION AND INSTALLER TRAINING REQUIREMENTS.)
8. POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE FILLED CMU CELLS SHALL BE STANDARD ASTM A36 THREADED RODS (OR APPROVED EQUAL) WITH A MINIMUM STEEL YIELD STRENGTH OF Fy=36ksi OR ASTM F593 STAINLESS STEEL ANCHORS WITH A MINIMUM STEEL YIELD STRENGTH OF fy=45ksi, UNLESS SHOWN OTHERWISE ON THE DRAWINGS. ADHESIVE SHALL BE HILTI "HIT-HY70" SYSTEM (REF: ICC-ES ESR-2682), SIMPSON STRONG-TIE "SET-XP" SYSTEM (REF: IAPMO UES ER-265), DEWALT "AC 100+ GOLD" SYSTEM (REF: ICC-ES ESR 3200), (OR APPROVED EQUAL). (SEE PRODUCT MANUALS FOR HOLE CLEANING, INSTALLATION AND INSTALLER TRAINING REQUIREMENTS.)
9. POST-INSTALLED ADHESIVE ANCHORS IN HOLLOW CMU OR CLAY MASONRY SHALL BE STANDARD ASTM A36 THREADED RODS (OR APPROVED EQUAL) WITH A MINIMUM STEEL YIELD STRENGTH OF Fy=36ksi OR ASTM F593 STAINLESS STEEL ANCHORS WITH A MINIMUM STEEL YIELD STRENGTH OF fy=45ksi, UNLESS SHOWN OTHERWISE ON THE DRAWINGS. ADHESIVE AND SCREEN TUBES SHALL BE HILTI "HIT-HY70" SYSTEM (REF: ICC-ES ESR-2682), SIMPSON STRONG-TIE "ET-HP" SYSTEM (REF: IAPMO UES ER-241), DEWALT "AC 100+ GOLD" SYSTEM (REF: ICC-ES ESR 3200), (OR APPROVED EQUAL). (SEE PRODUCT MANUALS FOR HOLE CLEANING, INSTALLATION AND INSTALLER TRAINING REQUIREMENTS.)
10. POST-INSTALLED EXPANSION ANCHORS IN CONCRETE SHALL BE HILTI "KWIK BOLT TZ" (REF: ICC-ES ESR-1917), SIMPSON STRONG-TIE "STRONG BOLT 2" (REF: ICC-ES ESR-3037), DEWALT "POWER STUD+ SD1+" (REF: ICC-ES ESR-2818), (OR APPROVED EQUAL) CARBON STEEL ANCHORS UNLESS SHOWN OTHERWISE ON THE DRAWINGS. (SEE PRODUCT MANUALS FOR HOLE CLEANING, INSTALLATION AND INSTALLER TRAINING REQUIREMENTS.)
11. POST-INSTALLED SCREW ANCHORS SHALL BE HILTI "KWIK HUS EZ" (REF: ICC-ES ESR-3027), SIMPSON STRONG-TIE "TITEN HD" (REF: ICC-ES ESR-2713), DEWALT "SCREW BOLT" (REF: ICC ESR-3889), (OR APPROVED EQUAL), UNLESS NOTED OTHERWISE. (SEE PRODUCT MANUALS FOR HOLE CLEANING, INSTALLATION AND INSTALLER TRAINING REQUIREMENTS.)
12. CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL BE DESIGNED CONSIDERING BOLT THREADS INCLUDED IN THE SHEAR PLANE (A325-N). ALL BOLTING SHALL BE INSTALLED BY THE TURN-OF-THE-NUT METHOD, REMOVABLE LOAD INDICATOR BOLTS; OR CALIBRATED WRENCH. SNUG TIGHT BOLTING WILL NOT BE PERMITTED UNLESS SPECIFICALLY DETAILED ON CONTRACT DRAWINGS.
13. ALL HIGH STRENGTH BOLTED CONNECTIONS (EXCEPT COMPOSITE FLOOR BEAM CONNECTIONS) SHALL BE BEARING TYPE SELECTED TO SUPPORT ONE-HALF (1/2) OF THE MAXIMUM TOTAL UNIFORM LOAD CAPACITY OF THE BEAMS AS SHOWN IN TABLE 3-6 OF THE AISC MANUAL, 14TH EDITION, FOR THE GIVEN BEAM SIZE, SPAN AND GRADE OF STEEL SPECIFIED. THE EFFECTS OF ANY CONCENTRATED LOADS MUST BE TAKEN INTO ACCOUNT. CONNECTIONS SHALL BE DESIGNED CONSIDERING THREADS INCLUDED IN THE SHEAR PLANE (A325-N).
14. ALL WELDS SHALL BE E70XX, MINIMUM AND SHALL BE PERFORMED BY AWS CERTIFIED WELDERS, CERTIFIED WITHIN THE PREVIOUS TWELVE (12) MONTHS. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO AVOID DAMAGE TO THE BUILDING AND COMPONENTS DUE TO FIRE HAZARDS FROM WELDING.
15. DO NOT PRIME PAINT STEEL THAT RECEIVES SPRAYED FIREPROOFING.
16. ALL STEEL LINTELS AND SHELF ANGLES SHALL BE COATED WITH A ZINC RICH PRIMER.
17. ALL STRUCTURAL STEEL EXPOSED TO WEATHER (SUCH AS MECHANICAL FRAMES) SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION.

METAL DECKING (05 31 00)

1. METAL DECKING SUPPLIER SHALL SUBMIT SHOP DRAWINGS PREPARED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARKANSAS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
2. POWDER ACTUATED OR PNEUMATIC FASTENERS MAY NOT BE SUBSTITUTED FOR PUDDLE WELDS.

LIGHT GAUGE METAL TRUSSES (05 44 00)

1. TRUSS FABRICATOR SHALL SUBMIT CALCULATIONS AND SHOP DRAWINGS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARKANSAS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
2. TRUSS DIMENSIONS AND LAYOUT, IF SHOWN, IS FOR ESTIMATING PURPOSES ONLY AND IS NOT NECESSARILY TO BE USED FOR FABRICATION. FABRICATOR SHALL BE RESPONSIBLE FOR ACTUAL DIMENSIONS OF TRUSSES. TRUSSES SHALL UTILIZE ONLY THE BEARING WALLS AND SUPPORTS SHOWN ON THE PLANS.
3. CONTRACTOR SHALL PROVIDE BRACING FOR TRUSS CHORDS AND WEB MEMBERS AS REQUIRED BY THE TRUSS FABRICATOR. SYSTEM IS NOT STABLE UNTIL SHEATHING AND PERMANENT BRACING ARE INSTALLED.
4. TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE TRUSS HANGERS WHERE TRUSSES ARE SUPPORTED BY OTHER TRUSSES.
5. TRUSS MANUFACTURER SHALL COORDINATE ALL HAT CHANNELS AND ATTACHMENT TO TOP AND BOTTOM CHORD OF TRUSS. ADDITIONAL TRUSSES SHALL BE ADDED IF HAT CHANNELS CANNOT CARRY ROOF LOADS OR CEILING LOADS AT TRUSS SPACING SHOWN ON PLAN. HAT CHANNEL DEFLECTION ON ROOF AND CEILING SHALL BE LIMITED TO L/240 TOTAL LOAD AND L/360 LIVE LOAD.
6. TRUSS MANUFACTURER SHALL DESIGN AND COORDINATE A COMPLETE ROOF SYSTEM INCLUDING ALL TRUSSES, HEADERS, CONNECTIONS AND HAT CHANNELS (IF APPLICABLE), ETC.

STRUCTURAL PANELS (06 12 00)

1. PNEUMATIC NAILING MAY BE SUBSTITUTED FOR COMMON NAILS UNDER THE FOLLOWING CONDITIONS:
   A. PNEUMATIC NAIL SUBSTITUTE FOR 8d COMMON NAILS SHALL HAVE A MINIMUM DIAMETER OF 0.131 INCHES AND LENGTH OF 2 ½INCHES.
   B. PNEUMATIC NAIL SUBSTITUTE FOR 10d COMMON NAILS SHALL HAVE A MINIMUM DIAMETER OF 0.148 INCHES AND LENGTH OF 3 INCHES.

   T-HEAD NAILS OR STAPLES ARE NOT ACCEPTABLE.

EARTHWORK & FOUNDATION NOTES (31 00 00)

EXCAVATION & FILL (31 22 00 & 31 23 23)

1. ALL UNDERCUTTING, SITE PREPARATION, FILL SELECTION, BACKFILLING AND COMPACTION SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE SPECIFICATIONS AND SOILS ENGINEER'S RECOMMENDATIONS.
2. SELECT FILL BENEATH THE BUILDING SHALL BE PLACED IN LIFTS NOT EXCEEDING 8" LOOSE THICKNESS AND COMPACTED TO AT LEAST 95% OF MAXIMUM MODIFIED PROCTOR DRY DENSITY (ASTM D1557). THE IN-PLACE DENSITY AND MOISTURE CONTENT SHALL BE ESTABLISHED AND APPROVED FOR EACH LIFT PRIOR TO PLACEMENT OF SUBSEQUENT LIFTS.

SPREAD FOOTINGS (31 24 50)

1. BOTTOM OF FOOTING ELEVATIONS (BF) SHOWN ON THE PLANS ARE FOR ESTIMATING PURPOSES ONLY AND ARE NOT NECESSARILY TO BE USED FOR CONSTRUCTION. THE SOILS ENGINEER OR HIS REPRESENTATIVE SHALL BE ENGAGED TO INSPECT ALL FOOTING EXCAVATIONS TO VERIFY THAT THE REQUIRED ALLOWABLE BEARING CAPACITY IS ATTAINABLE. BOTTOM OF FOOTING ELEVATIONS SHALL BE ADJUSTED PER THE ON-SITE RECOMMENDATIONS OF THE SOILS ENGINEER OR HIS REPRESENTATIVE.
2. ALL SPREAD FOOTINGS SHALL BE FOUNDED ON NATIVE VERY STIFF TO HARD, SHALEY CLAYS AND/OR TESED AND APPROVED NEW SELECT FILL WITH AN ALLOWABLE NET BEARING CAPACITY OF AT LEAST 2000 PSF. (REF: GEOTECHNICAL INVESTIGATION, JOB NO. 14-351 DATED MARCH 2023 BY GTS, INC.)
3. MAINTAIN FINISHED GRADE (AND/OR BOTTOM OF FOOTING ELEVATIONS) TO PROVIDE AT LEAST 1'-6" COVER ABOVE THE BOTTOM OF ALL EXTERIOR FOOTINGS FOR FROST PROTECTION.

RETAINING WALLS (32 32 13)

1. ALL RETAINING WALLS SHALL HAVE A PROPERLY INSTALLED DRAINAGE SYSTEM TO RELIEVE HYDROSTATIC PRESSURE.
2. BACKFILL BOTH SIDES OF WALLS EQUALLY UNTIL LOW SIDE IS UP TO GRADE.
3. PROVIDE ADDITIONAL SHORING FOR ALL FOUNDATION WALLS AS REQUIRED DURING CONSTRUCTION BACKFILLING AND COMPACTION OPERATIONS.
4. ALL FOUNDATION WALLS WITH AN ELEVATED CONCRETE SLAB FRAMING INTO THE TOP OF THEM MAY BE BACKFILLED ONLY AFTER THE ELEVATED SLAB IS IN PLACE AND CURED.
5. IF RETAINING WALLS ARE REQUIRED BY THE BUILDING OFFICIALS TO BE INSPECTED (AS-BUILT CERTIFICATION FORM), THE CONTRACTOR SHALL RETAIN THE SERVICES OF AN INDEPENDENT REGISTERED ENGINEER OR NOTIFY THE ENGINEER-OF-RECORD AT LEAST 3 DAYS PRIOR TO COVERING UP THE REBAR WITH WALL FACING MATERIAL (WHETHER CONCRETE OR MASONRY), SO THAT IN-PLACE REBAR MAY BE PROPERLY INSPECTED.

DESIGN LOADS:

DEAD LOADS:		WEIGHT OF THE STRUCTURE
ROOF LIVE LOAD:		20 PSF
FLOOR LIVE LOADS:		
LOBBIES		100 PSF
ASSEMBLY AREAS		100 PSF
STAIRS AND EXITS		100 PSF
GROUND SNOW LOAD	Pg:	10 PSF
WIND SPEED FOR RISK CATEGORY II & EXPOSURE C		Vult: 105 MPH Vasd: 82 MPH
BUILDING RISK CATEGORY		II
WIND EXPOSURE CATEGORY		C
INTERNAL PRESSURE COEFFICIENT	GCpi:	+/-0.18
COMP. & CLADDING WIND PRESSURE	Pnet30:	SEE ASCE 7-10, TABLE 30.7-2
MAPPED SPECTRAL RESPONSE ACCELERATIONS	Ss: S1:	0.578 0.177
SITE CLASS		C
SPECTRAL RESPONSE COEFFICIENTS	Sds: Sd1:	0.463 0.177
SEISMIC DESIGN CATEGORY		C
BASIC SEISMIC-FORCE-RESISTING SYSTEM (PER ASCE 7-10, TABLE 12.2-1)		SEE WALLACE ENGINEERING DOCUMENTS SEE WALLACE ENGINEERING DOCUMENTS
DESIGN BASE SHEAR SEISMIC RESPONSE COEFFICIENT RESPONSE MODIFICATION FACTOR ANALYSIS PROCEDURE		V: SEE WALLACE DESIGN COLLECTIVE DOCUMENTS Cs: SEE WALLACE DESIGN COLLECTIVE DOCUMENTS R: SEE WALLACE DESIGN COLLECTIVE DOCUMENTS EQUIVALENT LATERAL FORCE METHOD (PER ASCE 7-10, TABLE 12.6-1 & SECT. 12.8)

CODES: 2021 ARKANSAS FIRE PREVENTION CODE
A.C.A. 12-80-101 ET. SEQ. (ARKANSAS STATE LAW)

CONSTRUCTION DOCUMENTS PROVIDED BY ENGINEERING CONSULTANTS, INC. ARE FOR NEW STRUCTURAL ELEMENTS OUTSIDE THE EXISTING BUILDING ENVELOPE AND ARE DEPENDENT ON THE EXISTING STRUCTURE. WITHOUT STRUCTURAL MODIFICATIONS AND STRENGTHENING TO THE EXISTING STRUCTURE THE NEW STRUCTURAL ELEMENTS WILL NOT PERFORM AS DESIGNED. SEE CONSTRUCTION DOCUMENTS BY WALLACE DESIGN COLLECTIVE FOR REQUIRED STRUCTURAL MODIFICATIONS AND STRENGTHENING.

THE FOUNDATIONS AND STRUCTURAL FRAMING HAVE BEEN DESIGNED TO RESIST THE LOADS AND FORCES STATED ABOVE IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2012 ARKANSAS FIRE PREVENTION CODE AND A.C.A. 12-80-101 ET. SEQ.

PRE-FABRICATED LIGHT GAUGE METAL TRUSS DESIGN LOADS:

ROOF TRUSSES

DEAD LOAD:	5 PSF (TOP CHORD) 5 PSF (BOTTOM CHORD)
COLLATERAL LOAD:	5 PSF (TOP CHORD) 5 PSF (BOTTOM CHORD)
LIVE LOAD:	20 PSF (NON-REDUCIBLE) TOP CHORD 5 PSF (NON- REDUCIBLE) BOTTOM CHORD
WIND LOAD:	(SEE DESIGN LOADS ABOVE) DO NOT USE COLLATERAL LOAD IN COMBINATION WITH WIND LOAD. TRUSSES SHALL BE DESIGNED FOR COMPONENTS & CLADDING WIND PRESSURES
SNOW LOAD	(SEE DESIGN LOADS ABOVE)
SEISMIC LOAD:	(SEE DESIGN LOADS ABOVE) DO NOT USE COLLATERAL LOAD IN COMBINATION WITH SEISMIC LOAD
CODES:	2021 ARKANSAS FIRE PREVENTION CODE A.C.A. 12-80-101 ET. SEQ. (ARKANSAS STATE LAW)

SPECIAL INSPECTION NOTES

1. SPECIAL INSPECTIONS SHALL BE REQUIRED IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL INSPECTIONS WITH THE INSPECTION AGENTS.
2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE TO PERFORM THE REQUIRED INSPECTION TO THE SATISFACTION OF THE BUILDING OFFICIAL.
3. THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF INSPECTIONS. INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.
4. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT