

SPECIAL INSPECTIONS

- 1. Special Inspections shall be performed in accordance with Section 1705 of 2021 IBC. An independent testing agency shall be employed to provide Special Inspections during construction on the types of work listed under Section 1705. The following areas of work require Special Inspections in accordance with 2021 IBC.
2. Refer to project specification for additional quality control/quality assurance requirements.
3. Construction Manager/Contractor shall coordinate any additional Special Inspection requirements with the Owner and applicable building authorities.
4. Special Inspections are not the responsibility of the Structural Engineer of Record.
5. Special Inspections shall be paid for directly by the Construction Manager.
6. Copies of all Special Inspections Reports shall be emailed to the SEOR Andrew Deschenes, P.E., (asd@tswstructural.com) or their designate within seven (7) calendar days of completing the individual inspection(s).

STRUCTURAL STEEL

(IBC 1705.2.1, 1705.13.1 & 1705.14.1)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16; TABLE J6-1, AISC 341-16), Welder identification, Fit-up groove welds, Access holes, Fit-up of fillet welds, Check welding equipment, Welder qualification records and continuity records.

DURING WELDING (TABLE N5.4-2, AISC 360-16; TABLE J6-2, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Use of qualified welders, Control and handling of welding consumables, Cracked tack welds, Environmental conditions, WPS followed, Welding techniques, Steel headed stud anchors.

AFTER WELDING (TABLE N5.4-3, AISC 360-16; TABLE J6-3, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Welds cleaned, Size, length, and location of welds, Welds meet visual acceptance criteria, Arc strikes, k-area, Backing & weld tabs removed and finished, and fillet welds added (if required), Repair activities, Document acceptance or rejection of welded joint/member, Placement of reinforcing or contouring fillet welds, Weld access holes, Prohibited welds.

STEEL ELEMENTS OF COMPOSITE CONSTRUCTION (TABLES J9-1 thru J9-3, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Reinforcing steel, Composite member size.

OTHER STEEL INSPECTIONS (SECTION N5.7 & N5.8, AISC 360-16; TABLES J8-1 & J10-1, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Structural steel details (fabricated steel or steel frames), Anchor rods and other embedments supporting structural steel, Reduced beam sections (RBS), Protected zones, H-piles, Galvanized structural steel.

STRUCTURAL STEEL (CONT.)

(IBC 1705.2.1, 1705.13.1, & 1705.14.1)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-16; TABLE J7-1, AISC 341-16), Manufacturer's certifications, Fasteners marked, Fastener selection, Bolting procedure, Connecting surfaces, Pre-installation verification testing by installation personnel, Fastener storage.

DURING BOLTING (TABLE N5.6-2, AISC 360-16; TABLE J7-2, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Position of fasteners, Joint brought into snug-tight condition prior to the pretensioning operation, Fastener components not turned by the wrench are prevented from rotating, Pretensioning of fasteners.

AFTER BOLTING (TABLE N5.6-3, AISC 360-16; TABLE J7-3, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Row: Document acceptance or rejection of bolted connections.

STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

(IBC 1705.2)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: STEEL ROOF AND FLOOR DECKS (IBC TABLE 1705.2.2/SDI QA/QC 6.1), Material verification of cold-formed steel deck, Floor and roof deck welding, Floor and deck mechanical fasteners, Deck installation.

OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC TABLE 1705.2.3)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: End connections - welding or bolted, Bridging - horizontal or diagonal.

WELDING OF REINFORCING STEEL (IBC 1705.3.1, TABLE 1705.3)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verification of weldability, Inspect single pass fillet welds, max 5/16", Inspect all other welds.

COLD-FORMED STEEL CONSTRUCTION (IBC 1705.2.2, 1705.2.4, 1705.12.2, & 1705.13.3)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Trusses spanning > 60-feet, Welding in wind-force-resisting systems or seismic-force-resisting systems, Floor and roof deck welds.

CONCRETE CONSTRUCTION

(IBC 1705.3; TABLES J9-2 & J9-3, AISC 341-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Reinforcing steel, including prestressing tendons, Cast-in anchors, Post-installed anchors, Use of required mix design, Concrete sampling for strength tests, slump, air content, and temperature, Concrete & shotcrete placement, Curing temperature and techniques, Pre-stressed concrete, Erection of precast concrete, Strength verification, Formwork, Limits on water added at the truck or pump, Proper placement techniques to limit segregation, Embedded installation of the precast concrete diaphragm connections, Verification installation tolerances of precast concrete diaphragm connections.

INSPECTION OF FABRICATORS

(IBC 1704.2)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verification & Inspection, Submittal of certificate of compliance.

SOILS CONSTRUCTION

(IBC 1705.6)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verify subgrade is adequate to achieve design bearing capacity, Verify excavations extend to proper depth and material, Verify subgrade has been appropriately prepared prior to placing compacted fill, Perform classification and testing of compacted fill materials, Verify proper materials, densities and lift thicknesses.

MASONRY CONSTRUCTION

(IBC 1705.4)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Row: PRIOR TO CONSTRUCTION (ARTICLE 1.5, TMS-602-16)

AS CONSTRUCTION BEGINS (TABLE 4, TMS-602-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Proportions of site-prepared mortar, Grade, type, and size of reinforcement, connectors, anchor bolts, and prestressing tendons and anchorages, Sample panel construction.

DURING MASONRY CONSTRUCTION (TABLE 4, TMS-602-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Size and location of structural elements, Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction, Welding of reinforcement, Preparation, construction, and protection of masonry during cold weather (<40°F) or hot weather (>90°F), Observation of preparation of grout specimens/prism preparation performed as required by Article 1.4 of TMS 602-16, Confirm specimen/prism preparation performed as required by Article 1.4 of TMS 602-16, Continuous inspection is required for Risk Category IV buildings, Materials and procedures with the approved submittals.

PRIOR TO GROUTING (TABLE 4, TMS-602-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Grout space, Placement of reinforcement, connectors, and anchor bolts, Proportions of site-prepared grout.

MINIMUM TESTING (TABLE 3, TMS 602-16)

Table with columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verification of slump flow and Visual Stability Index (VSI) for self-consolidating grout, Verification of fm and FAAC, Verification of proportions of materials in grout and premixed or preblended mortar.

PRELIMINARY 30% CD

Hight Jackson ASSOCIATES 5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72768 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR BWHS - Locker Room Building Centerton, AR

DRAWN BY: TMW CHECKED BY: ASD ISSUE DATE: 02/02/2026 PROJECT NO: 0000

REVISION DATES

REQUIRED IBC SPECIAL INSPECTIONS SHEET S1.0 COPYRIGHT 2026 THESE DRAWINGS AND SPECIFICATIONS ARE THE EXCLUSIVE PROPERTY OF HIGHT JACKSON ASSOCIATES PA USE OR REPRODUCTION IS PROHIBITED WITHOUT WRITTEN CONSENT



STRUCTURAL ENGINEERS (479) 621-6128 ROGERS, ARKANSAS TSW #: 26004 PM: ASD DE: BWA

1/30/2026 4:35:05 PM C:\Users\mmw\Documents\26004 - BWHS Arts & Athletics Addition - Locker - Bentonville, AR\_STRUCTURE\_R25\_tmw\FKCK-vrt

**CONSTRUCTION SAFETY GENERAL NOTE**

THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, TEMPORARY SHORING/BRACING, OR FOR SAFETY PRECAUTIONS AND PROGRAMS, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

**Concrete General Notes 3100:**

- All detailing, fabrication and placing of reinforcing steel shall conform to the ACI Standard "Details and Detailing of Concrete Reinforcement" (ACI 315).
- Concrete at slab-on-grade shall develop a 28-day minimum compressive strength of 3,500 psi and have Ultra Fiber 500 manufactured by Solomon Colors, Inc. mixed into the concrete at 2.0 lb/cubic yard. Follow normal mixing time and speed as recommended by ASTM C94.
- All concrete at footings, pedestals and walls shall typically develop a 28-day minimum compressive strength of 3,000 psi.
- All concrete for footings, walls and slab-on-grade shall have a 5" maximum slump.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All reinforcing bar splices shall be 44 bar diameters for #6 and smaller diameter bars. Reinforcing bar splices shall be 48 bar diameters for #7 and larger bar diameters.
- All reinforcing bar hooks shall be ACI standard 90 degree hook, unless noted otherwise.
- Provide two #4 x 4'-0" long diagonal bars centered in slab, at all re-entrant corners and any other locations designated on the plans.
- Provide corner bars in footings, concrete stemwall and turn-down slab same size and spacing as longitudinal reinforcing.
- Provide (1) #4 hoop with 8" lap in slab-on-grade and elevated slab around floor drains, columns and all slab penetrations 3" in diameter or greater. Also install around electrical conduit groupings 3" in diameter or greater.
- Limit the width of conduit groups to 3'-0" as they pass under wall footings. As much as possible, align the conduit perpendicular to the footing as it passes under the footing. Provide a minimum spacing of 2'-0" between conduit groups as the conduit passes under a footing. **Do not extend conduits under column footings or spread footings**
- Plate dowel system shall be Diamond Dowel System by PNA Construction Technologies, the Speed Plate System by SIKA Corp. or approved equal. Install plate dowels at slab construction joints at 18" o.c.
- Smooth dowels shall be steel conforming to ASTM A36.
- All slots, sleeves and other embedded items shall be set before concrete is placed. See Architectural, Electrical, Mechanical, and Vendor's drawings for size and locations.
- Bar supports at footings, slabs-on-grade and elevated slabs shall be factory made wire bar supports, type "SBU" linear supports.
- Epoxy for doweling reinforcement shall be HY-200 by Hilti, AT 3G by Simpson Strong-Tie or AC208+ by Dewart, unless noted otherwise.
- Maximum net allowable bearing pressure = 2,000 psf for continuous footings and 2,500 psf for isolated spread footings. Footings shall bear on natural, stiff to very stiff, sandy clean clay and sandy clay soils as described in the Soils Report No. A25166.00135.000 dated January 26th, 2026.
- Use of compacted, free-draining pea gravel, crushed stone, or coarse sand underneath the building slab is recommended by TSW, Inc. Consult Geotechnical Engineer regarding potential substitution of free-draining coarse materials with approved subgrade. Slabs-On-Grade have been designed for a modulus of subgrade reaction (k-value) of 100 psi/in.

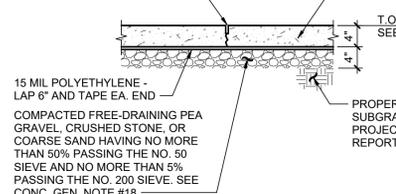
**NOTE:**  
EXPANSIVE CLAYS, LOW-STRENGTH SOILS, SOFT SOILS AND SOILS WITH ORGANIC MATERIAL ARE NOT SUITABLE IN SUPPORTING THE SLAB AND FOUNDATIONS. IF CONTRACTOR DISCOVERS UNSTABLE MATERIAL DURING EARTHWORK, A GEOTECHNICAL ENGINEER SHALL BE CONSULTED IN ORDER TO ARRIVE AT A SOLUTION THAT WILL NOT COMPROMISE THE STRUCTURAL INTEGRITY OF THE SLAB AND FOUNDATIONS.

**Concrete Masonry General Notes 4100:**

- All concrete masonry units shall be lightweight above finished floor and normal weight below grade. All hollow concrete masonry units shall conform to ASTM C90, Grade N, Type 1 with a minimum ultimate compressive prism strength (Fm) of 2,000 psi for the masonry assemblage. All concrete masonry shall be laid in Running (Common) Bond.
- Mortar at exterior walls, all load-bearing walls, walls below grade and non-load-bearing walls higher than 20'-0" shall be Type S mortar and have a minimum compressive strength of 1,800 psi. Mortar at interior non-load-bearing walls not higher than 20'-0" and mortar at masonry veneer shall be Type N mortar and have a minimum compressive strength of 750 psi. All mortar shall conform to ASTM C270. **Masonry cement shall not be used for mortar.**
- All grout shall be ready-mix concrete, with 3/8" diameter max. aggregate, have a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10" or preblended product (Core Fill Grout, Coarse CF-02, by Spec Mix) with a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10".
- All 8" CMU bond beam units shall be reinforced with one bar. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of bars for vertical wall reinforcement and bond beam requirements. Provide corner bars and lap bond beam reinforcing 48 bar diameters.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All bolts, anchors, reinforcement and embedded items shall be grouted in place.
- All reinforcing bar splices shall be 48 bar diameters, U.N.O.
- At all 8" CMU walls except at interior non-load bearing walls, provide (1) vertical bar each cell for the first (2) cells adjacent to control joints in walls, at ends of walls, wall corners and on each side of wall openings, unless noted otherwise. Vertical bars shall match reinforcement for remainder of wall. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of reinforcement.
- Provide control joints in brick veneer where shown on Architectural Drawings.
- Provide control joints in CMU walls where shown on Drawing S3.0. Place joints for CMU walls max. 24'-0" o.c.
- Provide horizontal joint reinforcement at 16" o.c. Reinforcement shall be ladder design, min. 9 gage welded steel wire, not dipped galvanized to 1.5 oz. width shall be 1 1/2" less than wall thickness.

**NOTE:**  
ALL SLAB-ON-GRADE SHALL HAVE ULTRAFIBER 500 REINFORCING FIBERS AT DOSAGE OF 2.0 LB./ CUBIC YD. AS MANUFACTURED BY SOLOMON COLORS, INC.

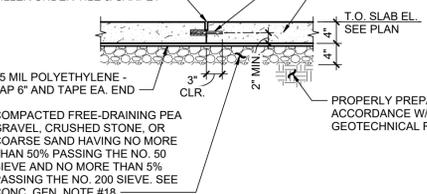
SAWCUT JOINT 1/8" x 1 1/2" DEEP TO PRODUCE VERTICAL SIDEWALL & HORIZONTAL SHELF TO SUPPORT JOINT FILLER. FILL W/ JOINT FILLER AT EXPOSED JOINTS & FILL W/ SUBFLOOR FILLER UNDER TILE & CARPET



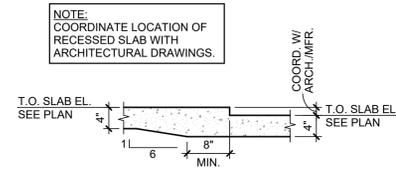
**1 TYP. SLAB-ON-GRADE CONTROL JT. DTL. (CJ)**  
NOT TO SCALE

**NOTE:**  
PROVIDE DIAMOND DOWEL SYSTEM AS MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES OR THE SPEED SYSTEM BY SIKA CORPORATION AT CONSTRUCTION JOINTS IN ALL FLOOR AREAS. COORDINATE WITH ARCHITECT FOR CONSTRUCTION JOINT LOCATIONS.

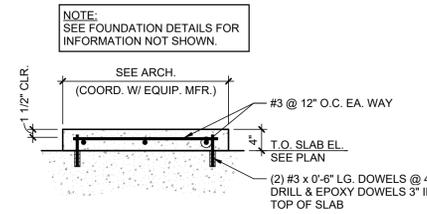
RESAW JOINT 1/8" x 1 1/2" DEEP TO PRODUCE VERTICAL SIDEWALL & HORIZONTAL SHELF TO SUPPORT JOINT FILLER. FILL W/ JOINT FILLER AT EXPOSED JOINTS & FILL W/ SUBFLOOR FILLER UNDER TILE & CARPET



**2 TYP. SLAB-ON-GRADE CONSTRUCTION JT. DTL. (CJ)**  
NOT TO SCALE



**3 TYP. RECESSED SLAB**  
NOT TO SCALE



**4 TYP. LOCKER PAD FOUNDATION**  
3/4" = 1'-0"

**TYPICAL STRUCTURAL ABBREVIATIONS**

A.R.	ANCHOR ROD	FDN	FOUNDATION	PED	PEDESTAL
ACI	AMERICAN CONCRETE INSTITUTE	FIN	FINISH	PL	PLATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	FLR	FLOOR	PLF	POUNDS PER FOOT
ARCH	ARCHITECT	FTG	FOOTING	PROJ	PROJECTION
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	GA	GAUGE	PSF	POUNDS PER SQUARE FOOT
BLDG	BUILDING	GALV	GALVANIZED	PSI	POUNDS PER SQUARE INCH
BM	BEAM	H.S.A.	HEADED STUD ANCHOR	R	RADIUS
BOTT	BOTTOM	HK	HOOK	REIN	REINFORCEMENT
BRC	BEARING	HORIZ	HORIZONTAL	REQ'D	REQUIRED
BTWN	BETWEEN	J.B.E.	JOIST BEARING	RTU	ROOF TOP UNIT
CFS	COLD-FORMED STEEL	JT	JOINT	S.O.G.	SLAB ON GRADE
CL	CENTER LINE	LD	LONG DIMENSION	SCHED	SCHEDULE
CLR	CLEAR	LDH	LONG DIMENSION HORIZONTAL	SECT	SECTION
COL	COLUMN	LDV	LONG DIMENSION VERTICAL	SEOR	STRUCTURAL ENGINEER OF RECORD
CONC	CONCRETE	LLV	LONG LENGTH VERTICAL	SIM	SIMILAR
CONN	CONNECTION	LG	LONG	SJI	STEEL JOIST INSTITUTE
CONT	CONTINUOUS	LLH	LONG LENGTH HORIZONTAL	SPA	SPACING
DEFL.	DEFLECTION	LLV	LONG LENGTH VERTICAL	SPECS	SPECIFICATIONS
DIA, or Ø	DIAMETER	LONG	LONGITUDINAL	STD	STANDARD
DIM	DIMENSION	MAX	MAXIMUM	STIFF	STIFFENER
DN	DOWN	MBM	METAL BUILDING MANUFACTURER	STL	STEEL
do	DITTO	MECH	MECHANICAL	TOC	TOP OF CONCRETE
DTL	DETAIL	MFR	MANUFACTURER	TOF	TOP OF FOOTING
DWG	DRAWING	MIN	MINIMUM	TOS	TOP OF STEEL
E.F.	EACH FACE	MISC	MISCELLANEOUS	TOTB	TOP OF TIE BEAM
E.W.	EACH WAY	MTL	METAL	TOW	TOP OF WALL
EA	EACH	N.S.	NEAR SIDE	TRANS	TRANSVERSE
EL	ELEVATION	O.C.	ON CENTER	TYP	TYPICAL
EQ	EQUAL	O.F.	OUTSIDE FACE	U.N.O.	UNLESS NOTED OTHERWISE
EXIST	EXISTING	OPNG	OPENING	VERT	VERTICAL
EXP	EXPANSION	P.A.F.	POWDERED ACTUATED FASTENER	W	WITH
F.S.	FAR SIDE	WP	WORK POINT	WF	WELDED WIRE FABRIC
F.V.	FIELD VERIFY				

**PRELIMINARY**  
30% CD

**Hight Jackson ASSOCIATES**  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72768 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
**BWHS - Locker Room Building**  
Centerton, AR

DRAWN BY:  
**TMW**  
CHECK BY:  
**ASD**  
ISSUE DATE:  
**02/02/2026**

PROJECT NO:  
**0000**

REVISION DATES

FOUNDATION GENERAL NOTES & TYP. DETAILS

SHEET  
**S1.1**

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE THE EXCLUSIVE PROPERTY OF HIGHT JACKSON ASSOCIATES PA. USE OR REPRODUCTION IS PROHIBITED WITHOUT WRITTEN CONSENT.

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

1/30/2026 4:35:06 PM C:\Users\mm\Documents\26004 - BWHS Arts & Athletics Addition - Locker - BWHS Arts & Athletics Addition - Locker - AR\_STRUCT\_PL25\_tmw\FKCK-vt

**Structural Steel General Notes 5100:**

- All detailing, fabrication and erection of structural steel shall conform to the requirements of the AISC Specification for the Design, Fabrication and Erection of Structural Steel for Buildings.
- Wide flanges and WT tees shall conform to ASTM A992 with a yield strength of 50 ksi.
- Hollow Structural Section (HSS) shall conform to ASTM A500, Grade C with a yield strength of 50 ksi.
- Steel pipes shall conform to ASTM A501 or ASTM A53, Grade B.
- Headed stud anchors (H.S.A.'s) shall conform to ASTM A108.
- All other structural steel shall conform to the requirements of ASTM A36. Angle, plate and beam lintels at exterior wall openings shall be hot-dipped galvanized.
- All anchor rods shall conform to ASTM F1554, Grade 36. Nuts for anchor rods shall conform to ASTM A563, Grade A, heavy hex and anchor rod washers shall conform to ASTM F844.
- All welding shall conform to the Specifications of the American Welding Society. Welding electrodes shall be E-70 low hydrogen series. Welding shall be done by a certified welder.
- High strength bolts shall typically be 3/4" diameter bolts conforming to ASTM A325. Connections shall be designed as bearing type with threads in shear plane. Holes shall be 1/16" larger than bolt size. See details for connections with 1" diameter bolts.
- All bolts shall be tightened to a snug-tight condition. A snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. All connected elements must be brought into snug contact.
- Bearing ends of all columns shall be square cut.
- No openings shall be cut in structural members unless shown on the drawings.
- Steel frame is non-self-supporting and column anchor rods are designed for a completed condition only. Metal roof deck and masonry shear walls are required to provide lateral stability for the frame and resistance to wind and seismic forces. Contractor shall provide all temporary bracing required to maintain stability of structural system.
- Anchor rod holes in base plates shall be sized in accordance with AISC "Detailing For Steel Construction".
- All exposed edges of plates, beams, etc., shall be shop ground smooth and uniform.
- All column base plates supported on footings, pedestals and concrete columns shall bear on 1 1/2" non-shrink non-metallic grout unless noted otherwise.
- 1/2"Ø sleeve anchors shall be 1/2"Ø x 4" long HLC-H Sleeve Anchor by Hilli. HLC-H Sleeve Anchor shall be carbon steel with zinc plating & have a 3" embedment depth.
- 1/2"Ø expansion bolts shall be 1/2"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 3 1/2" embedment depth.
- 3/4"Ø expansion bolts shall be 3/4"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 4 3/4" embedment depth.

GENERAL CONTRACTOR SHALL INCLUDE 1 TON AT A MINIMUM COST OF \$5,000/ TON OF MISCELLANEOUS STEEL BEAMS, CHANNELS AND ANGLES IN ADDITION TO THE FRAMING SHOWN ON THE PLANS AND DETAILS. GENERAL CONTRACTOR SHALL INCLUDE ERECTION, FABRICATION, DESIGN AND DETAILING COSTS FOR THIS ADDITIONAL FRAMING WITH THE BASE BID. THE USE OF MISCELLANEOUS STEEL IS TO BE RECORDED BY THE GENERAL CONTRACTOR AND ANY UNUSED AMOUNT IS TO BE CREDITED TO THE OWNER.

**Steel Deck General Notes 5300:**

- Typical roof deck shall be 1 1/2" deep, 22 gauge, wide rib type and shall have nested side laps (Vulcraft 1.5B22, New Millennium 1.5B22 or approved equal). See Roof Framing Plans for limits of roof deck.
- Roof deck shall be welded to the steel framing per the Roof Deck Fastening Pattern Detail 1/S1.2.
- Roof deck fastening pattern has been designed for a net wind uplift of 44.0 psf at corner zones, 35.1 psf at side zones and 20.3 psf at interior zones for roofs.
- All deck shall be fastened per Steel Deck Institute (SDI) requirements.
- Deck specified has been determined on basis of 3 span condition; deck supplier shall use heavier gauge if required for one and two span conditions.

**Light Gauge Steel General Notes 5400:**

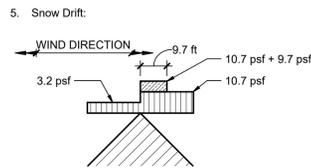
- All structural studs and headers shall be of the type, size, gauge and spacing as shown on the drawings.
- All framing members shall be formed from steel, corresponding to the requirements of ASTM C955.
- Steel for framing members shall have a minimum yield strength of 33 ksi, unless noted otherwise.
- Exterior stud walls and lintels shall be constructed with the following, unless noted otherwise.
  - 6" studs: 600S162-43
  - 6" track: 600T125-43
- See Architectural Drawings for summary of non-structural stud sizes; runner track gauge to match stud gauge.
- Load-bearing studs shall be braced in accordance with the manufacturer's specifications and recommendations. See Metal Stud Bridging Detail 3/S1.2.
- Slide clips shall be VertiClip SLB with vertical slots by The Steel Network, Inc. or approved equal.
- Rigid clips shall be StiffClip LB by The Steel Network, Inc. or approved equal.
- Powder actuated fasteners for attachment of bottom runner track shall have a 0.157" minimum shank diameter and a 1 1/2" minimum concrete embedment. Space fasteners at 16" o.c., unless noted otherwise.
- At interior stud walls extending to the underside of roof structures, provide a deflection track at the top of the wall.
- Provide bracing to structure for all stud walls and furr-downs.
- See Lintel Schedule on Drawing S3.0 for metal stud wall headers.

**Prefabricated Light Gauge Steel Truss General Notes 5410:**

- The material and fabrication criteria of trusses shall meet the requirements of "Specifications for the Design of Cold Formed Steel Structural Members" by AISI.
- Truss members shall be fabricated of structural quality steel sheet with a protective G60 zinc coating per ASTM A653. Steel shall have a minimum yield strength of 40,000 psi.
- Truss members shall have a 20 gauge minimum thickness.
- Fasteners at truss connections shall be self-drilling, self-tapping screws with corrosion-resistant plated finish per the truss manufacturer's recommendations.
- During erection, care shall be exercised to keep horizontal bending of trusses to a minimum. Proper erection bridging and bracing shall be installed to hold the trusses true and plumb and in a safe condition until permanent truss bridging and decking are installed.
- Install continuous bridging and permanent bracing per the truss manufacturer's requirements.
- Repair damaged galvanized coatings on truss members with galvanizing repair paint according to ASTM A780 and the truss manufacturer's instructions.
- Truss manufacturer shall design trusses for a 20 psf dead load (10 psf top chord and 10 psf bottom chord dead load).
- Trusses shall be designed for a 13.9 psf gross wind uplift.
- The truss manufacturer shall prepare complete fabrication and erection drawings, fully engineered and sealed by a registered structural engineer in the state of Arkansas.

**Design Loads**

- Typical Roof Dead Load: 20 psf
- Roof Live Load: 20 psf
- Rain Intensity, (15 min. i): 6.64 in/hr
- Snow Load: 15 psf
  - Ground Snow Load:
  - Flat-roof Snow Load at main roof (P<sub>f</sub>) = 11.6 psf
  - Slope Factor (C<sub>s</sub>) = 0.93
  - Snow Exposure Factor (C<sub>e</sub>) = 1.0
  - Snow Load Importance Factor (I<sub>s</sub>) = 1.1
  - Thermal Factor (C<sub>t</sub>) = 1.0

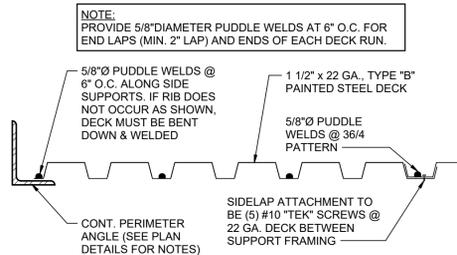


**Unbalanced Snow Load**

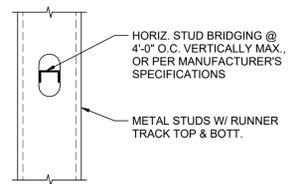
- Wind Load:
  - Ultimate Design Wind Speed (V<sub>ult</sub>): 115 mph
  - Nominal Design Wind Speed (V<sub>nom</sub>): 89.1 mph
  - Risk Category II
  - Wind Exposure C
  - Internal Pressure Coefficient, GC<sub>pi</sub> = ±0.18
- Components & Cladding Wind Load (Unfactored):
  - Width of Edge Zone, a = 5.60 ft
  - Wall Pressures (10 ft<sup>2</sup>):
    - End Zone Wall = 35.6 psf
    - Interior Zone = 28.9 psf
  - Wall Pressures (100 ft<sup>2</sup>):
    - End Zone Wall = 27.7 psf
    - Interior Zone = 25.0 psf
  - Roof Pressures (10 ft<sup>2</sup>):
    - Corner Zone = 93.4 psf
    - Eave & Rake Zone = 78.5 psf
    - Interior Zone = 53.8 psf
  - Roof Pressures (100 ft<sup>2</sup>):
    - Corner Zone = 48.9 psf
    - Eave & Rake Zone = 43.2 psf
    - Interior Zone = 16.8 psf

- Seismic:
  - Risk Category II
  - Seismic Importance Factor (I<sub>s</sub>) = 1.0
  - S<sub>s</sub> = 0.148
  - S<sub>1</sub> = 0.087
  - S<sub>0.5</sub> = 0.158
  - S<sub>0.1</sub> = 0.140
  - Site Class D (per Geotechnical Report)
  - Seismic Design Category C
  - Basic Structural System: Bearing Wall System
  - Seismic Resisting System: Intermediate Reinforced Masonry Shear Walls
  - Response Modification Coefficient (R): 3.500
  - Deflection Amplification Factor (C<sub>d</sub>): 2.250
  - Seismic Response Coefficient (C<sub>s</sub>): 0.045
  - Analysis Procedure: Equivalent Lateral Force Procedure
- Building Code:
  - 2021 Arkansas Fire Prevention Code, Volume II - Adopting 2021 International Building Code
  - ASCE 7-16

THIS FACILITY HAS BEEN DESIGNED FOR THE SEISMIC CRITERIA AND BUILDING CODE NOTED ON THIS DRAWING IN ACCORDANCE WITH THE REQUIREMENTS OF ACT 1100.

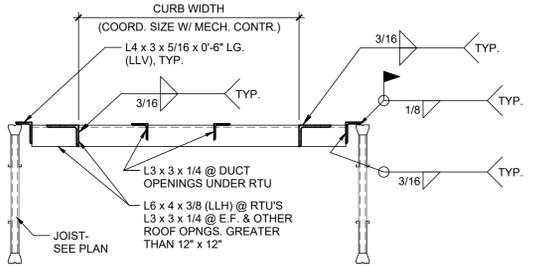


**1 1/2" ROOF DECK**  
**TYP. ROOF DECK FASTENING PATTERN DETAIL**  
NOT TO SCALE



**3 TYP. METAL STUD BRIDGING DETAIL**  
NOT TO SCALE

NOTES:  
1. AT L3 x 3 x 1/4 FRAMES, DELETE CLIP ANGLES, COPE VERTICAL LEG AND EXTEND HORIZONTAL LEG OVER FRAMING - WELD ALL AROUND.  
2. FOR ROOF OPENINGS OVER 6" TO 12" WIDE (I.E.: OVER-SIZED OPENING FOR ROOF DRAINS), PROVIDE L2 x 2 x 1/4 FRAME AROUND OPENING AND EXTEND TO JOISTS- WELD ALL AROUND.



**2 TYP. ROOF FRAMED OPNG. DETAIL**  
NOT TO SCALE

PRELIMINARY  
30% CD

Hight Jackson ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72761 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
**BWHS - Locker Room Building**  
Centerion, AR

DRAWN BY:  
**TMW**  
CHECK BY:  
**ASD**  
ISSUE DATE:  
**02/02/2026**

PROJECT NO:  
**0000**

REVISION DATES

FRAMING GENERAL NOTES & TYP. DETAILS  
SHEET  
**S1.2**  
COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE THE EXCLUSIVE PROPERTY OF HIGHT JACKSON ASSOCIATES PA. USE OR REPRODUCTION IS PROHIBITED WITHOUT WRITTEN CONSENT.

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

1/30/2026 4:35:07 PM C:\Users\tmw\Documents\26004 - BWHS Arts & Athletics Addition - Locker - Bentonville, AR\_STRUCTURE\_R25.tmw\FKCK.rvt

PRELIMINARY  
30% CD

Hight Jackson  
ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72781 | (479) 464-4965 | www.hjarch.com

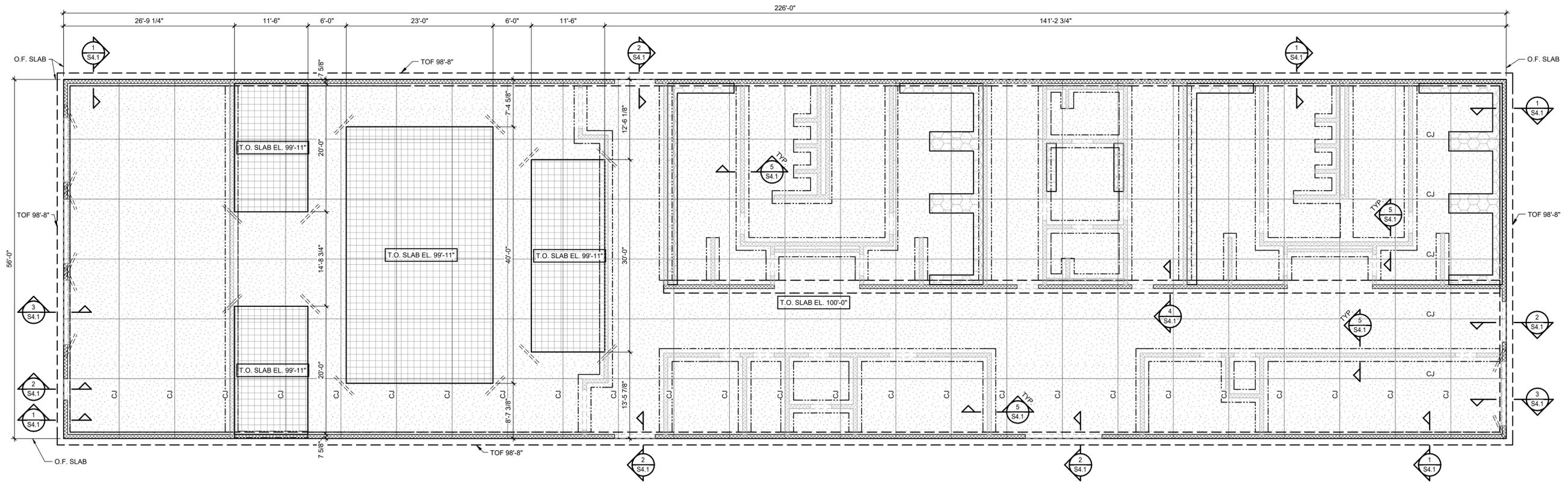
A NEW FACILITY FOR  
BWHS - Locker Room Building  
Centerton, AR

DRAWN BY:  
TMW  
CHECK BY:  
ASD  
ISSUE DATE:  
02/02/2026

PROJECT NO:  
0000  
REVISION DATES

FOUNDATION PLAN  
SHEET  
S2.1

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE  
THE EXCLUSIVE PROPERTY OF  
HIGHT JACKSON ASSOCIATES P.A.  
USE OR REPRODUCTION IS PROHIBITED  
WITHOUT WRITTEN CONSENT



**1 FOUNDATION PLAN**  
1/8" = 1'-0"

**PLAN NOTES:**

- 4" S.O.G. CONSTRUCTION: 4" CONCRETE SLAB REINFORCED W/ ULTRA FIBER 500 ON 15 MIL. POLYETHYLENE FILM (COORD. W/ ARCH. SPECS.) OVER 4" CRUSHED STONE.
- ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. \_\_\_\_\_ AS ESTABLISHED BY \_\_\_\_\_
- TOP OF FOOTING ELEVATION 98'-8". TYP. (U.N.O.).
- SEE ARCH. DWGS / OVERALL FOUNDATION PLAN FOR DIMENSIONS NOT SHOWN.
- "CJ" DENOTES CONTROL OR CONSTRUCTION JOINT. (SEE DTL. 1/S1.1 & 2/S1.1).
- COORDINATE LOCATION & LIMITS OF VENEER WITH ARCH. DWGS.
- COORDINATE DOOR LOCATIONS WITH ARCH. DWGS.
- SEE ARCH. DWGS. FOR LOCATIONS OF NON-LOAD-BEARING MASONRY WALLS BEARING ON THICKENED SLABS.
- SEE ARCH. / PLUMBING DWGS. FOR EXACT LOCATIONS OF ALL FLOOR DRAINS, SLOPED & RECESSED SLABS. COORDINATE LOCATIONS OF PLUMBING LINES W/ PLUMBER PRIOR TO POURING FOOTINGS & SLABS-ON-GRADE.
- SEE DWG. S1.1 FOR GENERAL NOTES, FOOTING SCHEDULE & TYP. DETAILS.
- SEE ARCH. FLOOR PATTERN PLANS FOR SLAB LOCATIONS WITH POLISHED CONCRETE.

**LEGEND:**

- FOOTING MARK (SEE FTG. SCHED. ON DWG. S1.1)
- O.F. DENOTES OUTSIDE FACE
- TOF DENOTES TOP OF FOOTING
- DENOTES LIMITS OF 4" SLAB (T.O. SLAB EL. 100'-0") SEE PLAN NOTES
- DENOTES LIMITS LOCKER PAD (T.O. PAD EL. 100'-4") (SEE DTL. 4/S1.1)
- DENOTES LIMITS OF RECESSED SLAB COORD. LOCATIONS & RECESS W/ ARCH. (SEE DTL. 3/S1.1)
- DENOTES THICKENED SLAB
- DENOTES LOAD-BEARING CMU WALL
- DENOTES NON-LOAD-BEARING CMU WALL
- DENOTES CONCRETE STEM WALL
- DENOTES AREA & DIRECTION OF SLOPE TO FLOOR DRAIN 1/16"/FT. MIN. (U.N.O.)
- F.D. DENOTES FLOOR DRAIN
- DENOTES FLOOR DRAIN DISH OUT 1/16"/FT. MIN. (U.N.O.)
- DENOTES DIRECTION OF SLAB SLOPE (COORD. SLOPE W/ ARCH.)
- DENOTES TOP OF SLAB EL.
- DENOTES LOCATION OF RE-ENTRANT CORNER BARS (SEE CONC. GEN. NOTE #8)

1/30/2026 4:35:08 PM C:\Users\tmw\Documents\26004 - BWHS Mats & Athletics Addition - Locker - Centerton, AR\_STRUC\_T\_P25\_1.mw\FKCK.vrt

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

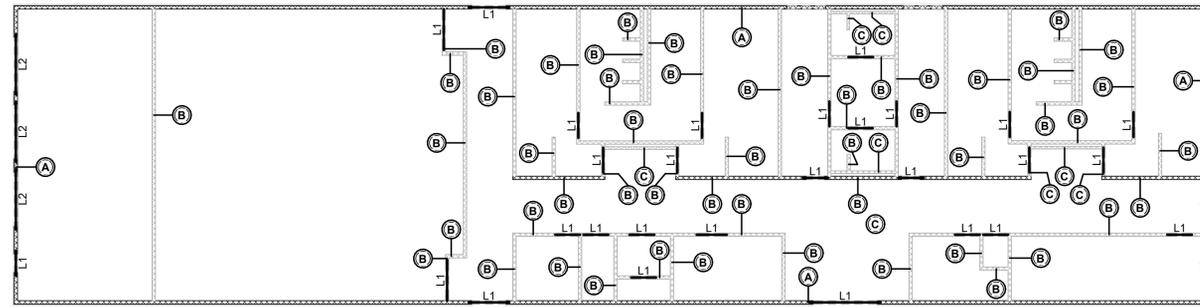
LINTEL SCHEDULE					
MARK	WALL LOCATIONS	TYPE & SIZE (THICKNESS x HEIGHT)	REINFORCEMENT OR ATTACHMENT	BRICK ANGLE OR PLATES	REMARKS
L1	8" CMU	CMU 8" x 8" BOND BM.	(1) #5 BOTT.	----	SEE NOTES #1, #2, #3, #4, #5, & #10
L2	8" CMU	CMU 8" x 8" OPEN BOTT. BOND BM. ON 8" x 8" CLOSED BOTT. BOND BM. (16" TOTAL DEPTH)	(2) #5 TOP & BOTTOM	----	SEE NOTES #1, #2, #3, #4 & #5
L3	8" CMU	W16 x 26	----	----	SEE NOTES #1, #2, #6, #7, #8 & #9 & DTL. 2a

- LINTEL SCHEDULE NOTES:**
- SEE ARCH. DWGS. FOR EXACT LOCATION OF ALL LINTELS.
  - COORDINATE ALL BOTTOM PLATE/BEAM/CMU/ANGLE ELEVATIONS WITH ARCH. DWGS.
  - ALL CMU LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
  - FILL ALL CMU LINTELS WITH 2,000 PSI GROUT.
  - ALL 8" CMU LINTELS SHALL HAVE A MIN. (2) CELLS OF (1) #5 JAMB STEEL EACH SIDE OF OPENING.
  - ALL STEEL LINTELS SHALL HAVE 6" MIN. BEARING EACH SIDE OF OPENING.
  - WELD TOP & BOTTOM PLATE TO FLANGE OF BEAM WITH 3/16" x 3" LG. FILLET WELD @ 12" O.C. ALONG EACH SIDE OF FLANGE.
  - ALL STEEL LINTELS SHALL HAVE 1/2" DIA. x 6" LG. H.S.A. @ 48" O.C. WELDED TO TOP FLANGE OR TOP PLATE.
  - AT EACH END OF STEEL LINTEL, STOP BOTTOM PLATE 1/2" CLEAR OF THE JAMB.
  - WHERE MECHANICAL DUCTS PASS THROUGH MASONRY WALLS, PROVIDE L1 LINTEL AT 8" CMU WALLS FOR OPENINGS NOT TO EXCEED 4'-0" WIDE. FOR OPENING WIDTHS FROM 4'-0" TO 6'-0", PROVIDE L1A LINTEL AT 8" CMU WALLS. FOR OPENING WIDTHS LARGER THAN 6'-0", COORDINATE WITH ARCH./ENGINEER.

MASONRY WALL REINFORCEMENT SCHEDULE						
MARK	WALL LOCATION	BOND BEAM REINF.	BOND BEAM LOCATIONS	VERT. REINF.	FOUNDATION DOWELS	REMARKS
A	8" CMU	(1) #5 BOTT.	TOP OF WALL, ROOF EL., & WHERE NOTED ON DWGS.	#5 @ 48" O.C.	#5 DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTE #2
B	8" CMU	(1) #5 BOTT.	TOP OF WALL	NONE	#4 x 2'-6" DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTES #1 & DTL. 3/S3.0
C	4" CMU	(1) #4 BOTT.	TOP OF WALL	NONE	#3 x 1'-6" DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTES #2, #3 & DTL. ___/S3.0

- MASONRY WALL REINFORCEMENT SCHEDULE NOTES:**
- WHERE TOP OF WALL IS UNSUPPORTED BY THE ROOF, BRACE TOP OF WALL WITH DIAGONAL L3 x 3 x 1/4 WELDED TO THE ROOF FRAMING ABOVE AT 8'-0" O.C. MAXIMUM (SEE DTL. 2/S3.0). WHERE WALL EXTENDS TO UNDERSIDE OF FLOOR OR ROOF DECK ABOVE, HOLD WALL DOWN 1/2" & SANDWICH TOP OF WALL WITH (2) L2 x 2 x 1/8 x 0'-6" LONG EACH ATTACHED TO DECK WITH (2) #8 SELF-TAPPING SCREWS (SPACE ANGLES AT 4'-0" O.C.). THIS BRACING IS PERMANENT BRACING REQUIRED FOR SEISMIC LOADS.
  - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 8" INTO SLAB.
  - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 6" INTO SLAB.

BEARING PLATE SCHEDULE					
LINTEL/BEAM SIZE	LINTEL BRG. PL SIZE (t x B x D)	BEAM BRG. PL SIZE (t x B x D)	HEADED STUD ANCHORS	SEE DTL.	REMARKS
ALL BEAM SIZES	3/8" x 7" x 11"	3/8" x 7" x 11"	(2) 1/2"Ø x 6" H.S.A.	4a/S3.0	8" CMU WALL
ALL BEAM SIZES	3/8" x 7" x 11"	3/8" x 7" x 11"	(2) 1/2"Ø x 6" H.S.A.	4b/S3.0	8" CMU WALL



### 1 LINTEL & MASONRY PLAN

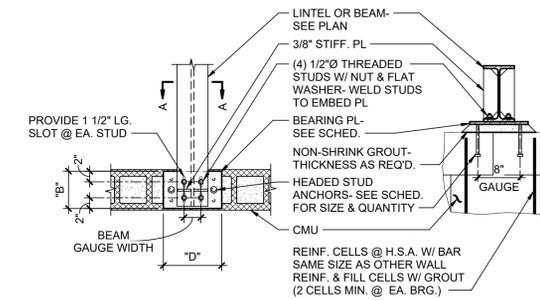
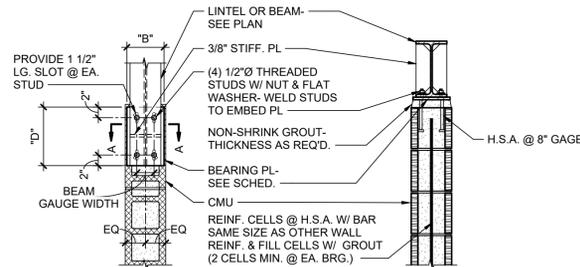
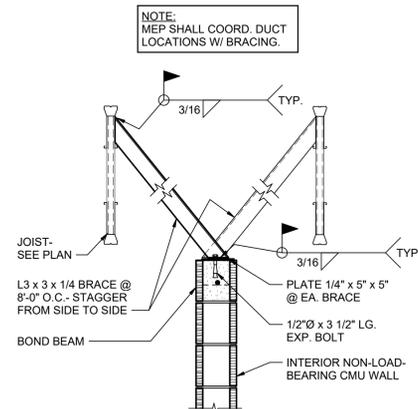
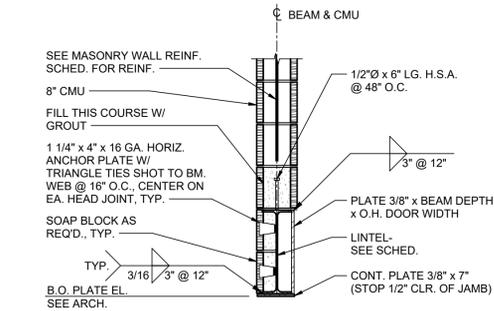
1/16" = 1'-0"

**PLAN NOTES:**

- SEE DWGS. S1.1 & S1.2 FOR GENERAL NOTES & TYP. DETAILS.

**LEGEND:**

- DENOTES LINTEL MARK, SEE LINTEL SCHED.
- DENOTES LOAD-BEARING CMU WALL
- DENOTES NON-LOAD-BEARING CMU WALL
- DENOTES MASONRY WALL REINF. MARK, SEE MASONRY WALL REINF. SCHED.
- DENOTES WALL CONTROL JOINT



1/30/2026 4:55:09 PM C:\Users\tmw\Documents\26004 - BWHS Addn - Locker - Bentonville, AR\_STRUC\_TPS.tmw\FKCK.rvt

PRELIMINARY  
30% CD

Hight Jackson  
ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72781 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
BWHS - Locker Room Building  
Centerton, AR

DRAWN BY:  
TMW  
CHECK BY:  
ASD  
ISSUE DATE:  
02/02/2026

PROJECT NO:  
0000

REVISION DATES

LINTEL & MASONRY PLAN  
& TYP. LINTEL & MASONRY  
DTLS.  
SHEET

S3.0

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE  
THE EXCLUSIVE PROPERTY OF  
HIGHT JACKSON ASSOCIATES PA  
USE OR REPRODUCTION IS PROHIBITED  
WITHOUT WRITTEN CONSENT

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

PRELIMINARY  
30% CD

Hight Jackson  
ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72758 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
BWHS - Locker Room Building  
Centeron, AR

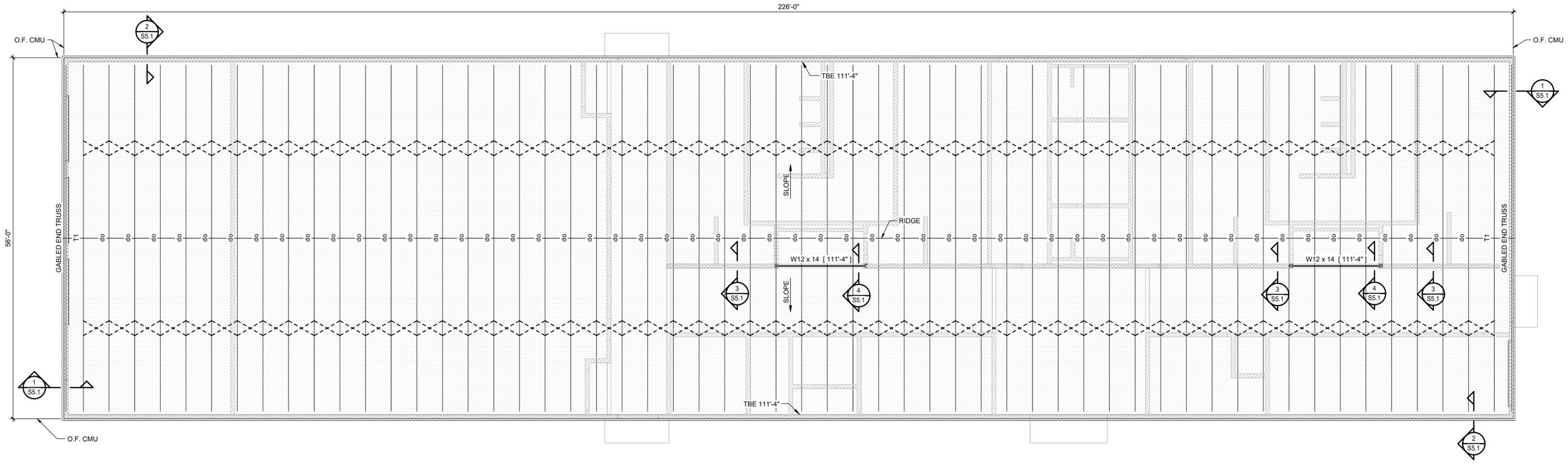
DRAWN BY:  
TMW  
CHECK BY:  
ASD  
ISSUE DATE:  
02/02/2026

PROJECT NO:  
0000  
REVISION DATES

ROOF FRAMING PLAN

SHEET  
S3.1

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE  
THE EXCLUSIVE PROPERTY OF  
HIGHT JACKSON ASSOCIATES PA  
USE OR REPRODUCTION IS PROHIBITED  
WITHOUT WRITTEN CONSENT



**1 ROOF FRAMING PLAN**  
1/8" = 1'-0"

**PLAN NOTES:**

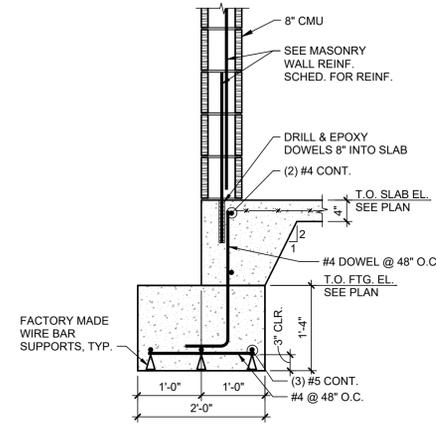
1. ROOF DECK: 1 1/2" DEEP, 22 GA. PAINTED WIDE RIB STEEL DECK (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS).
2. TOP OF STEEL DENOTES TOP OF MAIN STEEL.
3. TOP OF STEEL EL. VARIES, SEE PLAN.
4. LIGHT GAUGE METAL TRUSS BEARING ELEVATION 111'-4" TYP., (U.N.O.)
5. ALL ELEVATIONS BASED ON MAIN LEVEL FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. \_\_\_\_\_ AS ESTABLISHED BY \_\_\_\_\_.
6. SEE DWG. S1.2 FOR GENERAL NOTES AND TYPICAL DETAILS.
7. SEE ARCH. DWGS. & FOUNDATION PLAN FOR DIMENSIONS NOT SHOWN.

**LEGEND:**

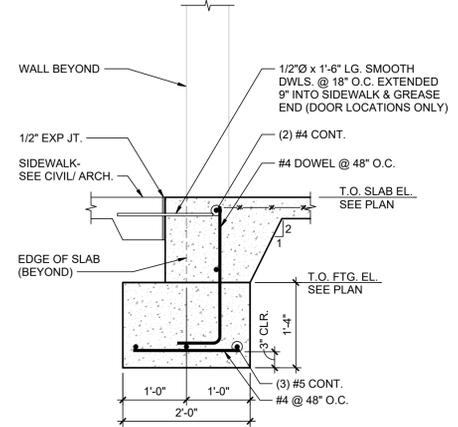
- O.F. DENOTES OUTSIDE FACE
- TBE DENOTES TRUSS BEARING ELEVATION
- TOS DENOTES TOP OF STEEL
- [111'-4"] DENOTES TOP OF BEAM ELEVATION
- DENOTES LIMITS OF 1 1/2" x 22 GA. ROOF DECK (SEE PLAN NOTES)
- ↙ SLOPE DENOTES DIRECTION OF ROOF SLOPE (COORD. PITCH W/ ARCH.)
- DENOTES BEARING PLATE (SEE BEARING PL. SCHED.)
- ▨ DENOTES LOAD-BEARING CMU WALL
- ▤ DENOTES NON-LOAD-BEARING CMU WALL
- ⋈ DENOTES DIAGONAL BRACING @ TRUSS BOTTOM CHORD TO ADJACENT TRUSS TOP CHORD. COORDINATE SIZE & LOCATION W/ MFR.
- ▧ DENOTES SUSPENDED EQUIPMENT-COORDINATE SIZE & LOCATION W/ MECH. CONTRACTOR (SEE DTL. \_\_\_/\_\_\_)
- T1 DENOTES LIGHT GAUGE METAL GABLED TRUSS BY OTHERS

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

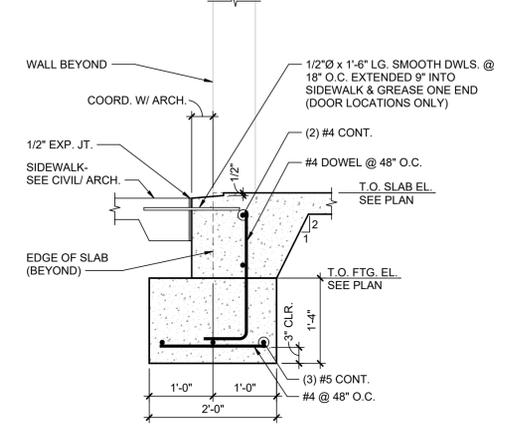
1/30/2026 4:35:10 PM C:\Users\mmw\Documents\26004 - BWHS Mats & Athletics Addition - Locker - Bentonville, AR\_STRUC\_T1\_R25\_mw\FKCK.vit



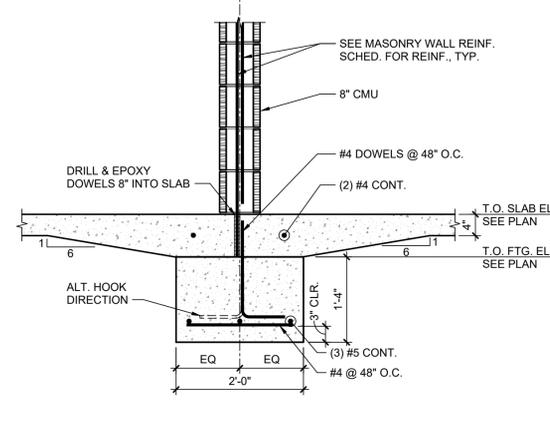
**1 SECTION**  
3/4" = 1'-0"



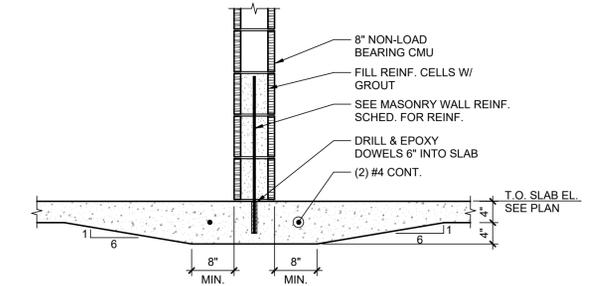
**2 SECTION**  
3/4" = 1'-0"



**3 SECTION**  
3/4" = 1'-0"



**4 SECTION**  
3/4" = 1'-0"



**5 SECTION**  
3/4" = 1'-0"

**PRELIMINARY**  
30% CD

Hight Jackson

ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72768 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
**BWHS - Locker Room Building**  
Centerion, AR

DRAWN BY:  
**TMW**

CHECKED BY:  
**ASD**

ISSUE DATE:  
**02/02/2026**

PROJECT NO:  
**0000**

REVISION DATES

FOUNDATION DETAILS

SHEET

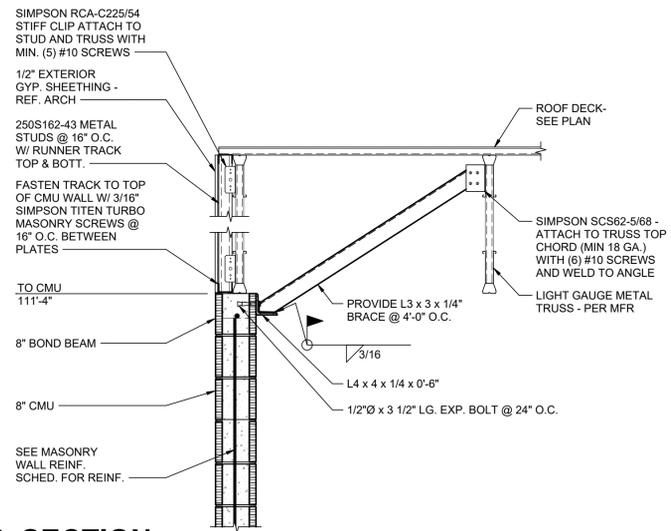
S4.1

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE  
THE EXCLUSIVE PROPERTY OF  
HIGHT JACKSON ASSOCIATES PA  
USE OR REPRODUCTION IS PROHIBITED  
WITHOUT WRITTEN CONSENT

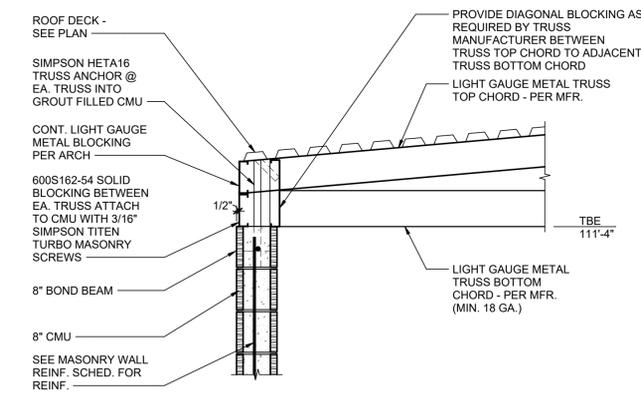
TATUM  
SMITH  
WELCHER

STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

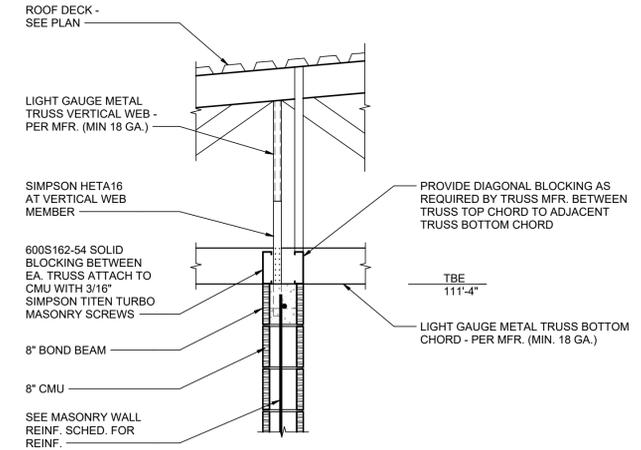
1/30/2026 4:35:11 PM C:\Users\tmw\Documents\26004 - BWHS Arts & Athletics Addition - Locker - Bentonville, AR\_STRUCUT\_R25\_tmw\FKCK.rvt



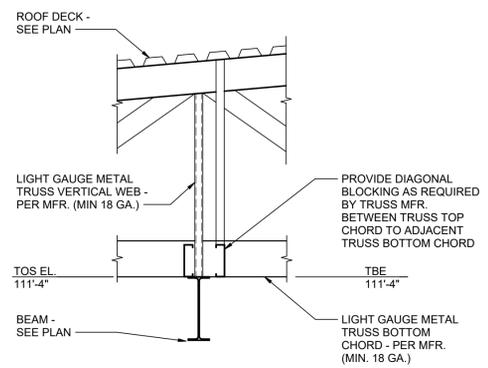
**1 SECTION**  
3/4" = 1'-0"



**2 SECTION**  
3/4" = 1'-0"



**3 SECTION**  
3/4" = 1'-0"



**4 SECTION**  
3/4" = 1'-0"

**PRELIMINARY**  
30% CD

**Hight Jackson**  
ASSOCIATES  
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72765 | (479) 464-4965 | www.hjarch.com

A NEW FACILITY FOR  
**BWHS - Locker Room Building**  
Centerton, AR

DRAWN BY:  
TMW  
CHECK BY:  
ASD  
ISSUE DATE:  
02/02/2026

PROJECT NO:  
0000

REVISION DATES

FRAMING DETAILS

SHEET  
**S5.1**

COPYRIGHT 2026  
THESE DRAWINGS AND SPECIFICATIONS ARE  
THE EXCLUSIVE PROPERTY OF  
HIGHT JACKSON ASSOCIATES P.A.  
USE OR REPRODUCTION IS PROHIBITED  
WITHOUT WRITTEN CONSENT

**TATUM SMITH WELCHER**  
STRUCTURAL ENGINEERS  
(479) 621-6128 ROGERS, ARKANSAS  
TSW #: 26004 PM: ASD DE: BWA

1/30/2026 4:35:11 PM C:\Users\mm\Documents\26004 - BWHS Arts & Athletics Addition - Locker - Bentonville, AR\_STRUCUT\_R25.imw\FKCK-v4