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SPECIAL INSPECTIONS			
1. _Special Inspections shall be performed in accordance with Section 1705 of 2021 IBC. An independent testing agency shall be employed to provide Special Inspections during construction on the types of work listed under Section 1705. The following areas of work require Special Inspections in accordance with 2021 IBC.			
2. Refer to project specification for additional quality control/quality assurance requirements.			
3. Construction Manager/Contractor shall coordinate any additional Special Inspection requirements with the Owner and applicable building authorities.			
4. Special Inspections are not the responsibility of the Structural Engineer of Record.			
5. Special Inspections shall be paid for directly by the Owner or Owner's Agent.			
6. Copies of all Special Inspections Reports shall be emailed to the SEOR Michael E. Whitten, P.E., (new@tswstructural.com) or their designate within seven (7) calendar days of completing the individual inspection(s).			

STRUCTURAL STEEL			
(IBC 1705.2.1, 1705.13.1 & 1705.14.1)			
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16; TABLE J6-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify welding procedures (WPS) and manufacturer certifications for welding consumable available	X	-----	-----
Verify type and grade of material.	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Welder identification	-----	X	A system shall be maintained by which a welder who has welded a joint or member can be identified. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fit-up groove welds	-----	X	Verify joint preparation, dimensions, cleanliness, tacking, and backing. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Access holes	-----	X	Verify configuration and finish. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fit-up of fillet welds	-----	X	Verify dimensions, cleanliness, and tacking. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Check welding equipment	-----	X	-----
Welder qualification records and continuity records	-----	X	-----
Fit-up CJP groove welds for HSS T-, Y-, and K-joints without backing (including joint geometry)	-----	X	Verify joint preparation, dimension (alignment, root opening, root face, bevel), cleanliness, and tacking (tack weld quality and location).
DURING WELDING (TABLE N5.4-2, AISC 360-16; TABLE J6-2, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Control and handling of welding consumables	-----	X	Verify packaging and exposure control. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Cracked tack welds	-----	X	Verify welding does not occur over cracked tack welds. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Environmental conditions	-----	X	Verify wind speed within limits, precipitation and temperature. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
WPS followed	-----	X	Verify settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Welding techniques	-----	X	Verify interpass and final cleaning, each pass within profile limitations, and quality of each pass. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Steel headed stud anchors	-----	X	Verify placement and installation.
AFTER WELDING (TABLE N5.4-3, AISC 360-16; TABLE J6-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Welds cleaned	-----	X	Verify welds properly cleaned. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Size, length, and location of welds	X	-----	-----
Welds meet visual acceptance criteria	X	-----	Verify crack prohibition, weld/base metal fusion, crater cross section, weld profiles, weld size, undercut, and porosity meet visual acceptance criteria.
Arc strikes	X	-----	-----
k-area	X	-----	-----
Backing & weld tabs removed and finished, and fillet welds added (if required)	X	-----	-----
Repair activities	X	-----	-----
Document acceptance or rejection of welded joint/member	X	-----	-----
Placement of reinforcing or contouring fillet welds	X	-----	Only required in components of seismic force resisting system.
Weld access holes	-----	X	After rolled heavy shapes are welded, visually inspect the weld access hole for cracks.
Prohibited welds	X	-----	Verify no prohibited welds have been added without approval of the EOR.
STEEL ELEMENTS OF COMPOSITE CONSTRUCTION (TABLES J9-1 thru J9-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Reinforcing steel	-----	X	Verify appropriate reinforcement size, type, grade, spacing, and orientation; not re-bent in field; correctly tied and supported; and required steel clearances provided. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Composite member size	-----	X	Verify required size. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
OTHER STEEL INSPECTIONS (SECTION N5.7 & N5.8, AISC 360-16; TABLES J8-1 & J10-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Structural steel details (fabricated steel or steel frames)	-----	X	Verify compliance with the details in construction documents in items including: braces, stiffeners, member locations, and proper application of joint details at each connection. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Anchor rods and other embedments supporting structural steel	-----	X	Verify compliance with construction documents. Verify diameter, grade, type, length of anchor rod or embedded item, and extent or depth of embedment prior to placement of concrete. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Reduced beam sections (RBS)	X	-----	For seismic force resisting system components: Verify contour and finish as well as dimensional tolerances.
Protected zones	X	-----	For seismic force resisting system components: Verify that no holes or unapproved attachments are made within the protected zone.
H-piles	X	-----	For seismic force resisting system components: Verify that no holes or unapproved attachments occur within the protected zones of piling.
Galvanized structural steel	-----	X	Verify exposed cut surfaces of galvanized structural steel main members and exposed corners of rectangular HSS have no cracks subsequent to galvanizing.

STRUCTURAL STEEL (CONT.)			
(IBC 1705.2.1, 1705.13.1, & 1705.14.1)			
PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-16; TABLE J7-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Manufacturer's certifications	X	-----	Verify certifications available for fastener materials.
Fasteners marked	-----	X	Verify marked in accordance with ASTM requirements. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fastener selection	-----	X	Verify proper selection for joint detail including grade, type, and bolt length if threads excluded from shear plane. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Bolting procedure	-----	X	Verify proper bolting procedure selected for joint detail. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Connecting surfaces	-----	X	Verify connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Pre-installation verification testing by installation personnel	X	-----	Observe and document for fastener assemblies and methods used.
Fastener storage	-----	X	Verify proper storage provided for bolts, nuts, washers, and other fastener components. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
DURING BOLTING (TABLE N5.6-2, AISC 360-16; TABLE J7-2, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Position of fasteners	-----	X	Verify fastener assemblies, of suitable condition, are placed in all holes and washers, if required, are positioned as required. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Joint brought into snug-tight condition prior to the pretensioning operation	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fastener components not turned by the wrench are prevented from rotating	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Pretensioning of fasteners	-----	X	Fasteners are pretensioned in accordance with the RCSC specification, progressing systematically from the most rigid point toward the free edges. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
AFTER BOLTING (TABLE N5.6-3, AISC 360-16; TABLE J7-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Document acceptance or rejection of bolted connections	X	-----	-----
NONDESTRUCTIVE TESTING (SECTION N5.5, AISC 360-16; SECTION J6, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
CJP welds (Risk Cat. II)	-----	X	Ultrasonic testing shall be performed on 10% of CJP groove welds in butt, T- and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. Testing rate must be increased if >5% of welds tested have unacceptable defects. See AISC 360-16 Section N5.5f for increase requirements.
CJP welds (Risk Cat. III or IV)	X	-----	Ultrasonic testing shall be performed on all CJP groove welds subject to transversely applied tension loading in butt, T- and corner joints, in materials 5/16-inch thick or greater. See AISC 360-16 Section N5.5e for reduction in rate of ultrasonic testing.
Welded joints subject to fatigue	X	-----	-----
Document all nondestructive testing	X	-----	Verify record indicates basis of rejection and location of defect for all rejected welds.
k-area	X	-----	For components of seismic force resisting system: Perform magnetic particle testing for cracks where welding of doubler plates, continuity plates, or stiffeners has been performed in the k-area. Testing area shall include the k-area base metal within 3 inches of weld. Inspection shall be performed no sooner than 48 hours following completion of welding.
CJP groove welds (all components of seismic force resisting system)	X	-----	Ultrasonic testing shall be performed on 100% of CJP groove welds in materials 5/16-inch thick or greater. Magnetic particle testing shall be performed on 25% of all beam-to-column CJP groove welds. See AWS D1.1/D1.1M Table 6.2 for acceptance/ rejection criteria. See Sections J6-2g and J6-2h in AISC 341-16 for potential reduction in the rate of magnetic particle and ultrasonic testing.
Base metal (>1 1/2")	X	-----	For components of seismic force resisting system: Ultrasonic testing for discontinuities shall be performed, after joint completion, behind and adjacent to fusion line of CJP groove welds where base metal (>1 1/2") is loaded in tension in through-thickness direction in T- and corner joints and the connection material is >3/4" thick. See AWS D1.1/D1.1M Table 6.2 for acceptance/ rejection criteria.
Beam cope and access holes (flange >1 1/2" for rolled shapes, web thickness >1 1/2" for built-up sections)	X	-----	For components of seismic force resisting system: Magnetic particle testing or penetrant testing shall be performed.
Reduced beam section repair	X	-----	For components of seismic force resisting system: Magnetic particle testing shall be performed on any weld and adjacent area of the reduced beam section cut surface that has been repaired by welding, or on the base metal of the reduced beam section cut surface if sharp notch has been removed by grinding.
Weld tab removal sites	X	-----	For components of seismic force resisting system: Magnetic particle testing shall be performed on same beam-to-column joints receiving ultrasonic testing under the CJP groove welds for components of seismic force resisting system listed in this table. See Section J6-2f and Section J6-2b of AISC 341-16 for reference. See Sections J6-2g and J6-2h in AISC 341-16 for potential reduction in the rate of magnetic particle and ultrasonic testing.

STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL			
(IBC 1705.2)			
STEEL ROOF AND FLOOR DECKS (IBC TABLE 1705.2.2/SDI QA/QC 6.1)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Material verification of cold-formed steel deck	-----	X	Verify deck materials are represented by appropriate mill certifications.
Floor and roof deck welding	-----	X	Verify weld meets acceptance criteria of AWS D.3. Verify welder qualifications.
Floor and deck mechanical fasteners	-----	X	Verify fastener installation in accordance with SDI.
Deck installation	-----	X	Verify deck installation in accordance with applicable drawings and documents.
OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC TABLE 1705.2.3):			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
End connections - welding or bolted	-----	X	Verify installation compliance with SJI specifications listed in IBC 2207.1
Bridging - horizontal or diagonal	-----	X	Verify installation compliance with SJI specifications listed in IBC 2207.1 as applicable.
WELDING OF REINFORCING STEEL (IBC 1705.3.1, TABLE 1705.3)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verification of weldability	-----	X	Verify weldability of reinforcing steel other than ASTM A706 in accordance with AWS D1.4.
Inspect single pass fillet welds, max 5/16"	-----	X	Verify weld meets acceptance criteria of AWS D1.4.
Inspect all other welds	X	-----	Verify weld meets acceptance criteria of AWS D1.4.
COLD-FORMED STEEL CONSTRUCTION (IBC 1705.2.2, 1705.2.4, 1705.12.2, & 1705.13.3)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Trusses spanning > 60-feet	-----	X	Verify temporary and permanent truss bracing is installed in accordance with approved truss package.
Welding in wind-force-resisting systems or seismic-force-resisting systems	-----	X	Verify proper screw attachment, bolting, anchoring and other fastening of shear walls, diaphragms, drag struts, braces, shear panels and holdowns. See IBC 1705.12.2 for exceptions.
Floor and roof deck welds	-----	X	Verify weld meets acceptance criteria of SDI QA/QC. Verify welder qualifications.

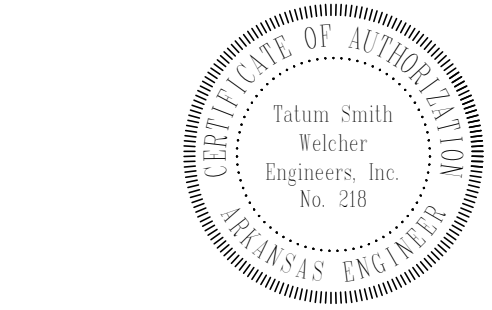
CONCRETE CONSTRUCTION			
(IBC 1705.3; TABLES J9-2 & J9-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Reinforcing steel, including prestressing tendons	-----	X	Verify, prior to placing concrete, reinforcing is of specified type, grade and size; free of oil, dirt and rust; located and spaced properly; hooks, bends, ties, stirrups and supplemental reinforcement placed correctly; lap lengths, stagger and offsets provided; and all mechanical connections installed per the manufacturer's instructions and/or evaluation report.
Cast-in anchors	-----	X	Verify anchor installation complies with ACI 318: 17.8.2.
Post-installed anchors	-----	X	All post-installed anchors shall be specially inspected as required by the approved ICC-ES report. Anchors installed horizontally or in upwardly inclined orientations to resist tension loads require continuous inspection per ACI 318:17.8.2.4. Verify all other mechanical and adhesive anchors comply with ACI 318: 17.8.2.
Use of required mix design	-----	X	Verify mixes comply with the approved construction documents; ACI 318: Ch. 19, 26.4.3, 26.4.4 and IBC 1904.1, 1904.2.
Concrete sampling for strength tests, slump, air content, and temperature	X	-----	Verify sampling in accordance with ASTM C172 and ASTM C31. See ACI 318: 26.12 for evaluation and acceptance of concrete. See ACI 318: 26.5 for mixing requirements of concrete.
Concrete & shotcrete placement	X	-----	Verify proper application techniques. See ACI 318: 26.5.
Curing temperature and techniques	-----	X	Verify concrete surface temperature (other than high-early-strength) is kept >50°F in moist condition for at least 7 days after placement unless accelerated curing is used. High-early-strength concrete shall be kept >50°F in moist condition for at least 3 days unless accelerated curing is used. Verify compliance with cold weather requirements in ACI 318: 26.5.4 or hot weather requirements in ACI 318: 26.5.5, whichever is applicable.
Pre-stressed concrete	X	-----	Verify application of prestressing force and grouting of bonded prestressing tendons in accordance with ACI 318: 26.10.
Erection of precast concrete	-----	X	Verify all precast elements are lifted, assembled and braced in accordance with the approved construction documents. See ACI 318: 26.9.
Strength verification	-----	X	Verify adequate strength has been achieved prior to the removal of shores and forms or the stressing of post-tensioned tendons. See ACI 318: 26.11.2.
Formwork	-----	X	Verify forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents. See ACI 318: 26.11.1,2.
Limits on water added at the truck or pump	-----	X	Verify during concrete placement. Applicable to composite construction in seismic force resisting system components. Perform on a random daily basis per AISC 341-16 Section J5.1. See Table J9-2, AISC 341-16.
Proper placement techniques to limit segregation	-----	X	Verify during concrete placement. Applicable to composite construction in seismic force resisting system components. Perform on a random daily basis per AISC 341-16 Section J5.1. See Table J9-2, AISC 341-16.
Verify installation of the embedded parts, completion of the continuity of reinforcement across joints, and completion of connections in the field	X	-----	For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to seismic design category C, D, E, or F. See ACI 318:26.13.1.3 and ACI 550.5.
Verify installation tolerances of precast concrete diaphragm connections	-----	X	See ACI 550.5 for compliance.

INSPECTION OF FABRICATORS			
(IBC 1704.2)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify fabricator maintains detailed fabrication and quality control procedures	-----	X	See IBC 1704.2.5.1.
Submit all of certificate of compliance	-----	X	Where work is done on premises of "Approved" fabricator. Fabricator shall submit a Certificate of Compliance to the building official stating work was performed in accordance with the approved construction documents. See IBC 1704.2.5.1.

WOOD CONSTRUCTION			
(IBC 1705.5, 1705.12.1 & 1705.13.2)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
High-load diaphragms	-----	X	Verify thickness and grade of sheathing, size of framing members at panel edges, nail/staple diameters and length, and the number of fastener lines and fastener spacing per approved plans. See IBC 1705.5.1.
Wood trusses spanning >60 ft.	-----	X	Verify that temporary and permanent truss bracing is installed in accordance with approved truss package.
Structural wood	-----	X	If fastener spacing is < 4" o.c.: Verify that proper nailing, bolting, anchoring and other fastening of shear walls, diaphragms, drag struts, braces, and holdowns. Continuous inspection required during field gluing of elements.
Review procedures for completeness and adequacy relative to the Code requirements for the fabricators scope of work	-----	X	-----

SOILS CONSTRUCTION			
(IBC 1705.6)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify subgrade is adequate to achieve design bearing capacity	-----	X	Prior to placement of concrete; per Geotechnical Report.
Verify excavations extend to proper depth and material	-----	X	Prior to placement of compacted fill or concrete; per Geotechnical Report
Verify subgrade has been appropriately prepared prior to placing compacted fill	-----	X	Prior to placement of compacted fill; per Geotechnical Report
Perform classification and testing of compacted fill materials	-----	X	All materials shall be checked at each lift for proper classifications and gradations not less than once for each 10,000 sq ft. of surface area unless otherwise noted; per Geotechnical Report
Verify proper materials, densities and lift thicknesses	X	-----	During placement and compaction on compacted fill; per Geotechnical Report

MASONRY CONSTRUCTION			
(IBC 1705.4)			
PRIOR TO CONSTRUCTION (ARTICLE 1.5, TMS-602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Review material certificates, mix designs, test results and construction procedures	-----	X	Verify materials conform to requirements of approved construction documents. Mix design, test results, material certificates, and construction procedures submitted for review. Mortar mix designs conform to ASTM C 270; grout conforms to ASTM C 476. Material certificates provided for: reinforcement; anchors, ties, fasteners, and metal accessories; masonry units; mortar and grout materials. Construction procedures for cold-weather or hot-weather construction reviewed. Qualification of field testing personnel, and special inspector reviewed.
AS CONSTRUCTION BEGINS (TABLE 4, TMS-602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Proportions of site-prepared mortar	-----	X	Verify mortar is type and color specified on construction documents, conforms to ASTM C 270, and is mixed in accordance with Article 2.6 A and Article 2.6 C of TMS 602-16.
Grade and size of prestressing tendons and anchors	-----	X	Verify prestressing tendons comply with Article 2.4 B of TMS-602-16 and anchorages, couplers, and end blocks comply with Article 2.4 H.
Grade, type, and size of reinforcement, connectors, anchor bolts, and prestressing tendons and anchorages	-----	X	Verify reinforcement is placed in accordance with Article 3.4 of TMS-602-16. Prestressing tendons placed per Article 3.6 A.
Sample panel construction	-----	X	Verify sample panel complies with Article 1.6 D of TMS-602-16.
DURING MASONRY CONSTRUCTION (TABLE 4, TMS-602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Size and location of structural elements	-----	X	Verify locations of structural elements comply with approved plans. Confirm tolerances meet the requirements of Article 3.3 F of TMS 602-16.
Type, size, and location of reinforcement, connectors, anchor bolts, and prestressing tendons and anchorages	-----	X	Verify anchorages and connections are provided per approved plans, Section 1.2 (1e), 6.1.4.3 and 6.2.1 of TMS 402-16. Continuous inspection required for Risk Category IV buildings.
Welding of reinforcement	X	-----	Verify welded splice has bars butted and welded to develop at least 125% of yield strength of bar in tension or compression. See Section 6.1.6.1.2 of TMS 402-16.
Preparation, construction, and protection of masonry during cold weather (<40°F) or hot weather (>90°F).	-----	X	Verify cold-weather construction performed in accordance with Article 1.8 C of TMS 602-16 and hot weather construction per Article 1.8 D of TMS 602-16.
Observation of preparation of grout specimens, mortar specimens, and/or prisms	-----	X	Confirm specimen/prism preparation performed as required by Article 1.4 of TMS 602-16. Continuous inspection is required for Risk Category IV buildings.
Placement of masonry unit and construction of mortar joints	-----	X	Verify placement in accordance with Article 3.3 B of TMS 602-16.
Materials and procedures with the approved submittals	-----	X	Verify materials and procedures conform to approved submittals. See Article 1.5 of TMS 602-16.
MINIMUM TESTING (TABLE 3, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verification of slump flow and Visual Stability Index (VSI) for self-consolidating grout	-----	X	Compressive strength tests should be performed in accordance with ASTM C 1019; slump flow and VSI performed in accordance with ASTM C 1611.
Verification of fm and FAAC	-----	X	Determine compressive strength for each wythe by "unit strength method" or by the "prism test method" as specified in Article 1.4 B of TMS 602-16 prior to construction. For Risk Category IV buildings this should be verified at every 5,000 sq. ft. of construction.
Verification of proportions of materials in grout and premixed or preblended mortar	-----	X	Verify that proportions for mortar meet ASTM C 270 and proportions for grout meet ASTM C 476. This applies to Risk Category IV buildings only.
PRIOR TO GROUTING (TABLE 4, TMS-602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Grout space	-----	X	Verify grout space is free of mortar droppings, debris, loose aggregate, and other deleterious materials and cleanouts are provided per Article 3.2 D and 3.2 F of TMS-602-16. Continuous inspection is required for Risk Category IV buildings.
Placement of prestressing tendons and anchorages	-----	X	Verify reinforcement, cover, and protection of prestressing tendons, Sections 10.8 and 10.9 of TMS 402-16, and Articles 2.4 and 3.6 of TMS 602-16.
Placement of reinforcement, connectors, and anchor bolts	-----	X	Verify reinforcement, joint reinforcement, wall ties, anchor bolts and veneer anchors are installed in accordance with the approved construction documents, Section 6.1, 6.3.1, 6.3.6, and 6.3.7 of TMS 402-16, and Articles 3.2 E, and 3.4 of TMS 602-16. Continuous inspection is required for Risk Category IV buildings.
Proportions of site-prepared grout and prestressing grout for bonded tendons	-----	X	Verify grout is proportioned per ASTM C 476 and has a slump between 8-11 inches. Self-consolidated grout shall not be proportioned onsite. See Article 2.6 B of TMS 602-16. Verify prestressing grout complies with Article 2.4 G.1.b of TMS 602-16.



STRUCTURAL ENGINEERS
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sheet S1.0
REQUIRED IBC SPECIAL INSPECTIONS
13 April 2026

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CONSTRUCTION SAFETY GENERAL NOTE

THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, TEMPORARY SHORING/BRACING, OR FOR SAFETY PRECAUTIONS AND PROGRAMS, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

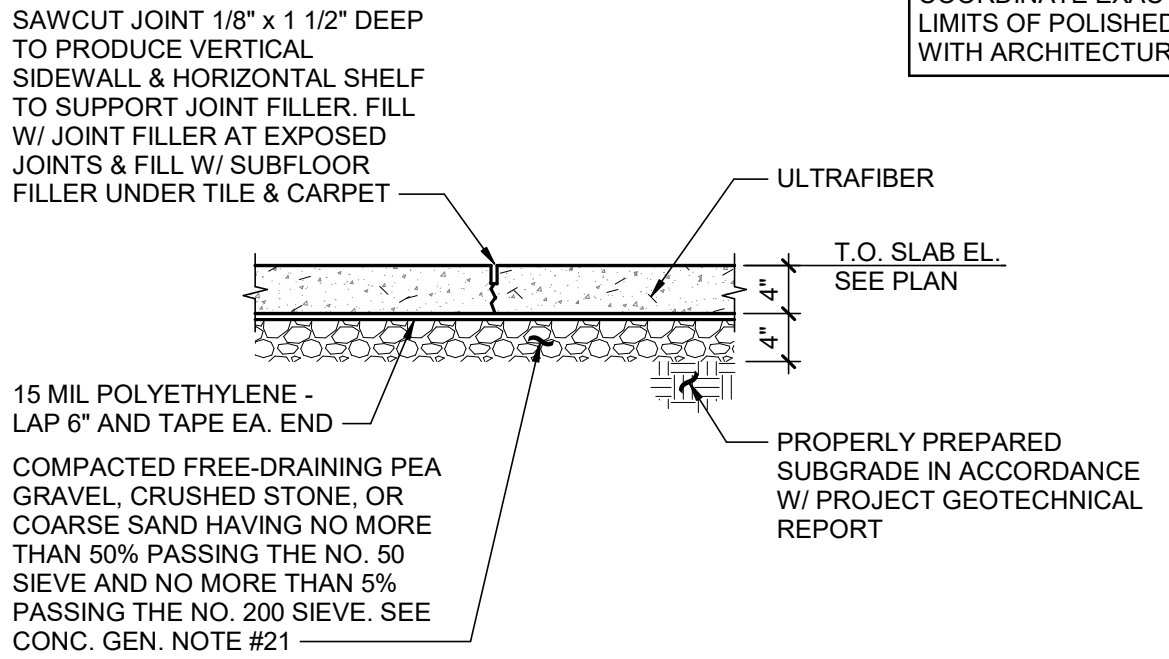
Concrete General Notes 3100:

- All detailing, fabrication and placing of reinforcing steel shall conform to the ACI Standard "Details and Detailing of Concrete Reinforcement" (ACI 315).
- Concrete at slab-on-grade shall develop a 28-day minimum compressive strength of 3,500 psi and have Ultra Fiber 500 manufactured by Solomon Colors, Inc. mixed into the concrete at 2.0 lb/cubic yard. Follow normal mixing time and speed as recommended by ASTM C94.
- Concrete at elevated slab on metal deck shall develop a 28-day minimum compressive strength of 3,500 psi. Elevated slab concrete shall be normal weight.
- All concrete at footings, pedestals and walls shall typically develop a 28-day minimum compressive strength of 3,000 psi.
- All concrete for footings, walls and slab-on-grade shall have a 5" maximum slump. Concrete at elevated slabs shall have a 4" maximum slump.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All reinforcing bar splices shall be 44 bar diameters for #6 and smaller diameter bars. Reinforcing bar splices shall be 48 bar diameters for #7 and larger bar diameters.
- All reinforcing bar hooks shall be ACI standard 90 degree hook, unless noted otherwise.
- Provide two #4 x 4'-0" long diagonal bars centered in slab, at all re-entrant corners and any other locations designated on the plans.
- Provide corner bars in footings, concrete stemwall and turn-down slab same size and spacing as longitudinal reinforcing.
- Provide (1) #4 hoop with 8" lap in slab-on-grade and elevated slab around floor drains, columns and all slab penetrations 3" in diameter or greater. Also install around electrical conduit groupings 3" in diameter or greater.
- Limit the width of conduit groups to 3'-0" as they pass under wall footings. As much as possible, align the conduit perpendicular to the footing as it passes under the footing. Provide a minimum spacing of 2'-0" between conduit groups as the conduit passes under a footing. **Do not extend conduits under column footings or spread footings.**
- Plate dowel system shall be Diamond Dowel System by PNA Construction Technologies, the Speed Plate System by SIKA Corp. or approved equal. Install plate dowels at slab construction joints at 18" o.c.
- Welded wire fabric shall conform to ASTM A1064. Provide mesh in flat sheets.
- Wire fabric reinforcing shall lap 6" and be securely wired at each side and end.
- Smooth dowels shall be steel conforming to ASTM A36.
- All slots, sleeves and other embedded items shall be set before concrete is placed. See Architectural, Electrical, Mechanical, and Vendor's drawings for size and locations.
- Bar supports at footings, slabs-on-grade and elevated slabs shall be factory made wire bar supports, type "SBU" linear supports.
- Epoxy for doweling reinforcement shall be HY-200 by Hilti, AT 3G by Simpson Strong-Tie or AC208+ by Dewart, unless noted otherwise.
- Maximum net allowable bearing pressure = 2,000 psf. for continuous footings and 2,500 psf for isolated spread footings. Footings shall bear on moderate shear strength clays/sands/gravels (existing fill) or new select fill as described in the Geotechnical & Testing Services, Inc. Soils Report No. 26-15041 dated May 22, 2026.
- Use of compacted, free-draining pea gravel, crushed stone, or coarse sand underneath the building slab is recommended by TSW, Inc. Consult Geotechnical Engineer regarding potential substitution of free-draining coarse materials with approved subgrade. Slabs-On-Grade have been designed for a modulus of subgrade reaction (k-value) of 100 psi/in.

NOTE:
EXPANSIVE CLAYS, LOW-STRENGTH SOILS, SOFT SOILS AND SOILS WITH ORGANIC MATERIAL ARE NOT SUITABLE IN SUPPORTING THE SLAB AND FOUNDATIONS. IF CONTRACTOR DISCOVERS UNSTABLE MATERIAL DURING EARTHWORK, A GEOTECHNICAL ENGINEER SHALL BE CONSULTED IN ORDER TO ARRIVE AT A SOLUTION THAT WILL NOT COMPROMISE THE STRUCTURAL INTEGRITY OF THE SLAB AND FOUNDATIONS.

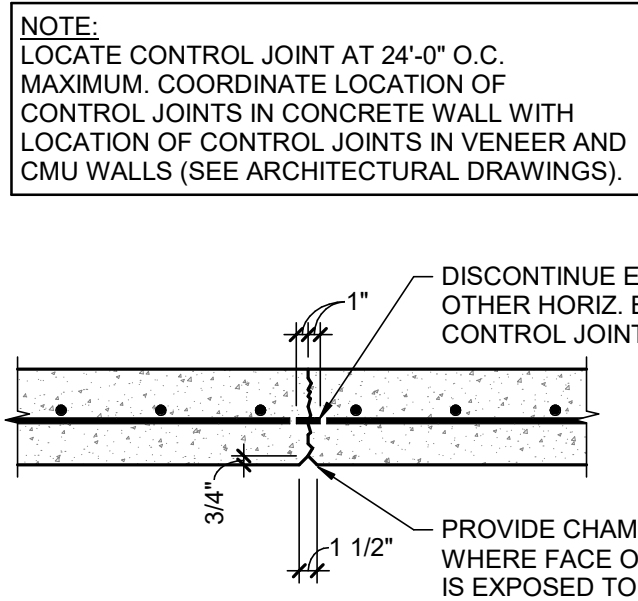
Concrete Masonry General Notes 4100:

- All concrete masonry units shall be lightweight above finished floor and normal weight below grade. All hollow concrete masonry units shall conform to ASTM C90, Grade N, Type 1 with a minimum ultimate compressive prism strength (f'm) of 2,000 psi for the masonry assemblage. All concrete masonry shall be laid in Running (Common) Bond.
- Mortar at exterior walls, all load-bearing walls, walls below grade and non-load-bearing walls higher than 20'-0" shall be Type S mortar and have a minimum compressive strength of 1,800 psi. Mortar at interior non-load-bearing walls not higher than 20'-0" and mortar at masonry veneer shall be Type N mortar and have a minimum compressive strength of 750 psi. All mortar shall conform to ASTM C270. Masonry cement shall not be used for mortar.
- All grout shall be ready-mix concrete, with 3/8" diameter max. aggregate, have a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10".
- All 8" CMU bond beam units shall be reinforced with one bar. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of bars for vertical wall reinforcement and bond beam requirements. Provide corner bars and lap bond beam reinforcing 48 bar diameters.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All bolts, anchors, reinforcement and embedded items shall be grouted in place.
- All reinforcing bar splices shall be 48 bar diameters, U.N.O.
- At all 8" CMU walls except at interior non-load bearing walls, provide (1) vertical bar each cell for the first (2) cells adjacent to control joints in walls, at ends of walls, wall corners and on each side of wall openings, unless noted otherwise. Vertical bars shall match reinforcement for remainder of wall. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of reinforcement.
- Provide control joints in brick veneer where shown on Architectural Drawings.
- Provide control joints in CMU walls where shown on Architectural Drawings. Place joints for CMU walls max. 24'-0" o.c.
- Provide horizontal joint reinforcement at 16" o.c. Reinforcement shall be ladder design, min. 9 gage welded steel wire, hot dipped galvanized to 1.5 oz. width shall be 1 1/2" less than wall thickness.



1 TYP. SLAB-ON-GRADE CONTROL JT. DTL. (CJ)

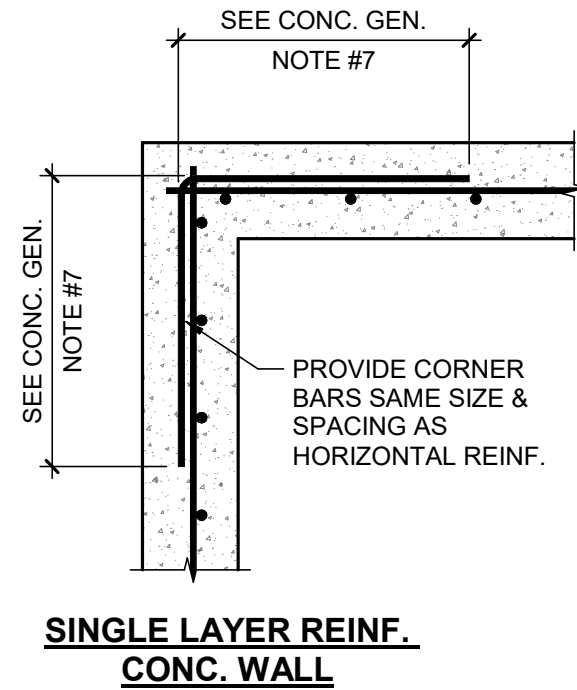
NOT TO SCALE



SINGLE LAYER REINF. CONC. WALL

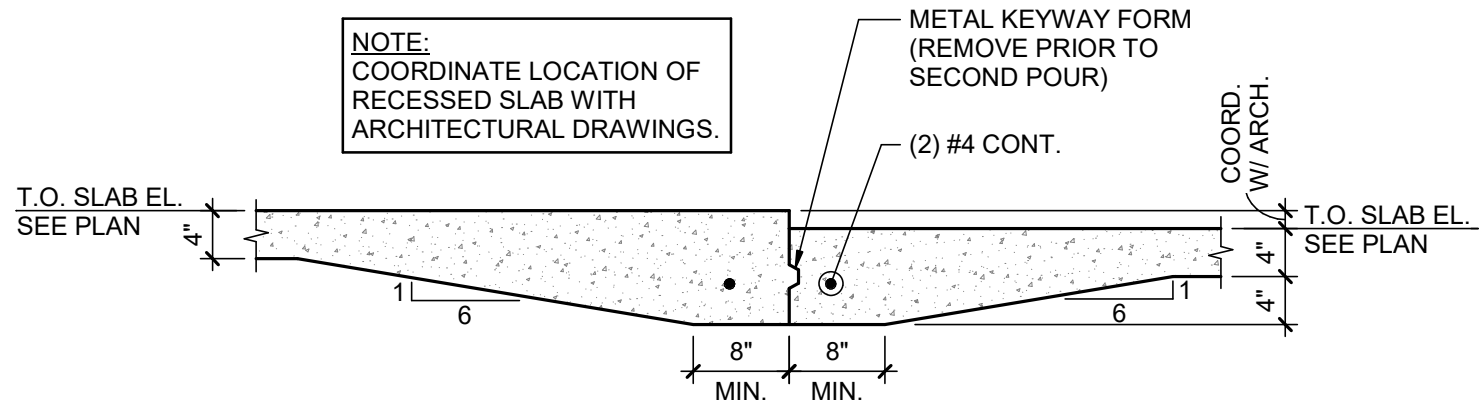
3 TYP. CONCRETE WALL CONTROL JT. DTL.

3/4" = 1'-0"



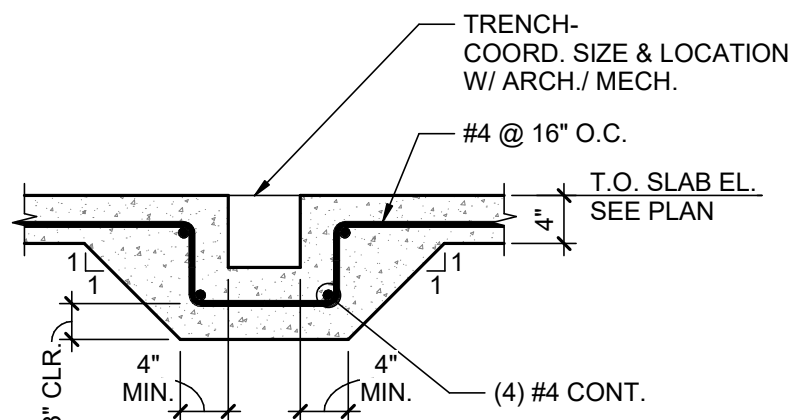
5 TYP. CONCRETE WALL CORNER DETAIL

NOT TO SCALE



7 TYP. RECESSED SLAB

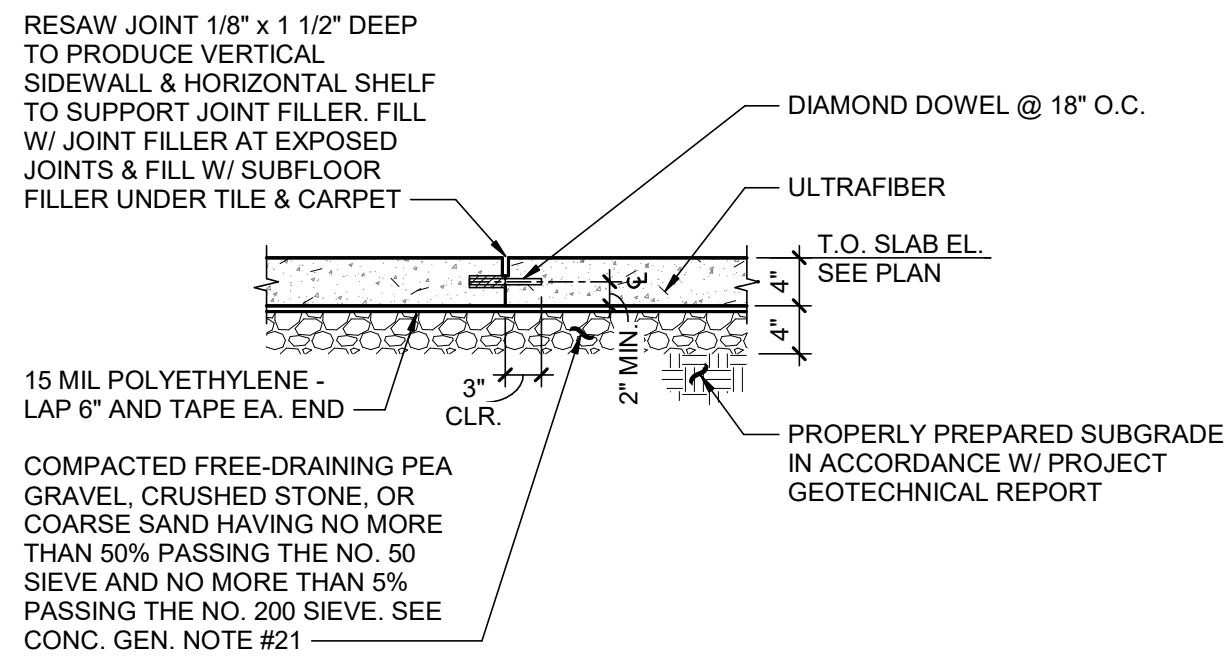
NOT TO SCALE



9 TYP. TRENCH DETAIL

NOT TO SCALE

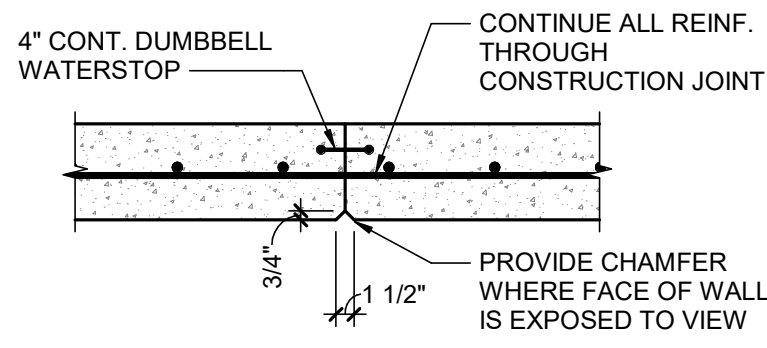
NOTE: PROVIDE DIAMOND DOWEL SYSTEM AS MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES OR THE SPEED SYSTEM BY SIKA CORPORATION AT CONSTRUCTION JOINTS IN ALL FLOOR AREAS. COORDINATE WITH ARCHITECT FOR CONSTRUCTION JOINT LOCATIONS.



2 TYP. SLAB-ON-GRADE CONSTRUCTION JT. DTL. (CJ)

NOT TO SCALE

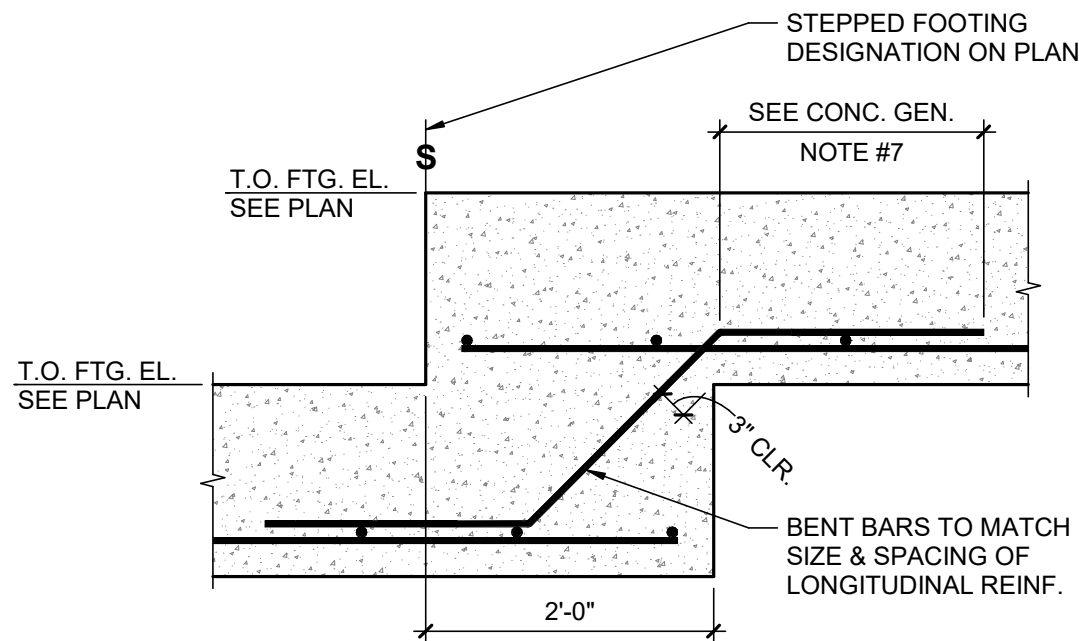
NOTE: CONSTRUCTION JOINT MAY BE SUBSTITUTED FOR CONTROL JOINT.



SINGLE LAYER REINF. CONC. WALL

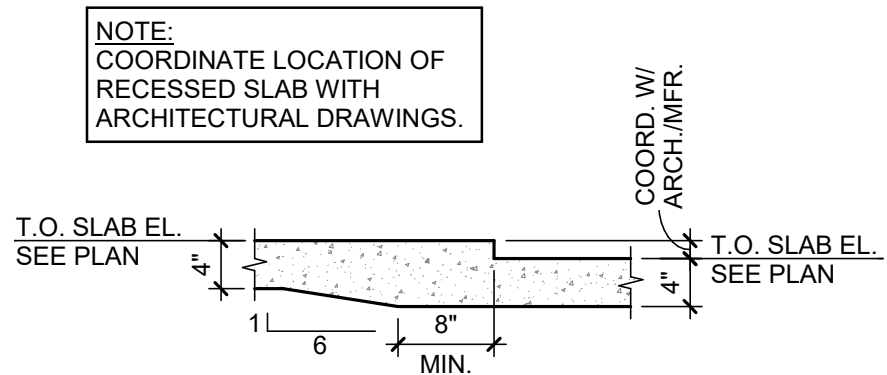
4 TYP. CONCRETE WALL CONSTRUCTION JT. DTL.

NOT TO SCALE



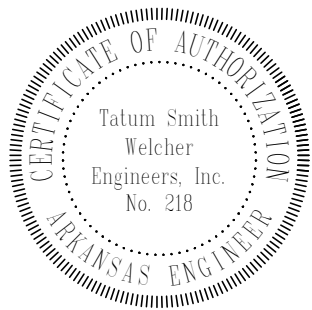
6 TYP. STEPPED FOOTING DETAIL

NOT TO SCALE



8 TYP. RECESSED SLAB

NOT TO SCALE



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sheet S1.1
FOUNDATION GENERAL NOTES & TYP. DETAILS
13 April 2026

Structural Steel General Notes 5100:

- All detailing, fabrication and erection of structural steel shall conform to the requirements of the AISC Specification for the Design, Fabrication and Erection of Structural Steel for Buildings.
- Wide flanges, Channels, and WT tees shall conform to ASTM A992 with a yield strength of 50 ksi.
- Hollow Structural Section (HSS) shall conform to ASTM A500, Grade C with a yield strength of 50 ksi.
- Steel pipes shall conform to ASTM A501 or ASTM A53, Grade B.
- Headed stud anchors (H.S.A.'s) shall conform to ASTM A108.
- All angles shall be ASTM A572 with a yield strength of 50 ksi. All other structural steel shall conform to the requirements of ASTM A36. Angle, plate and beam lintels at exterior wall openings shall be hot-dipped galvanized.
- All anchor rods shall conform to ASTM F1554, Grade 36. Nuts for anchor rods shall conform to ASTM A563, Grade A, heavy hex and anchor rod washers shall conform to ASTM F844.
- All welding shall conform to the Specifications of the American Welding Society. Welding electrodes shall be E-70 low hydrogen series. Welding shall be done by a certified welder.
- High strength bolts shall typically be 3/4" diameter bolts conforming to ASTM A325. Connections shall be designed as bearing type with threads in shear plane. Holes shall be 1/16" larger than bolt size. See details for connections with 1" diameter bolts.
- All bolts shall be tightened to a snug-tight condition. A snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. All connected elements must be brought into snug contact.
- Bearing ends of all columns shall be square cut.
- No openings shall be cut in structural members unless shown on the drawings.
- Steel frame is non-self-supporting and column anchor rods are designed for a completed condition only. Metal roof deck, elevated slabs, steel moment frames, braced frames and masonry shear walls are required to provide lateral stability for the frame and resistance to wind and seismic forces. Contractor shall provide all temporary bracing required to maintain stability of structural system.
- Anchor rod holes in base plates shall be sized in accordance with AISC "Detailing For Steel Construction".
- All exposed edges of plates, beams, etc., shall be shop ground smooth and uniform.
- All column base plates supported on footings, pedestals and concrete columns shall bear on 1 1/2" non-shrink non-metallic grout unless noted otherwise.
- 1/2"Ø sleeve anchors shall be 1/2"Ø x 4" long HLC-H Sleeve Anchor by Hilli. HLC-H Sleeve Anchor shall be carbon steel with zinc plating & have a 3" embedment depth.
- 1/2"Ø expansion bolts shall be 1/2"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 3 1/2" embedment depth.
- 3/4"Ø expansion bolts shall be 3/4"Ø x 5 1/2" long Kwik Bolt 3 by Hilli. Expansion Anchor shall be carbon steel with zinc plating & have a 4 3/4" embedment depth.

GENERAL CONTRACTOR SHALL INCLUDE 2 TON AT A MINIMUM COST OF \$10,000/ TON OF MISCELLANEOUS STEEL BEAMS, CHANNELS AND ANGLES IN ADDITION TO THE FRAMING SHOWN ON THE PLANS AND DETAILS. GENERAL CONTRACTOR SHALL INCLUDE ERECTION, FABRICATION, DESIGN AND DETAILING COSTS FOR THIS ADDITIONAL FRAMING WITH THE BASE BID. THE USE OF MISCELLANEOUS STEEL IS TO BE RECORDED BY THE GENERAL CONTRACTOR AND ANY UNUSED AMOUNT IS TO BE CREDITED TO THE OWNER.

Steel Joist General Notes 5200:

- The design, fabrication and erection of steel joists shall conform to the Standard Specifications for Open Web Steel Joists of the Steel Joist Institute. Joists have been designed according to the ASD Load Tables. Joist loads shown on the drawings are ASD loads.
- All hangers supported from open web joists shall be located at bottom chord panel points and shall be connected without field welding or drilling holes in the bottom chord.
- Joists shall have bridging per SJI Specifications. Roof joists shall have additional bridging, as required to resist stress reversal for the net uplift load indicated.
- Roof joists shall be designed for a net wind uplift of 30 psf. One-third stress increase of joists for wind uplift will not be allowed.
- "K" Joists framing into or nearest to columns shall be field bolted with (2) 1/2" diameter A307 bolts and have a 1/4" x 6" x 6" bottom chord stabilizer plate to provide lateral stability during construction. Do not weld bottom chord to stabilizer plate.
- Joist manufacturer shall design joists for mechanical and equipment loads noted on the framing plans in addition to the uniform dead and live loads. Coordinate equipment size and locations with mechanical contractor.
- Joist manufacturer shall design roof joists for the snow drift and sliding snow loadings shown in the Design Loads. See Snow Drift Plan (on this sheet) for locations of drifts and sliding snow.
- Attach joist top and bottom chord bridging to masonry walls with L2 1/2 x 2 1/2 x 3/16 x joist depth and (3) 1/2"Ø sleeve anchors, minimum.

Steel Deck General Notes 5300:

- Typical roof deck shall be 1 1/2" deep, 22 gauge, wide rib type and shall have nested side laps (Vulcraft 1.5B22, New Millennium 1.5B22 or approved equal). See Roof Framing Plans for limits of roof deck.
- Roof deck shall be welded to the steel framing per the Roof Deck Fastening Pattern Detail 3/S1.2.
- Roof deck fastening pattern has been designed for a net wind uplift of 38.7 psf at corner zones, 38.7 psf at side zones and 28.4 psf at interior zones for roofs.
- Deck at elevated concrete cap shall be 1 1/2" deep 22 gauge, galvanized steel VL Composite deck. (Vulcraft 1.5VL22, New Millennium 1.5CD22 or approved equal)
- Deck at Elevated Concrete Cap shall be attached to the steel per the Composite Deck Fastening pattern, see detail 1/S1.2.
- Floor deck at intermediate landings for stairs shall be 1" deep, 26 gauge, galvanized steel form deck (Vulcraft 1.0C26, New Millennium 1.0FD26 or approved equal). Form deck shall be welded to the steel framing at 18" o.c. with 3/8" diameter puddle welds & welding washers. Sidelap stitch connector shall be one #10 self-drilling, self-tapping screw between framing.
- All deck shall be fastened per Steel Deck Institute (SDI) requirements.
- Deck specified has been determined on basis of 3 span condition; deck supplier shall use heavier gauge if required for one and two span conditions.

Light Gauge Steel General Notes 5400:

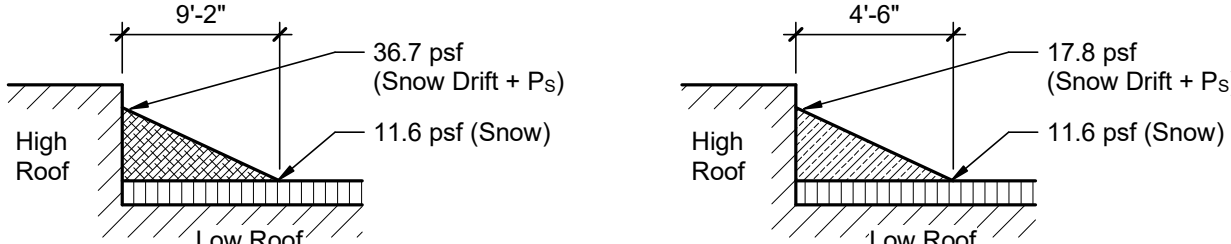
- All structural studs and headers shall be of the type, size, gauge and spacing as shown on the drawings.
- All framing members shall be formed from steel, corresponding to the requirements of ASTM C955.
- Steel for framing members shall have a minimum yield strength of 33 ksi, unless noted otherwise.
- Exterior stud walls and lintels shall be constructed with the following, unless noted otherwise.
 - 6" studs: 600S162-54
 - 6" track: 600T125-54
- See Architectural Drawings for summary of non-structural stud sizes; runner track gauge to match stud gauge, except at walls at strap bracing.
- Load-bearing studs shall be braced in accordance with the manufacturer's specifications and recommendations. See Metal Stud Bridging Detail 3/S1.3.
- Slide clips shall be VertiClip SLB with vertical slots by The Steel Network, Inc. or approved equal.
- Rigid clips shall be StiffClip LB by The Steel Network, Inc. or approved equal.
- Powder actuated fasteners for attachment of bottom runner track shall have a 0.157" minimum shank diameter and a 1 1/2" minimum concrete embedment. Space fasteners at 16" o.c., unless noted otherwise.
- At interior stud walls extending to the underside of roof structures, provide a deflection track at the top of the wall.
- Provide bracing to structure for all stud walls and furr-downs.
- See Lintel Schedule on Drawing S3.0 for metal stud wall headers.

Wood Framing General Notes 6100:

- All framing lumber shall be Southern Yellow Pine (SYP) No. 2. Lumber shall have a moisture content between 7% and 19%.
- All nailing is to be in accordance with the 2021 International Building Code Wood Fastening Schedule, Table 2304.9.1, unless noted otherwise.
- All wood connectors are to be installed according to the manufacturer's specifications and recommendations with the maximum number and size of nails.
- All multi-member beams and headers shall be glued and nailed together with (3) rows of 16d nails at 12" o.c.. Nail both sides of all 3-ply members.
- All laminated veneer lumber (LVL) shall be 1.9E Microlam LVL as manufactured by TrusJoist or approved equal.
- All exterior walls shall be sheathed with 1/2" APA rated plywood sheathing along outside face of stud wall for full height of wall. Attach plywood to wood stud wall with 8d common nails at 6" o.c. along panel edges and 12" o.c. at intermediate studs. Attach plywood to metal stud wall with #10 screws at 6" o.c. along panel edges and 12" o.c. at intermediate studs.
- Where noted on the plan interior load bearing walls shall be sheathed with 1/2" APA rated plywood sheathing on one side full height from finish floor to bottom of second floor deck and from the second floor deck to bottom of roof sheathing. Attach plywood to stud walls with 8d common nails at 6" o.c. around perimeter of panel and 12" o.c. at intermediate studs.

Design Loads

- | | |
|--|--------------------------|
| 1. Typical Roof Dead Load: | 20 psf |
| 2. Roof Live Load: | 20 psf |
| 3. Typical Elevated Floor Dead Load (5" slab):
Stage Dead Load: | 65 psf
10 psf |
| 4. Typical Elevated Floor Live Load:
Stage Live Load: | 100 psf
150 psf |
| 5. Rain Intensity, (15 min,):: | 6.52 in/hr
3.38 in/hr |
| 6. Snow Load: <ul style="list-style-type: none">Ground Snow Load:Flat-roof Snow Load at main roof (P_g) = 16.5 psfSlope Factor (C_s) = 1.0Snow Exposure Factor (C_e) = 1.0Snow Load Importance Factor (I_s) = 1.1Thermal Factor (C_t) = 1.0 | 15 psf |
| 7. Snow Drift: | |

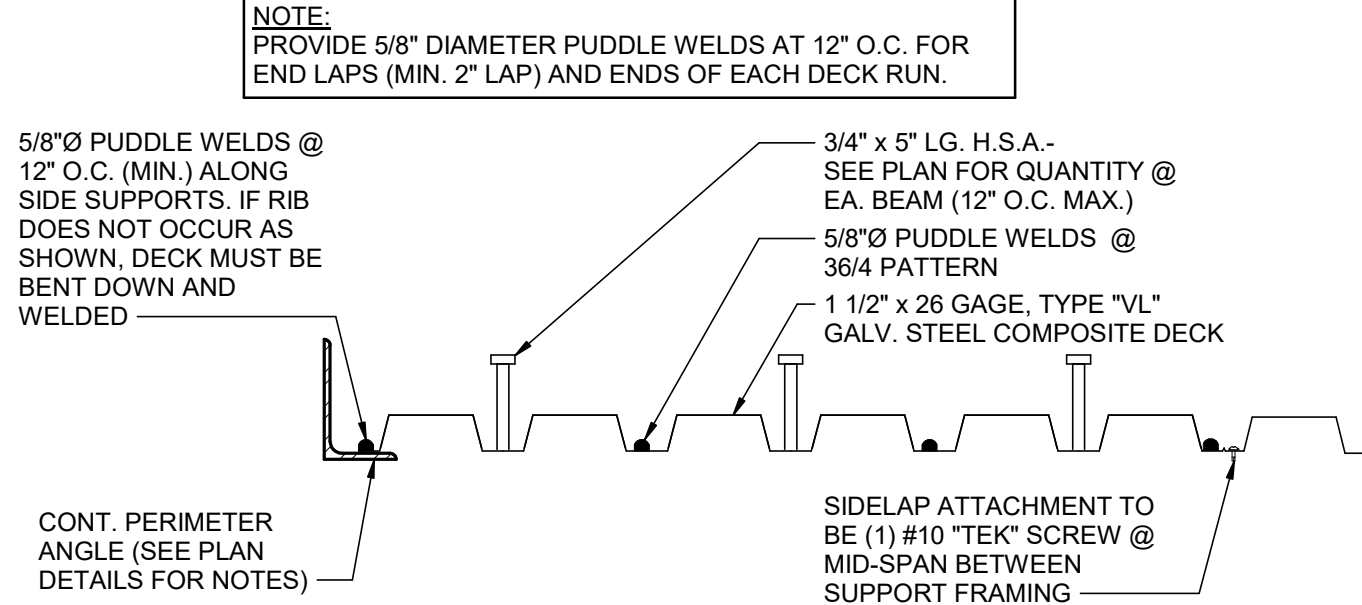


A Drift @ Low Roof

B Drift @ Low Roof

- | | |
|--|--|
| 8. Wind Load: <ul style="list-style-type: none">Ultimate Design Wind Speed (V_{ult}):Nominal Design Wind Speed (V_{nom}):Risk Category IIIWind Exposure BInternal Pressure Coefficient, GC_p = ±0.18 | 115 mph
89.9 mph |
| 9. Components & Cladding Wind Load (Unfactored): <ul style="list-style-type: none">Width of Edge Zone, a =Wall Pressures (10 ft²)<ul style="list-style-type: none">End Zone Wall =Interior Zone =Wall Pressures (100 ft²)<ul style="list-style-type: none">End Zone Wall =Interior Zone =Roof Pressures (10 ft²)<ul style="list-style-type: none">Corner Zone =Eave & Rake Zone =Interior Zone =Roof Pressures (100 ft²)<ul style="list-style-type: none">Corner Zone =Eave & Rake Zone =Interior Zone = | 3 ft
29.4 psf
23.9 psf
22.1 psf
22.1 psf
50.7 psf
50.7 psf
38.4 psf
39.8 psf
39.8 psf
30.0 psf |
| 10. Seismic: <ul style="list-style-type: none">Risk Category IIISeismic Importance Factor (I_s) = 1.0S_s = 0.154S₁ = 0.090S_{0.2} = 0.165S_{0.1} = 0.143Site Class D (presumed per IBC Sec. 1613.2.2)Seismic Design Category CBasic Structural System:<ul style="list-style-type: none">Seismic Resisting System:Response Modification Coefficient (R):Deflection Amplification Factor (C_d):Seismic Response Coefficient (C_s):Analysis Procedure: | Bearing Wall System
Intermediate Reinforced Masonry Shear Walls
3.5
2.25
0.059
Equivalent Lateral Force Procedure |
| 11. Building Code: <ul style="list-style-type: none">2021 International Building CodeASCE 7-162012 Arkansas Fire Prevention Code, Volume II | |

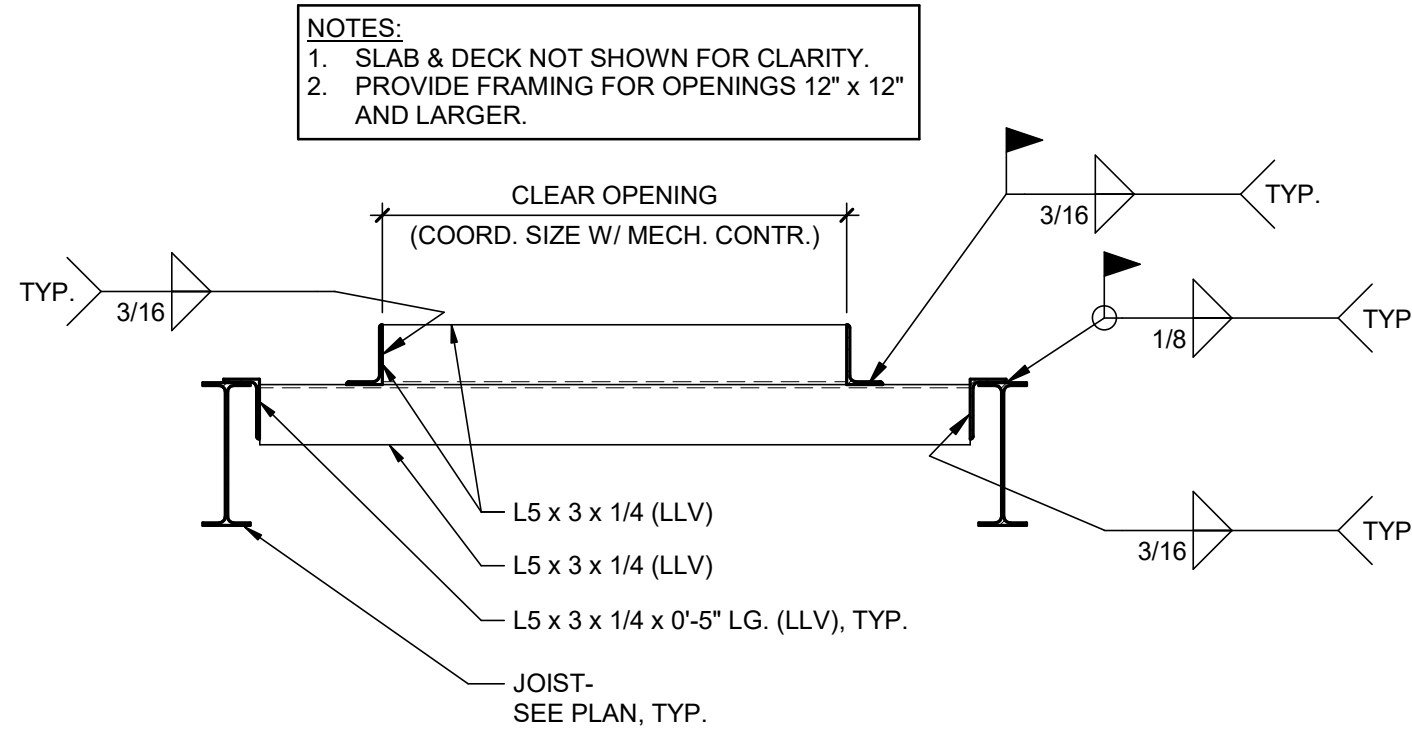
THIS FACILITY HAS BEEN DESIGNED FOR THE SEISMIC CRITERIA AND BUILDING CODE NOTED ON THIS DRAWING IN ACCORDANCE WITH THE REQUIREMENTS OF ACT 1100.



1 1/2" VL COMPOSITE FLOOR DECK

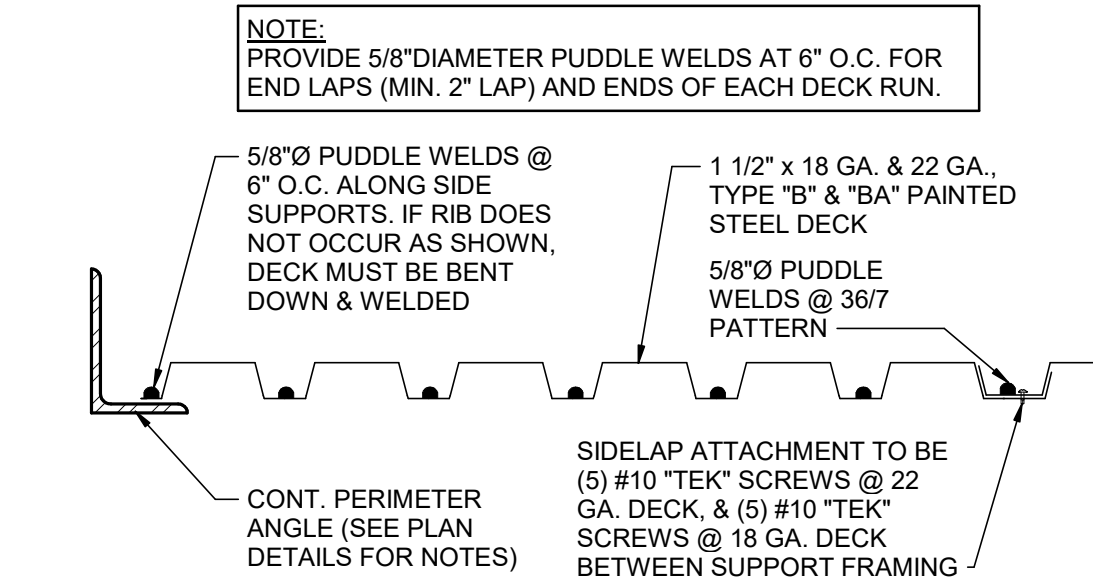
1 TYP. COMPOSITE FLOOR DECK FASTENING PATTERN DETAIL

NOT TO SCALE



2 TYP. FLOOR FRAMED OPNG. DETAIL

NOT TO SCALE

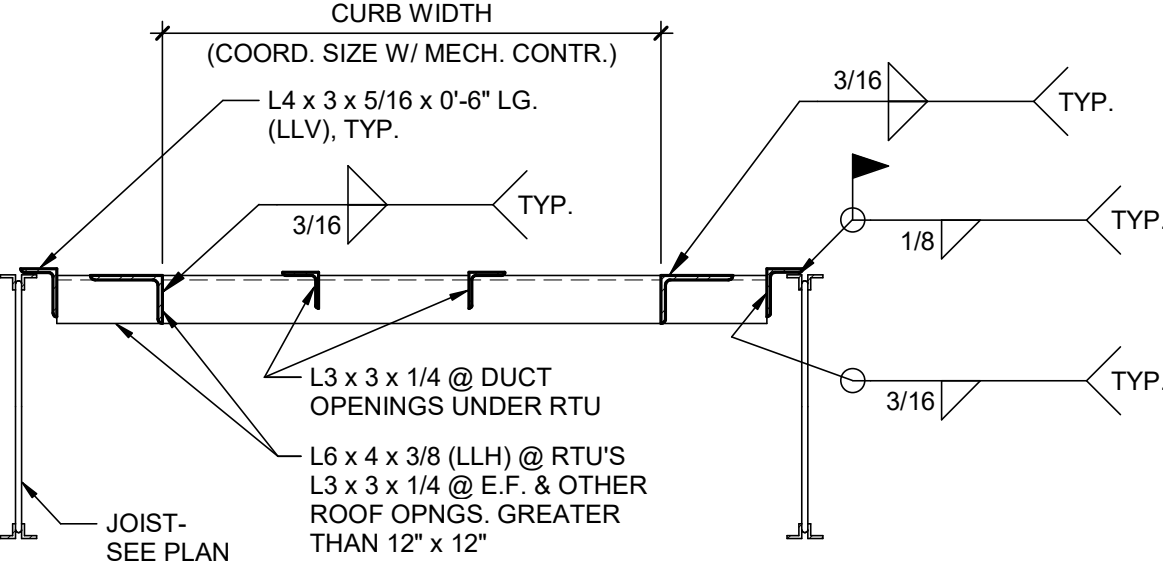


1 1/2" ROOF DECK

3 TYP. ROOF DECK FASTENING PATTERN DETAIL

NOT TO SCALE

- NOTES:
- AT L3 x 3 x 1/4 FRAMES, DELETE CLIP ANGLES, COPE VERTICAL LEG AND EXTEND HORIZONTAL LEG OVER FRAMING - WELD ALL AROUND.
 - FOR ROOF OPENINGS OVER 6" TO 12" WIDE (I.E.: OVER-SIZED OPENING FOR ROOF DRAINS), PROVIDE L2 x 2 x 1/4 FRAME AROUND OPENING AND EXTEND TO JOISTS- WELD ALL AROUND.



4 TYP. ROOF FRAMED OPNG. DETAIL

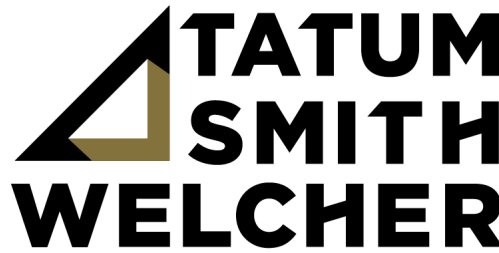
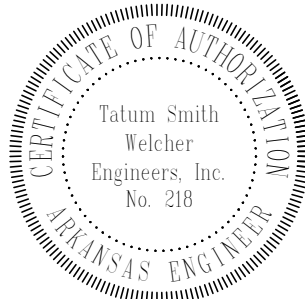
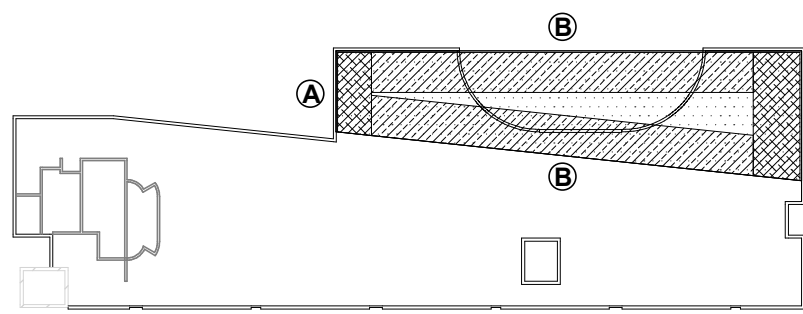
NOT TO SCALE

- LEGEND:**
- DENOTES LIMITS OF SLIDING SNOW @ LOW ROOF (SEE DESIGN LOADS)
 - DENOTES LIMITS OF SNOW DRIFT @ LOW ROOF (SEE DESIGN LOADS)



5 SNOW DRIFT & SLIDING SNOW PLAN

1" = 40'-0"



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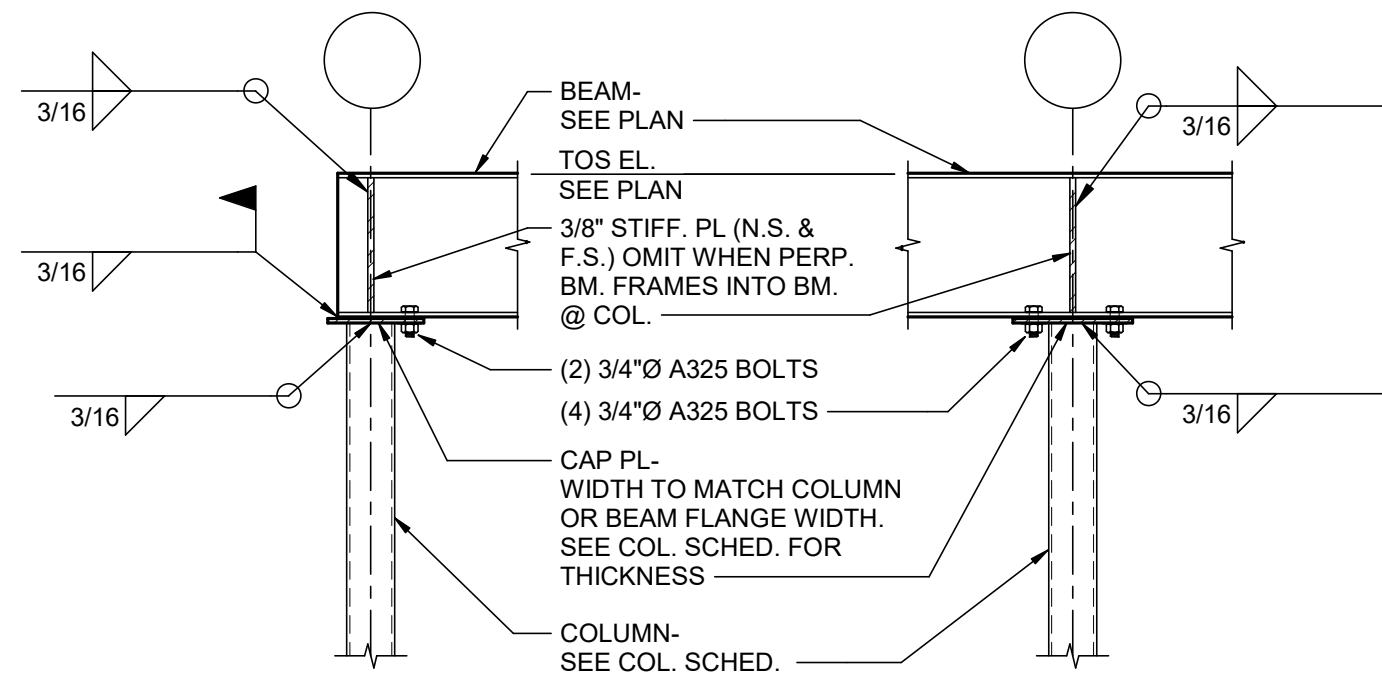
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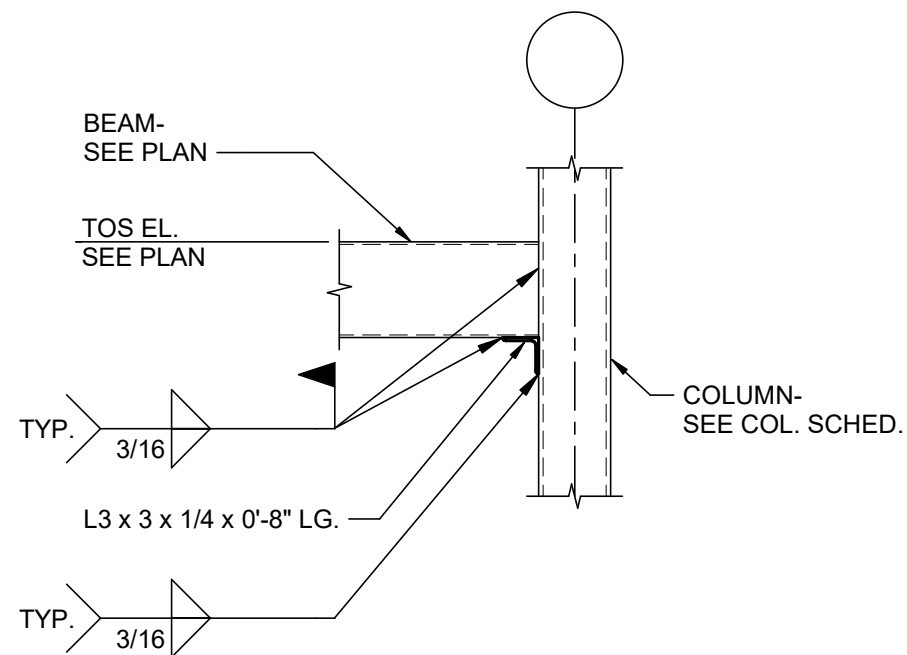
sheet S1.2
FRAMING GENERAL NOTES & TYP. DETAILS
13 April 2026



**TYP. HSS COL. CAP
PLATE DETAILS**

1

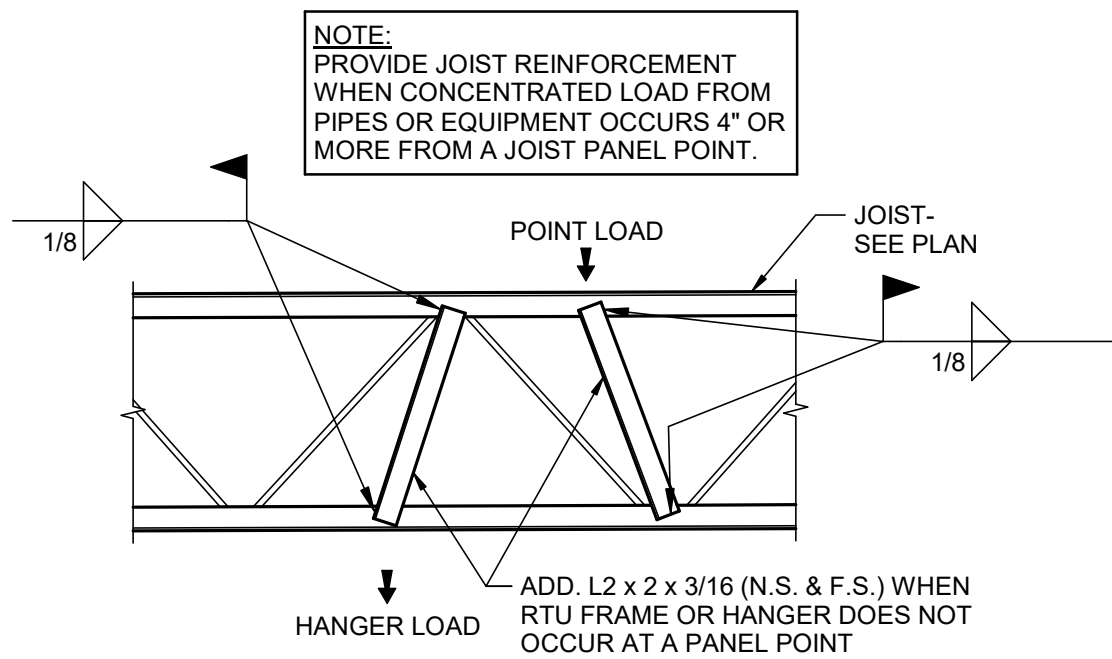
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**TYP. HSS BEAM TO HSS COLUMN
CONNECTION DETAIL**

2

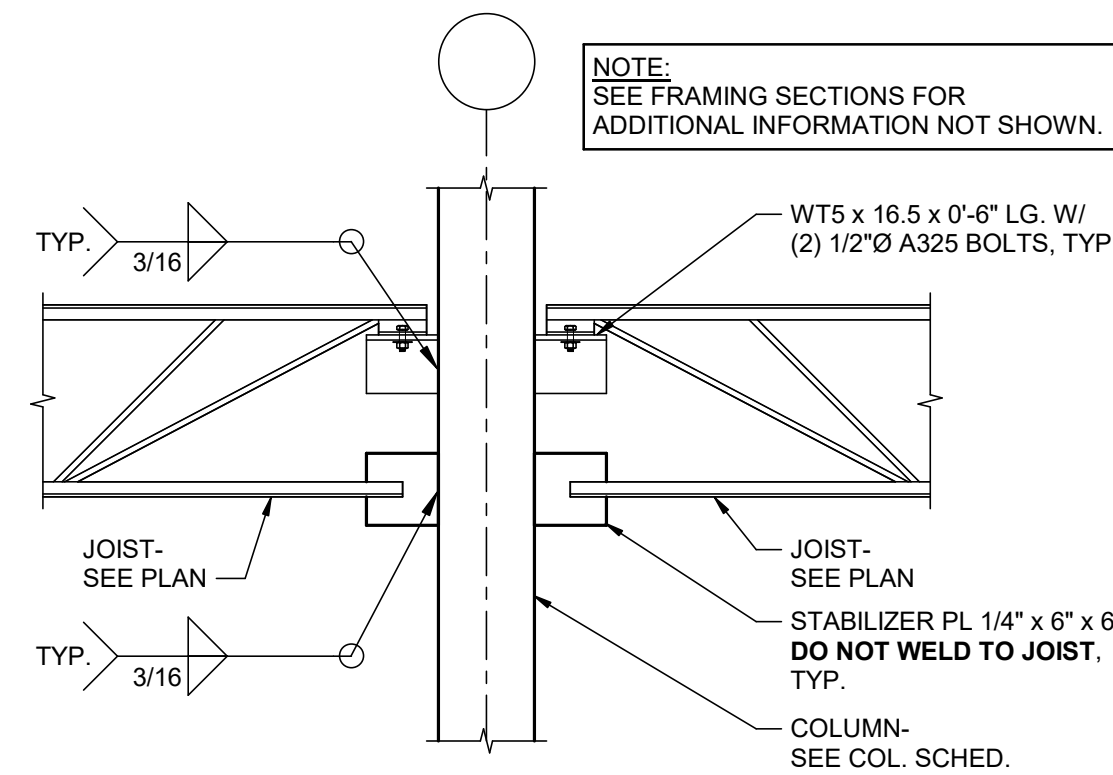
NOT TO SCALE



**TYP. JOIST REINFORCEMENT
AT CONCENTRATED LOADS**

3

NOT TO SCALE



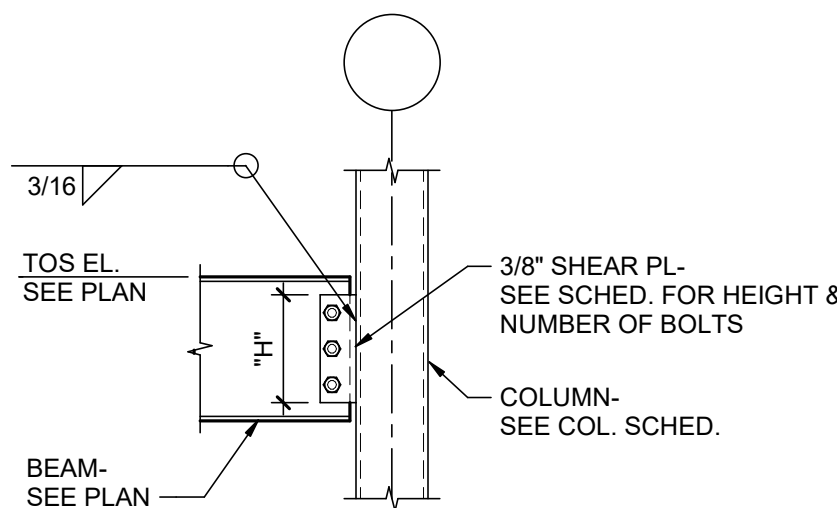
**TYP. JOIST TO COL.
CONNECTION DETAIL**

4

NOT TO SCALE

MISC. BEAM TO HSS COL. CONN. SCHED.			
BEAM SIZE	SHEAR PL SIZE (H)	NUMBER OF BOLTS	REMARKS
C8, W8, C10, W10	6"	(2) 3/4"Ø A325	-----
C12, W12, MC12, W14	9"	(3) 3/4"Ø A325	-----
C15, W16, W18, MC18	1'-0"	(4) 3/4"Ø A325	-----
W21, W24, W27	1'-3"	(5) 3/4"Ø A325	-----
W30	1'-6"	(6) 3/4"Ø A325	-----

NOTE:
TYPICAL BEAM TO COLUMN CONNECTION IS WITH CAP
PLATE. USE THIS CONNECTION AT MISCELLANEOUS BEAMS
FRAMING INTO COLUMNS BETWEEN MAIN FRAMING LEVELS.

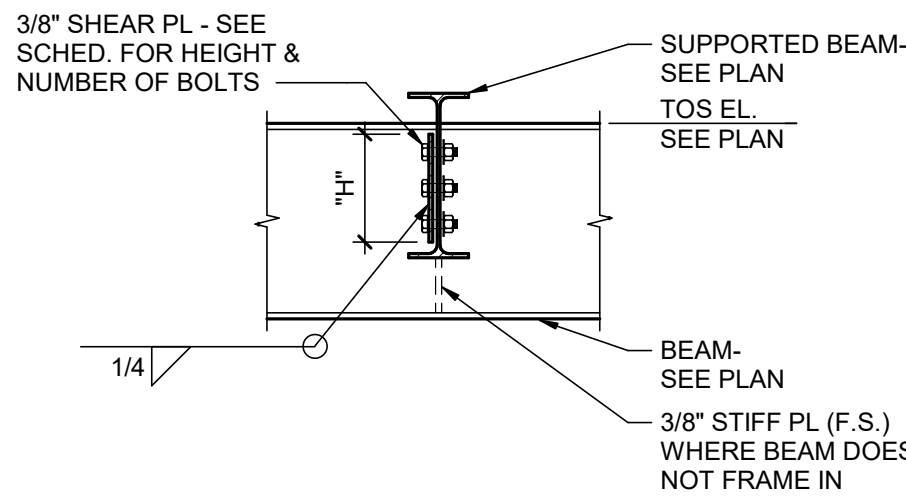


**TYP. MISC. BEAM TO HSS
COLUMN CONNECTION DETAIL**

5

NOT TO SCALE

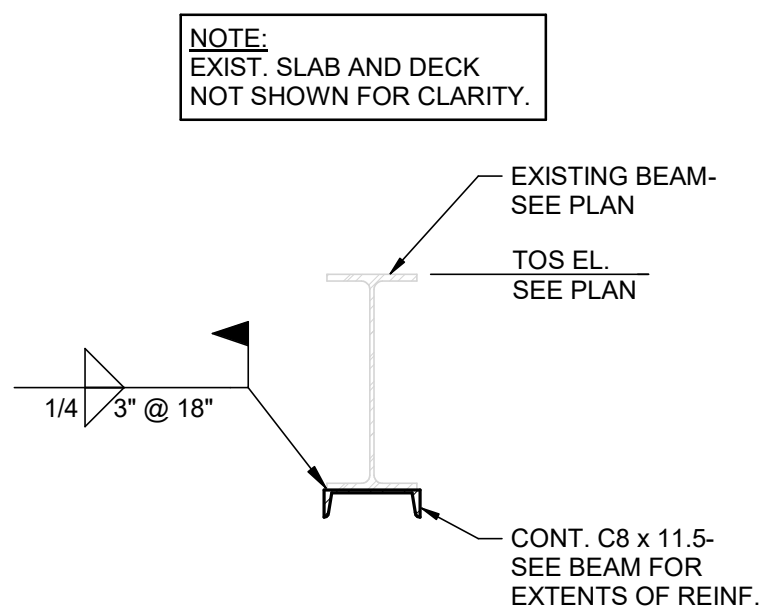
BEAM TO BEAM CONN. SCHED.			
SUPPORTED BEAM SIZE	SHEAR PL SIZE (H)	NUMBER OF BOLTS	REMARKS
C8, W8, C10, W10	6"	(2) 3/4"Ø A325	-----
C12, W12, MC12, W14	9"	(3) 3/4"Ø A325	-----
C15, W16, W18, MC18	1'-0"	(4) 3/4"Ø A325	-----
W21, W24, W27	1'-3"	(5) 3/4"Ø A325	-----
W30	1'-6"	(6) 3/4"Ø A325	-----



**TYP. BEAM TO BEAM
CONNECTION DETAIL**

6

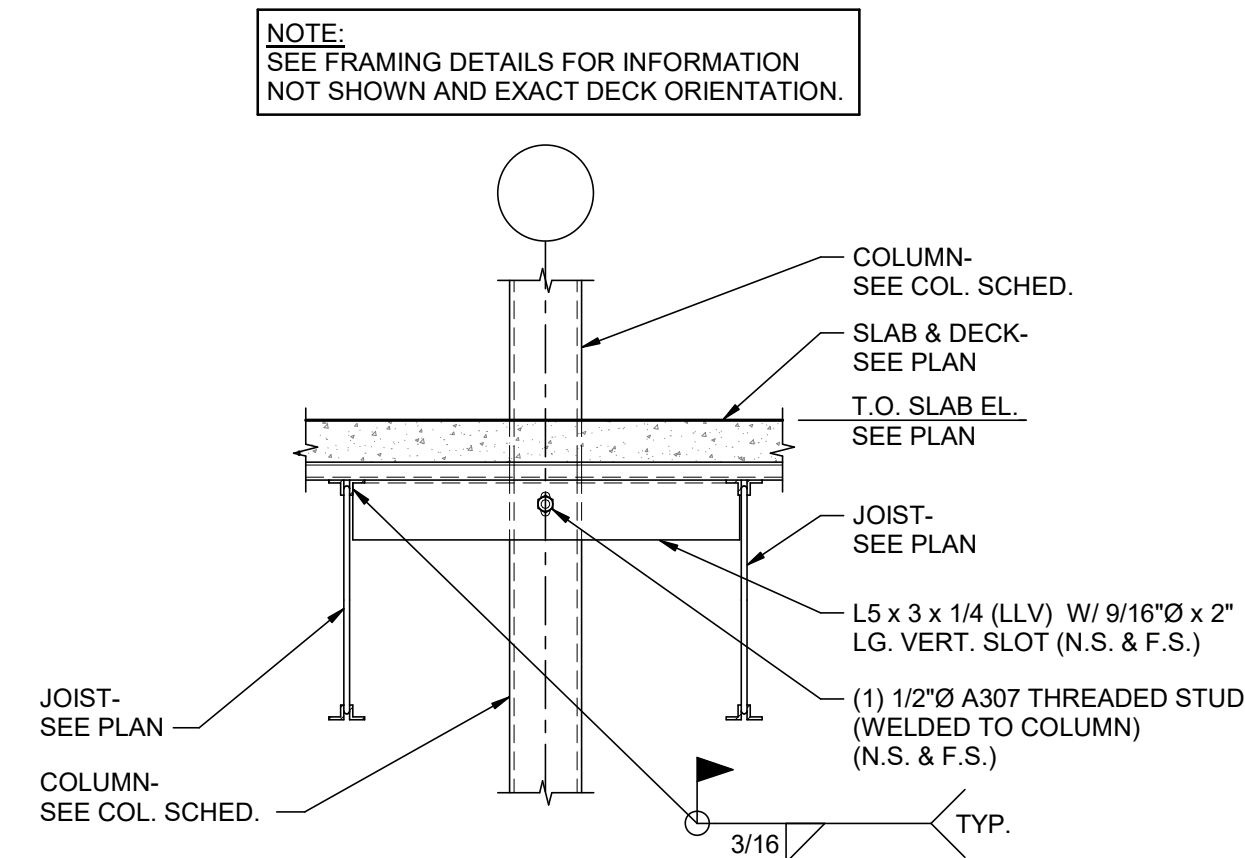
NOT TO SCALE



**TYP. EXIST. BEAM
REINFORCING DETAIL**

7

NOT TO SCALE

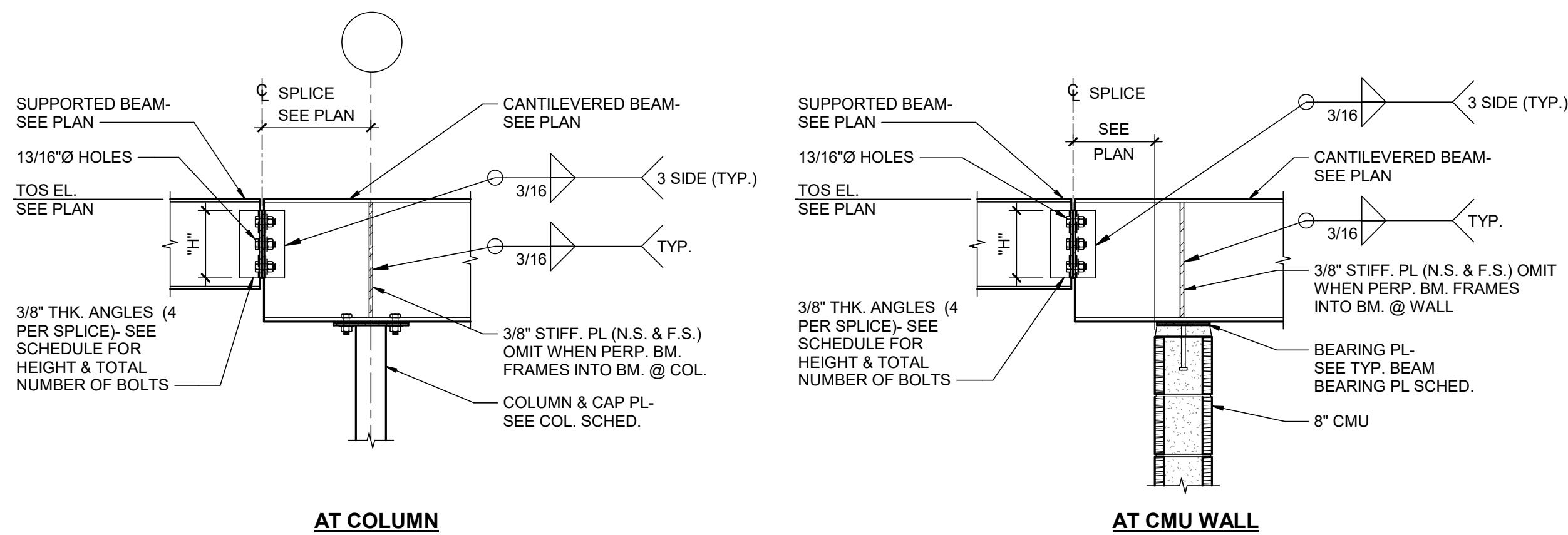


**TYP. HSS COLUMN BLOCKOUT @
ELEVATED SLAB DETAIL**

8

NOT TO SCALE

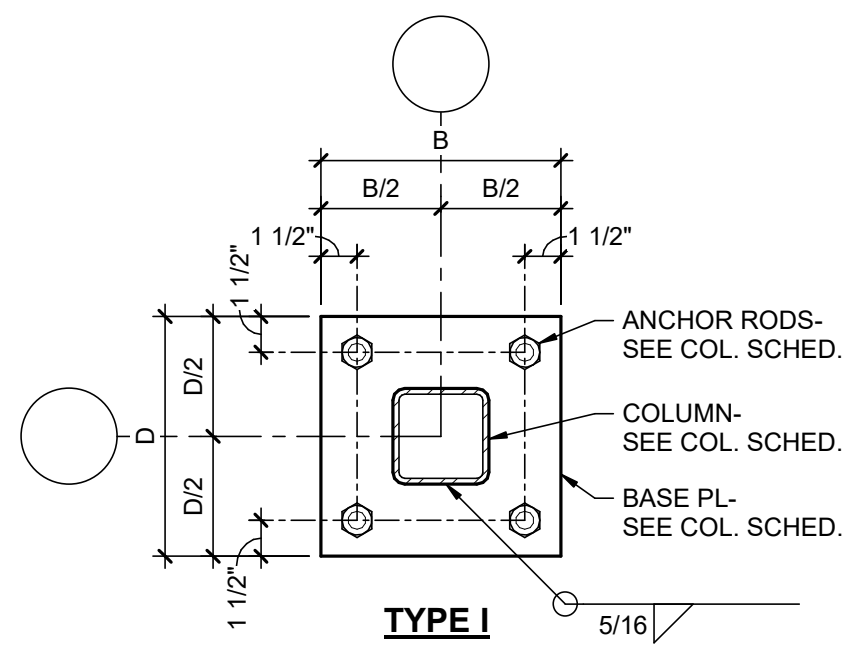
CANTILEVERED BEAM SPLICE SCHED.			
SUPPORTED BEAM SIZE	SHEAR PL SIZE (H)	NUMBER OF BOLTS (EA. SIDE)	REMARKS
C8, W8, C10, W10	6"	(2) 3/4"Ø A325	-----
C12, W12, MC12, W14	9"	(3) 3/4"Ø A325	-----
C15, W16, W18, MC18	1'-0"	(4) 3/4"Ø A325	-----
W21, W24, W27	1'-3"	(5) 3/4"Ø A325	-----
W30	1'-6"	(6) 3/4"Ø A325	-----



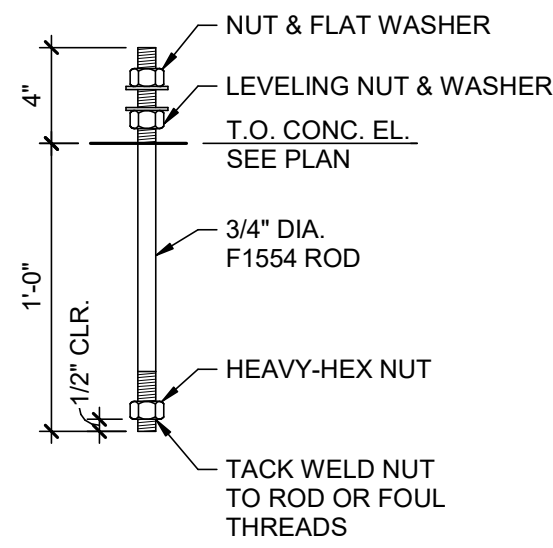
TYP. CANTILEVERED BEAM SPLICE

9

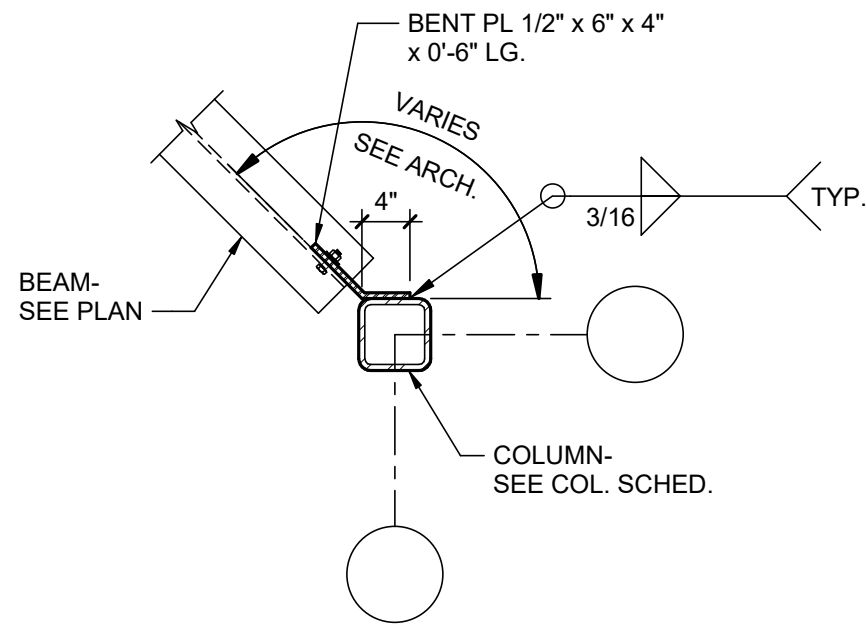
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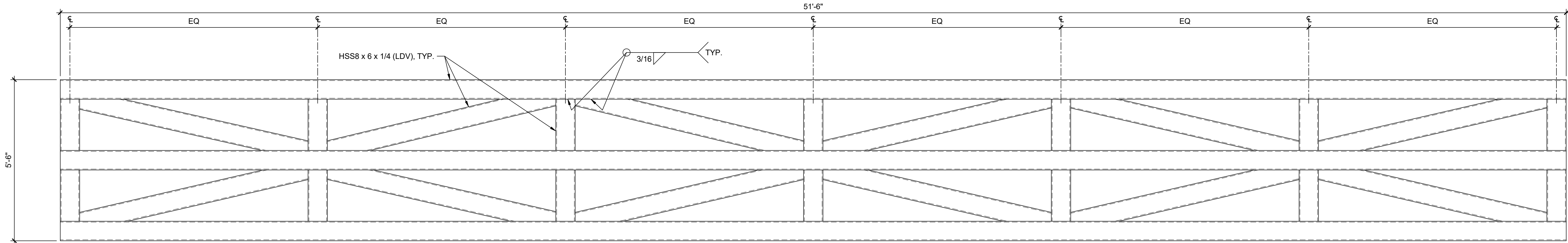
1 TYP. BASE PLATE DETAILS
1 1/2" = 1'-0"



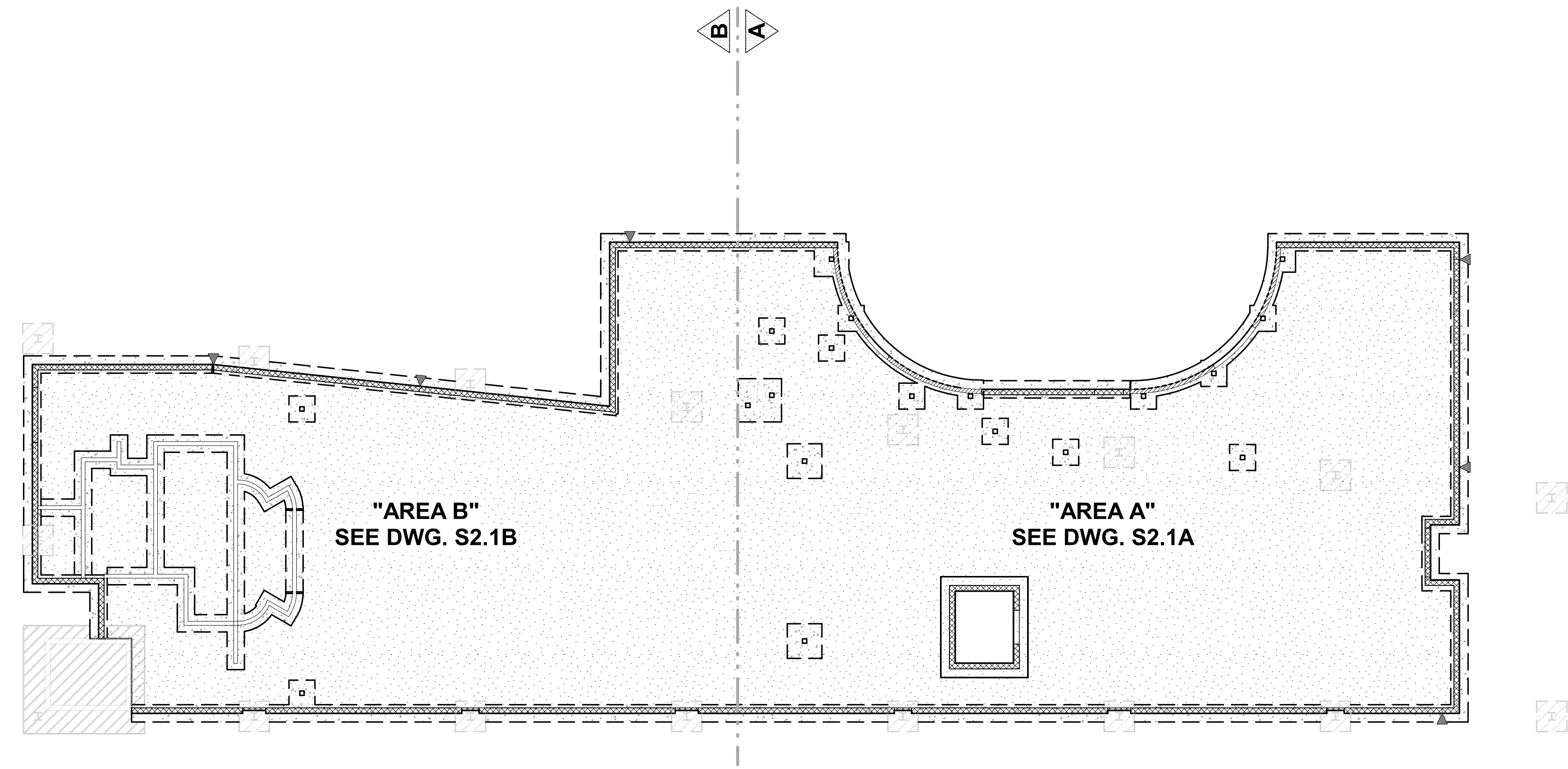
2 TYP. ANCHOR ROD DETAIL
1 1/2" = 1'-0"



3 TYP. BM. TO COL. ANGLED CONNECTIONS
NOT TO SCALE



4 TRUSS PROFILE
1/2" = 1'-0"

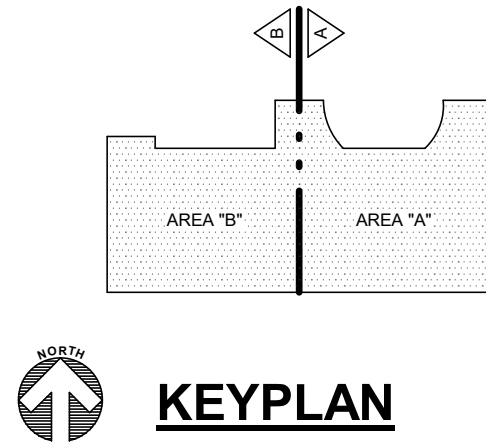


1 FOUNDATION PLAN - OVERALL

1" = 10'-0"

PLAN NOTES:

1. THIS DRAWING IS FOR REFERENCE ONLY.
2. SEE REFERENCED AREAS FOR ENLARGED PLANS.



KEYPLAN



sheet S2.1
FOUNDATION PLAN - OVERALL
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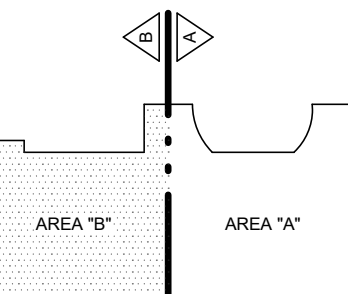
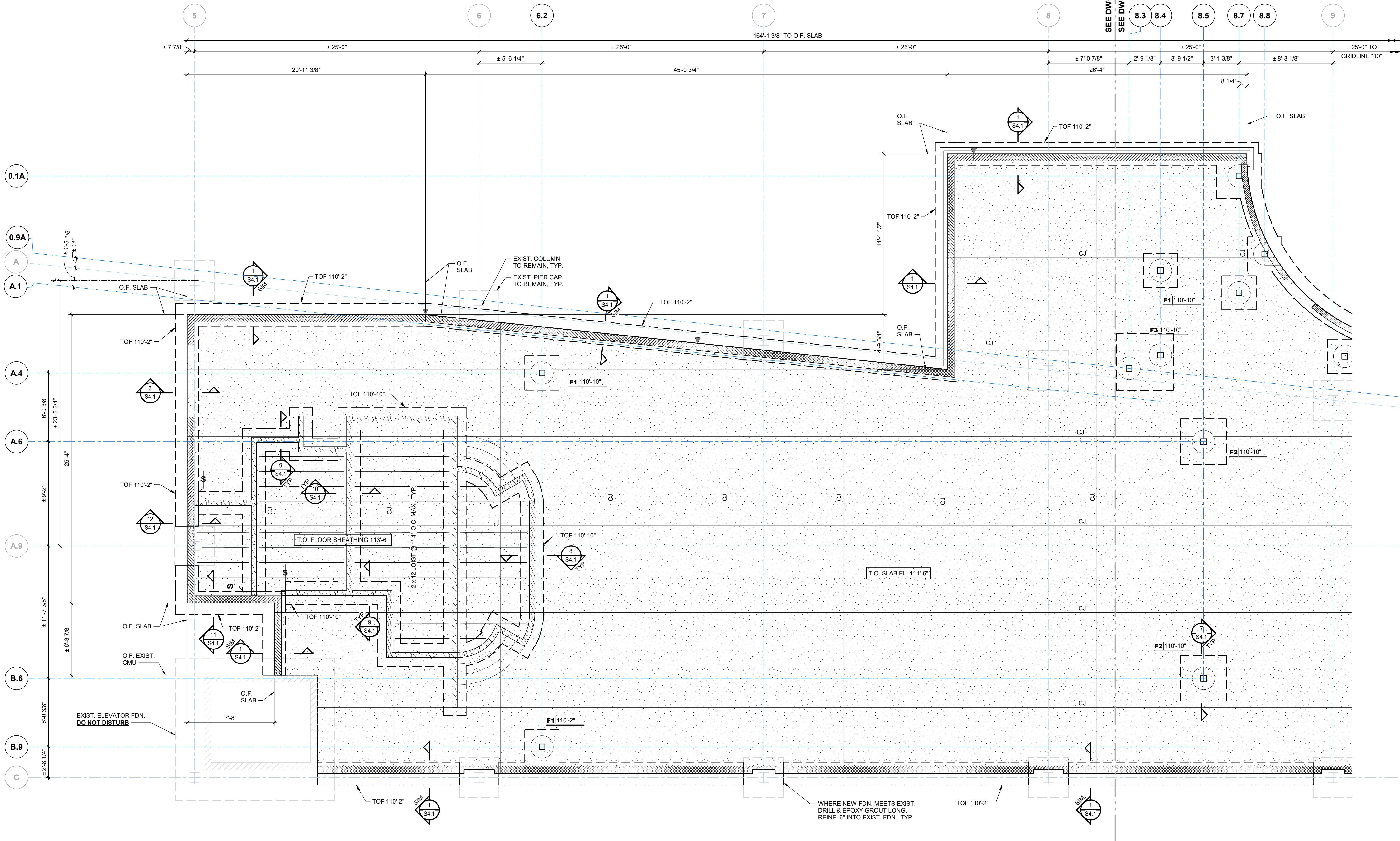
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1 FOUNDATION PLAN - AREA "B"

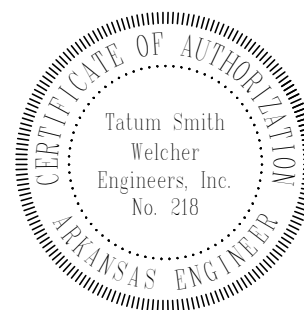
1/4" = 1'-0"

NOTE:
SEE DRAWING S2.1A FOR
LEGEND AND PLAN NOTES.



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sheet S2.1B
FOUNDATION PLAN - AREA "B"
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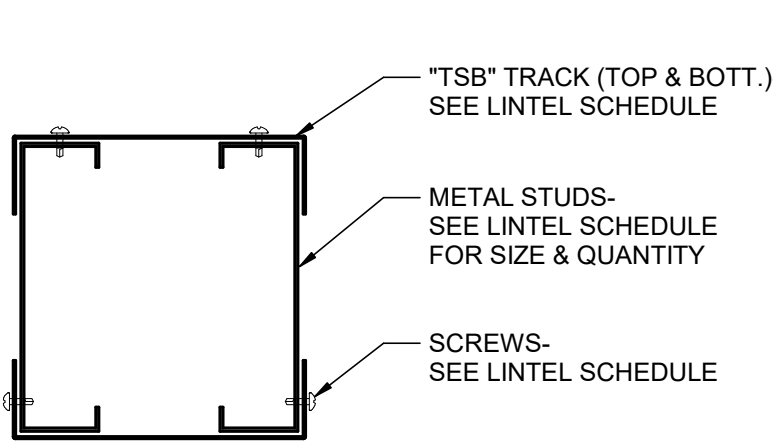
LINTEL SCHEDULE					
MARK	WALL LOCATIONS	TYPE & SIZE (THICKNESS x HEIGHT)	REINFORCEMENT OR ATTACHMENT	BRICK ANGLE OR PLATES	REMARKS
L1A	6" MTL. STD.	(2) 600S162-54 MTL. STUDS W/ 6" x 18 GA "TSB" TRACKS TOP & BOTT	(4) #8 SCREWS @ 12" O.C.	----	SEE NOTES #1, #2, #8 & DTL. 1
L1B	6" MTL. STD.	(3) 800S162-54 MTL. STUDS W/ 6" x 16 GA "TSB" TRACKS TOP & BOTT	(4) 1/8" x 1 1/2" FILLET WELDS & (2) #8 SCREWS @ 12" O.C.	----	SEE NOTES #1, #2, #8 & DTL. 2
L2A	8" CMU	CMU 8" x 8" BOND BM.	(2) #4 BOTT.	----	SEE NOTES #1, #2, #4, #5, #6, & #7
L2B	8" CMU	CMU 8" x 8" OPEN BOTT. BOND BM. ON 8" x 8" CLOSED BOTT. BOND BM. (16" TOTAL DEPTH)	(2) #5 TOP & BOTTOM	L5 x 3 1/2 x 5/16 (LLH)	SEE NOTES #1, #2, #4, #5, & #6

- LINTEL SCHEDULE NOTES:
- SEE ARCH. DWGS. FOR EXACT LOCATION OF ALL LINTELS.
 - COORDINATE ALL BOTTOM PLATE/BEAM/CMU/ANGLE ELEVATIONS WITH ARCH. DWGS.
 - BRICK LINTELS SHALL HAVE 6" MIN. BEARING EACH SIDE OF OPENINGS.
 - ALL CMU LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
 - FILL ALL CMU LINTELS WITH 2,000 PSI GROUT.
 - ALL 8" CMU LINTELS SHALL HAVE A MIN. (3) CELLS OF (2) #5 JAMB STEEL EACH SIDE OF OPENING.
 - WHERE MECHANICAL DUCTS PASS THROUGH MASONRY WALLS, PROVIDE L2A LINTEL AT 8" CMU WALLS FOR OPENINGS NOT TO EXCEED 4'-0" WIDE. FOR OPENING WIDTHS FROM 4'-0" TO 6'-0", PROVIDE L2B LINTEL AT 8" CMU WALLS. FOR OPENING WIDTHS LARGER THAN 6'-0", COORDINATE WITH ARCH./ENGINEER.
 - PROVIDE (2) KING STUDS & (2) JACK STUDS @ JAMBS, TYP. (U.N.O.)
 - SEE GENERAL NOTES FOR STUD AND TRACK SIZES.

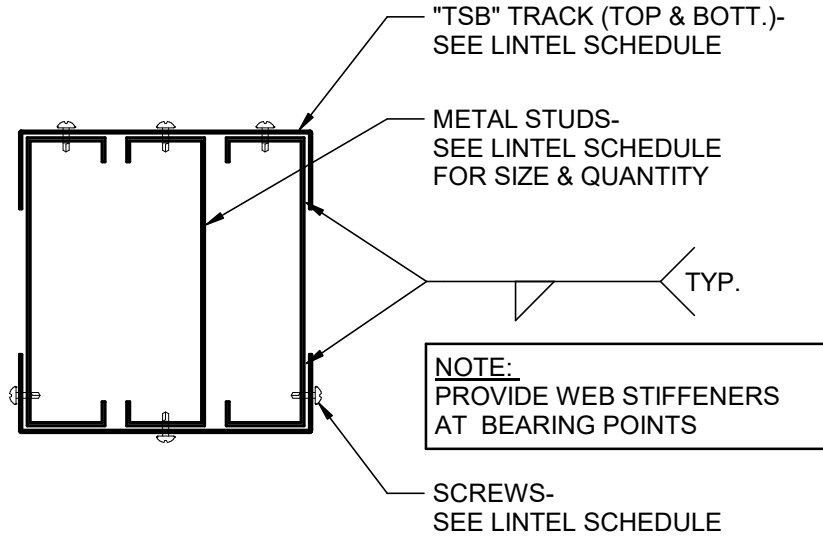
MASONRY WALL REINFORCEMENT SCHEDULE						
MARK	WALL LOCATION	BOND BEAM REINF.	BOND BEAM LOCATIONS	VERT. REINF.	FOUNDATION DOWELS	REMARKS
A	4" CMU	(1) #4 BOTT.	TOP OF WALL	----	#3 x 1'-6" DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTE #3
B	8" CMU	(1) #5 BOTT.	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#5 @ 32" O.C.	#5 DOWEL @ 32" O.C. CENTER IN WALL	SEE NOTE #2
C	8" CMU	(1) #5 BOTT.	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#5 @ 48" O.C.	#5 DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTE #2

- MASONRY WALL REINFORCEMENT SCHEDULE NOTES:
- WHERE TOP OF WALL IS UNSUPPORTED BY THE ROOF OR FLOOR FRAMING, BRACE TOP OF WALL WITH DIAGONAL L3 x 3 x 1/4 WELDED TO THE ROOF OR FLOOR FRAMING ABOVE AT 8'-0" O.C. MAXIMUM (2) MINIMUM, SEE DTL. 3A/S3.0). WHERE WALL EXTENDS TO UNDERSIDE OF FLOOR OR ROOF DECK ABOVE, HOLD WALL DOWN 1/2" & SANDWICH TOP OF WALL WITH (2) L2 x 2 x 1/8 x 0'- 6" LONG EACH ATTACHED TO DECK WITH (2) #8 SELF-TAPPING SCREWS (SPACE ANGLES AT 4'-0" O.C.), THIS BRACING IS PERMANENT BRACING REQUIRED FOR SEISMIC LOADS.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 8" INTO SLAB.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 6" INTO SLAB.

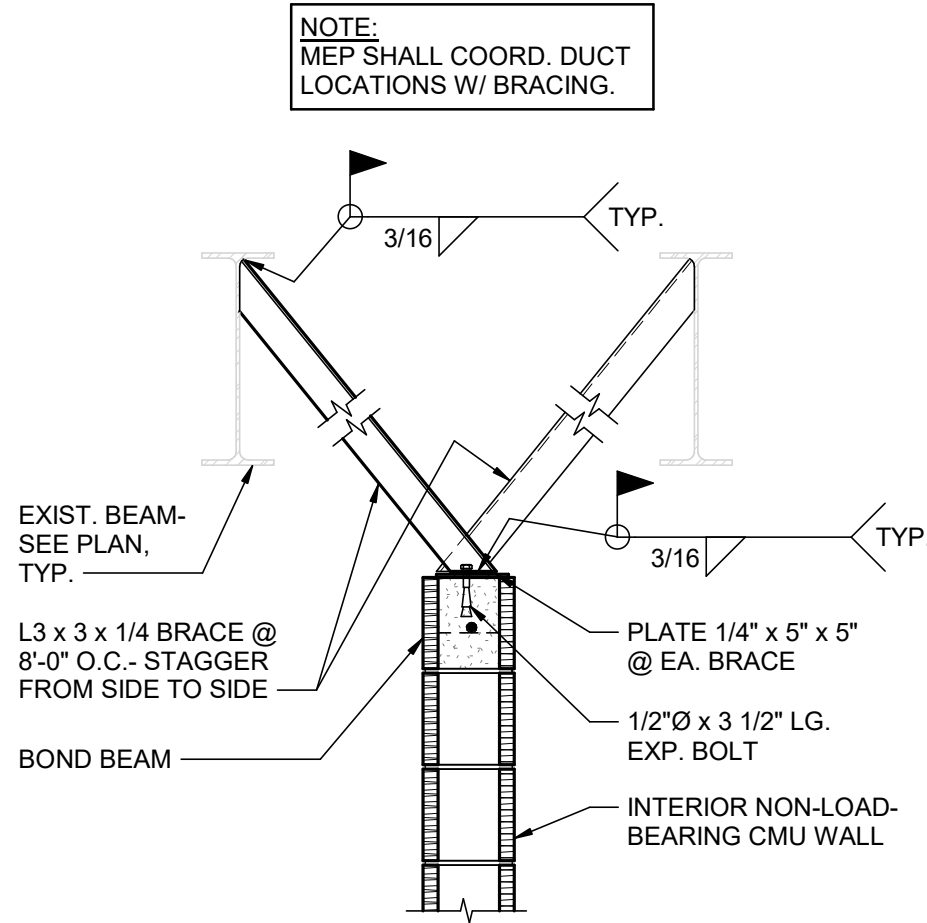
BEARING PLATE SCHEDULE					
LINTEL/BEAM SIZE	LINTEL BRG. PL SIZE (t x B x D)	BEAM BRG. PL SIZE (t x B x D)	HEADED STUD ANCHORS	SEE DTL	REMARKS
ALL BEAM SIZES	3/8" x 7" x 11"	3/8" x 7" x 11"	(2) 1/2"Ø x 6" H.S.A.	4a/S3.0	8" CMU WALL
ALL BEAM SIZES	3/8" x 11" x 11"	3/8" x 11" x 11"	(4) 1/2"Ø x 6" H.S.A.	4a/S3.0	12" CMU WALL
ALL BEAM SIZES	3/8" x 7" x 11"	3/8" x 7" x 11"	(2) 1/2"Ø x 6" H.S.A.	4b/S3.0	8" CMU WALL
ALL BEAM SIZES	3/8" x 11" x 11"	3/8" x 11" x 11"	(4) 1/2"Ø x 6" H.S.A.	4b/S3.0	12" CMU WALL
ALL BEAM SIZES	3/8" x 8" x 11"	3/8" x 7" x 11"	(4) 1/2"Ø x 6" H.S.A.	4c/S3.0	12" CMU WALL



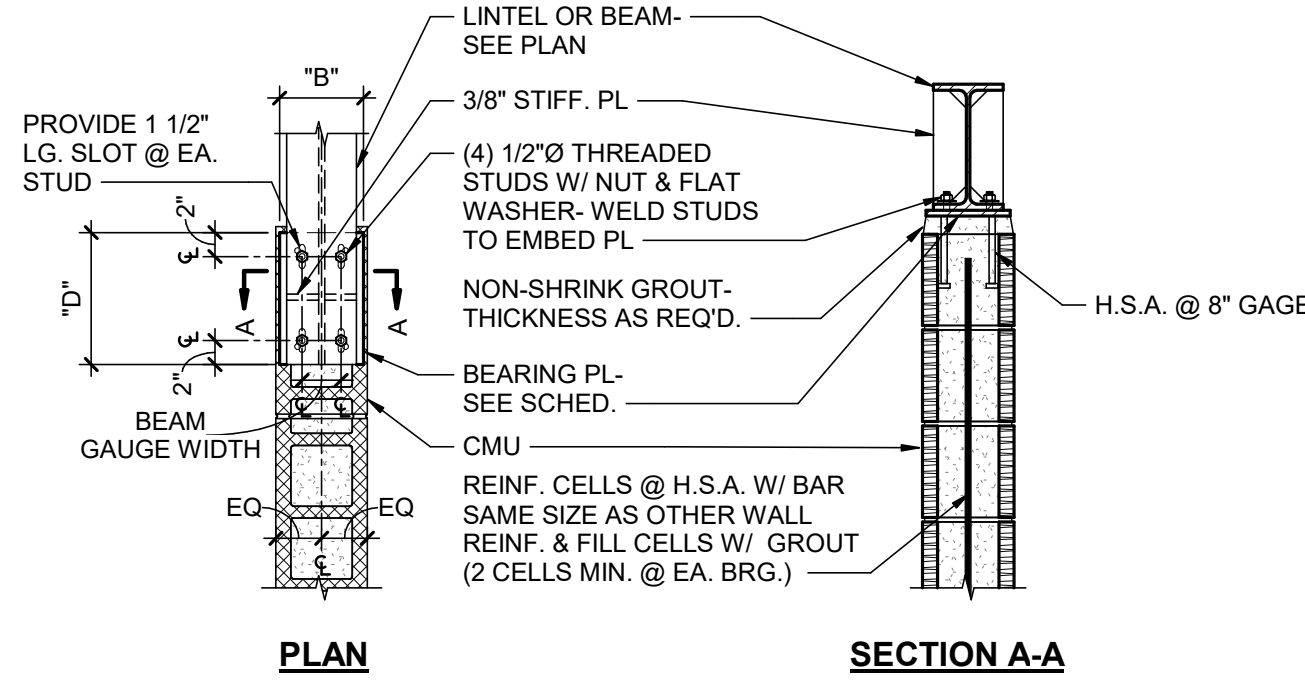
1 STUD LINTEL DTL.
NOT TO SCALE



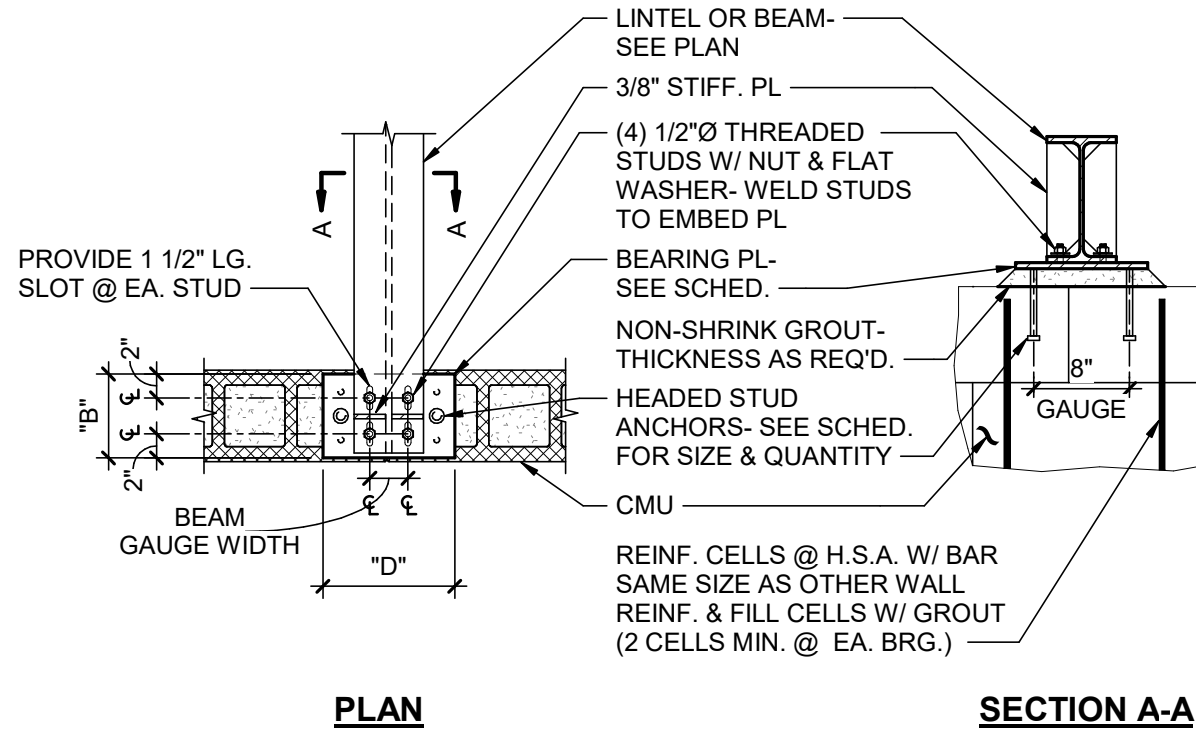
2 STUD LINTEL DTL.
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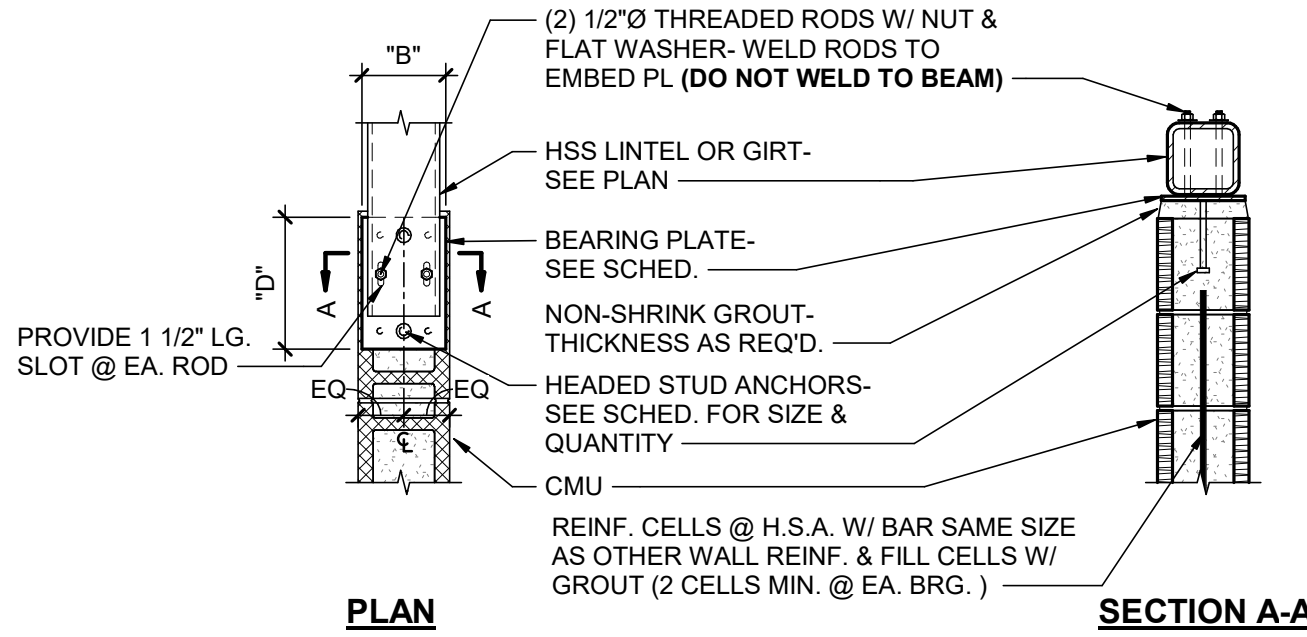
3 TYP. CMU WALL BRACING
NOT TO SCALE



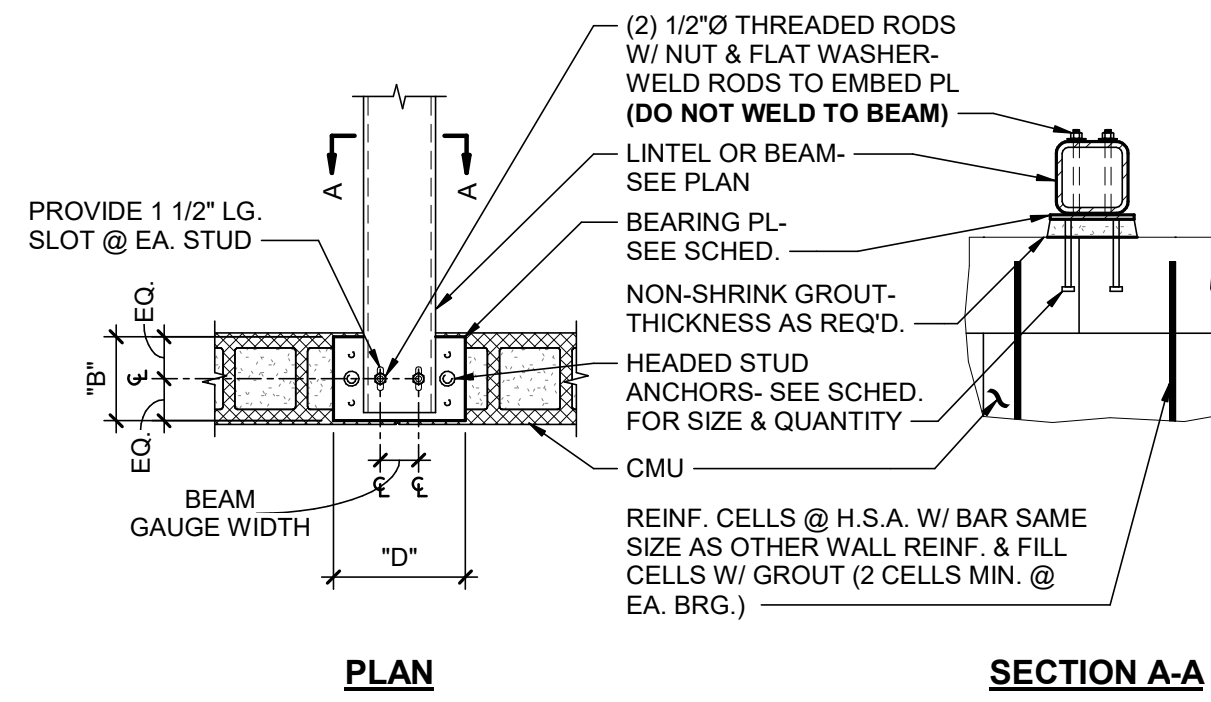
4 TYP. WF LINTEL & BEAM BEARING PLATE DETAILS
NOT TO SCALE



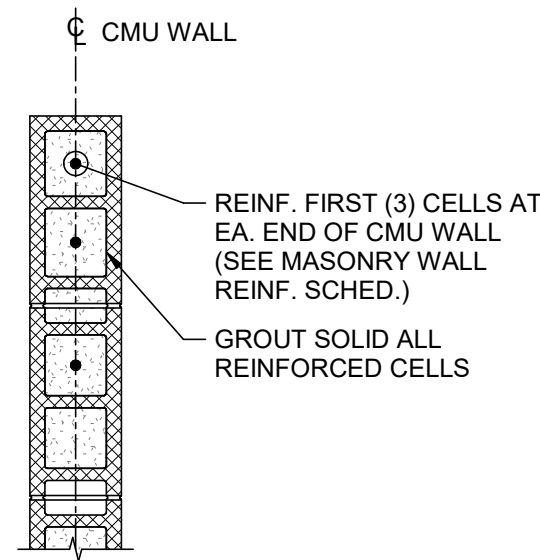
5 TYP. WF LINTEL & BEAM BEARING PLATE DETAILS
NOT TO SCALE



6 TYP. HSS LINTEL & BEAM BEARING PLATE DETAILS
NOT TO SCALE



7 TYP. HSS LINTEL & BEAM BEARING PLATE DETAILS
NOT TO SCALE



8 TYP. CMU REINF. LAYOUT - CENTERED
NOT TO SCALE

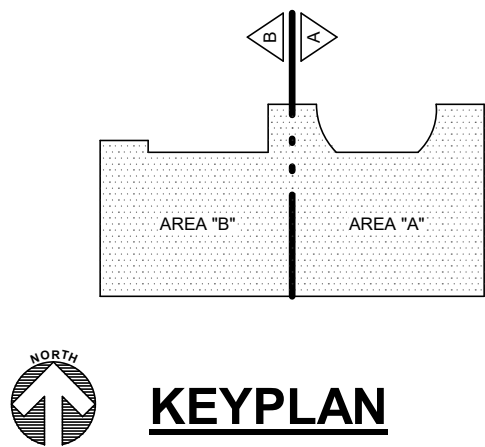
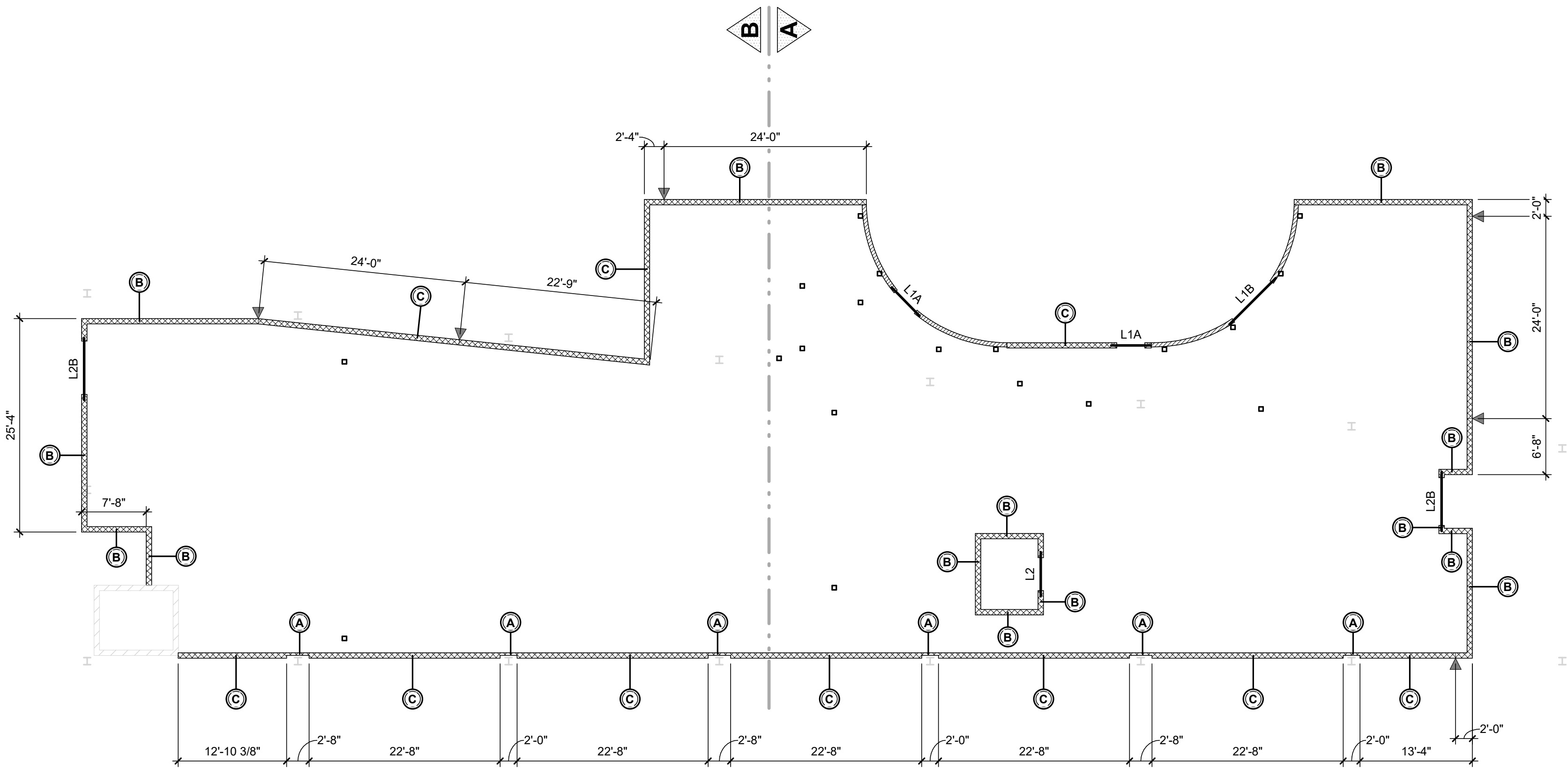


1 FIRST LEVEL LINTEL & MASONRY PLAN

3/32" = 1'-0"

- LEGEND:**
- L1
DENOTES LINTEL MARK, SEE LINTEL SCHED.
 - DENOTES LOAD-BEARING CMU WALL
 - DENOTES NON-LOAD-BEARING CMU WALL
 - (A)
DENOTES MASONRY WALL REINF. MARK, SEE MASONRY WALL REINF. SCHED.
 - ▼
DENOTES WALL CONTROL JOINT

- PLAN NOTES:**
- SEE DWGS. S1.1, S1.2, S1.3 & S1.4 FOR GENERAL NOTES & TYP. DETAILS.



KEYPLAN



sheet S3.0.1
FIRST LEVEL LINTEL & MASONRY PLAN AREAS
13 April 2026



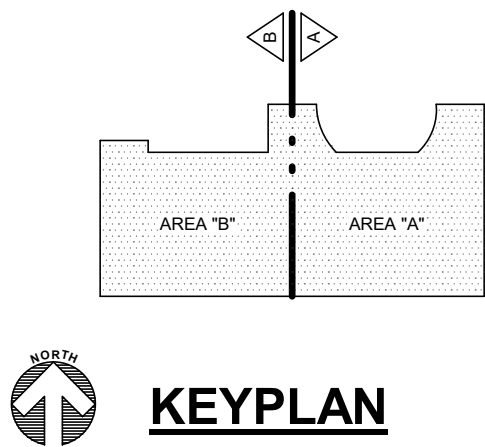
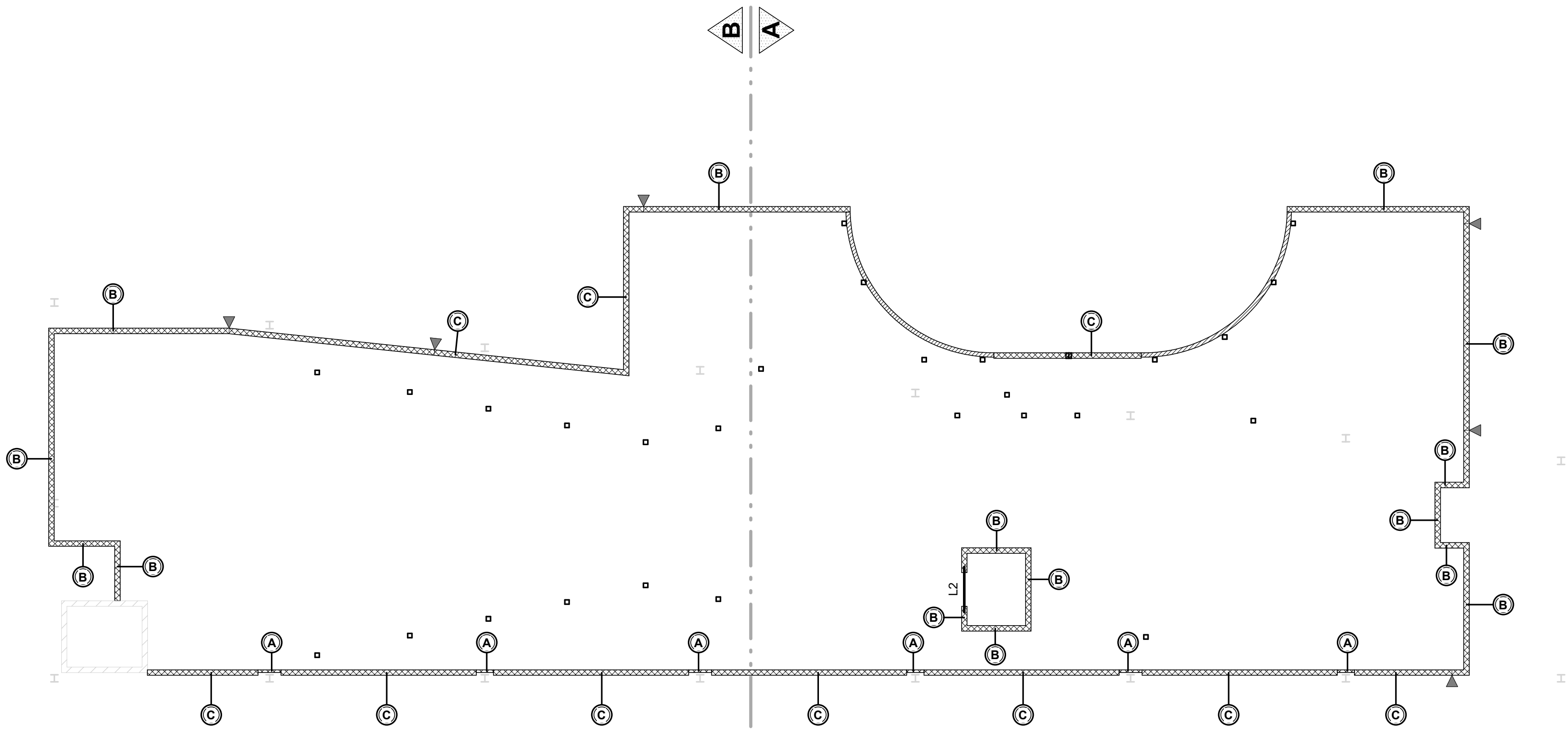
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1 SECOND LEVEL LINTEL & MASONRY PLAN
3/32" = 1'-0"

NOTE:
SEE DRAWING S3.0.1 FOR
LEGEND AND PLAN NOTES.

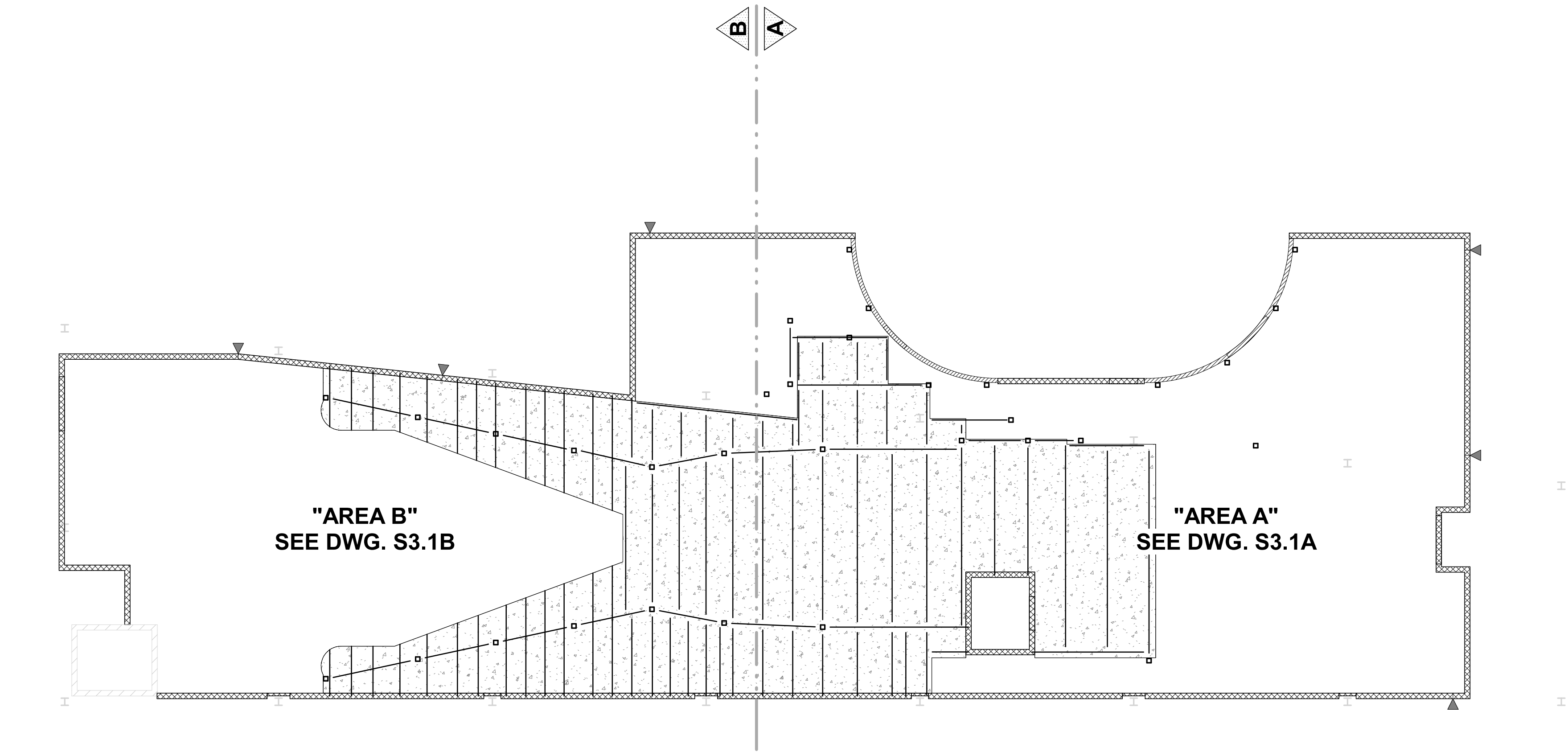


sheet S3.0.2
SECOND LEVEL LINTEL & MASONRY PLAN AREAS
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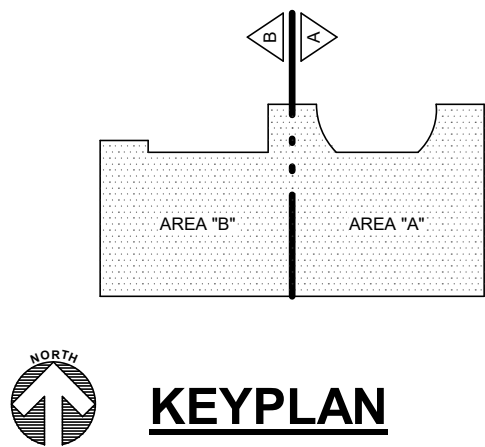
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MEZZANINE LEVEL FRAMING PLAN - OVERALL

1" = 10'-0"

PLAN NOTES:

1. THIS DRAWING IS FOR REFERENCE ONLY.
2. SEE REFERENCED AREAS FOR ENLARGED PLANS.



KEYPLAN



sheet S3.1

MEZZANINE LEVEL FRAMING PLAN - OVERALL

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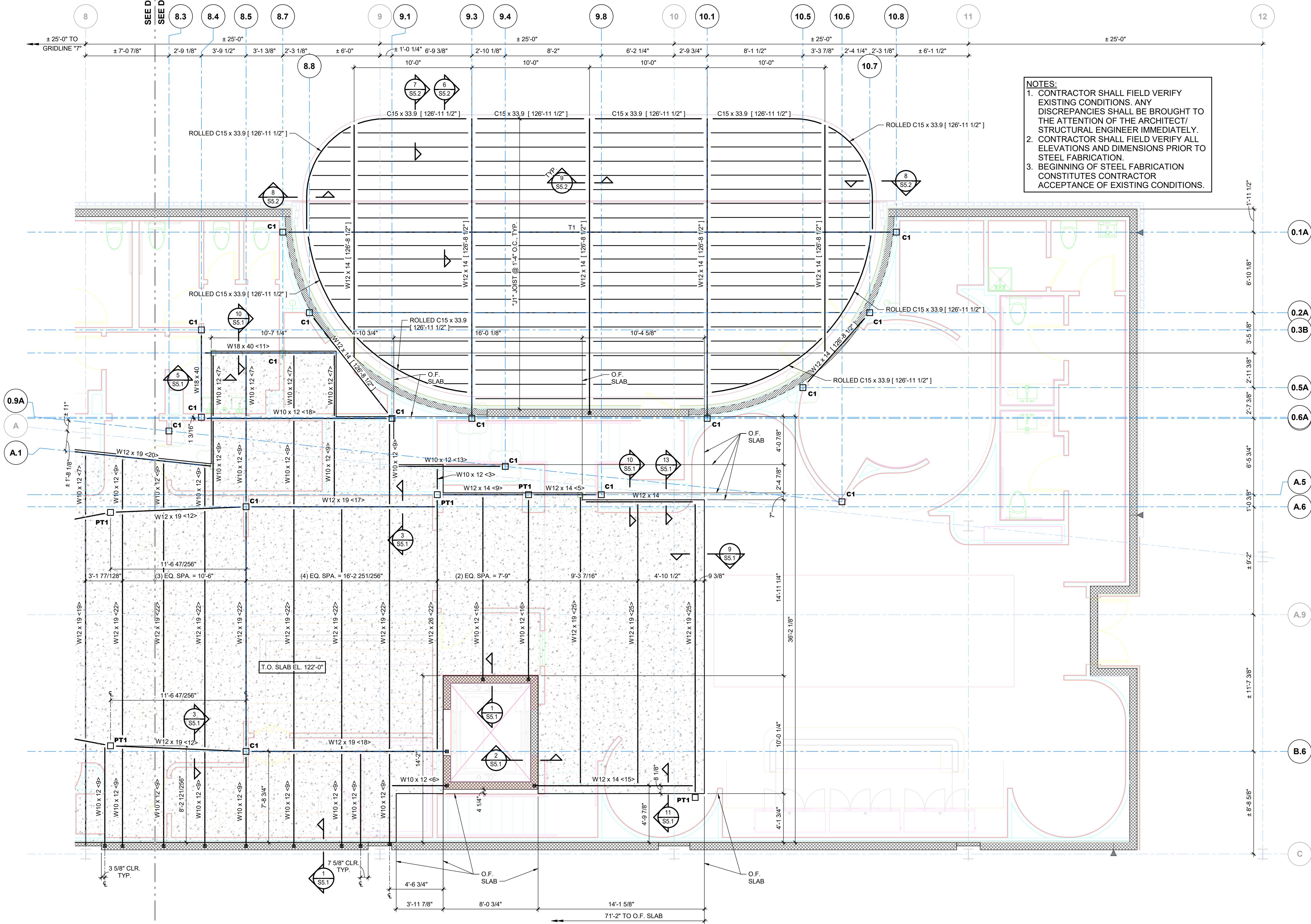
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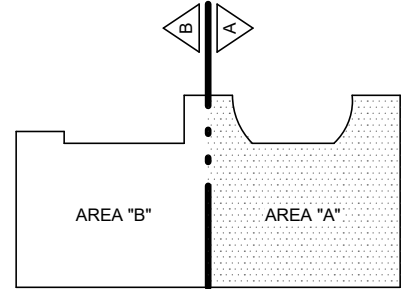
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1/4" = 1'-0"

MEZZANINE LEVEL FRAMING PLAN - AREA "A"



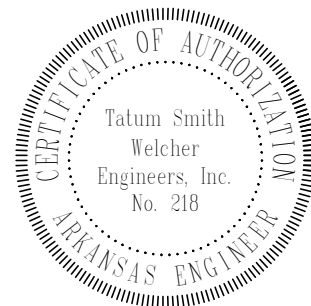
NOTES:
1. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ STRUCTURAL ENGINEER IMMEDIATELY.
2. CONTRACTOR SHALL FIELD VERIFY ALL ELEVATIONS AND DIMENSIONS PRIOR TO STEEL FABRICATION.
3. BEGINNING OF STEEL FABRICATION CONSTITUTES CONTRACTOR ACCEPTANCE OF EXISTING CONDITIONS.

COLUMN SCHEDULE						
MARK	COLUMN SIZE	BASE PL SIZE (t x B x D)	BASE PL TYPE (SEE DTL. 1/S1.4)	ANCHOR RODS (SEE DTL. 2/S1.4)	BOTT. BASE PL EL.	CAP PL THICKNESS
C1	HSS6 x 6 x 1/4	3/4" x 12" x 12"	TYPE I	(4) 3/4" F1554	110'-3 1/2"	1/4"



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TSW #: 26021 PM: MEW DE: AVK



- PLAN NOTES:**
- TYP. ELEVATED SLAB CONSTRUCTION: 5" NORMAL WEIGHT CONCRETE SLAB REINFORCED W/ #4 @ 12" O.C. E.W. ON 1 1/2" DEEP, 22 GAUGE, GALVANIZED COMPOSITE DECK. PLACE REINFORCING 1 1/2" CLEAR FROM TOP OF SLAB. (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS.)
 - TOP OF ELEVATED SLAB EL. SEE PLAN
 - TOP OF STL. EL. 121'-7". (TYP. U.N.O.)
 - ALL ELEVATIONS BASED ON FINISH FLOOR EL. 111'-6". ACTUAL FIN. FLR. EL. 1284.00' AS INDICATED IN THE ORIGINAL CONSTRUCTION DRAWINGS DATED 05/06/2019.
 - SEE DWGS. S1.2, S1.3, & S1.4 FOR GENERAL NOTES & TYPICAL DETAILS.
 - SEE THIS DWG. FOR COLUMN SCHEDULE.
 - SEE ARCH. DWGS. & FOUNDATION PLAN FOR DIMENSIONS NOT SHOWN.
 - SEE DWGS. S3.0 & S3.01 FOR LINTEL PLANS & DETAILS.
 - CONTRACTOR SHALL SAWCUT 1/8" x 1 1/4" DEEP JOINTS. CONTRACTOR SHALL SUBMIT JOINT PLAN TO ARCHITECT FOR APPROVAL PRIOR TO THE ELEVATED SLAB CONSTRUCTION.

NOTE:
SEAL ALL VOIDS AT FLOOR PENETRATIONS. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.

NOTE:
1. CONTROL JOINTS IN ELEVATED SLABS SHALL BE LOCATED AT ALL GIRDER LOCATIONS AND AT EVERY THIRD FLOOR JOIST WITH A MAXIMUM SPACING OF 12'-0" O.C. BETWEEN CONTROL JOINTS.

- LEGEND:**
- C1 DENOTES COLUMN MARK (SEE COL. SCHED. ON DWG. S1.4)
 - PT1 DENOTES HSS6 x 6 x 1/4 POST
 - O.F. DENOTES OUTSIDE FACE
 - TOS DENOTES TOP OF STEEL
 - JBE DENOTES JOIST BEARING ELEVATION
 - J1 DENOTES 1200S162-54 METAL STUD JOIST
 - J2 DENOTES HSS10 x 5 x 5/16 (LDV) JOIST
 - J3 DENOTES HSS14 x 6 x 3/8 (LDV) JOIST
 - <45> DENOTES NUMBER OF H.S.A.'s WELDED TO BEAM TOP FLANGE (SEE COMPOSITE BEAM GENERAL NOTE #1)
 - [118'-8"] DENOTES TOP OF BEAM EL.
 - H.P. DENOTES HIGH POINT
 - L.P. DENOTES LOW POINT
 - 114'-0" DENOTES TOP OF SLAB EL.
 - [Symbol] DENOTES BEAM BEARING PLATE (SEE BEARING PL. SCHED.)
 - [Symbol] DENOTES BEAM SPLICE (SEE DTL. 9/S1.3)
 - [Symbol] DENOTES LOAD-BEARING CMU WALL
 - [Symbol] DENOTES NON-LOAD-BEARING CMU WALL
 - [Symbol] DENOTES LIMITS OF 5" CONC. SLAB & DECK (SEE PLAN NOTES)
 - [Symbol] DENOTES LIMITS OF FRAMED OPENING IN FLOOR- COORDINATE SIZE & LOCATION W/ MECH. (SEE DTL. 2/S1.2)
 - F.O. [Symbol]
 - [Symbol] DENOTES CMU WALL CONTROL JOINT

STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
MICHAEL E. WHITEN
No. 15426
07/02/2026

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sheet S3.1A
MEZZANINE LEVEL FRAMING PLAN - AREA "A"
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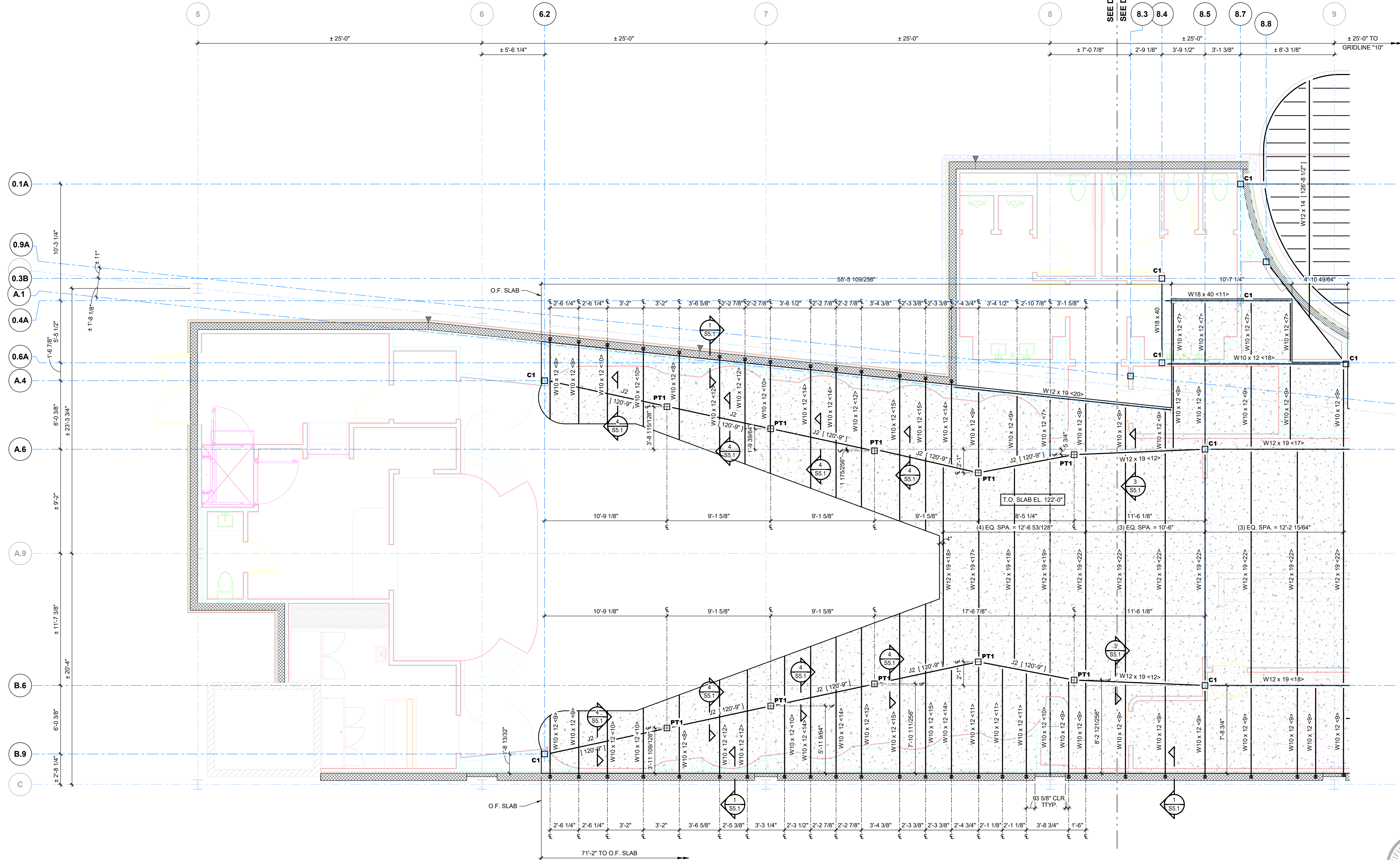


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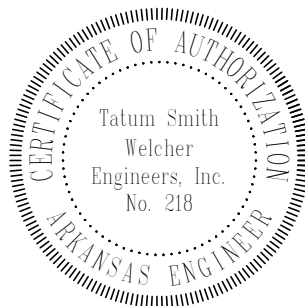
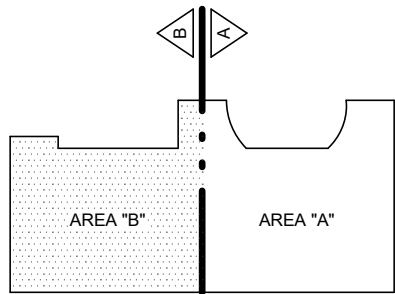
MEZZANINE LEVEL FRAMING PLAN - AREA "B"

1/4" = 1'-0"

NOTE:
SEE DRAWING S3.1A FOR
LEGEND AND PLAN NOTES



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sheet S3.1B

MEZZANINE LEVEL FRAMING PLAN - AREA "B"

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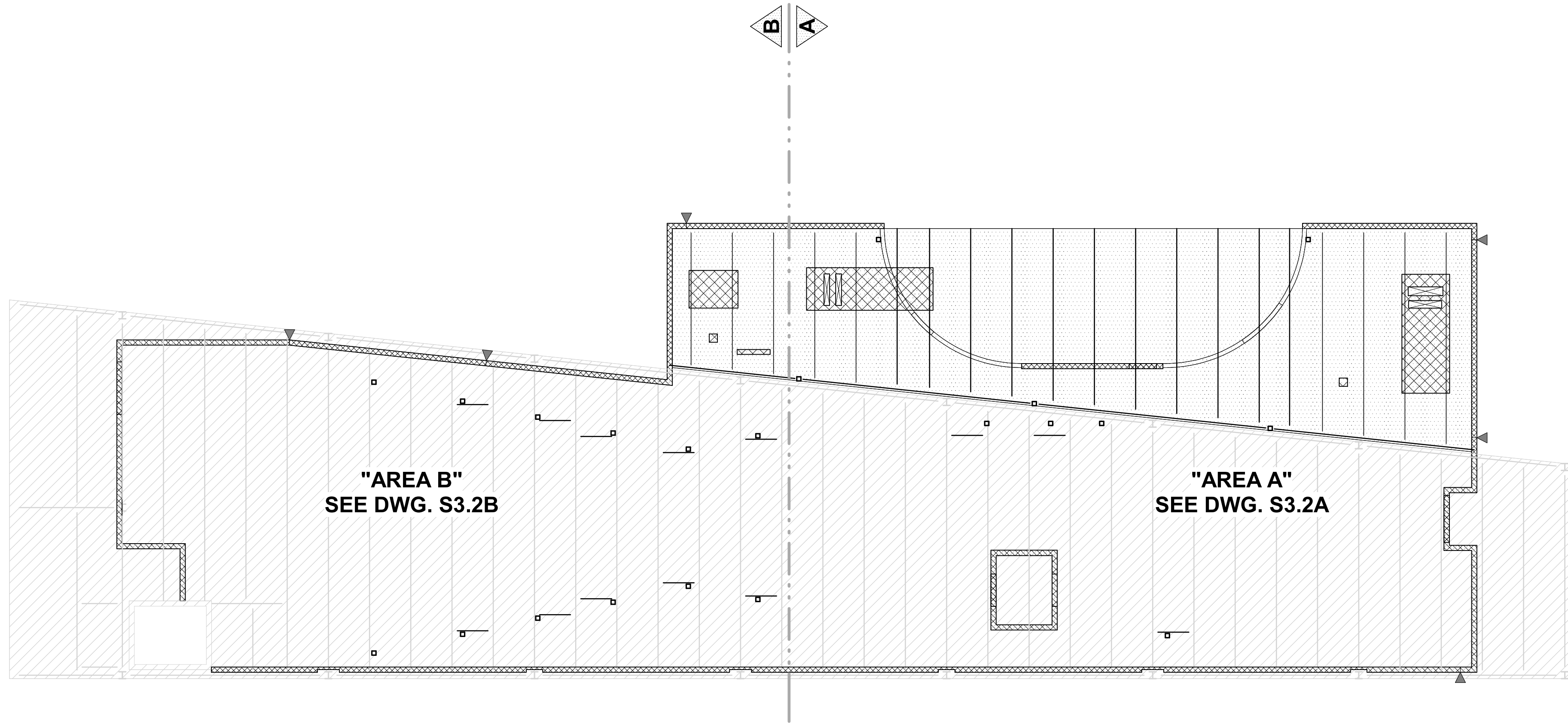
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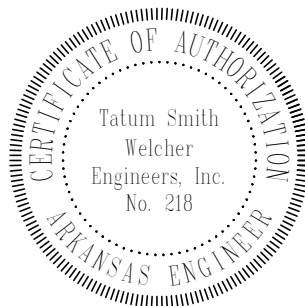
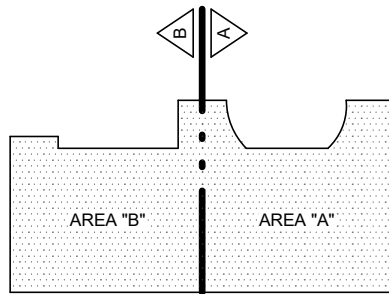
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SECOND LEVEL FRAMING PLAN - OVERALL

1" = 10'-0"



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sheet S3.2

SECOND LEVEL FRAMING PLAN - OVERALL

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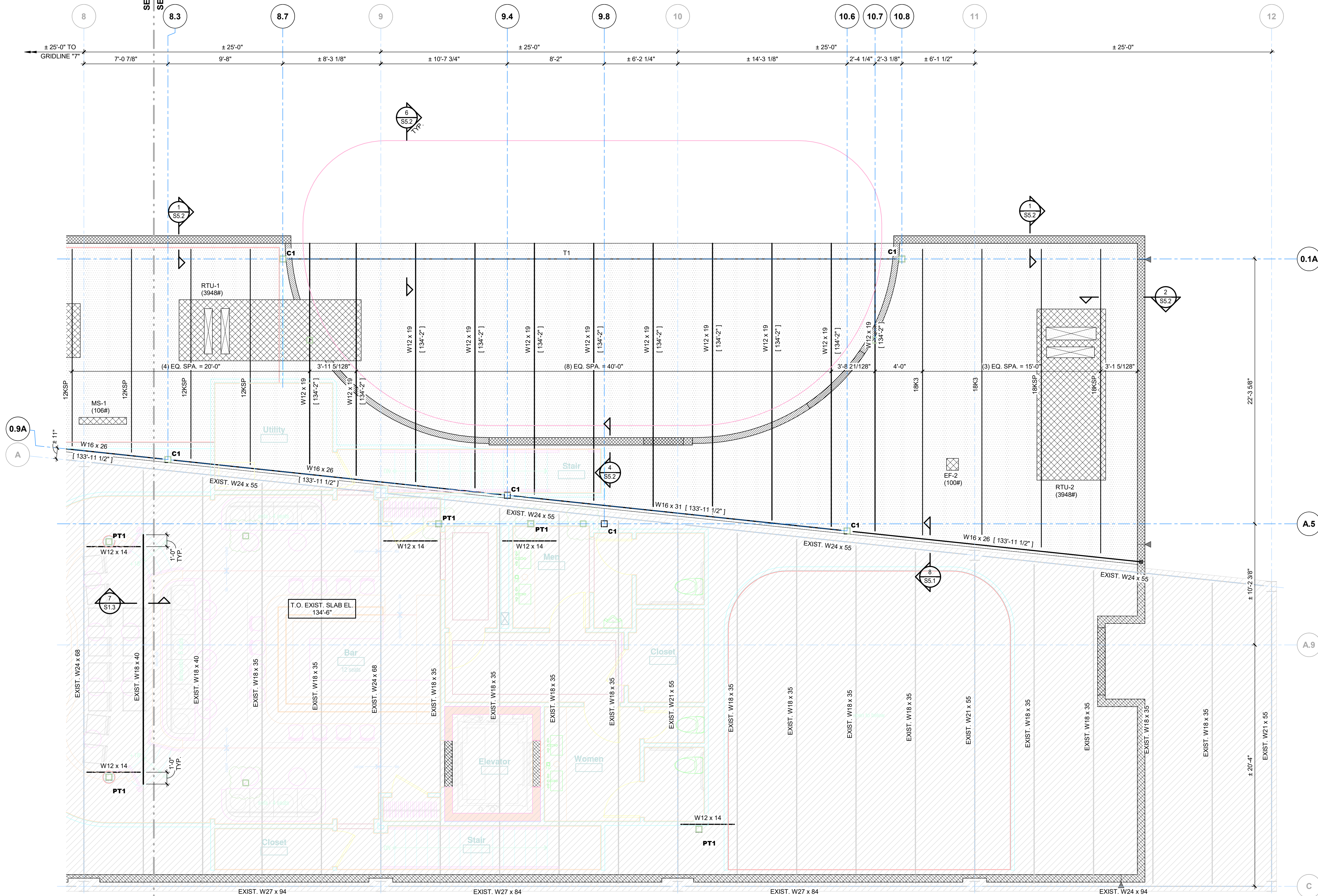
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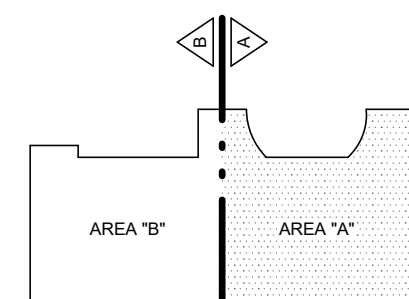
1 SECOND LEVEL FRAMING PLAN - AREA "A"

1/4" = 1'-0"

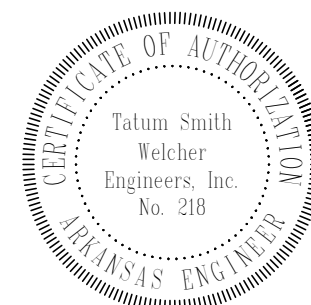


- LEGEND:**
- DENOTES COLUMN MARK (SEE COL. SCHED. ON DWG. S1.4)
 - O.F. DENOTES OUTSIDE FACE
 - JBE DENOTES JOIST BEARING ELEVATION
 - TOS DENOTES TOP OF STEEL
 - [128'-9"] DENOTES TOP OF BEAM ELEVATION
 - DENOTES LIMITS OF 1 1/2" x 22 GA. ROOF DECK (SEE PLAN NOTES)
 - SLOPE DENOTES DIRECTION OF ROOF SLOPE (COORD. PITCH W/ ARCH.)
 - (2,500#) DENOTES EQUIPMENT WEIGHT IN POUNDS
 - DENOTES BEARING PLATE (SEE BEARING PL. SCHED.)
 - DENOTES LOAD-BEARING CMU WALL
 - (0.5k) DENOTES POINT LOAD IN KIPS
 - DENOTES CMU WALL CONTROL JOINT
 - DENOTES SUSPENDED EQUIPMENT-COORDINATE SIZE & LOCATION W/ MECH. CONTRACTOR
 - DENOTES FRAMED OPENING IN ROOF-COORDINATE SIZE & LOCATION W/ MECH. CONTRACTOR (SEE DTL. 4 / S1.2)

- PLAN NOTES:**
- ROOF DECK: 1 1/2" DEEP, 22 GA. PAINTED WIDE RIB STEEL DECK (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS).
 - TOP OF STEEL DENOTES TOP OF MAIN STEEL.
 - TOP OF STEEL EL. VARIES. SEE PLAN.
 - TOP OF "K" SERIES JOISTS (+2 1/2") TYP., (U.N.O.)
 - JOIST BEARING ELEVATION VARIES. SEE PLAN.
 - ALL ELEVATIONS BASED ON MAIN LEVEL FINISH FLOOR EL. 111'-6". ACTUAL FIN. FLR. EL. 1284.00' AS INDICATED IN THE ORIGINAL CONSTRUCTION DRAWINGS DATED 05/08/2019.
 - SEE DWGS. S1.2, S1.3 & S1.4 FOR GENERAL NOTES AND TYPICAL DETAILS.
 - SEE DWG. S1.4 FOR COLUMN SCHEDULE.
 - SEE ARCH. DWGS., FOUNDATION, SECOND FLOOR FRAMING & OVERALL ROOF FRAMING PLANS FOR DIMENSIONS NOT SHOWN.



KEYPLAN



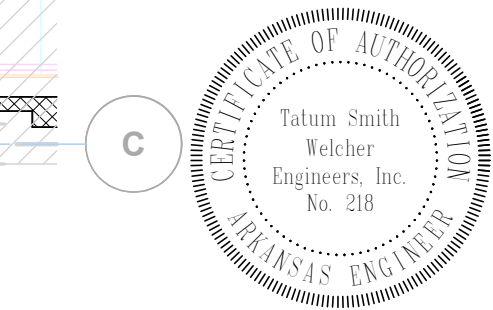
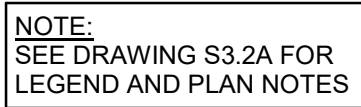
TATUM SMITH WELCH
STRUCTURAL ENGINEERS
(479) 621-6128 ROGERS, ARKANSAS
TSW #: 26021 PM: MEW DE: AVK



WAC
Lounge/Bar
5079 W Northgate Road
Rogers, AR 72758

sheet S3.2A
SECOND LEVEL FRAMING PLAN - AREA "A"
13 April 2026

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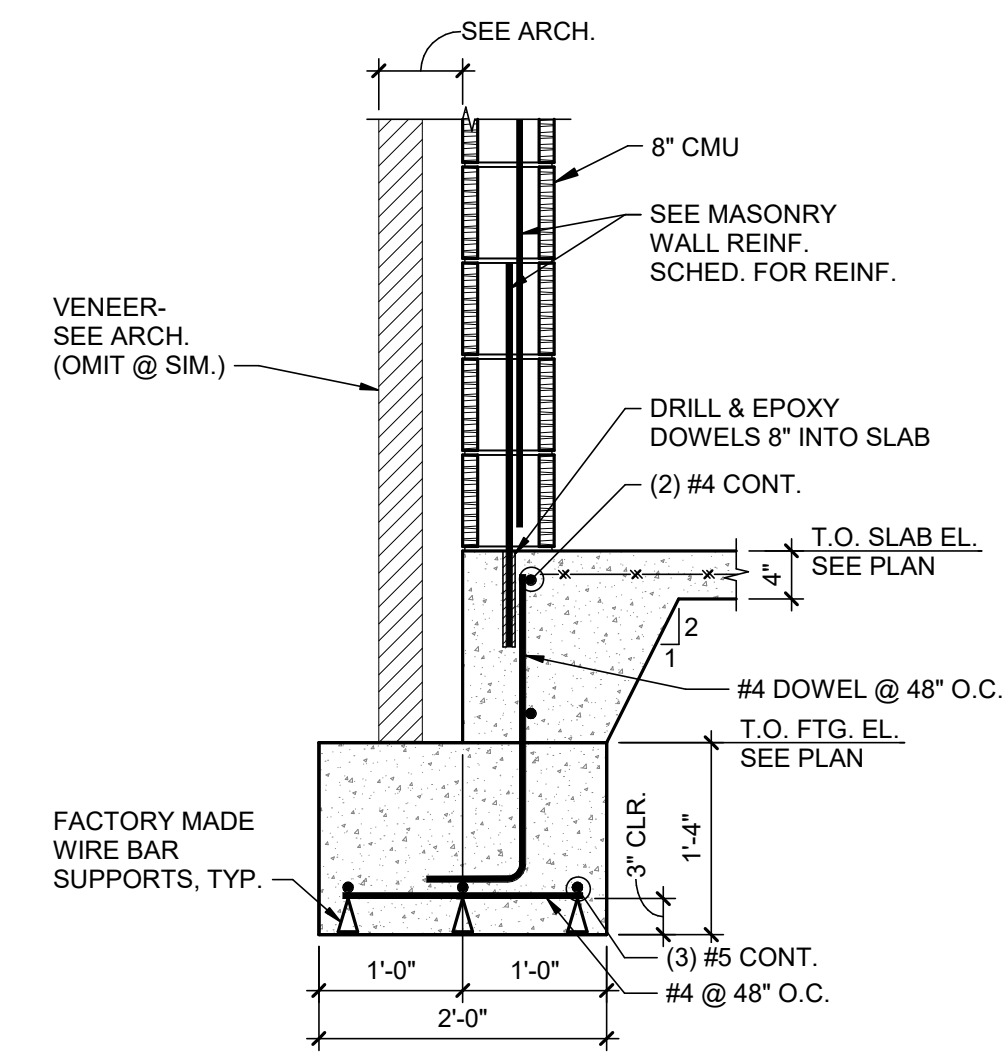
**TATUM
SMITH
WELCHER**
STRUCTURAL ENGINEERS
(479) 621-6128 ROGERS, ARKANSAS
TSW #: 26021 PM: MEW DE: AVH

SECOND LEVEL FRAMING PLAN - AREA "B"
13 April 2026

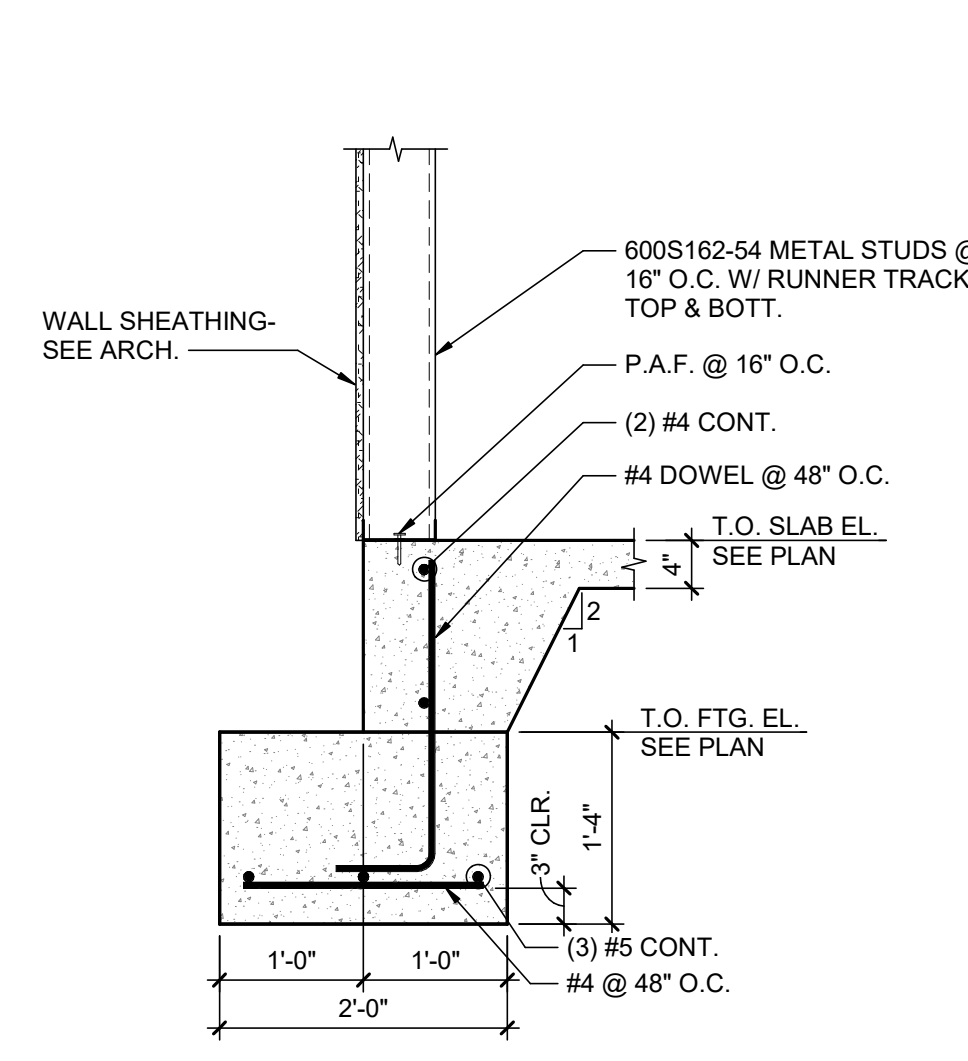
WAC
Lounge/Bar
5079 W Northgate Road
Rogers, AR 72758



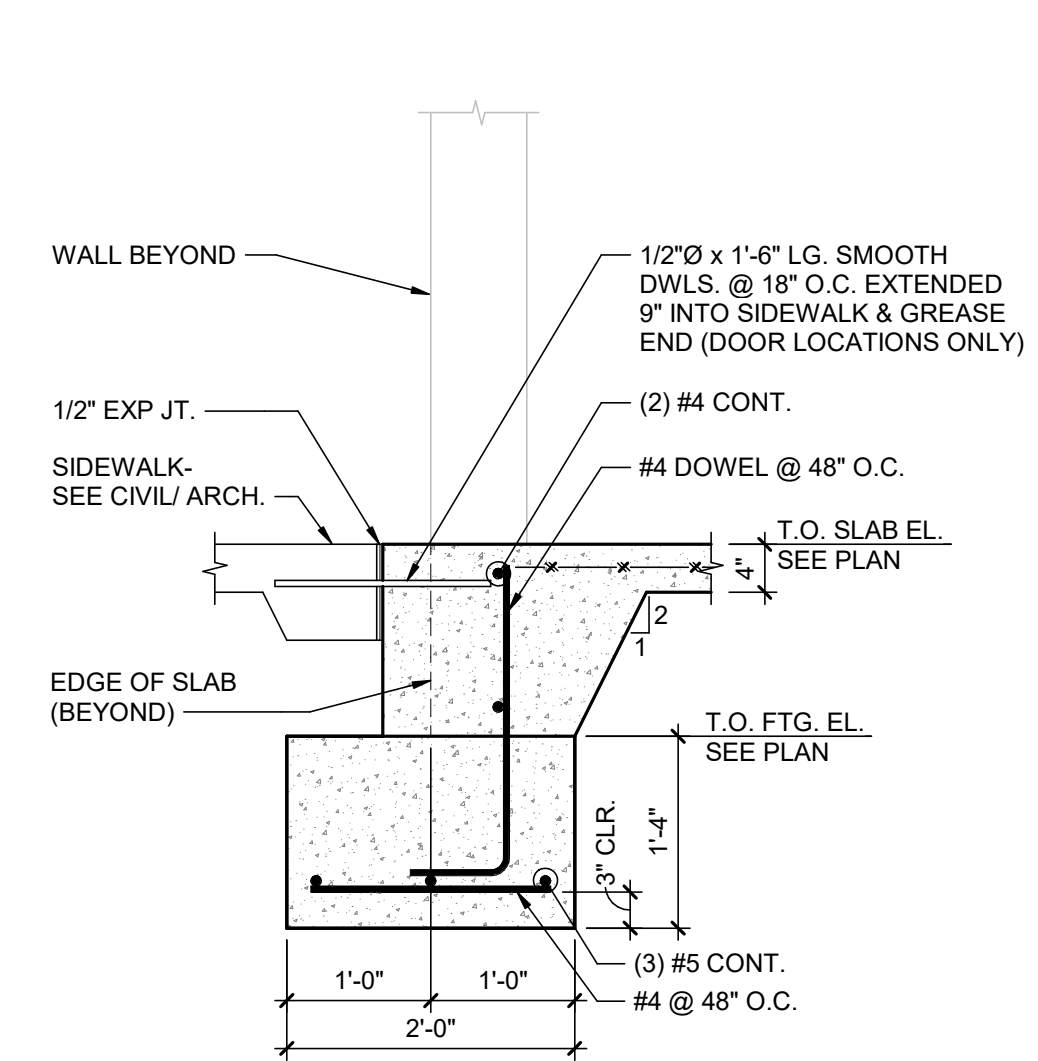
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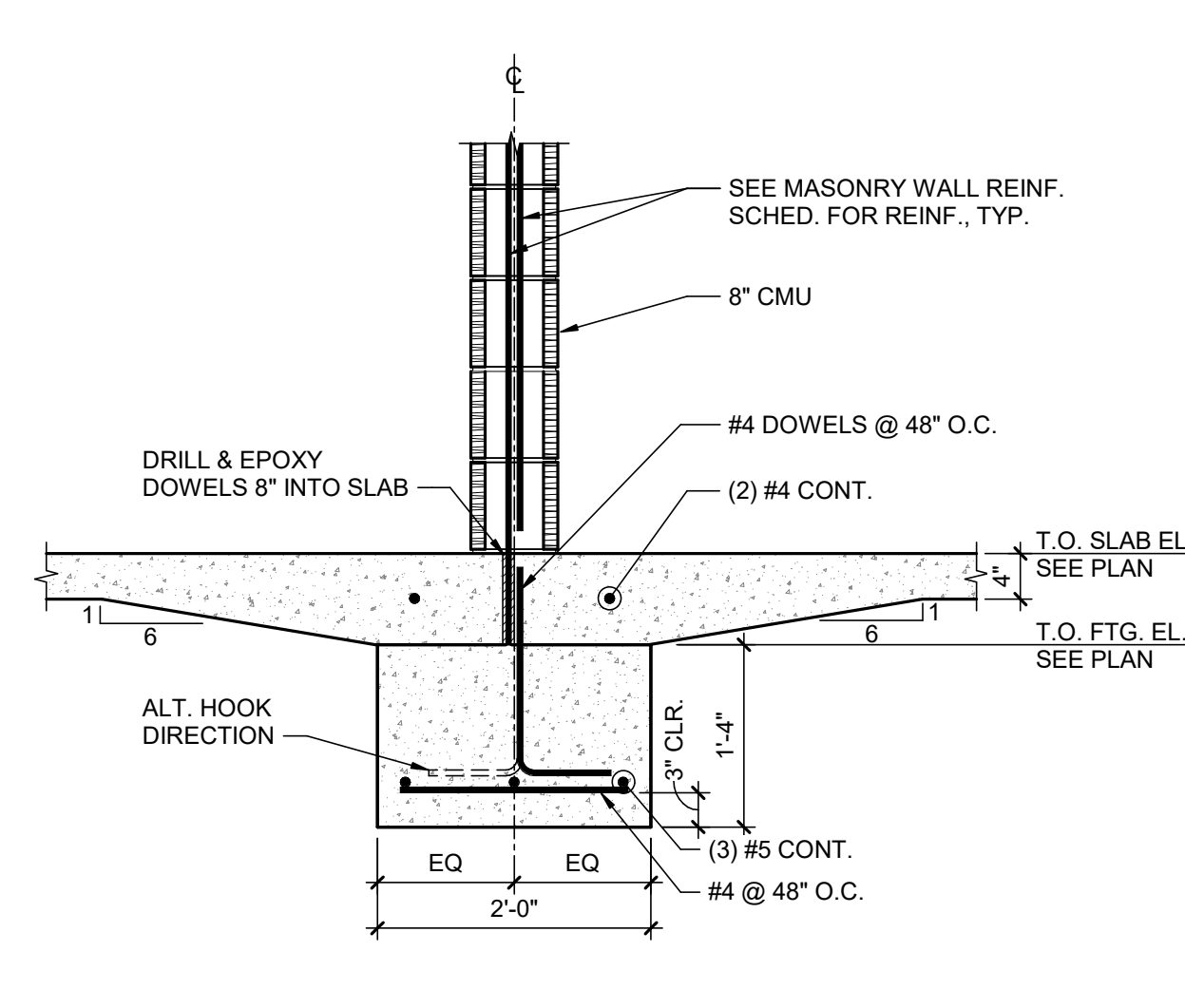
1 PARTIAL ELEVATION
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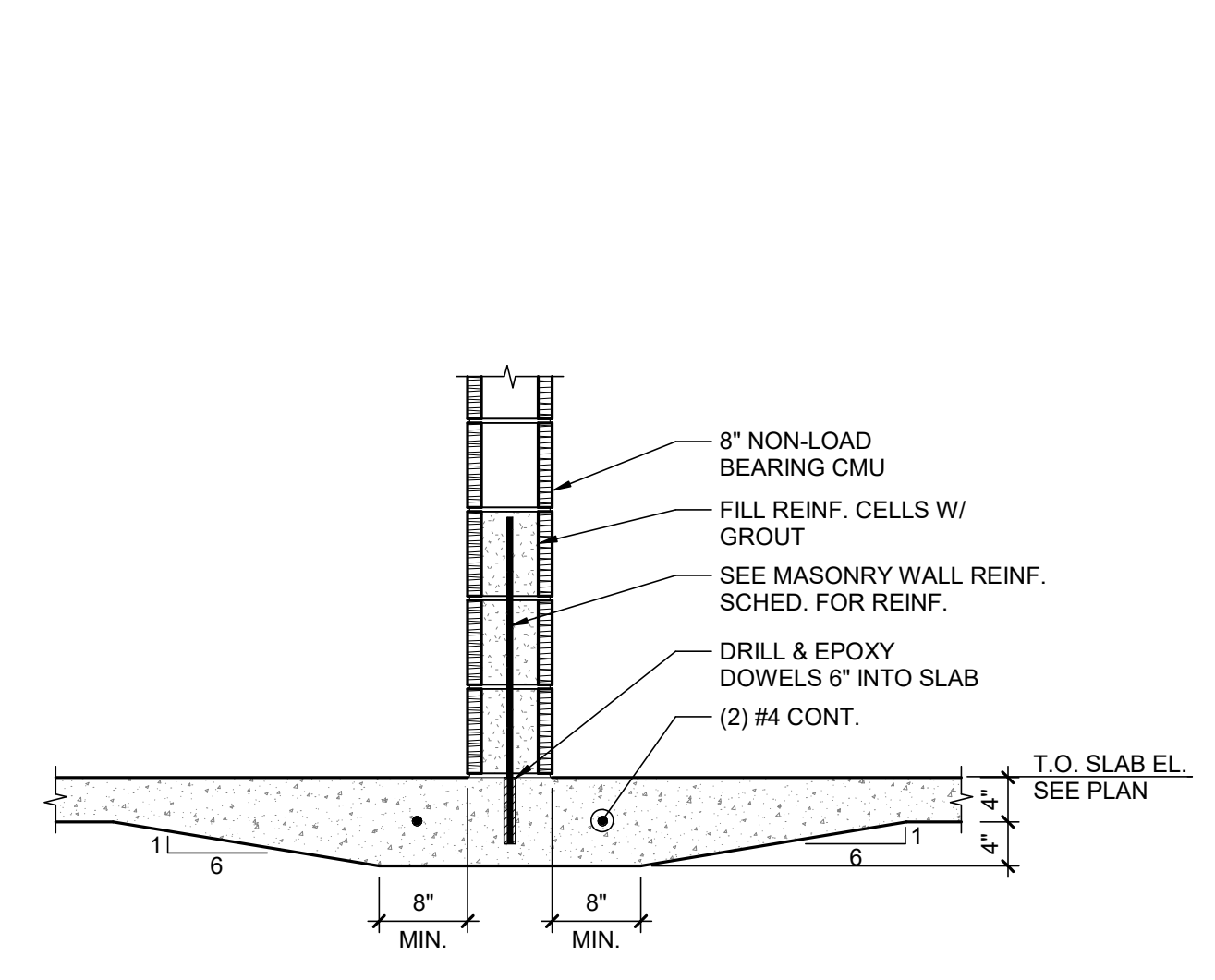
2 SECTION
3/4" = 1'-0"



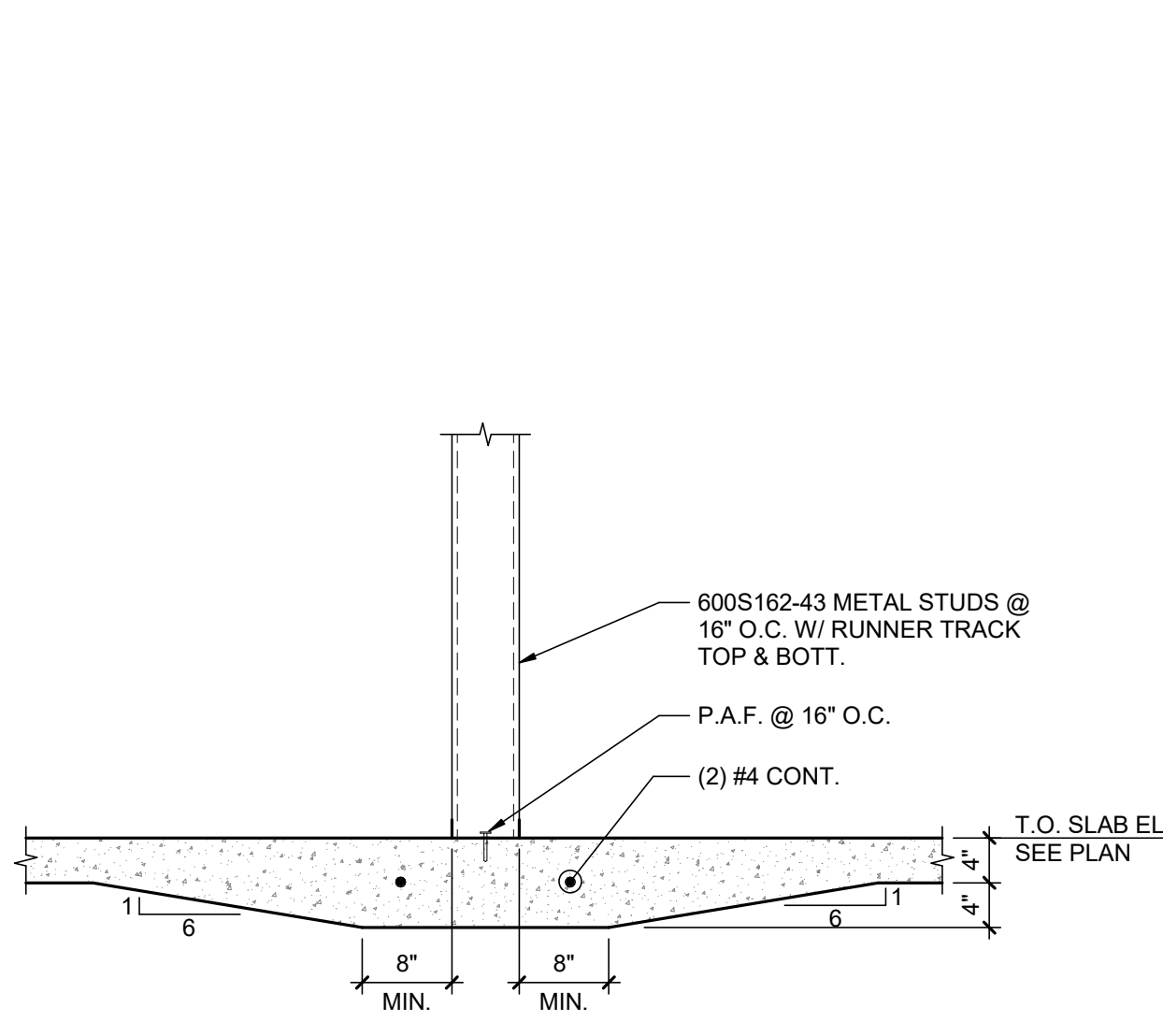
3 SECTION
3/4" = 1'-0"



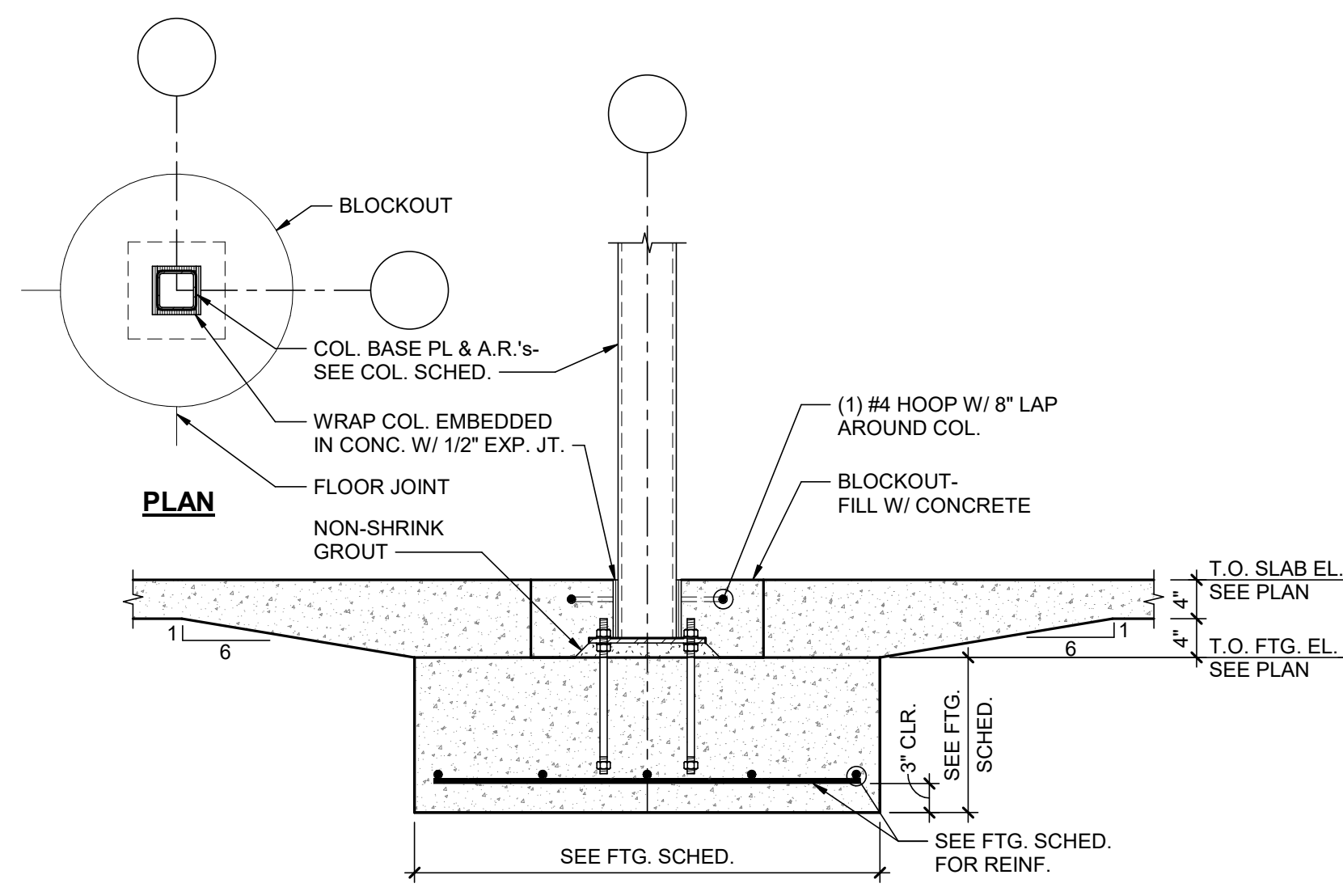
4 SECTION
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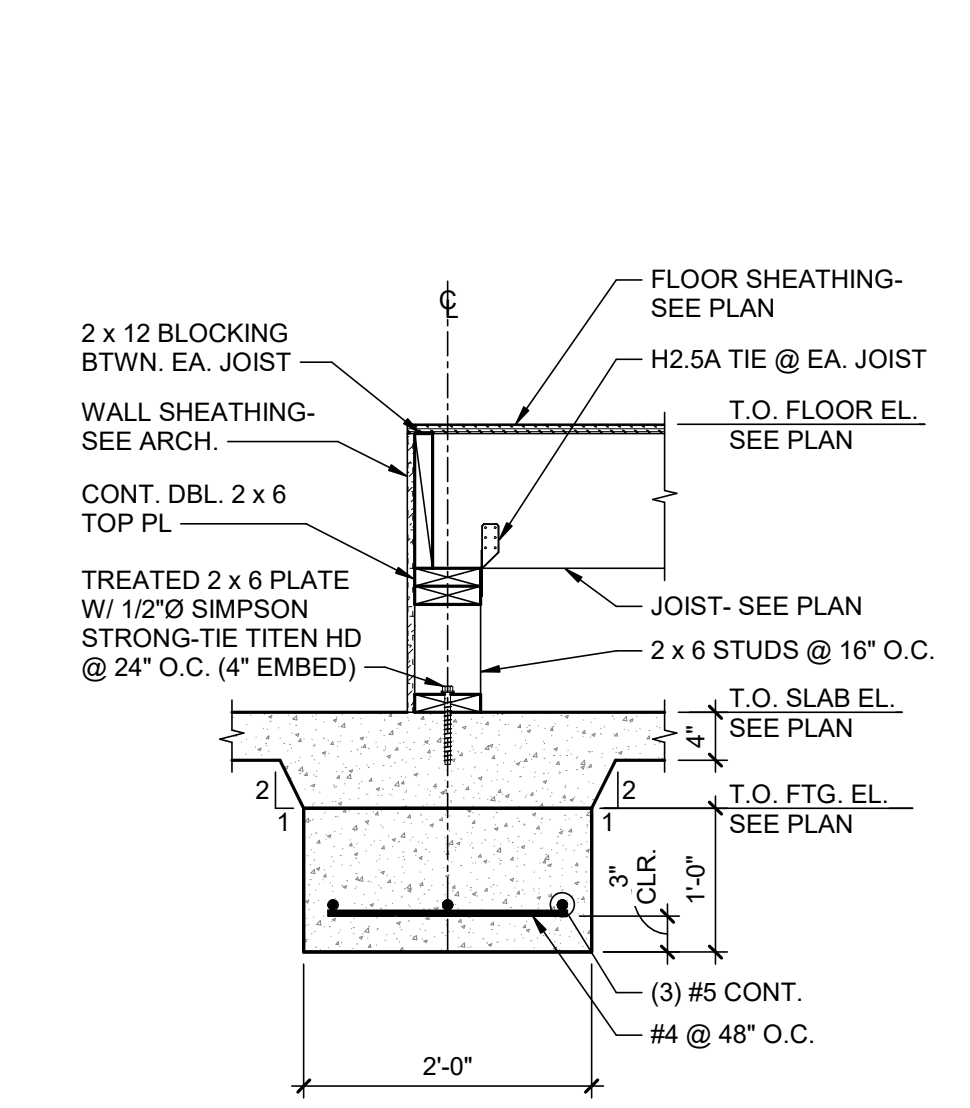
5 SECTION
3/4" = 1'-0"



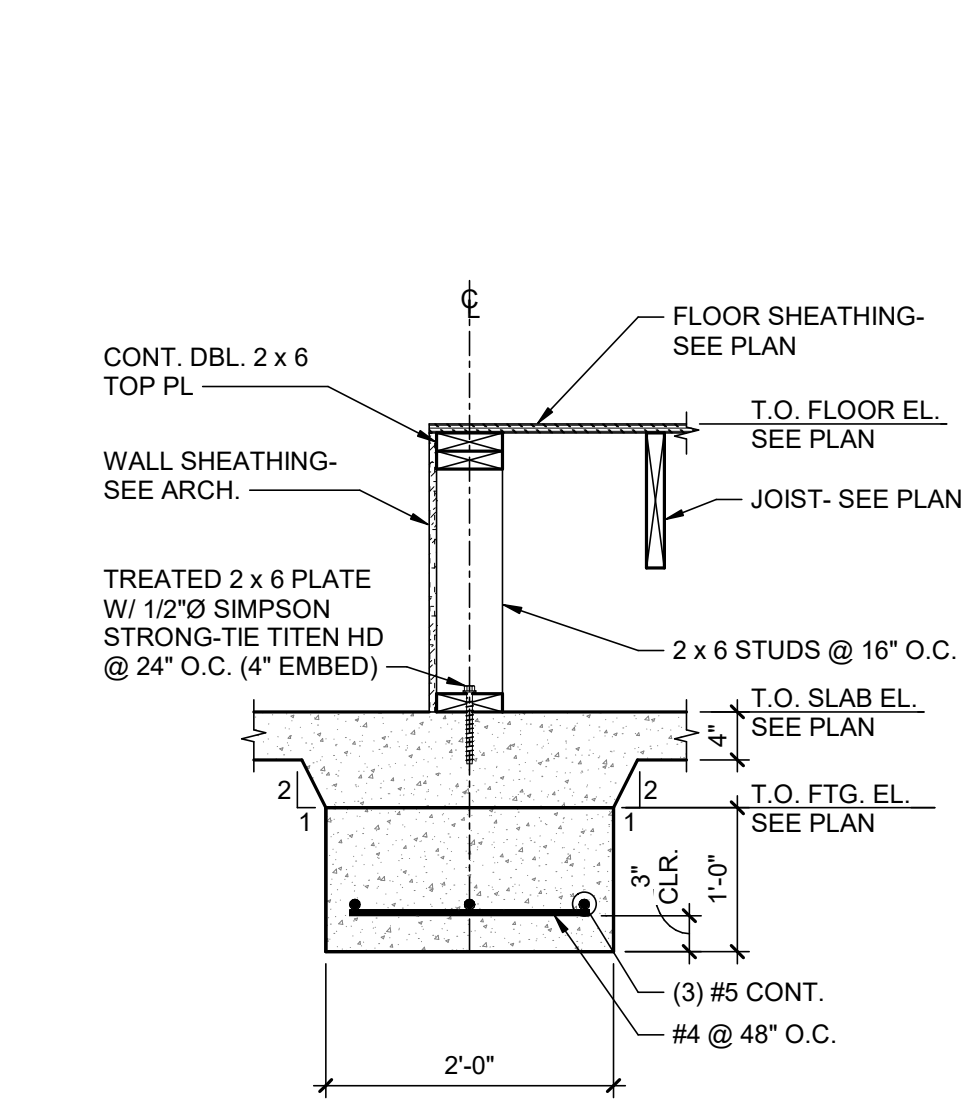
6 SECTION
3/4" = 1'-0"



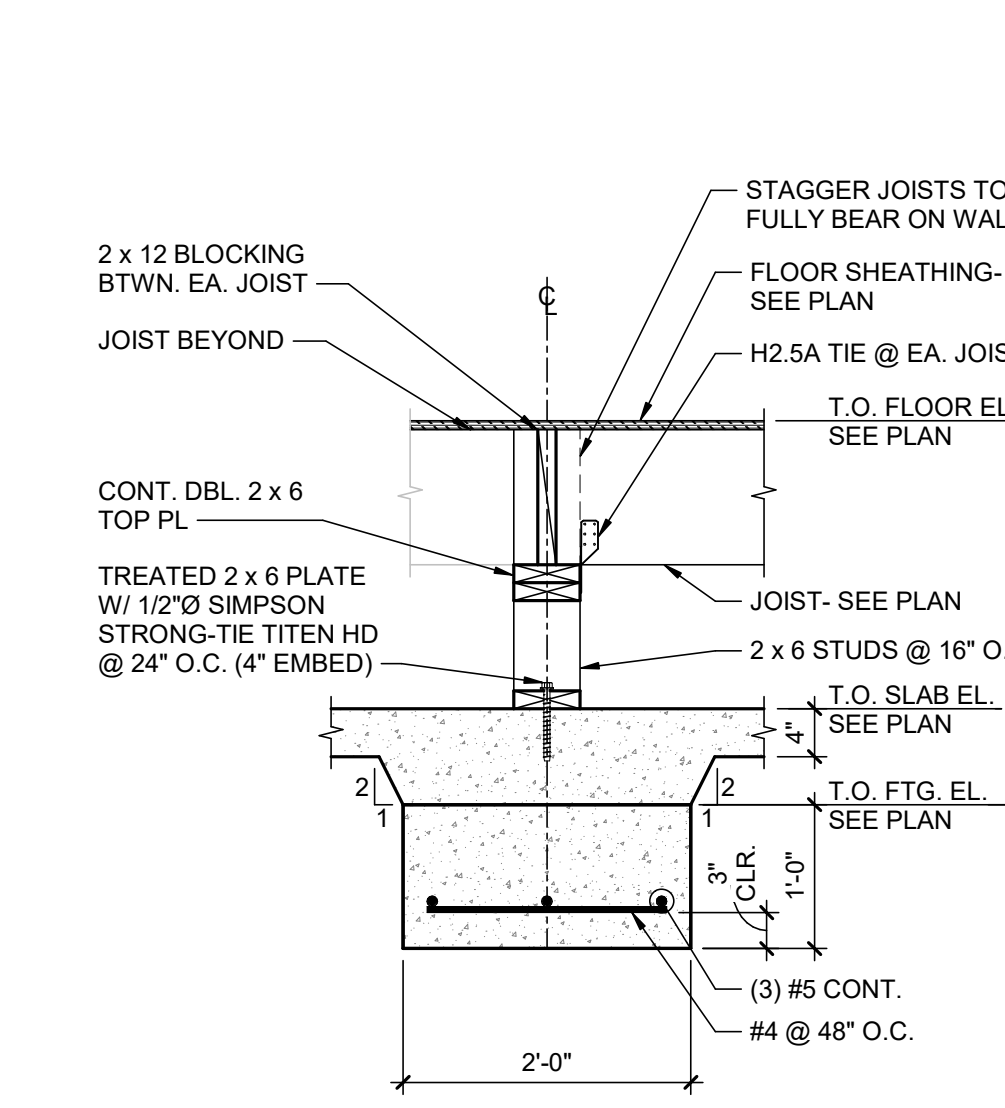
7 SECTION
3/4" = 1'-0"



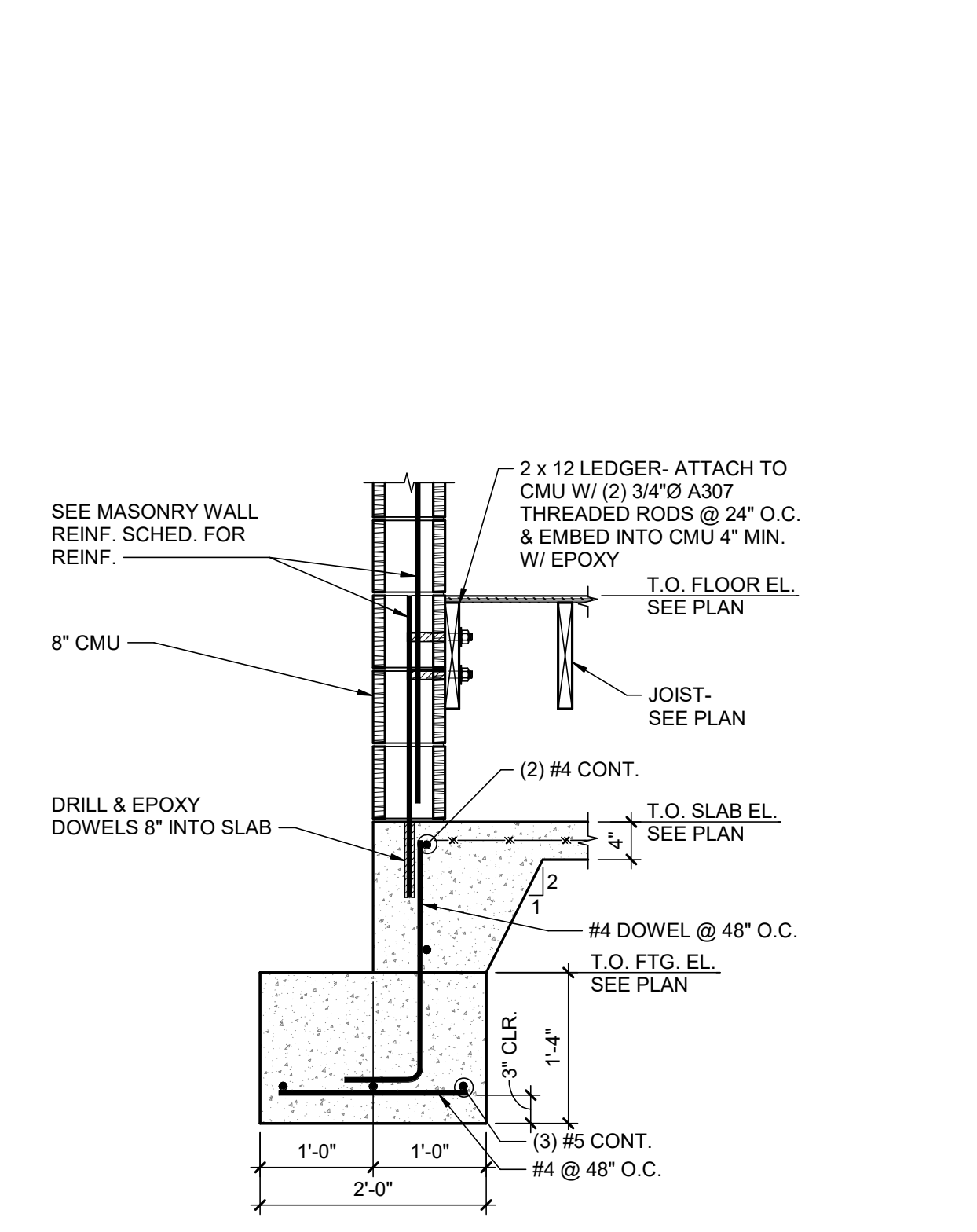
8 SECTION
3/4" = 1'-0"



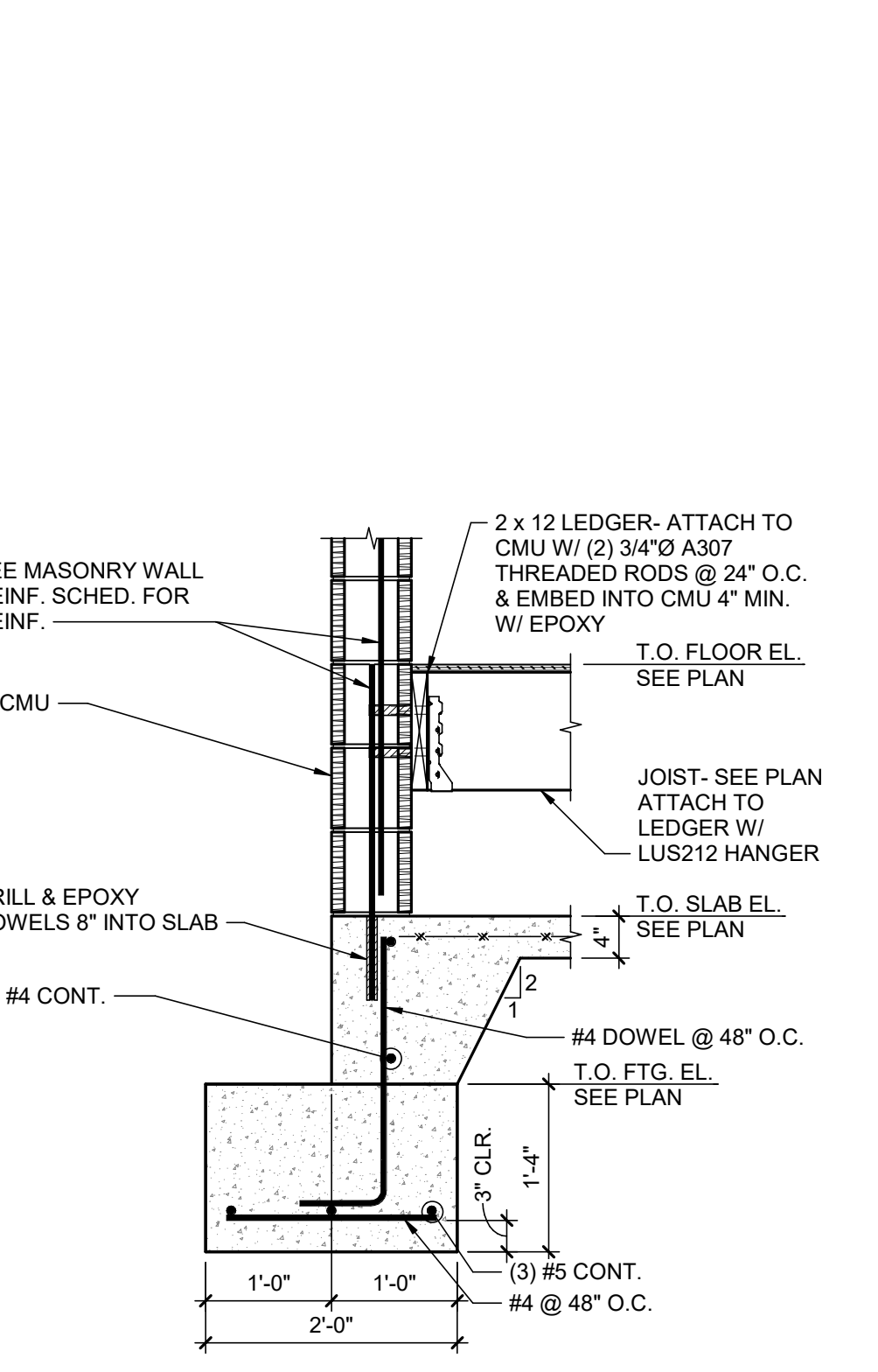
9 SECTION
3/4" = 1'-0"



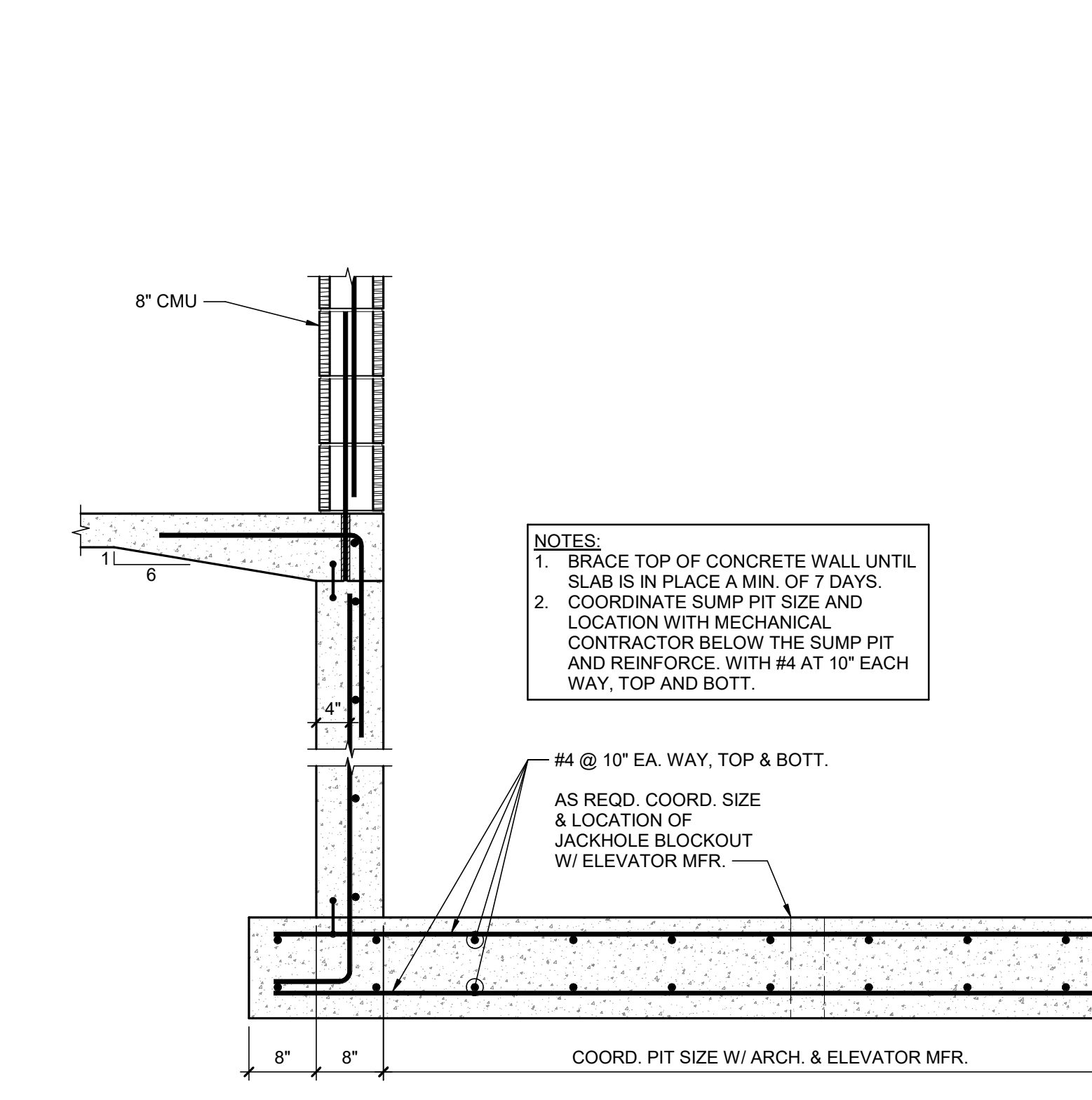
10 SECTION
3/4" = 1'-0"



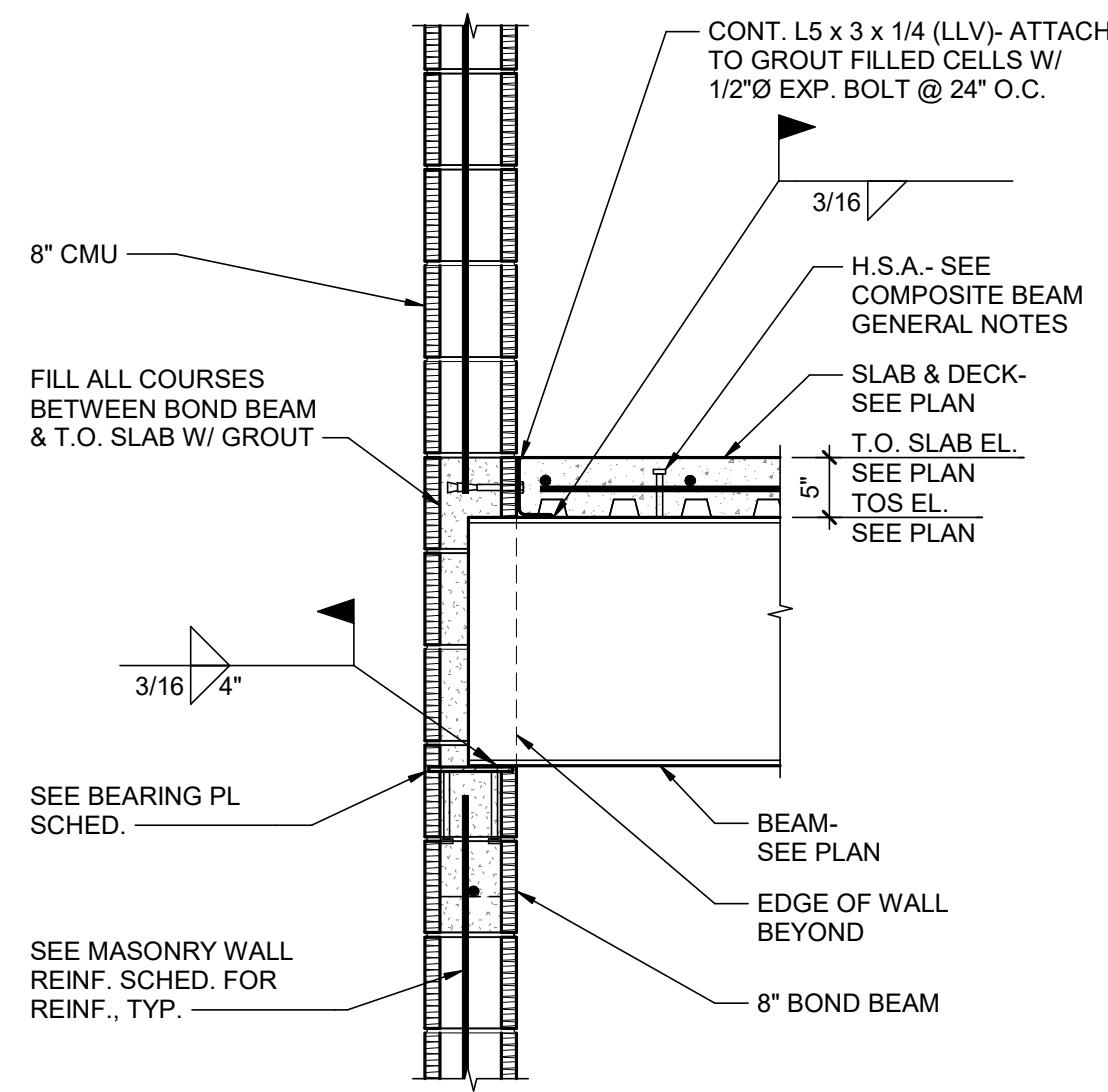
11 SECTION
3/4" = 1'-0"



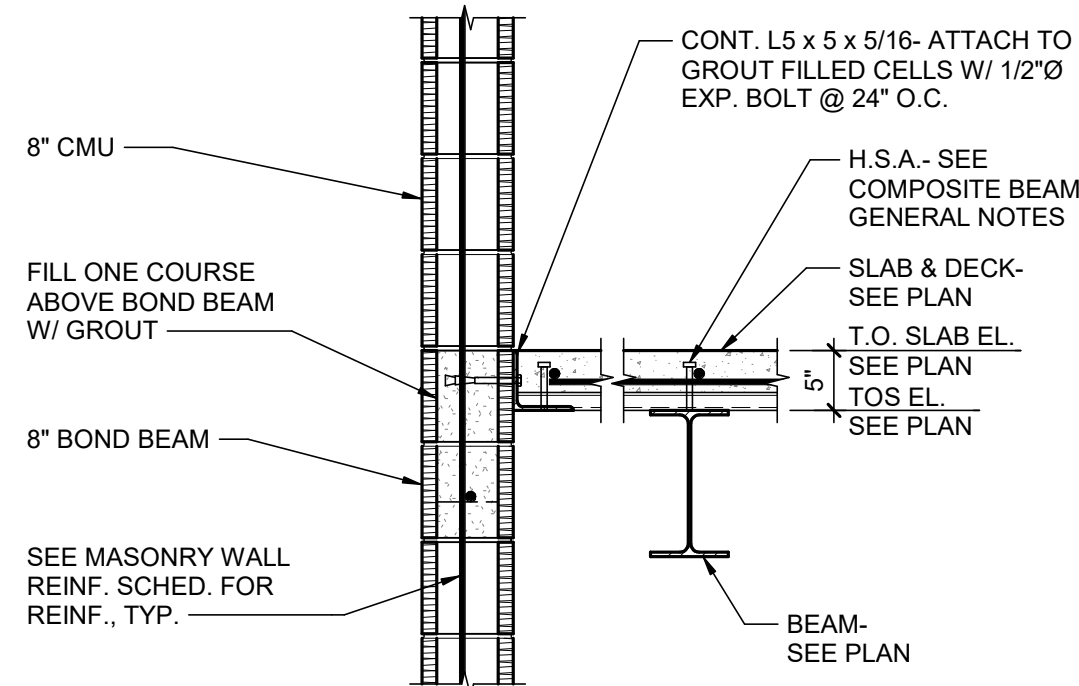
12 SECTION
3/4" = 1'-0"



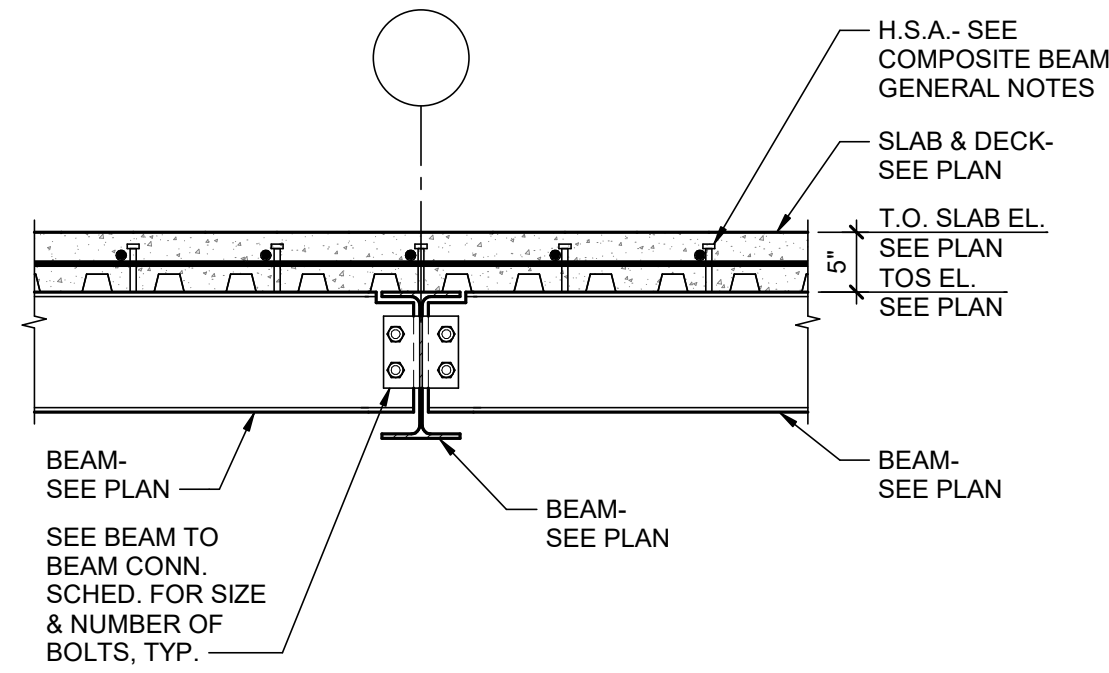
13 SECTION
3/4" = 1'-0"



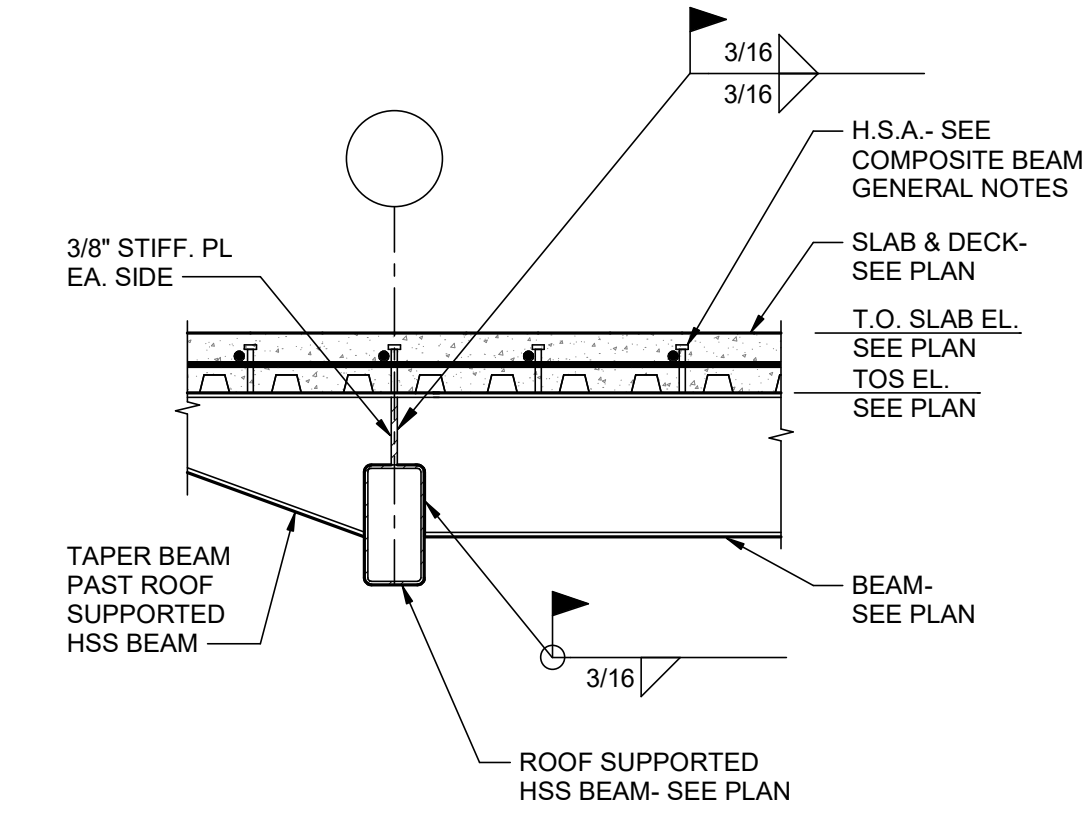
1 SECTION
3/4" = 1'-0"



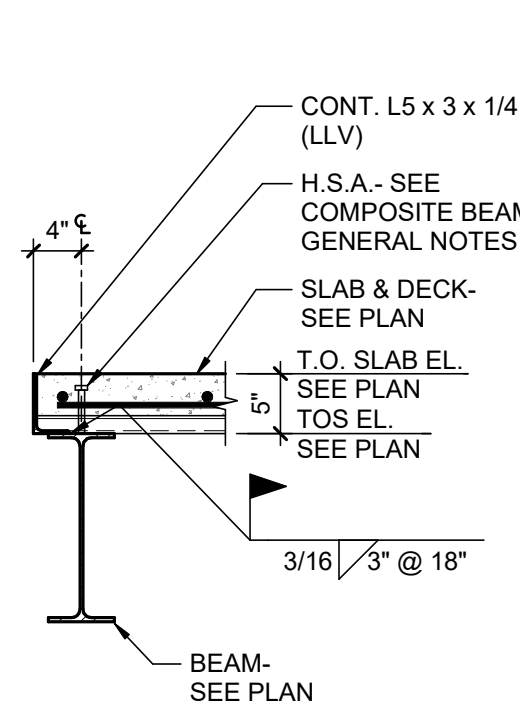
2 SECTION
3/4" = 1'-0"



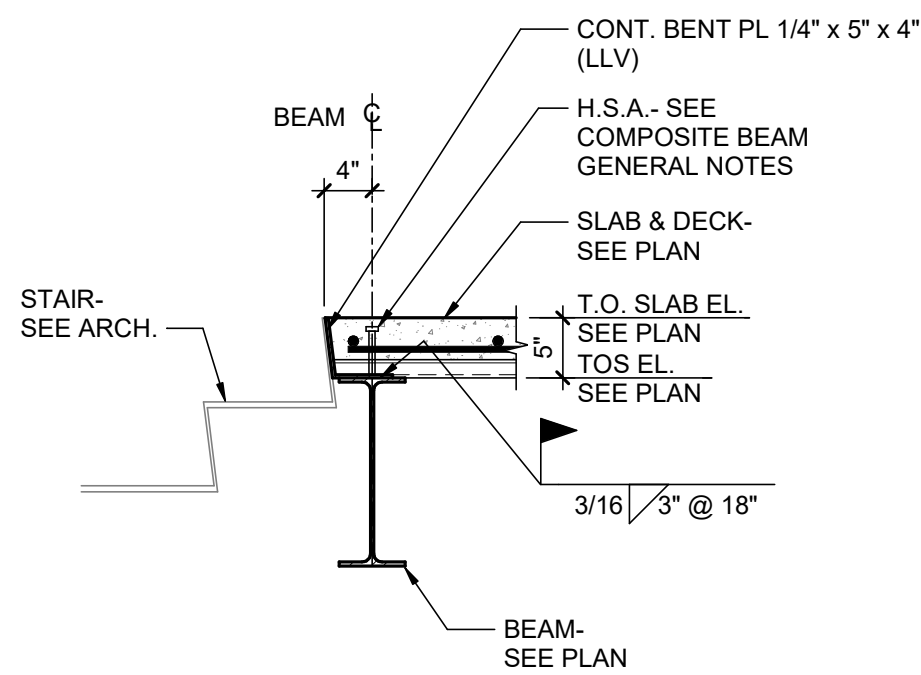
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3/4" = 1'-0"



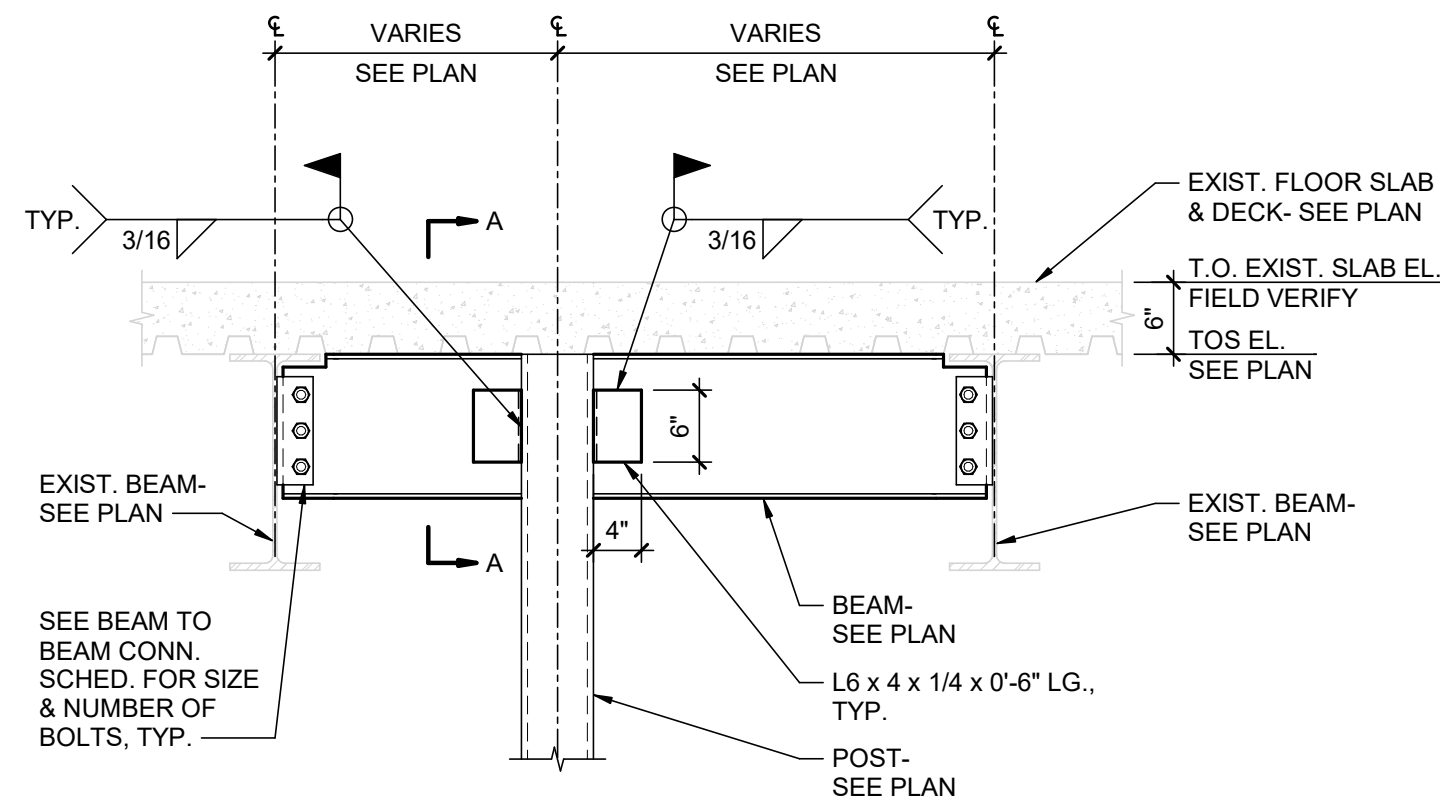
4 SECTION
3/4" = 1'-0"



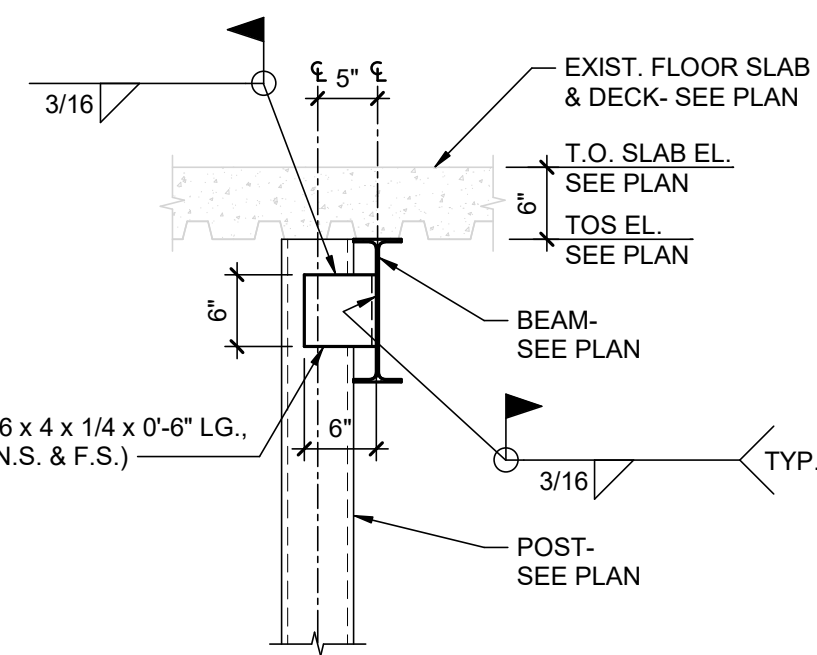
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3/4" = 1'-0"



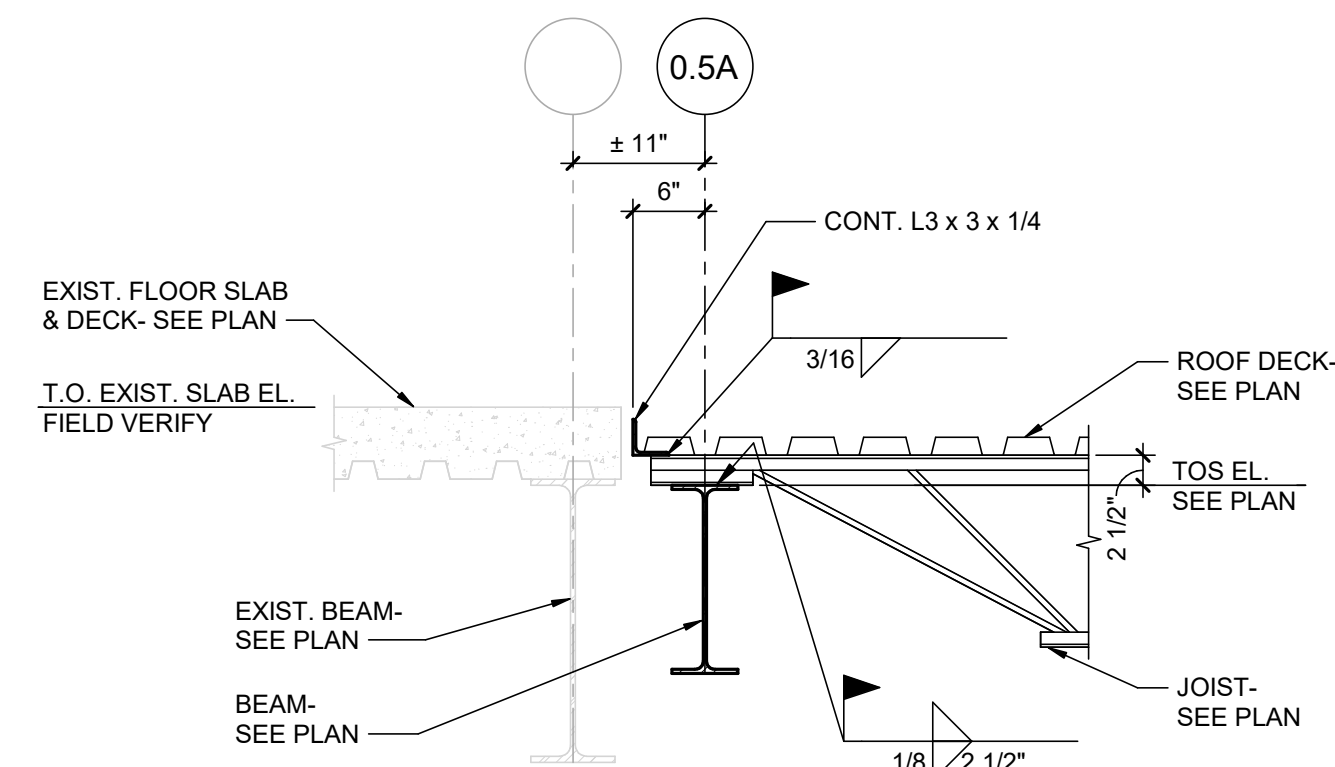
6 SECTION
3/4" = 1'-0"



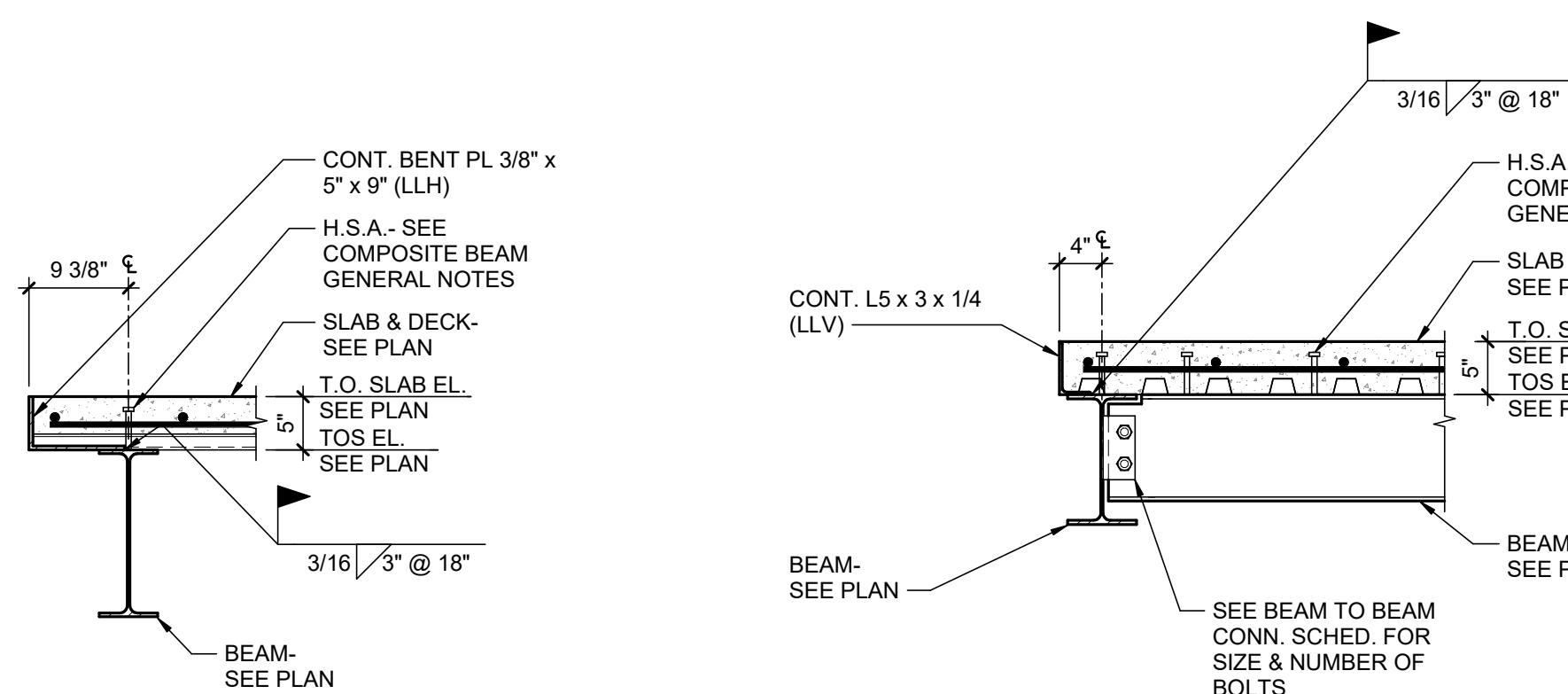
7 SECTION
3/4" = 1'-0"



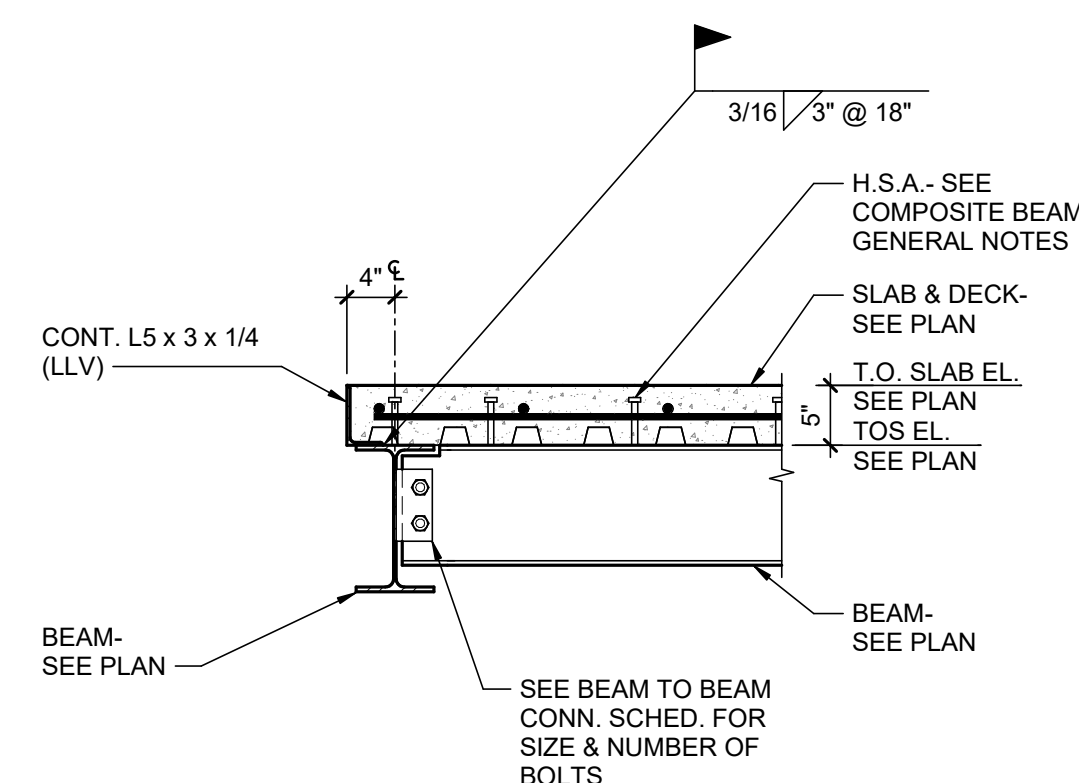
SECTION A-A



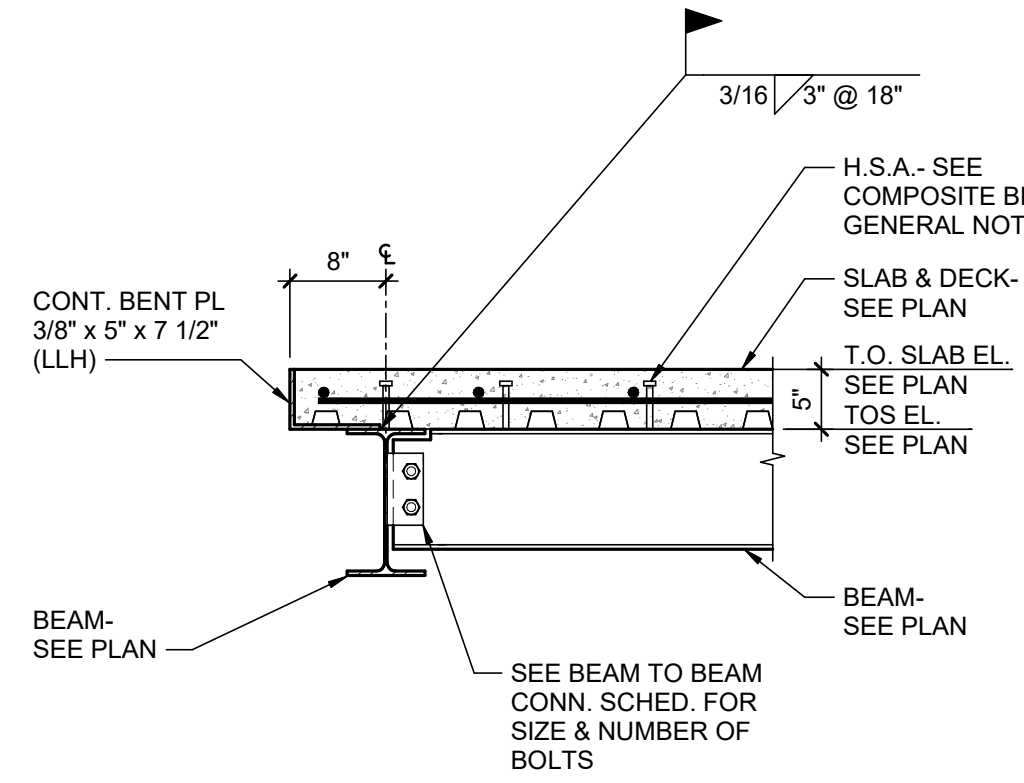
8 SECTION
3/4" = 1'-0"



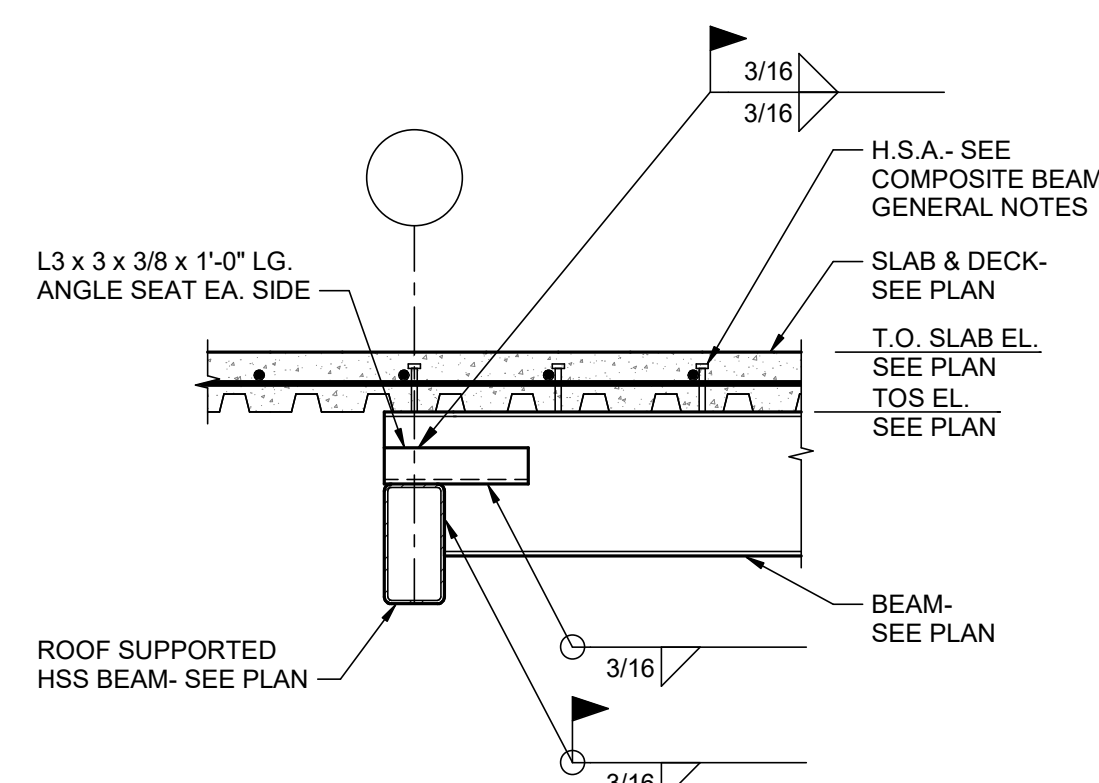
9 SECTION
3/4" = 1'-0"



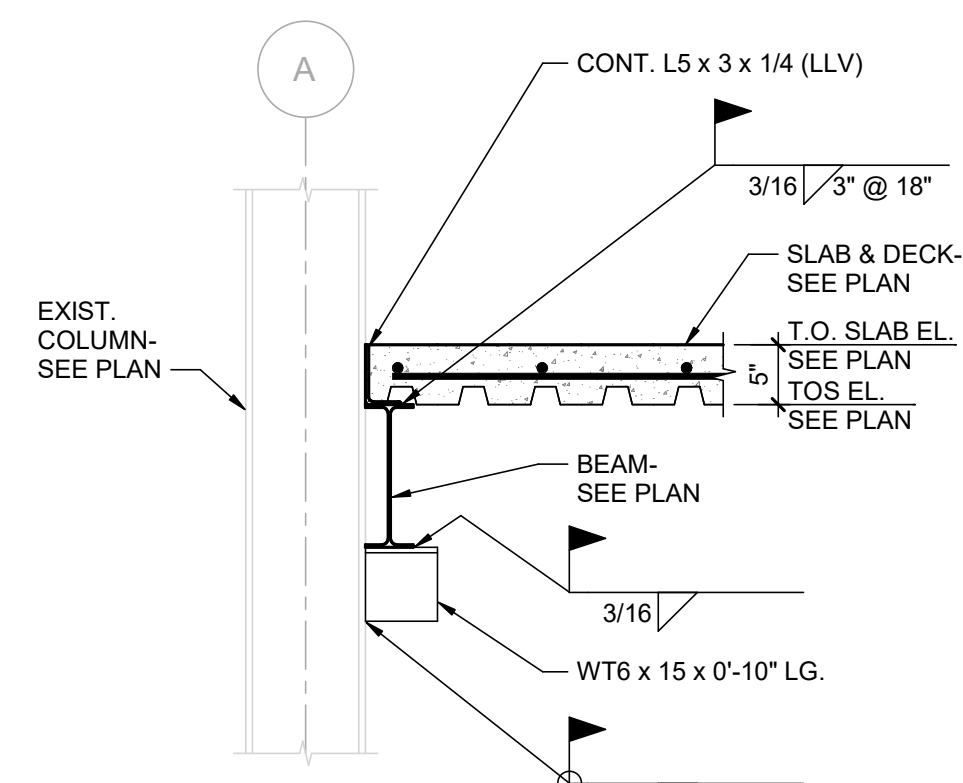
10 SECTION
3/4" = 1'-0"



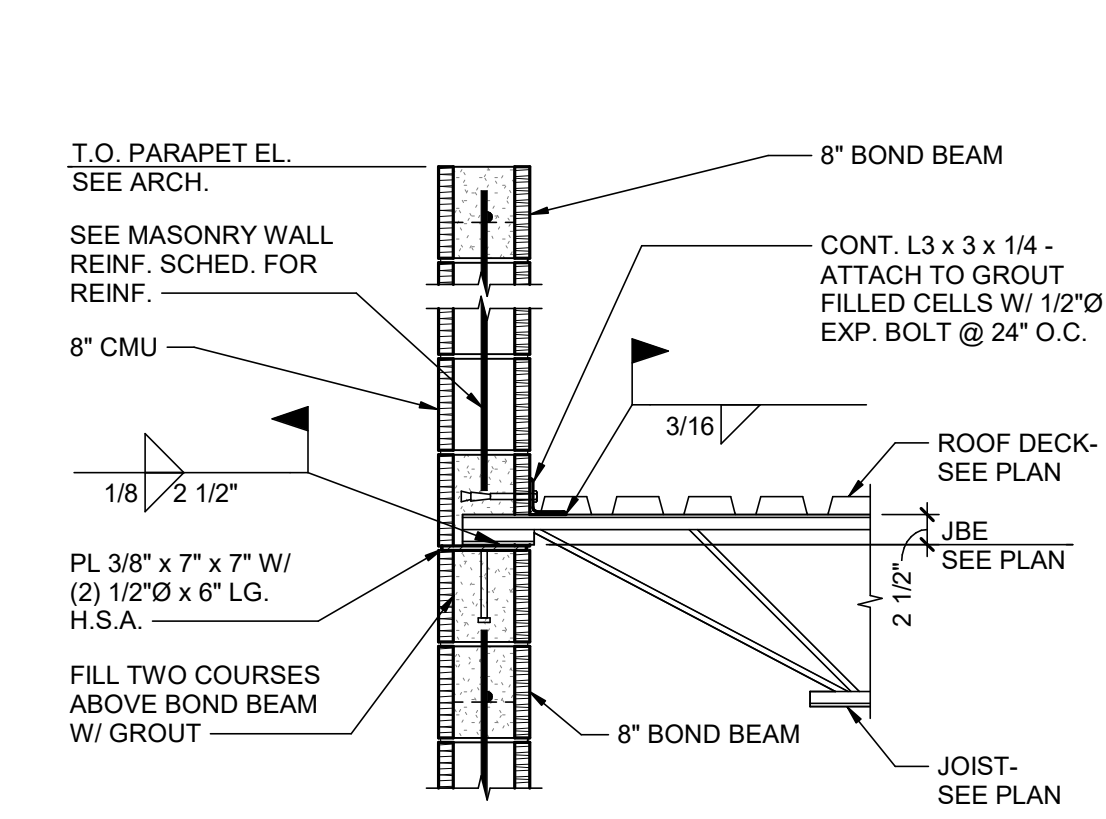
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3/4" = 1'-0"



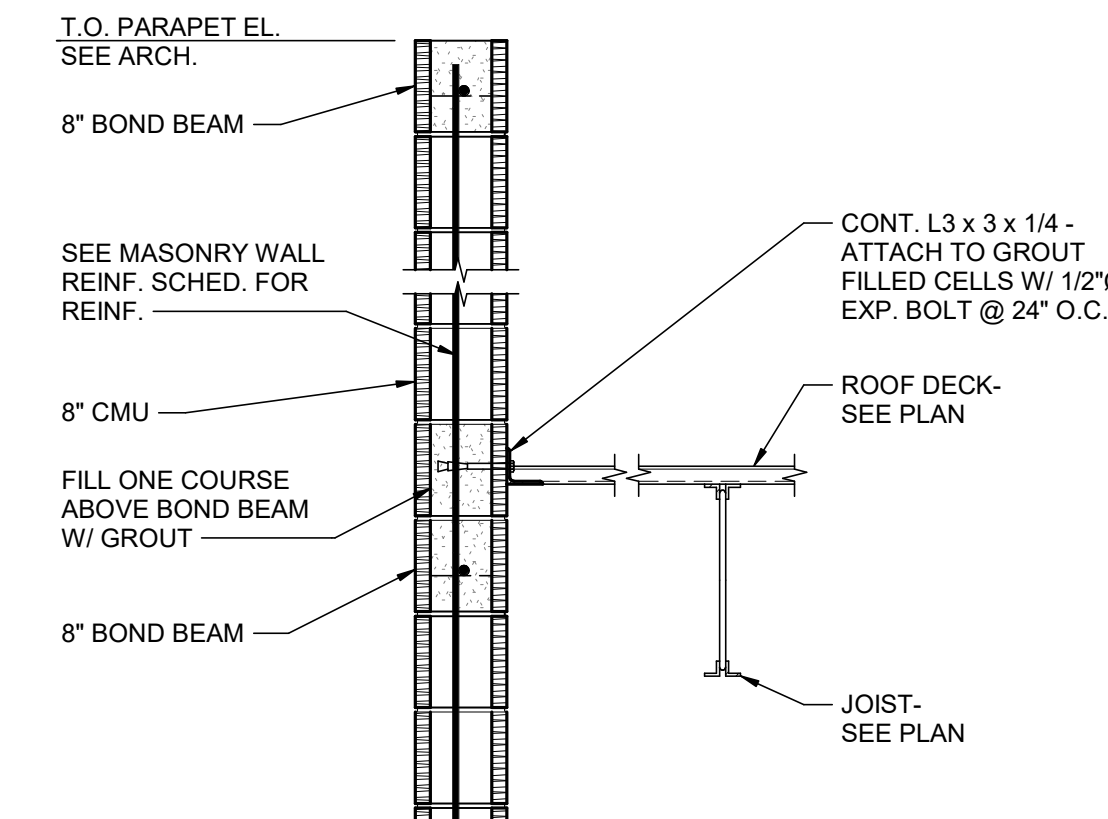
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3/4" = 1'-0"



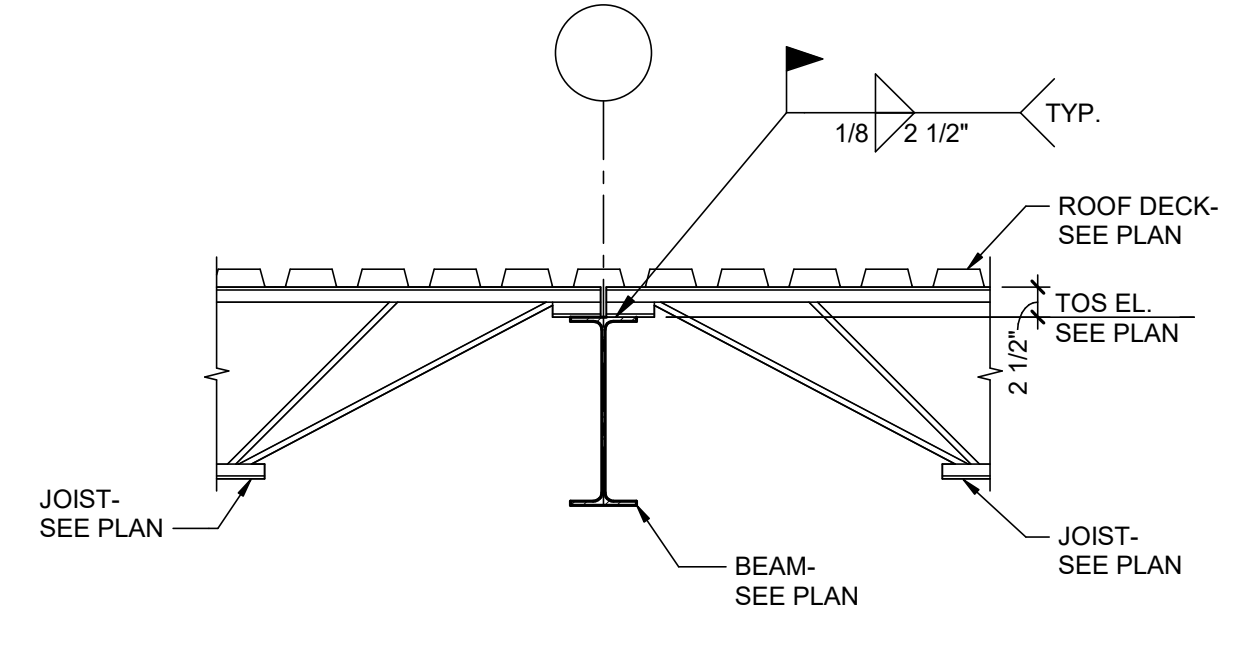
13 SECTION
3/4" = 1'-0"



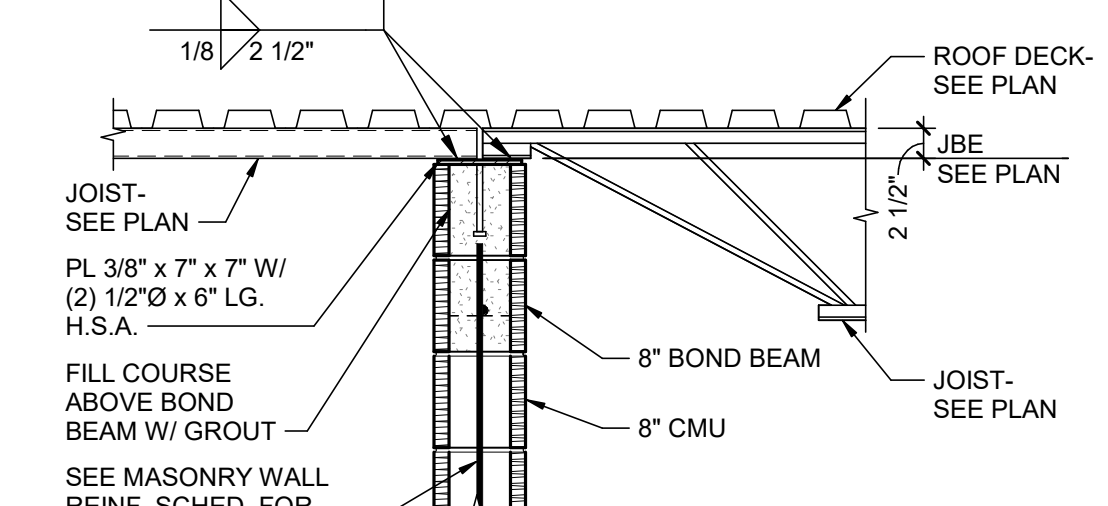
1 SECTION
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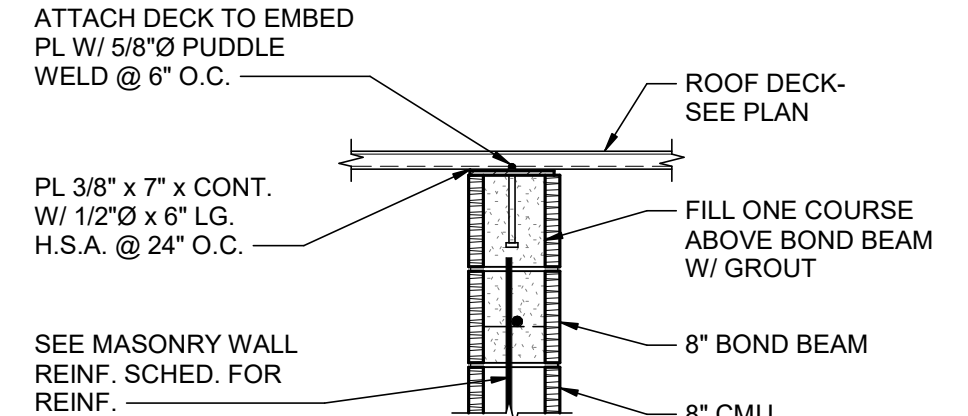
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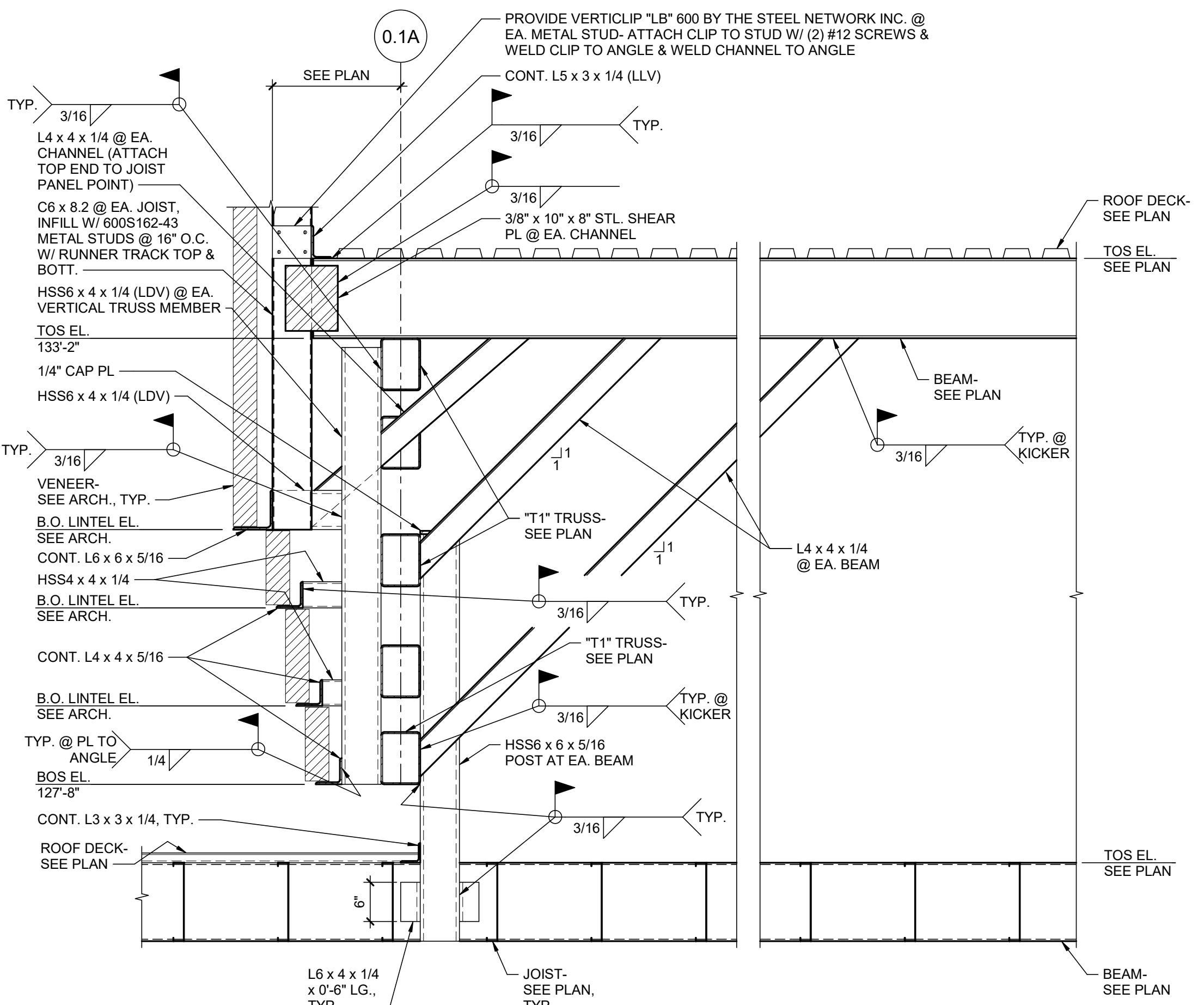
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 3/4" = 1'-0"



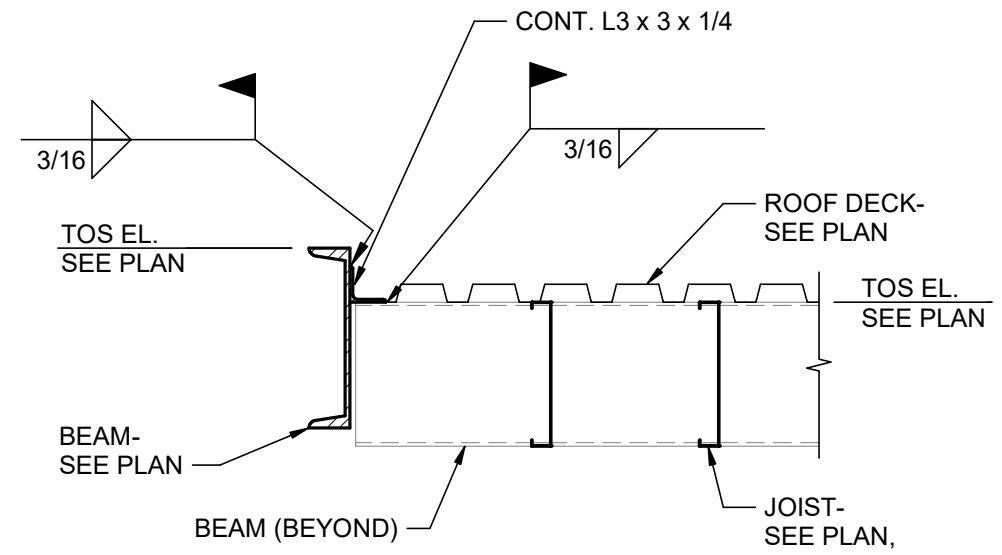
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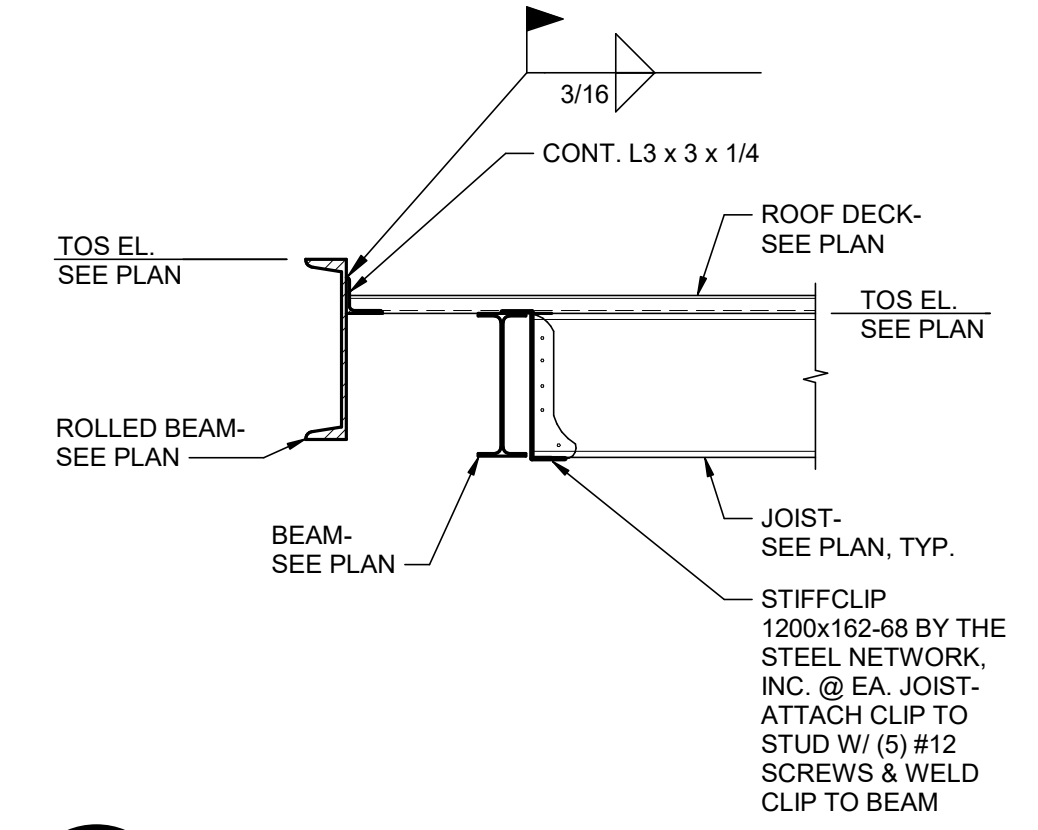
5 SECTION
 3/4" = 1'-0"



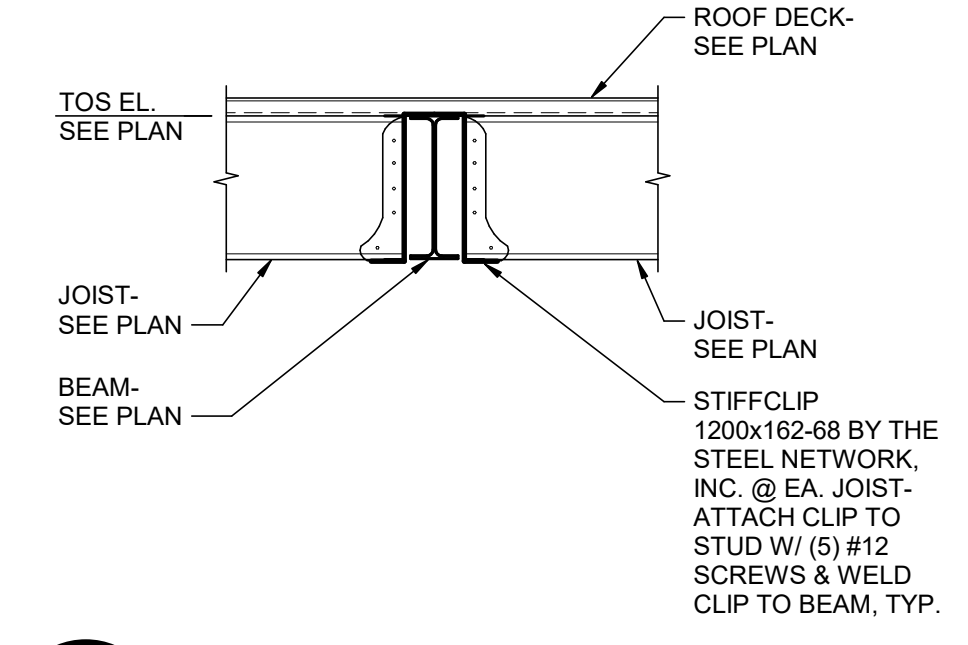
6 SECTION
 3/4" = 1'-0"



7 SECTION
 3/4" = 1'-0"



8 SECTION
 3/4" = 1'-0"



9 SECTION
 3/4" = 1'-0"

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 TSW #: 26021 PM: MEW DE: AVK

CERTIFICATE OF AUTHORIZATION
 Tatum Smith
 Welcher
 Engineers, Inc.
 No. 218
 ARKANSAS ENGINEER

sheet S5.2
 FRAMING DETAILS
 13 April 2026