

SPECIAL INSPECTIONS

- 1. Special Inspections shall be performed in accordance with Section 1705 of 2021 IBC. An independent testing agency shall be employed to provide Special Inspections during construction on the types of work listed under Section 1705. The following areas of work require Special Inspections in accordance with 2021 IBC.
2. Refer to project specification for additional quality control/quality assurance requirements.
3. Construction Manager/Contractor shall coordinate any additional Special Inspection requirements with the Owner and applicable building authorities.
4. Special Inspections are not the responsibility of the Structural Engineer of Record.
5. Special Inspections shall be paid for directly by the Construction Manager.
6. Copies of all Special Inspections Reports shall be emailed to the SEOR Andrew Deschenes, P.E., (asd@tswstructural.com) or their designate within seven (7) calendar days of completing the individual inspection(s).

STRUCTURAL STEEL

(IBC 1705.2.1, 1705.13.1 & 1705.14.1)

PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16; TABLE J6-1, AISC 341-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verify welding procedures (WPS) and manufacturer certifications for welding consumable available, Verify type and grade of material, Welder identification, Fit-up groove welds, Access holes, Fit-up of fillet welds, Check welding equipment, Welder qualification records and continuity records.

DURING WELDING (TABLE N5.4-2, AISC 360-16; TABLE J6-2, AISC 341-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Use of qualified welders, Control and handling of welding consumables, Cracked tack welds, Environmental conditions, WPS followed, Welding techniques, Steel headed stud anchors.

AFTER WELDING (TABLE N5.4-3, AISC 360-16; TABLE J6-3, AISC 341-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Welds cleaned, Size, length, and location of welds, Welds meet visual acceptance criteria, Arc strikes, k-area, Backing & weld tabs removed and finished, and fillet welds added (if required), Repair activities, Document acceptance or rejection of welded joint/member, Placement of reinforcing or contouring fillet welds, Weld access holes, Prohibited welds.

OTHER STEEL INSPECTIONS (SECTION N5.7 & N5.8, AISC 360-16; TABLES J8-1 & J10-1, AISC 341-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Structural steel details (fabricated steel or steel frames), Anchor rods and other embedments supporting structural steel, Reduced beam sections (RBS), Protected zones, H-piles, Galvanized structural steel.

STRUCTURAL STEEL (CONT.)

(IBC 1705.2.1, 1705.12.1, & 1705.13.1)

PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-10; TABLE J7-1, AISC 341-10)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Manufacturer's certifications, Fasteners marked, Fastener selection, Bolting procedure, Connecting surfaces, Pre-installation verification testing by installation personnel, Fastener storage.

DURING BOLTING (TABLE N5.6-2, AISC 360-10; TABLE J7-2, AISC 341-10)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Position of fasteners, Joint brought into snug-tight condition prior to the pretensioning operation, Fastener components not turned by the wrench are prevented from rotating, Pretensioning of fasteners.

AFTER BOLTING (TABLE N5.6-3, AISC 360-10; TABLE J7-3, AISC 341-10)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Document acceptance or rejection of bolted connections.

STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL (IBC 1705.2)

STEEL ROOF AND FLOOR DECKS (IBC TABLE 1705.2.2/SDI/QA/QC 6.1)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Material verification of cold-formed steel deck, Floor and roof deck welding, Floor and deck mechanical fasteners, Deck installation.

OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC TABLE 1705.2.3)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: End connections - welding or bolted, Bridging - horizontal or diagonal.

WELDING OF REINFORCING STEEL (IBC 1705.3.1, TABLE 1705.3)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verification of weldability, Inspect single pass fillet welds, max 5/16", Inspect all other welds.

CONCRETE CONSTRUCTION

(IBC 1705.3; TABLES J9-2 & J9-3, AISC 341-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Reinforcing steel, Cast-in anchors, Post-installed anchors, Use of required mix design, Concrete sampling for strength tests, slump, air content, and temperature, Concrete & shotcrete placement, Curing temperature and techniques, Pre-stressed concrete, Erection of precast concrete, Strength verification, Formwork, Limits on water added at the truck or pump, Proper placement techniques to limit segregation, Verify installation of the embedded parts, completion of the continuity of reinforcement across joints, and completion of connections in the field, Verify installation tolerances of precast concrete diaphragm connections.

INSPECTION OF FABRICATORS (IBC 1704.2)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verify fabricator maintains detailed fabrication and quality control procedures, Submit a certificate of compliance.

SOILS CONSTRUCTION (IBC 1705.6)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verify subgrade is adequate to achieve design bearing capacity, Verify excavations extend to proper depth and material, Verify subgrade has been appropriately prepared prior to placing compacted fill, Perform classification and testing of compacted fill materials, Verify proper materials, densities and lift thicknesses.

MASONRY CONSTRUCTION

(IBC 1705.4)

PRIOR TO CONSTRUCTION (ARTICLE 1.5, TMS-602-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Review material certificates, mix designs, test results and construction procedures.

AS CONSTRUCTION BEGINS (TABLE 4, TMS-602-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Proportions of site-prepared mortar, Grade, type, and size of reinforcement, connectors, and anchor bolts, Sample panel construction.

DURING MASONRY CONSTRUCTION (TABLE 4, TMS-602-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Verify locations of structural elements comply with approved plans, Confirm tolerances meet the requirements of Article 3.3 F of TMS 602-16, Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction, Welding of reinforcement, Preparation, construction, and protection of masonry during cold weather (<40°F) or hot weather (>90°F), Observation of preparation of grout specimens, mortar specimens, and/or prisms, Placement of masonry unit and construction of mortar joints, Materials and procedures with the approved submittals.

PRIOR TO GROUTING (TABLE 4, TMS-602-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Grout space, Placement of reinforcement, connectors, and anchor bolts, Proportions of site-prepared grout.

MINIMUM TESTING (TABLE 3, TMS 602-16)

Table with 4 columns: Verification & Inspection, Continuous, Periodic, Detailed Instructions. Rows include: Compressive strength tests should be performed in accordance with ASTM C 1019; slump flow and VSI performed in accordance with ASTM C 1611, Determine compressive strength for each wythe by "unit strength method" or by the "prism test method" as specified in Article 1.4 B of TMS 602-16 prior to construction, For Risk Category IV buildings this should be verified at every 5,000 sq. ft. of construction, Verify that proportions for mortar meet ASTM C 270 and proportions for grout meet ASTM C 476. This applies to Risk Category IV buildings only.

PRELIMINARY 30% CD

Hight Jackson ASSOCIATES 5201 W Village Parkway, Suite 3001, Rogers, Arkansas 72768 | (479) 464-4965 | www.hjarch.com

An Addition for BWHS BWHS Band Addition 1351 GAMBLE RD CENTERTON, AR

DRAWN BY: TMW CHECK BY: ASD ISSUE DATE: 02/02/2026

PROJECT NO: 2421

REVISION DATES

REQUIRED IBC SPECIAL INSPECTIONS

SHEET S1.0

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STRUCTURAL ENGINEERS (479) 621-6128 ROGERS, ARKANSAS TSW #: 26004 PM: ASD DE: BWA

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CONSTRUCTION SAFETY GENERAL NOTE

THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, TEMPORARY SHORING/BRACING, OR FOR SAFETY PRECAUTIONS AND PROGRAMS, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

Concrete General Notes 3100:

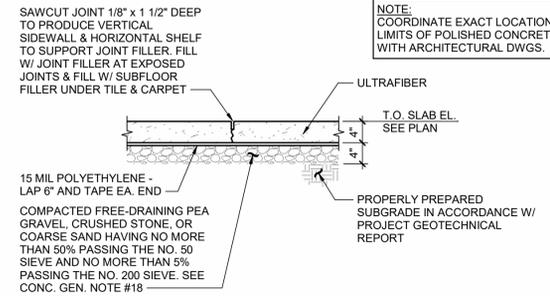
- All detailing, fabrication and placing of reinforcing steel shall conform to the ACI Standard "Details and Detailing of Concrete Reinforcement" (ACI 315).
- Concrete at slab-on-grade shall develop a 28-day minimum compressive strength of 3,500 psi and have Ultra Fiber 500 manufactured by Solomon Colors, Inc. mixed into the concrete at 2.0 lb/cubic yard. Follow normal mixing time and speed as recommended by ASTM C94.
- All concrete at footings, pedestals and walls shall typically develop a 28-day minimum compressive strength of 3,000 psi.
- All concrete for footings, walls and slab-on-grade shall have a 5" maximum slump.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All reinforcing bar splices shall be 44 bar diameters for #6 and smaller diameter bars. Reinforcing bar splices shall be 48 bar diameters for #7 and larger bar diameters.
- All reinforcing bar hooks shall be ACI standard 90 degree hook, unless noted otherwise.
- Provide two #4 x 4'-0" long diagonal bars centered in slab, at all re-entrant corners and any other locations designated on the plans.
- Provide corner bars in footings, concrete stemwall and turn-down slab same size and spacing as longitudinal reinforcing.
- Provide (1) #4 hoop with 8" lap in slab-on-grade and elevated slab around floor drains, columns and all slab penetrations 3" in diameter or greater. Also install around electrical conduit groupings 3" in diameter or greater.
- Limit the width of conduit groups to 3'-0" as they pass under wall footings. As much as possible, align the conduit perpendicular to the footing as it passes under the footing. Provide a minimum spacing of 2'-0" between conduit groups as the conduit passes under a footing. **Do not extend conduits under column footings or spread footings.**
- Plate dowel system shall be Diamond Dowel System by PNA Construction Technologies, the Speed Plate System by SIKA Corp. or approved equal. Install plate dowels at slab construction joints at 18" o.c.
- Smooth dowels shall be steel conforming to ASTM A36.
- All slots, sleeves and other embedded items shall be set before concrete is placed. See Architectural, Electrical, Mechanical, and Vendor's drawings for size and locations.
- Bar supports at footings, slabs-on-grade and elevated slabs shall be factory made wire bar supports, type "SBU" linear supports.
- Epoxy for doweling reinforcement shall be HY-200 by Hilti, AT 3G by Simpson Strong-Tie or AC208+ by Dewart, unless noted otherwise.
- Maximum net allowable bearing pressure = 1,500 psf for continuous footings and 1,800 psf for isolated spread footings. Footings shall bear on natural, stiff to very stiff, sandy clean clay and sandy clay soils as described in the Soils Report No. A25186.00135.000 dated January 26th, 2026.
- Use of compacted, free-draining pea gravel, crushed stone, or coarse sand underneath the building slab is recommended by TSW, Inc. Consult Geotechnical Engineer regarding potential substitution of free-draining coarse materials with approved subgrade. Slabs-On-Grade have been designed for a modulus of subgrade reaction (k-value) of 100 psi/in.

NOTE:
EXPANSIVE CLAYS, LOW-STRENGTH SOILS, SOFT SOILS AND SOILS WITH ORGANIC MATERIAL ARE NOT SUITABLE IN SUPPORTING THE SLAB AND FOUNDATIONS. IF CONTRACTOR DISCOVERS UNSTABLE MATERIAL DURING EARTHWORK, A GEOTECHNICAL ENGINEER SHALL BE CONSULTED IN ORDER TO ARRIVE AT A SOLUTION THAT WILL NOT COMPROMISE THE STRUCTURAL INTEGRITY OF THE SLAB AND FOUNDATIONS.

Concrete Masonry General Notes 4100:

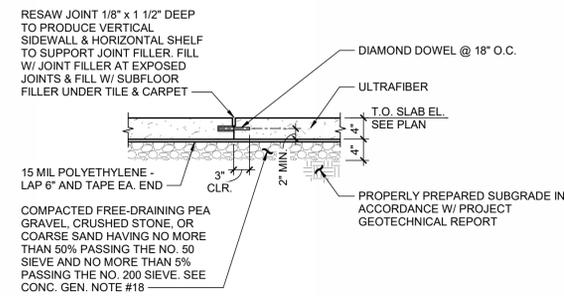
- All concrete masonry units shall be lightweight above finished floor and normal weight below grade. All hollow concrete masonry units shall conform to ASTM C90, Grade N, Type 1 with a minimum ultimate compressive prism strength (f'm) of 2,000 psi for the masonry assemblage. All concrete masonry shall be laid in Running (Common) Bond.
- Mortar at exterior walls, all load-bearing walls, walls below grade and non-load-bearing walls higher than 20'-0" shall be Type S mortar and have a minimum compressive strength of 1,800 psi. Mortar at interior non-load-bearing walls not higher than 20'-0" and mortar at masonry veneer shall be Type N mortar and have a minimum compressive strength of 750 psi. All mortar shall conform to ASTM C270. **Masonry cement shall not be used for mortar.**
- All grout shall be ready-mix concrete, with 3/8" diameter max. aggregate, have a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10" or preblended product (Core Fill Grout, Coarse CF-02, by Spec Mix) with a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10".
- All 12" CMU bond beam units shall be reinforced with two bars and all 8" CMU bond beam units shall be reinforced with one bar. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of bars for vertical wall reinforcement and bond beam requirements. Provide corner bars and lap bond beam reinforcing 48 bar diameters.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All bolts, anchors, reinforcement and embedded items shall be grouted in place.
- All reinforcing bar splices shall be 48 bar diameters, U.N.O.
- At all 8" CMU walls except at interior non-load bearing walls, provide (1) vertical bar each cell for the first (2) cells adjacent to control joints in walls, at ends of walls, wall corners and on each side of wall openings, unless noted otherwise. Vertical bars shall match reinforcement for remainder of wall. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of reinforcement.
- Provide control joints in brick veneer where shown on Architectural Drawings.
- Provide control joints in CMU walls where shown on S3.0. Place joints for CMU walls max. 24'-0" o.c.
- Provide horizontal joint reinforcement at 16" o.c. Reinforcement shall be ladder design, min. 9 gage welded steel wire, hot dipped galvanized to 1.5 oz. width shall be 1 1/2" less than wall thickness.

NOTE:
ALL SLAB-ON-GRADE SHALL HAVE ULTRAFIBER 500 REINFORCING FIBERS AT DOSAGE OF 2.0 LB / CUBIC YD. AS MANUFACTURED BY SOLOMON COLORS, INC.

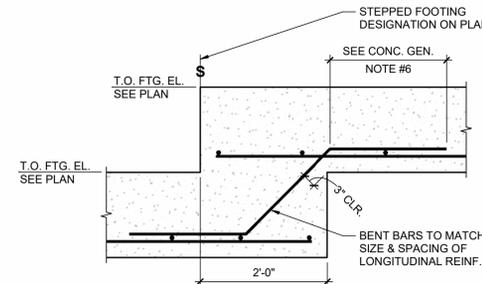


1 TYP. SLAB-ON-GRADE CONTROL JT. DTL. (CJ)
NOT TO SCALE

NOTE:
PROVIDE DIAMOND DOWEL SYSTEM AS MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES OR THE SPEED SYSTEM BY SIKA CORPORATION AT CONSTRUCTION JOINTS IN ALL FLOOR AREAS. COORDINATE WITH ARCHITECT FOR CONSTRUCTION JOINT LOCATIONS.



2 TYP. SLAB-ON-GRADE CONSTRUCTION JT. DTL. (CJ)
NOT TO SCALE



3 TYP. STEPPED FOOTING DETAIL
NOT TO SCALE

TYPICAL STRUCTURAL ABBREVIATIONS

| | | | | | |
|-----------|--|--------|-----------------------------|--------|-------------------------------|
| A.R. | ANCHOR ROD | FDN | FOUNDATION | PED | PEDESTAL |
| ACI | AMERICAN CONCRETE INSTITUTE | FIN | FINISH | PL | PLATE |
| AISC | AMERICAN INSTITUTE OF STEEL CONSTRUCTION | FLR | FLOOR | PLF | POUNDS PER FOOT |
| ARCH | ARCHITECT | FTG | FOOTING | PROJ | PROJECTION |
| ASTM | AMERICAN SOCIETY FOR TESTING AND MATERIALS | GA | GAUGE | PSF | POUNDS PER SQUARE FOOT |
| BLDG | BUILDING | GALV | GALVANIZED | PSI | POUNDS PER SQUARE INCH |
| BM | BEAM | H.S.A. | HEADED STUD ANCHOR | R | RADIUS |
| BOTT | BOTTOM | HK | HOOK | REIN | REINFORCEMENT |
| BRG | BEARING | HORIZ | HORIZONTAL | REQ'D | REQUIRED |
| BTWN | BETWEEN | J.B.E. | JOIST BEARING | RTU | ROOF TOP UNIT |
| CFS | COLD-FORMED STEEL | JT | JOINT | S.O.G. | SLAB ON GRADE |
| CL | CENTER LINE | L | ANGLE | SCHED | SCHEDULE |
| CLR | CLEAR | LDH | LONG DIMENSION HORIZONTAL | SECT | SECTION |
| COL | COLUMN | LDV | LONG DIMENSION VERTICAL | SEOR | STRUCTURAL ENGINEER OF RECORD |
| CONC | CONCRETE | LLV | LONG LENGTH VERTICAL | SIM | SIMILAR |
| CONN | CONNECTION | LONG | LONG | SJI | STEEL JOIST INSTITUTE |
| CONT | CONTINUOUS | LLH | LONG LENGTH HORIZONTAL | SPA | SPACING |
| DEFL. | DEFLECTION | LLV | LONG LENGTH VERTICAL | SPECS | SPECIFICATIONS |
| DIA. or Ø | DIAMETER | LONG | LONGITUDINAL | STD | STANDARD |
| DIM | DIMENSION | MAX | MAXIMUM | STIFF | STIFFENER |
| DN | DOWN | MBM | METAL BUILDING MANUFACTURER | STL | STEEL |
| do | DITTO | MECH | MECHANICAL | TOC | TOP OF CONCRETE |
| DTL | DETAIL | MFR | MANUFACTURER | TOF | TOP OF FOOTING |
| DWG | DRAWING | MIN | MINIMUM | TOS | TOP OF STEEL |
| E.F. | EACH FACE | MISC | MISCELLANEOUS | TOTB | TOP OF TIE BEAM |
| E.W. | EACH WAY | MTL | METAL | TOW | TOP OF WALL |
| EA | EACH | N.S. | NEAR SIDE | TRANS | TRANSVERSE |
| EL | ELEVATION | O.C. | ON CENTER | TYP | TYPICAL |
| EQ | EQUAL | O.F. | OUTSIDE FACE | U.N.O. | UNLESS NOTED OTHERWISE |
| EXIST | EXISTING | OPNG | OPENING | VERT | VERTICAL |
| EXP | EXPANSION | P.A.F. | POWDERED ACTUATED FASTENER | W | WITH |
| F.S. | FAR SIDE | WP | WORK POINT | WF | WELDED WIRE FABRIC |
| F.V. | FIELD VERIFY | | | | |

PRELIMINARY
30% CD

Hight Jackson ASSOCIATES
5201 W Village Parkway, Suite 300 | Rogers, Arkansas 72781 | (479) 464-4965 | www.hjarch.com

An Addition for BWHS
BWHS Band Addition
1351 GAMBLE RD CENTERTON, AR

DRAWN BY:

TMW

CHECK BY:

ASD

ISSUE DATE:

02/02/2026

PROJECT NO:

2421

REVISION DATES:

FOUNDATION GENERAL NOTES & TYP. DETAILS

SHEET

S1.1

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STRUCTURAL ENGINEERS
(479) 621-6128 ROGERS, ARKANSAS
TSW #: 26004 PM: ASD DE: BWA

Structural Steel General Notes 5100:

- All detailing, fabrication and erection of structural steel shall conform to the requirements of the AISC Specification for the Design, Fabrication and Erection of Structural Steel for Buildings.
- Wide flanges and WT tees shall conform to ASTM A992 with a yield strength of 50 ksi.
- Hollow Structural Section (HSS) shall conform to ASTM A500, Grade C with a yield strength of 50 ksi.
- Steel pipes shall conform to ASTM A501 or ASTM A53, Grade B.
- Headed stud anchors (H.S.A.'s) shall conform to ASTM A108.
- All other structural steel shall conform to the requirements of ASTM A36. Angle, plate and beam lintels at exterior wall openings shall be hot-dipped galvanized.
- All anchor rods shall conform to ASTM F1554, Grade 36. Nuts for anchor rods shall conform to ASTM A563, Grade A, heavy hex and anchor rod washers shall conform to ASTM F844.
- All welding shall conform to the Specifications of the American Welding Society. Welding electrodes shall be E-70 low hydrogen series. Welding shall be done by a certified welder.
- High strength bolts shall typically be 3/4" diameter bolts conforming to ASTM A325. Connections shall be designed as bearing type with threads in shear plane. Holes shall be 1/16" larger than bolt size. See details for connections with 1" diameter bolts.
- All bolts shall be tightened to a snug-tight condition. A snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. All connected elements must be brought into snug contact.
- Bearing ends of all columns shall be square cut.
- No openings shall be cut in structural members unless shown on the drawings.
- Steel frame is non-self-supporting and column anchor rods are designed for a completed condition only. Metal roof deck and masonry shear walls are required to provide lateral stability for the frame and resistance to wind and seismic forces. Contractor shall provide all temporary bracing required to maintain stability of structural system.
- Anchor rod holes in base plates shall be sized in accordance with AISC "Detailing For Steel Construction".
- All exposed edges of plates, beams, etc., shall be shop ground smooth and uniform.
- All column base plates supported on footings, pedestals and concrete columns shall bear on 1 1/2" non-shrink non-metallic grout unless noted otherwise.
- 1/2"Ø sleeve anchors shall be 1/2"Ø x 4" long HLC-H Sleeve Anchor by Hilli. HLC-H Sleeve Anchor shall be carbon steel with zinc plating & have a 3" embedment depth.
- 1/2"Ø expansion bolts shall be 1/2"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 3 1/2" embedment depth.
- 3/4"Ø expansion bolts shall be 3/4"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 4 3/4" embedment depth.

GENERAL CONTRACTOR SHALL INCLUDE 1 TON AT A MINIMUM COST OF \$5,000/ TON OF MISCELLANEOUS STEEL BEAMS, CHANNELS AND ANGLES IN ADDITION TO THE FRAMING SHOWN ON THE PLANS AND DETAILS. GENERAL CONTRACTOR SHALL INCLUDE ERECTION, FABRICATION, DESIGN AND DETAILING COSTS FOR THIS ADDITIONAL FRAMING WITH THE BASE BID. THE USE OF MISCELLANEOUS STEEL IS TO BE RECORDED BY THE GENERAL CONTRACTOR AND ANY UNUSED AMOUNT IS TO BE CREDITED TO THE OWNER.

Steel Joist General Notes 5200:

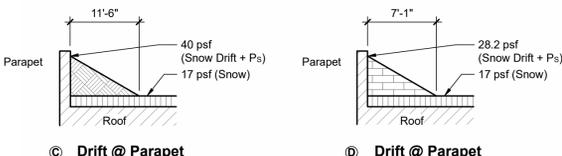
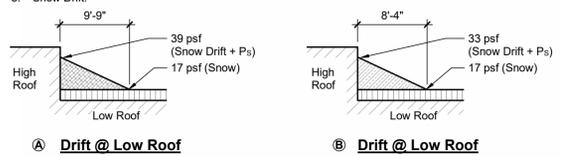
- The design, fabrication and erection of steel joists shall conform to the Standard Specifications for Open Web Steel Joists of the Steel Joist Institute. Joists have been designed according to the ASD Load Tables. Joist loads shown on the drawings are ASD loads.
- All hangers supported from open web joists shall be located at bottom chord panel points and shall be connected without field welding or drilling holes in the bottom chord.
- Joists shall have bridging per SJI Specifications. Roof joists shall have additional bridging, as required to resist stress reversal for the net uplift load indicated.
- Roof joists shall be designed for a net wind uplift of 11.7 psf. One-third stress increase of joists for wind uplift will not be allowed.
- Joist manufacturer shall design joists for mechanical and equipment loads noted on the framing plans in addition to the uniform dead and live loads. Coordinate equipment size and locations with mechanical contractor.
- Joist manufacturer shall design roof joists for the snow drift and sliding snow loadings shown in the Design Loads. See Snow Drift Plan (on this sheet) for locations of drifts and sliding snow.
- Attach joist top and bottom chord bridging to masonry walls with L2 1/2 x 2 1/2 x 3/16 x joist depth and (3) 1/2"Ø sleeve anchors, minimum. At "LH" use L3 x 3 x 1/4 x joist depth and 1/2"Ø expansion anchor and 18" o.c. (minimum) (3) bolts per angle

Steel Deck General Notes 5300:

- Typical roof deck shall be 1 1/2" deep, 22 gauge, wide rib type and shall have nested side laps (Vulcraft 1.5B22, New Millennium 1.5B22 or approved equal). See Roof Framing Plans for limits of roof deck.
- Roof deck shall be welded to the steel framing per the Roof Deck Fastening Pattern Detail 1/S1.3.
- Roof deck fastening pattern has been designed for a net wind uplift of 28.0 psf at corner zones, 28.0 psf at side zones and 18.4 psf at interior zones for roofs.
- All deck shall be fastened per Steel Deck Institute (SDI) requirements.
- Deck specified has been determined on basis of 3 span condition; deck supplier shall use heavier gauge if required for one and two span conditions.

Design Loads

- Typical Roof Dead Load: 20 psf
- Roof Live Load: 20 psf
- Rain Intensity, (15 min. i): 6.84 in/hr
- Snow Load:
 - Ground Snow Load: 15 psf
 - Flat-roof Snow Load at main roof (P_g) = 11.6 psf
 - Slope Factor (C_s) = 1.0
 - Snow Exposure Factor (C_e) = 1.0
 - Snow Load Importance Factor (I_s) = 1.1
 - Thermal Factor (C_t) = 1.0
- Snow Drift:



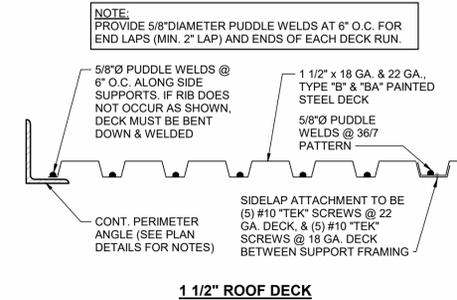
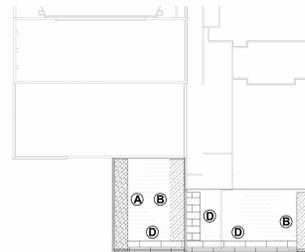
- Wind Load:
 - Ultimate Design Wind Speed (V_{ult}): 115 mph
 - Nominal Design Wind Speed (V_{nd}): 89.1 mph
 - Risk Category II
 - Wind Exposure C
 - Internal Pressure Coefficient, GC_{pi} = ±0.18
- Components & Cladding Wind Load (Unfactored):
 - Width of Edge Zone, a = 11.7 ft
 - Wall Pressures (10 ft²):
 - End Zone Wall = 38.8 psf
 - Interior Zone = 31.5 psf
 - Roof Pressures (100 ft²):
 - End Zone Wall = 30.2 psf
 - Interior Zone = 27.2 psf
 - Corner Zone = 66.7 psf
 - Eave & Rake Zone = 66.7 psf
 - Interior Zone = 50.6 psf
 - Roof Pressures (100 ft²):
 - Corner Zone = 52.5 psf
 - Eave & Rake Zone = 52.5 psf
 - Interior Zone = 39.5 psf
- Seismic:
 - Risk Category II
 - Seismic Importance Factor (I_s) = 1.0
 - S_s = 0.148
 - S₁ = 0.087
 - S_{0.5} = 0.158
 - S_{0.1} = 0.140
 - Site Class D (per Geotechnical Report)
 - Seismic Design Category C
 - Basic Structural System: Bearing Wall System
 - Seismic Resisting System: Intermediate Reinforced Masonry Shear Walls
 - Response Modification Coefficient (R): 3.500
 - Deflection Amplification Factor (C_d): 2.250
 - Seismic Response Coefficient (C_s): 0.045
 - Analysis Procedure: Equivalent Lateral Force Procedure
- Building Code:
 - 2021 Arkansas Fire Prevention Code, Volume II - Adopted 2021 International Building Code
 - ASCE 7-16

THIS FACILITY HAS BEEN DESIGNED FOR THE SEISMIC CRITERIA AND BUILDING CODE NOTED ON THIS DRAWING IN ACCORDANCE WITH THE REQUIREMENTS OF ACT 1100.

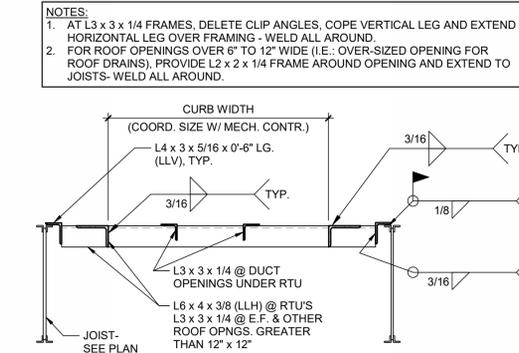
- LEGEND:**
- A** DENOTES LIMITS OF SLIDING SNOW @ LOW ROOF (SEE DESIGN LOADS)
 - B** DENOTES LIMITS OF SNOW DRIFT @ LOW ROOF (SEE DESIGN LOADS)
 - C** DENOTES LIMITS OF SNOW DRIFT @ PARAPET (SEE DESIGN LOADS)
 - D** DENOTES LIMITS OF SNOW DRIFT @ PARAPET (SEE DESIGN LOADS)



3 SNOW DRIFT & SLIDING SNOW PLAN
1" = 50'-0"



1 TYP. ROOF DECK FASTENING PATTERN DETAIL
NOT TO SCALE



2 TYP. ROOF FRAMED OPNG. DETAIL
NOT TO SCALE

PRELIMINARY
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An Addition for BWHS
BWHS Band Addition
1351 GAMBLE RD CENTERTON, AR

DRAWN BY: TMW
CHECK BY: ASD
ISSUE DATE: 02/02/2026

PROJECT NO: 2421

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FRAMING GENERAL NOTES & TYP. DETAILS
SHEET
S1.2
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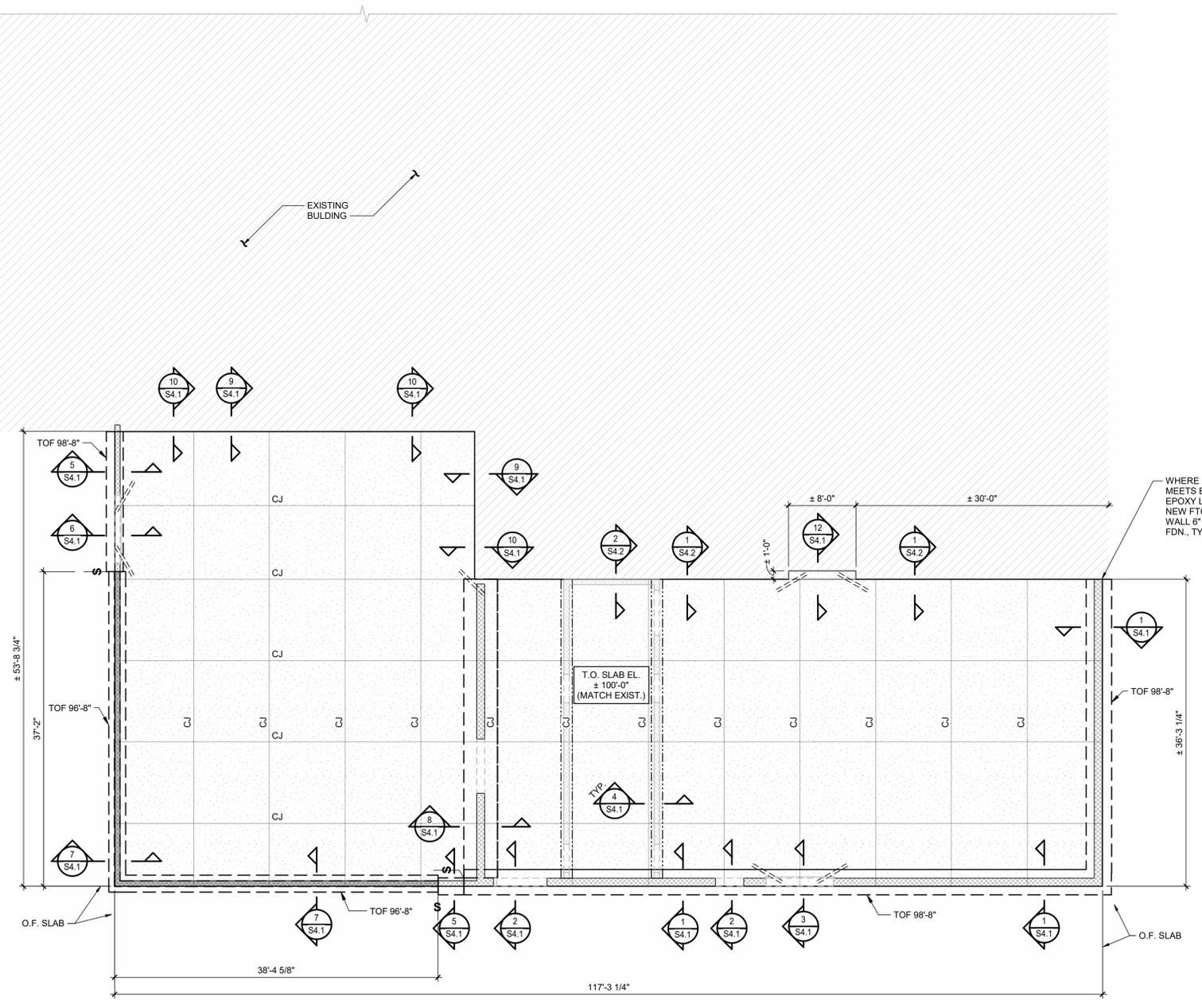
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1 FOUNDATION PLAN

1/8" = 1'-0"



- LEGEND:**
- FOOTING MARK (SEE FTG. SCHED. ON THIS SHEET)
 - TOP OF FOOTING ELEVATION
 - O.F. DENOTES OUTSIDE FACE
 - TOF DENOTES TOP OF FOOTING
 - ± DENOTES DIMENSION OR ELEVATION TO BE FIELD VERIFIED
 - DENOTES LIMITS OF 4" SLAB (T.O. SLAB EL. 100'-0") SEE PLAN NOTES
 - DENOTES LIMITS OF RECESSED SLAB COORD. LOCATIONS & RECESS W/ ARCH. (SEE DTL. 8/S1.1)
 - DENOTES LIMITS EXISTING SLAB
 - DENOTES THICKENED SLAB
 - DENOTES LOAD-BEARING CMU WALL
 - DENOTES NON-LOAD-BEARING CMU WALL
 - DENOTES CONCRETE STEM WALL
 - DENOTES SLAB ELEVATION CHANGE
 - DENOTES FTG. STEP MARK (SEE DTL. 3/S1.1)
 - DENOTES AREA & DIRECTION OF SLOPE TO FLOOR DRAIN 1/16"/FT. MIN. (U.N.O.)
 - F.D. DENOTES FLOOR DRAIN
 - DENOTES FLOOR DRAIN DISH OUT 1/16"/FT. MIN. (U.N.O.)
 - DENOTES TOP OF SLAB EL.
 - DENOTES LOCATION OF RE-ENTRANT CORNER BARS (SEE CONC. GEN. NOTE #8)

- PLAN NOTES:**
- 4" S.O.G. CONSTRUCTION: 4" CONCRETE SLAB REINFORCED W/ ULTRA FIBER 500 ON 15 MIL. POLYETHYLENE FILM (COORD. W/ ARCH. SPECS.) OVER 4" CRUSHED STONE
 - ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. _____ AS ESTABLISHED BY _____
 - TOP OF FOOTING ELEVATION 98'-8". TYP. (U.N.O.)
 - SEE ARCH. DWGS. / OVERALL FOUNDATION PLAN FOR DIMENSIONS NOT SHOWN.
 - "CJ" DENOTES CONTROL OR CONSTRUCTION JOINT. (SEE DTL. 1/S1.1 & 2/S1.1)
 - COORDINATE LOCATION & LIMITS OF VENEER WITH ARCH. DWGS.
 - COORDINATE DOOR LOCATIONS WITH ARCH. DWGS.
 - SEE ARCH. DWGS. FOR LOCATIONS OF NON-LOAD-BEARING MASONRY WALLS BEARING ON THICKENED SLABS.
 - SEE ARCH. / PLUMBING DWGS. FOR EXACT LOCATIONS OF ALL FLOOR DRAINS, SLOPED & RECESSED SLABS.
 - COORDINATE LOCATIONS OF PLUMBING LINES W/ PLUMBER PRIOR TO POURING FOOTINGS & SLABS-ON-GRADE.
 - SEE DWG. S1.1 FOR GENERAL NOTES & TYP. DETAILS.
 - SEE ARCH. FLOOR PATTERN PLANS FOR SLAB LOCATIONS WITH POLISHED CONCRETE.

WHERE NEW FOUNDATION MEETS EXISTING, DRILL & EPOXY LONG REINF. FROM NEW FTG. AND CONC. WALL 6" MIN. INTO EXIST. FDN., TYP

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30% CD

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BWHS Band Addition
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02/02/2026

PROJECT NO:
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REVISION DATES

FOUNDATION PLAN
SHEET
S2.1

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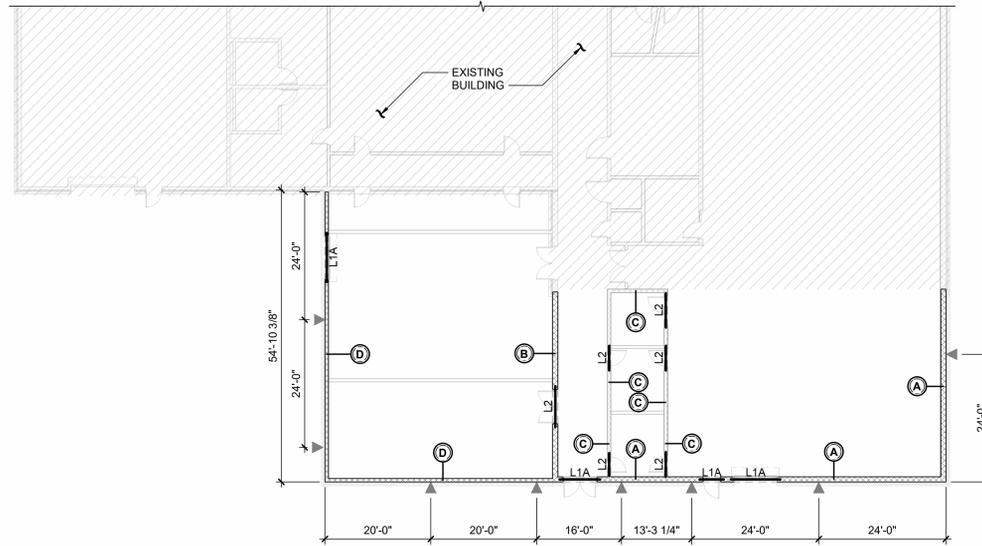
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| LINTEL SCHEDULE | | | | | |
|-----------------|----------------|--|-----------------------------|-----------------------|------------------------------------|
| MARK | WALL LOCATIONS | TYPE & SIZE (THICKNESS x HEIGHT) | REINFORCEMENT OR ATTACHMENT | BRICK ANGLE OR PLATES | REMARKS |
| L1 | 12" CMU | CMU 12" x 8" BOND BM. | (2) #5 BOTT. | ---- | SEE NOTES #1, #2, #4, #5 & #6 & #7 |
| L2 | 12" CMU | CMU 12" x 8" OPEN BOTT. BOND BM. ON 12" x 8" CLOSED BOTT. BOND BM. (16" TOTAL DEPTH) | (2) #5 TOP & BOTTOM | ---- | SEE NOTES #1, #2, #4, #5, #6 & #7 |
| L3 | 8" CMU | CMU 8" x 8" BOND BM. | (1) #5 BOTT. | ---- | SEE NOTES #1, #2, #4, #5, #6 & #7 |
| L4 | 8" CMU | CMU 8" x 8" OPEN BOTT. BOND BM. ON 8" x 8" CLOSED BOTT. BOND BM. (16" TOTAL DEPTH) | (1) #5 TOP & BOTT. | ---- | SEE NOTES #1, #2, #4, #5, #6 & #7 |

- LINTEL SCHEDULE NOTES:**
- SEE ARCH. DWGS. FOR EXACT LOCATION OF ALL LINTELS.
 - COORDINATE ALL BOTTOM PLATE/BEAM/CMU/ANGLE ELEVATIONS WITH ARCH. DWGS.
 - BRICK LINTELS SHALL HAVE 6" MIN. BEARING EACH SIDE OF OPENING.
 - ALL CMU LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
 - FILL ALL CMU LINTELS WITH 2,000 PSI GROUT.
 - ALL 8" CMU LINTELS SHALL HAVE A MIN. (2) CELLS OF (1) #5 JAMB STEEL EACH SIDE OF OPENING. 12" CMU LINTELS SHALL HAVE (2) CELLS OF (2) #5 EACH FACE EACH SIDE OF OPENING. U.N.O. ON THE DRAWING.
 - WHERE MECHANICAL DUCTS PASS THROUGH MASONRY WALLS, PROVIDE L3 LINTEL AT 8" CMU WALLS AND L1 LINTEL AT 12" CMU WALLS FOR OPENINGS NOT TO EXCEED 4'-0" WIDE. FOR OPENING WIDTHS FROM 4'-0" TO 6'-0", PROVIDE L4 LINTEL AT 8" CMU WALLS AND L2 LINTEL AT 12" CMU WALLS. FOR OPENING WIDTHS LARGER THAN 6'-0", COORDINATE WITH ARCH./ENGINEER.

| MASONRY WALL REINFORCEMENT SCHEDULE | | | | | | |
|-------------------------------------|---------------|------------------|---|------------------------|--|---------------------------------|
| MARK | WALL LOCATION | BOND BEAM REINF. | BOND BEAM LOCATIONS | VERT. REINF. | FOUNDATION DOWELS | REMARKS |
| A | 12" CMU | (2) #5 BOTT. | TOP OF WALL, ROOF EL., & WHERE NOTED ON DWGS. | #5 @ 24" O.C. EA. FACE | #5 DOWEL @ 24" O.C. EA. FACE | SEE NOTES #2 |
| B | 12" CMU | (2) #5 BOTT. | TOP OF WALL, ROOF EL., & WHERE NOTED ON DWGS. | #5 @ 48" O.C. EA. FACE | #5 DOWEL @ 40" O.C. EA. FACE | SEE NOTES #2 |
| C | 8" CMU | (1) #5 BOTT. | TOP OF WALL | NONE | #4 x 2'-6" DOWEL @ 48" O.C. CENTER IN WALL | SEE NOTES #1 & #3 & DTL. 2/S3.0 |
| D | 8" CMU | (1) #5 BOTT. | TOP OF WALL, ROOF EL., & WHERE NOTED ON DWGS. | #5 @ 48" O.C. | #5 DOWEL @ 48" O.C. CENTER IN WALL | SEE NOTE #2 |

- MASONRY WALL REINFORCEMENT SCHEDULE NOTES:**
- WHERE TOP OF WALL IS UNSUPPORTED BY THE ROOF OR FLOOR FRAMING, BRACE TOP OF WALL WITH DIAGONAL L3 x 3 x 1/4 WELDED TO THE ROOF OR FLOOR FRAMING ABOVE AT 8'-0" O.C. MAXIMUM (SEE DTL. 4/S3.0). WHERE WALL EXTENDS TO UNDERSIDE OF FLOOR OR ROOF DECK ABOVE, HOLD WALL DOWN 1/2" & SANDWICH TOP OF WALL WITH (2) L2 x 2 x 1/8 x 0'-6" LONG EACH ATTACHED TO DECK WITH (2) #8 SELF-TAPPING SCREWS (SPACE ANGLES AT 4'-0" O.C.). THIS BRACING IS PERMANENT BRACING REQUIRED FOR SEISMIC LOADS.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 8" INTO SLAB.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 6" INTO SLAB.



1 LINTEL & MASONRY PLAN

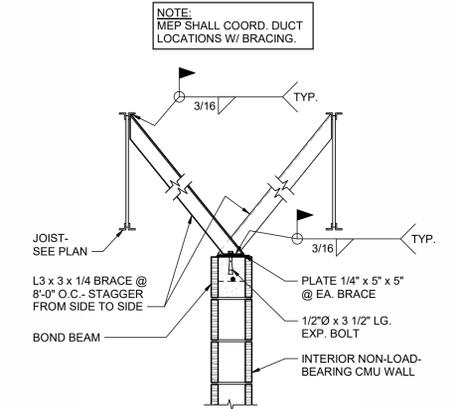
1/16" = 1'-0"

PLAN NOTES:

- SEE DWGS. S1.1 & S1.2 FOR GENERAL NOTES & TYP. DETAILS.

LEGEND:

- DENOTES LINTEL MARK, SEE LINTEL SCHED.
- DENOTES LOAD-BEARING CMU WALL
- DENOTES NON-LOAD-BEARING CMU WALL
- DENOTES MASONRY WALL REINF. MARK, SEE MASONRY WALL REINF. SCHED.
- DENOTES WALL CONTROL JOINT



2 TYP. CMU WALL BRACING

NOT TO SCALE

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30% CD

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BWHS Band Addition
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| ISSUE DATE: 02/02/2026 |
| PROJECT NO: 2421 |
| REVISION DATES |

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STRUCTURAL ENGINEERS
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LINTEL & MASONRY PLAN & DTL.S.
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S3.0
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1 ROOF FRAMING PLAN

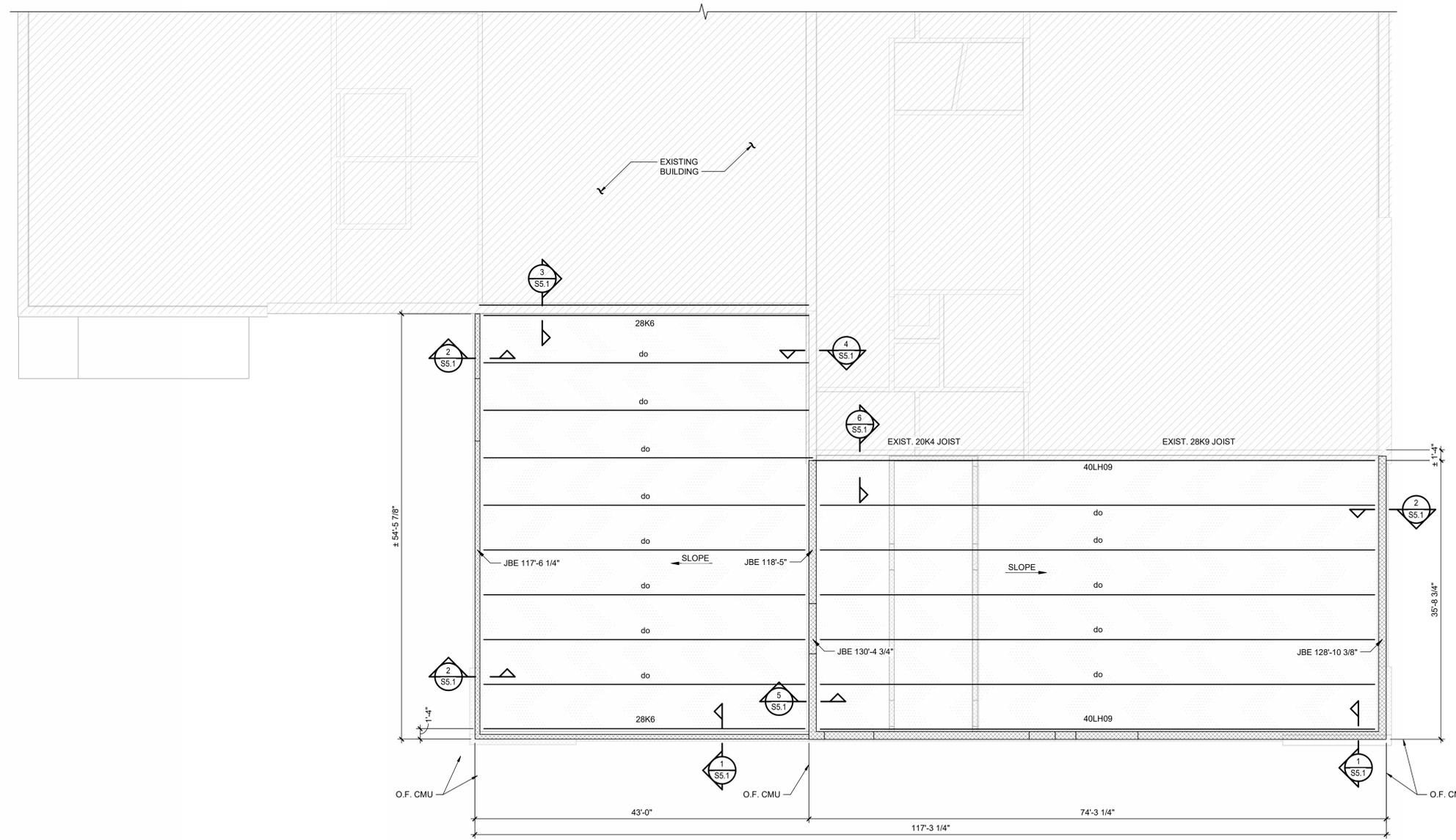
1/8" = 1'-0"

LEGEND:

- O.F. DENOTES OUTSIDE FACE
- JBE DENOTES JOIST BEARING ELEVATION
- TOS DENOTES TOP OF STEEL
- DENOTES FRAMED OPENING IN ROOF-COORDINATE SIZE & LOCATION W/ MECH. CONTRACTOR (SEE DTL. 2 / S1.2)
- DENOTES LIMITS OF 1 1/2" x 22 GA. ROOF DECK (SEE PLAN NOTES)
- DENOTES DIRECTION OF ROOF SLOPE (COORD. PITCH W/ ARCH.)
- (2,500#) DENOTES EQUIPMENT WEIGHT IN POUNDS
- DENOTES BEARING PLATE (SEE BEARING PL SCHED.)
- DENOTES LOAD-BEARING CMU WALL
- DENOTES NON-LOAD-BEARING CMU WALL

PLAN NOTES:

1. ROOF DECK: 1 1/2" DEEP, 22 GA. PAINTED WIDE RIB STEEL DECK (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS).
2. TOP OF STEEL DENOTES TOP OF MAIN STEEL.
3. TOP OF STEEL EL. VARIES, SEE PLAN.
4. TOP OF "K" SERIES JOISTS (+2 1/2") TYP. (U.N.O.)
5. TOP OF "LH" SERIES JOISTS (+5") TYP. (U.N.O.)
6. JOIST BEARING ELEVATION VARIES, SEE PLAN.
7. ALL ELEVATIONS BASED ON MAIN LEVEL FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. _____ AS ESTABLISHED BY _____
8. SEE DWGS. S1.2, S1.3 & S1.4 FOR GENERAL NOTES AND TYPICAL DETAILS.
9. SEE DWG. S1.4 FOR COLUMN SCHEDULE.
10. SEE ARCH. DWGS., FOUNDATION, SECOND FLOOR FRAMING & OVERALL ROOF FRAMING PLANS FOR DIMENSIONS NOT SHOWN.



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ROOF FRAMING PLAN

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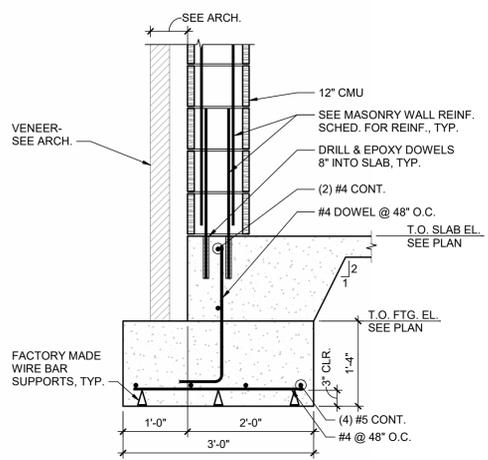
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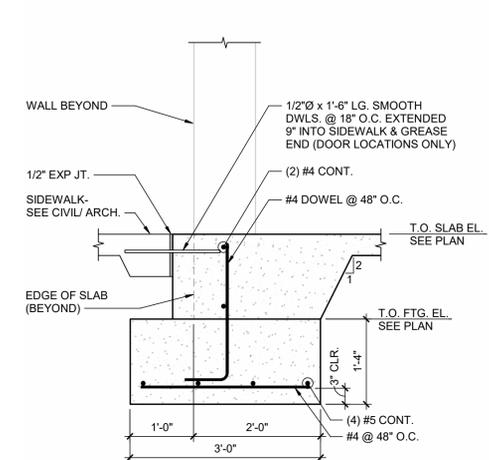
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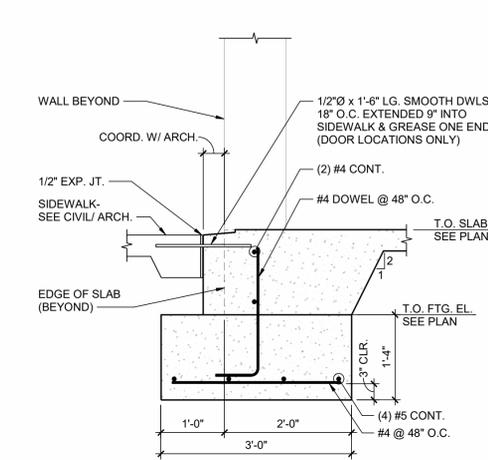
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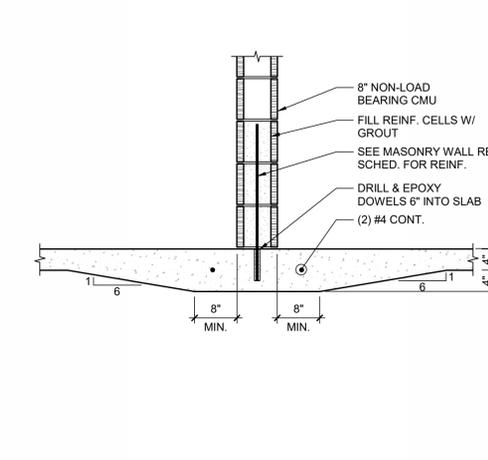
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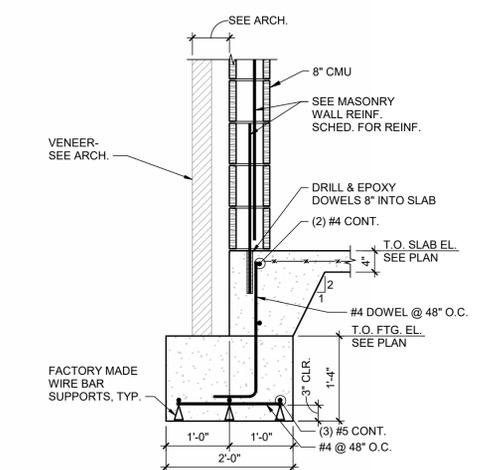
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3/4" = 1'-0"



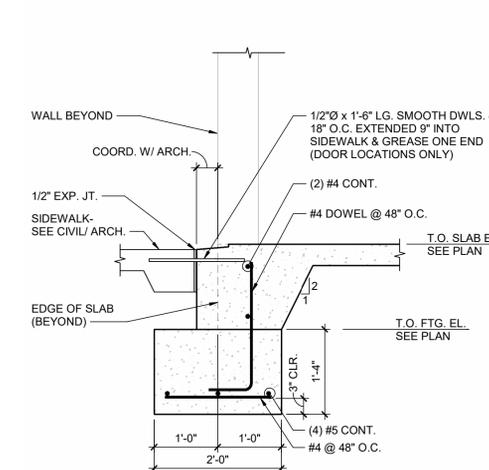
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3/4" = 1'-0"



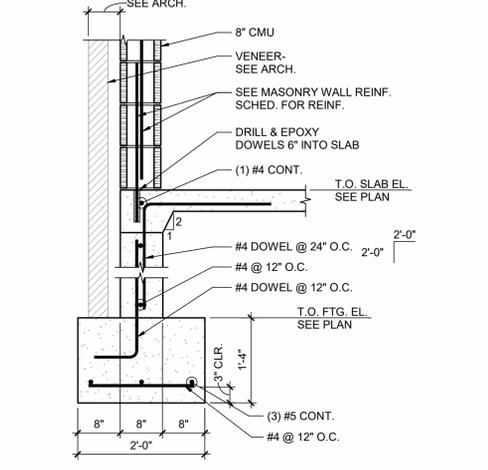
4 SECTION
3/4" = 1'-0"



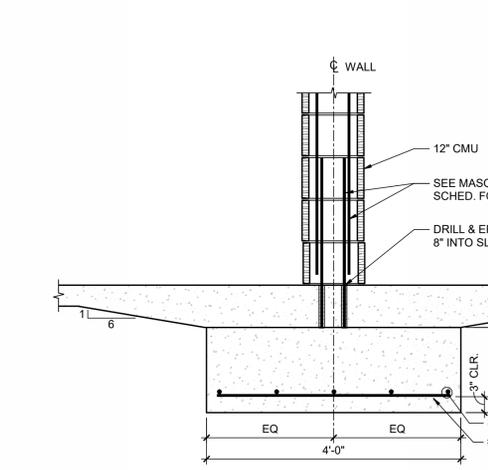
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3/4" = 1'-0"



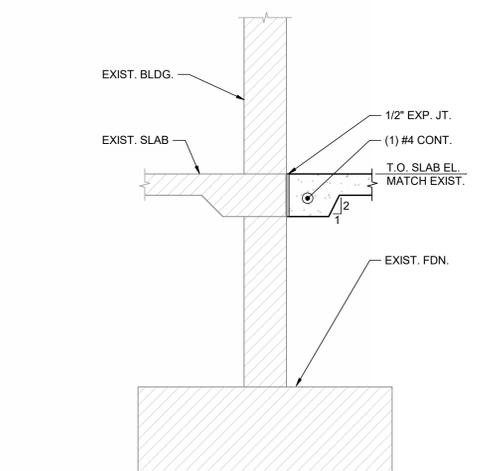
6 SECTION
3/4" = 1'-0"



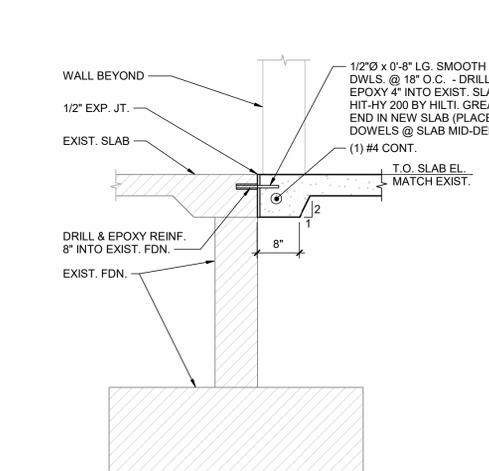
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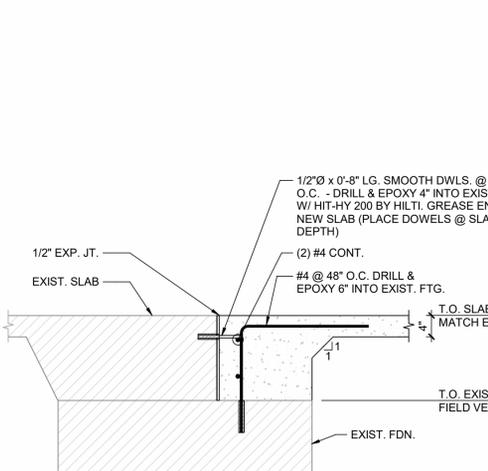
8 SECTION
3/4" = 1'-0"



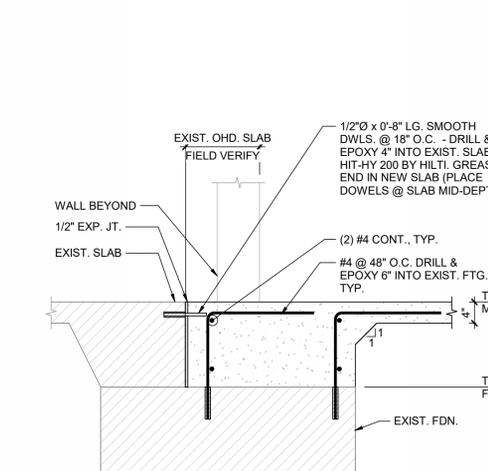
9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"



11 SECTION
3/4" = 1'-0"

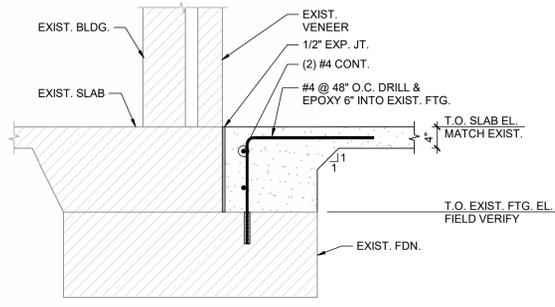


12 SECTION
3/4" = 1'-0"

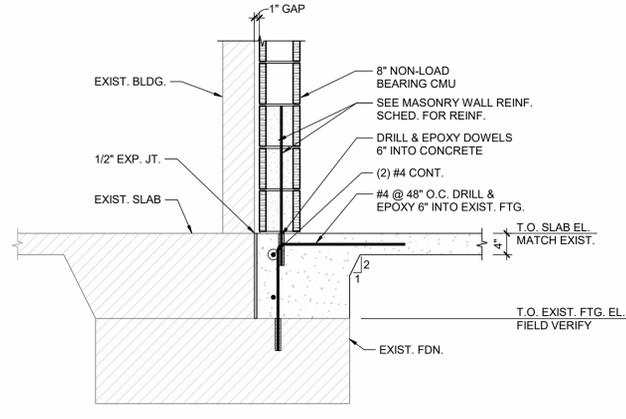
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1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"

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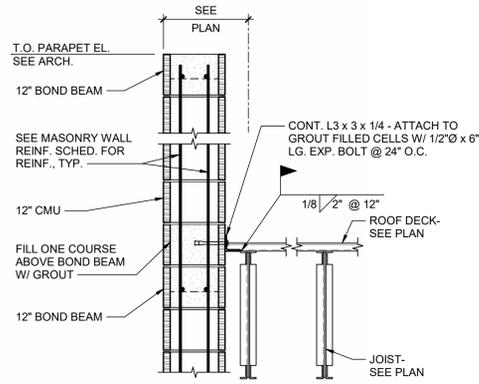
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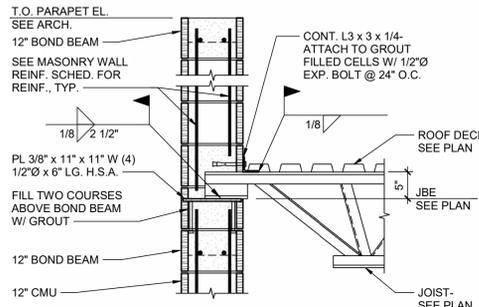
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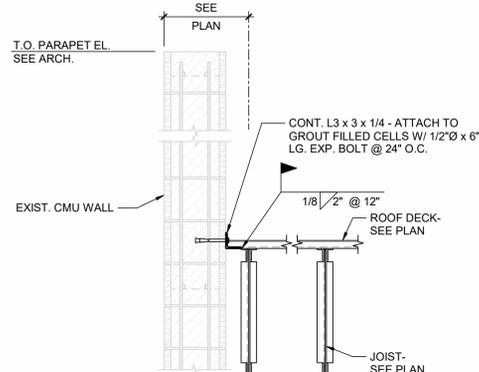
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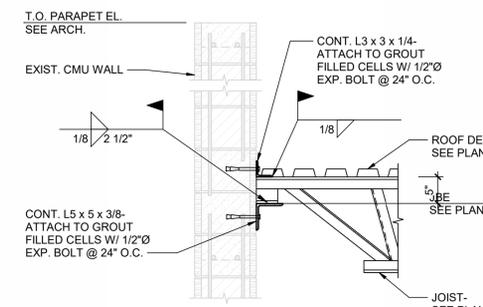
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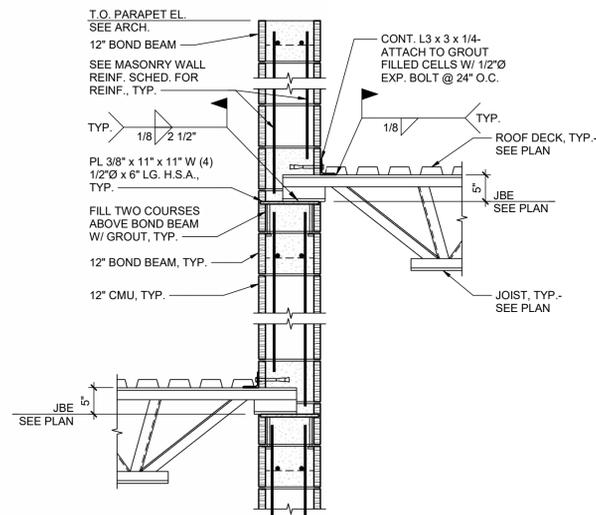
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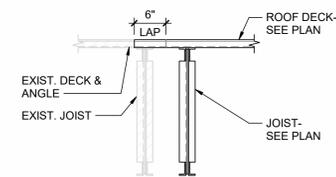
3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"



5 SECTION
3/4" = 1'-0"



6 SECTION
3/4" = 1'-0"

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