

**CONSTRUCTION SAFETY GENERAL NOTE**  
THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

**Drilled Pier General Notes 2200:**

- Drilled pier construction shall conform to the "Specification for the Construction of Drilled Piers" (ACI 336).
- Drilled pier concrete shall develop a 28-day minimum compressive strength of 4,000 psi. This concrete shall have a designed slump of 6" ± 1" and utilize a water-reducing admixture or a high range water reducing admixture.
- Reinforcing steel shall be deformed bars conforming to ASTM A615, grade 60.
- When steel casing is required, the casing shall conform to ASTM A283, grade C or ASTM A36.
- Drilled piers have been designed using an allowable end bearing capacity of 35,000 psf. Drilled piers shall bear in hard limestone. See Soils Report No. 21-1-5-188 dated Dec. 14, 2021 by GTS, Inc.
- Bottom of drilled piers shall bear in hard limestone as called for in the Typical Drilled Pier Detail 5/S1.1. Depth of penetration into the hard limestone shall be as called for in the Typical Drilled Pier Detail 5/S1.1.
- Bottom of drilled pier elevations are estimated from boring logs to extend from 17 ft to 21 ft for bid purposes. Final elevations shall be determined by the contractor's investigation to provide the bearing capacity on the bearing material noted above and the minimum penetration noted in the Typical Pier Detail 5/S1.1. All pier bottoms shall be inspected by the Geotechnical Engineer.
- Temporary casing of piers is anticipated to prevent soil from sloughing off into excavation. Dewatering of excavation may be required before placing of concrete.
- If excessive ground water is encountered, use tremie tube to place concrete for drilled pier. Withdraw casing only as excavation is filled with concrete. Maintain adequate head of concrete to balance outside soil to water pressure above bottom of casing during withdraw.
- Drilled piers shall have a minimum length of 3 pier diameters.

**Concrete General Notes 3100:**

- All detailing, fabrication and placing of reinforcing steel shall conform to the ACI Standard "Details and Detailing of Concrete Reinforcement" (ACI 315).
- Concrete at slab-on-grade shall develop a 28-day minimum compressive strength of 3,500 psi and have Ultra Fiber 500 manufactured by Solomon Colors, Inc. mixed into the concrete at 2.0 lb/cubic yard. Follow normal mixing time and speed as recommended by ASTM C94.
- All concrete at elevated slab on metal deck shall typically develop a 28-day minimum compressive strength of 3,500 psi.
- All concrete at grade beams, pedestals, pier caps and walls shall typically develop a 28-day minimum compressive strength of 4,000 psi.
- All concrete for grade beams, pier caps, pedestals, elevated slab, walls and slab-on-grade shall have a 5" maximum slump.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All reinforcing bar splices shall be 44 bar diameters for #6 and smaller diameter bars. Reinforcing bar splices shall be 48 bar diameters for #7 and larger bar diameters.
- All reinforcing bar hooks shall be ACI standard 90 degree hook, unless noted otherwise.
- Provide two #4 x 4'-0" long diagonal bars centered in slab, at all re-entrant corners and any other locations designated on the plans.
- Provide corner bars in grade beams, concrete stemwall and turn-down slab same size and spacing as longitudinal reinforcing.
- Provide (1) #4 hoop with 8" lap in slab-on-grade and elevated slab around floor drains, columns and all slab penetrations 3" in diameter or greater. Also install around electrical conduit groupings 3" in diameter or greater.
- Limit the width of conduit groups to 3'-0" as they pass under grade beams. As much as possible, align the conduit perpendicular to the footing as it passes under the footing. Provide a minimum spacing of 2'-0" between conduit groups as the conduit passes under a footing. Do not extend conduits under column footings or spread footings.
- Plate dowel system shall be Diamond Dowel System by PNA Construction Technologies, the Speed Plate System by Sika Corp. or approved equal. Install plate dowels at slab construction joints at 16" O.C.
- Welded wire fabric shall conform to ASTM A1064. Provide mesh in flat sheets.
- Wire fabric reinforcing shall lap 6" and be securely wired at each side and end.
- Smooth dowels shall be steel conforming to ASTM A36.
- All slots, sleeves and other embedded items shall be set before concrete is placed. See Architectural, Electrical, Mechanical, and Vendor's drawings for size and locations.
- Bar supports at footings and slabs-on-grade shall be factory made wire bar supports, type "SBU" linear supports.
- Epoxy for doweling reinforcement shall be HY-200 by Hilli, unless noted otherwise.
- Construction joints in grade beams shall occur at mid-span. Continue all longitudinal reinforcement through construction joint and maintain proper bar splice lengths.
- Grade beam reinforcement splices shall be over the drilled piers for the bottom bars and at the center of the span for all other bars.
- Use of compacted, free-draining pea gravel, crushed stone, or coarse sand underneath the building slab is recommended by TSWE, Inc. Consult Geotechnical Engineer regarding potential substitution of free-draining coarse materials with approved subgrade. Slabs-On-Grade have been designed for a modulus of subgrade reaction (R-value) of 125 psin. Slab-on-grade shall be supported on a minimum of 2'-0" of compacted, tested, and approved select fill as described in Soils Report No. 21-1-5-188 dated December 14, 2021 by GTS, Inc.

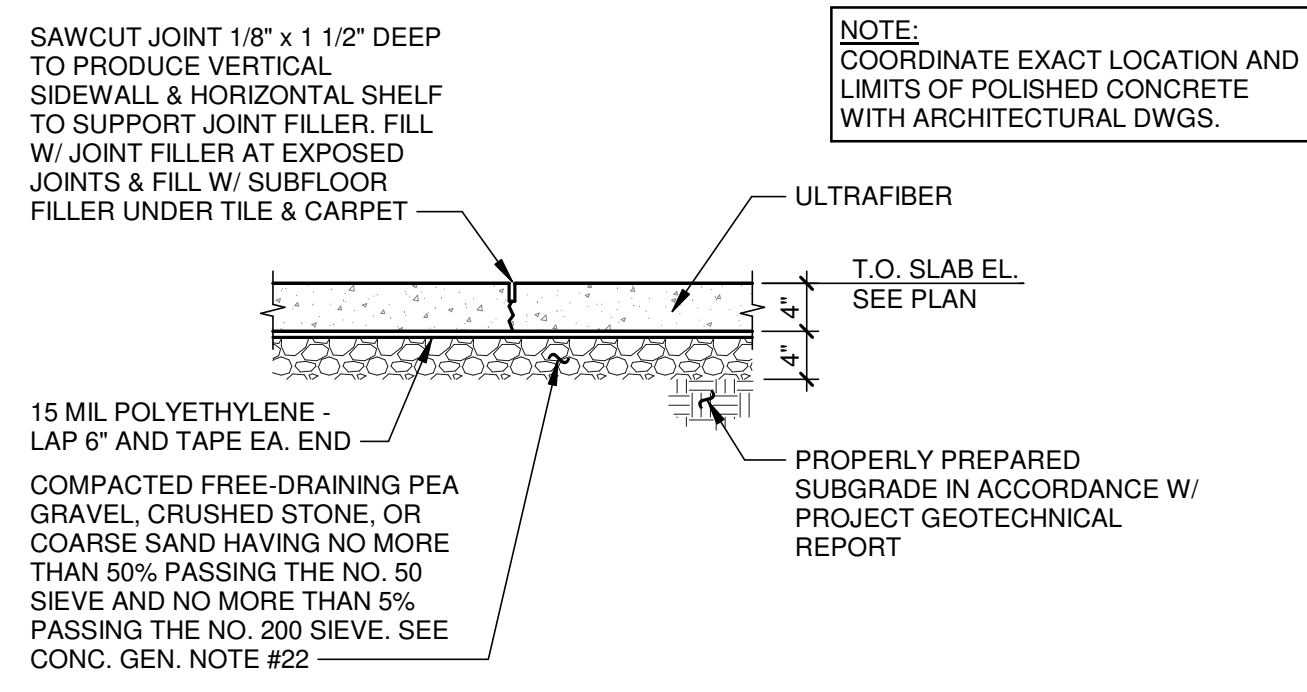
**Concrete Masonry General Notes 4100:**

- All concrete masonry units shall be lightweight above finished floor and normal weight below grade. All hollow concrete masonry units shall conform to ASTM C90, Grade N, Type 1 with a minimum ultimate compressive strength (f'm) of 2000 psi for the masonry assemblage. All concrete masonry shall be laid in Running (Common) Bond. CMU shall also comply with requirements of fire wall rating. See arch. drawings for firewall rating required.
- Mortar at walls shall be Type S mortar and have a minimum compressive strength of 1,800 psi. Mortar at interior non-load-bearing walls not higher than 20'-0" and mortar at masonry veneer shall be Type N mortar and have a minimum compressive strength of 750 psi. All mortar shall conform to ASTM C270. Masonry cement shall not be used for mortar. Mortar shall also comply with requirements of fire wall rating. See arch. drawings for firewall rating required.
- All grout shall be ready-mix concrete, with 3/8" diameter max. aggregate, have a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10" or preblended product (Core Fill Grout, Coarse CF-02, by Spec Mix) with a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10".
- All 8" CMU bond beam units shall be reinforced with one bar. See details for size of bars for vertical wall reinforcement and bond beam requirements. Provide corner bars and lap bond beam reinforcing 48 bar diameters.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All bolts, anchors, reinforcement and embedded items shall be grouted in place.
- All reinforcing bar splices shall be 48 bar diameters, U.N.O.
- At all 8" CMU walls provide (1) vertical bar each cell for the first (2) cells adjacent to control joints in walls, at ends of walls, wall corners and on each side of wall openings, unless noted otherwise. Vertical bars shall match reinforcement for remainder of wall. See details for size of reinforcement. Wall cell fill shall also comply with requirements of fire wall rating. See arch. drawings for firewall rating required.
- Provide control joints in brick veneer where shown on Architectural Drawings.
- Provide control joints in CMU walls where shown on Architectural Drawings. Place joints for CMU walls max. 24'-0" O.C.
- Provide horizontal joint reinforcement at 16" O.C. Reinforcement shall be ladder design, min. 9 gage welded steel wire, hot dipped galvanized to 1.5 oz. width shall be 1 1/2" less than wall thickness.

**TYPICAL STRUCTURAL ABBREVIATIONS**

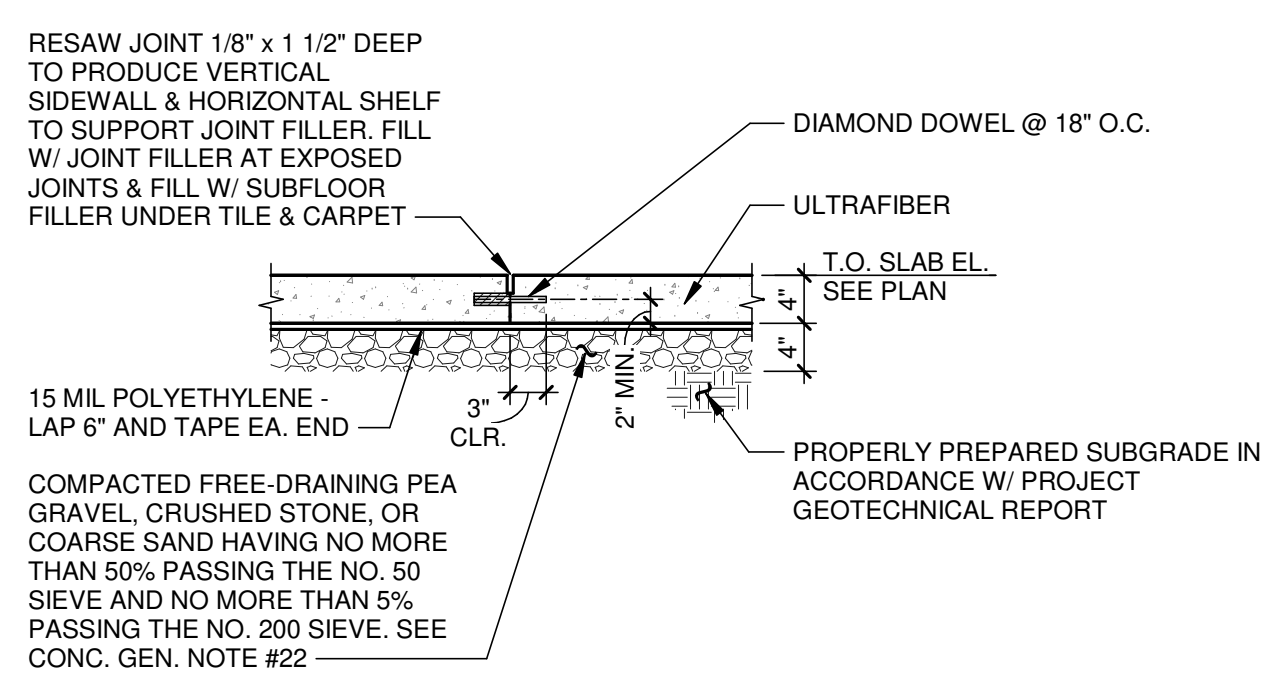
A.R.	ANCHOR ROD	F.S.	FAR SIDE	PLF	POUNDS PER FOOT
ACI	AMERICAN CONCRETE INSTITUTE	FDN	FOUNDATION	PROJ	PROJECTION
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	FIN	FINISH	PSF	POUNDS PER SQUARE FOOT
ARCH	ARCHITECT	FLR	FLOOR	PSI	POUNDS PER SQUARE INCH
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	FTG	FOOTING	R	RADIUS
BLDG	BUILDING	GALV	GALVANIZED	REINF	REINFORCEMENT
BM	BEAM	H.A.	HEADED STUD ANCHOR	RECD	REQUIRED
BOTT	BOTTOM	HK	HOOK	RTU	ROOF TOP UNIT
BRG	BEARING	HORIZ	HORIZONTAL	S.O.G.	SLAB ON GRADE
BTWN	BETWEEN	J.B.E.	JOIST BEARING ELEVATION	SCHED	SCHEDULE
CFS	COLD-FORMED STEEL	JST	JOIST	SECT	SECTION
CL or C	CENTER LINE	JT	JOINT	SEOR	STRUCTURAL ENGINEER OF RECORD
CLR	CLEAR	LG	LONG LEG	SIM	SIMILAR
COL	COLUMN	LLH	LONG LEG HORIZONTAL	SJI	STEEL JOIST INSTITUTE
CONC	CONCRETE	LLV	LONG LEG VERTICAL	SPA	SPACING
CONN	CONNECTION	LONG	LONGITUDINAL	SPECS	SPECIFICATIONS
CONT	CONTINUOUS	MAX	MAXIMUM	STD	STANDARD
DIA. or Ø	DIAMETER	MEM	METAL BUILDING MANUFACTURER	STIFF	STIFFENER
DIM	DIMENSION	MECH	MECHANICAL	STL	STEEL
DN	DOWN	MFR	MANUFACTURER	TOF	TOP OF FOOTING
Ø	DIAMETER	MNI	MINIMUM	TOG	TOP OF GRADE BEAM
DTL	DETAIL	MISC	MISCELLANEOUS	TOW	TOP OF WALL
DWG	DRAWING	MTL	METAL	TRANS	TRANSVERSE
E.F.	EACH FACE	N.S.	NEAR SIDE	TYP	TYPICAL
E.W.	EACH WAY	O.C.	ON CENTER	U.N.O.	UNLESS NOTED OTHERWISE
EA	EACH	OPNG	OPENING	VERT	VERTICAL
EL	ELEVATION	OPNG	OPENING	W/	WITH
EQ	EQUAL	PL or P	PLATE	WP	WORK POINT
EXIST	EXISTING			WWF	WELDED WIRE FABRIC
EXP	EXPANSION				

**NOTE:**  
ALL SLAB-ON-GRADE SHALL HAVE ULTRAFIBER 500 REINFORCING FIBERS AT DOSAGE OF 2.0 LB. / CUBIC YD. AS MANUFACTURED BY SOLOMON COLORS, INC.



**1 TYP. SLAB-ON-GRADE CONTROL JT. DTL. (CJ)**  
NOT TO SCALE

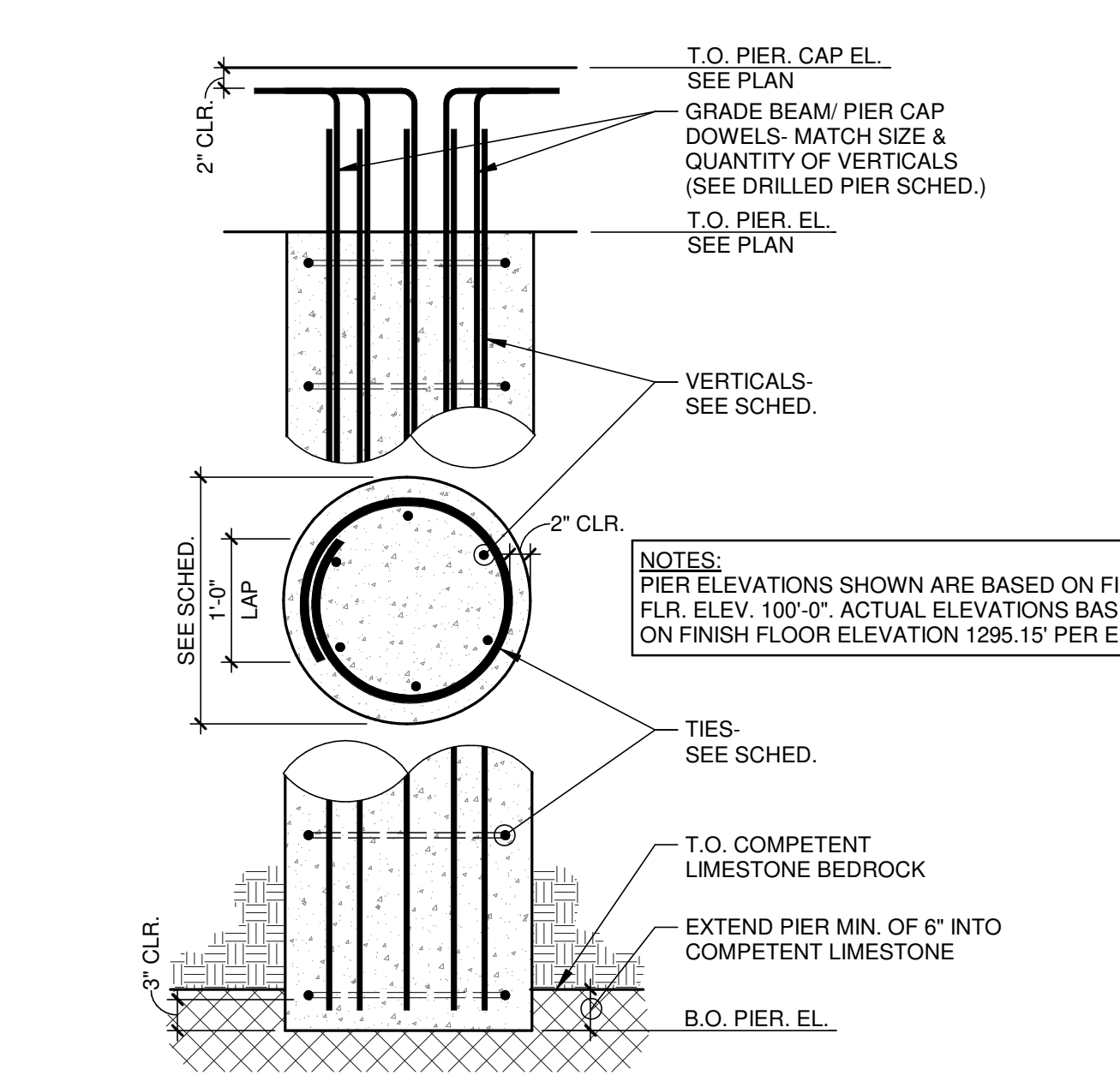
**NOTE:**  
PROVIDE DIAMOND DOWEL SYSTEM AS MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES OR THE SPEED SYSTEM BY SIKI CORPORATION AT CONSTRUCTION JOINTS IN ALL FLOOR AREAS. COORDINATE WITH ARCHITECT FOR CONSTRUCTION JOINT LOCATIONS.



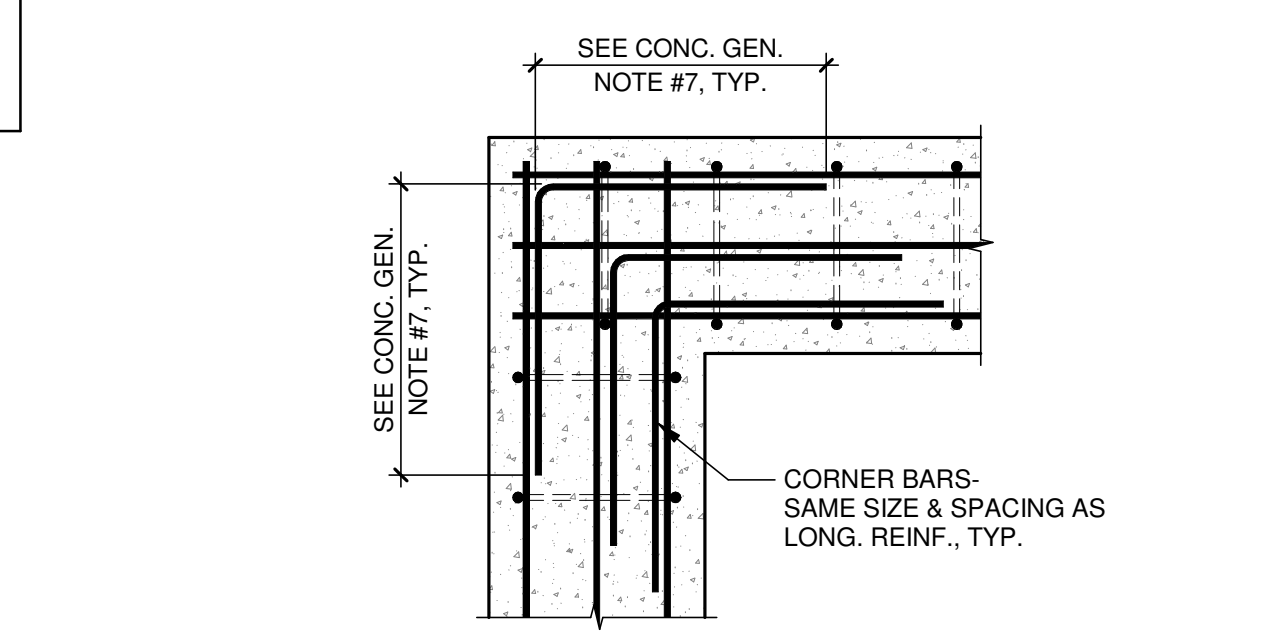
**2 TYP. SLAB-ON-GRADE CONSTRUCTION JT. DTL. (CJ)**  
NOT TO SCALE

**DRILLED PIER SCHEDULE**

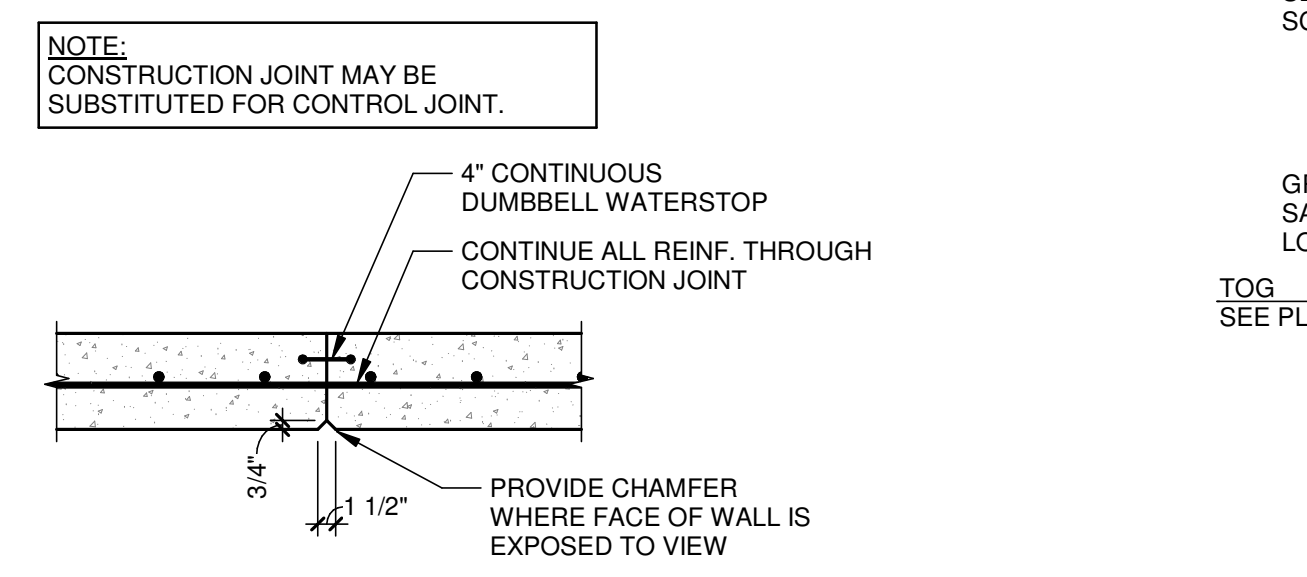
MARK	DIAMETER	VERTICALS	TIES	VERT. LAP LENGTH	REMARKS
DP1	2'-0"	(6) #6	#4 @ 12" O.C.	12"	<varies>
DP2	2'-6"	(9) #6	#4 @ 12" O.C.	12"	----



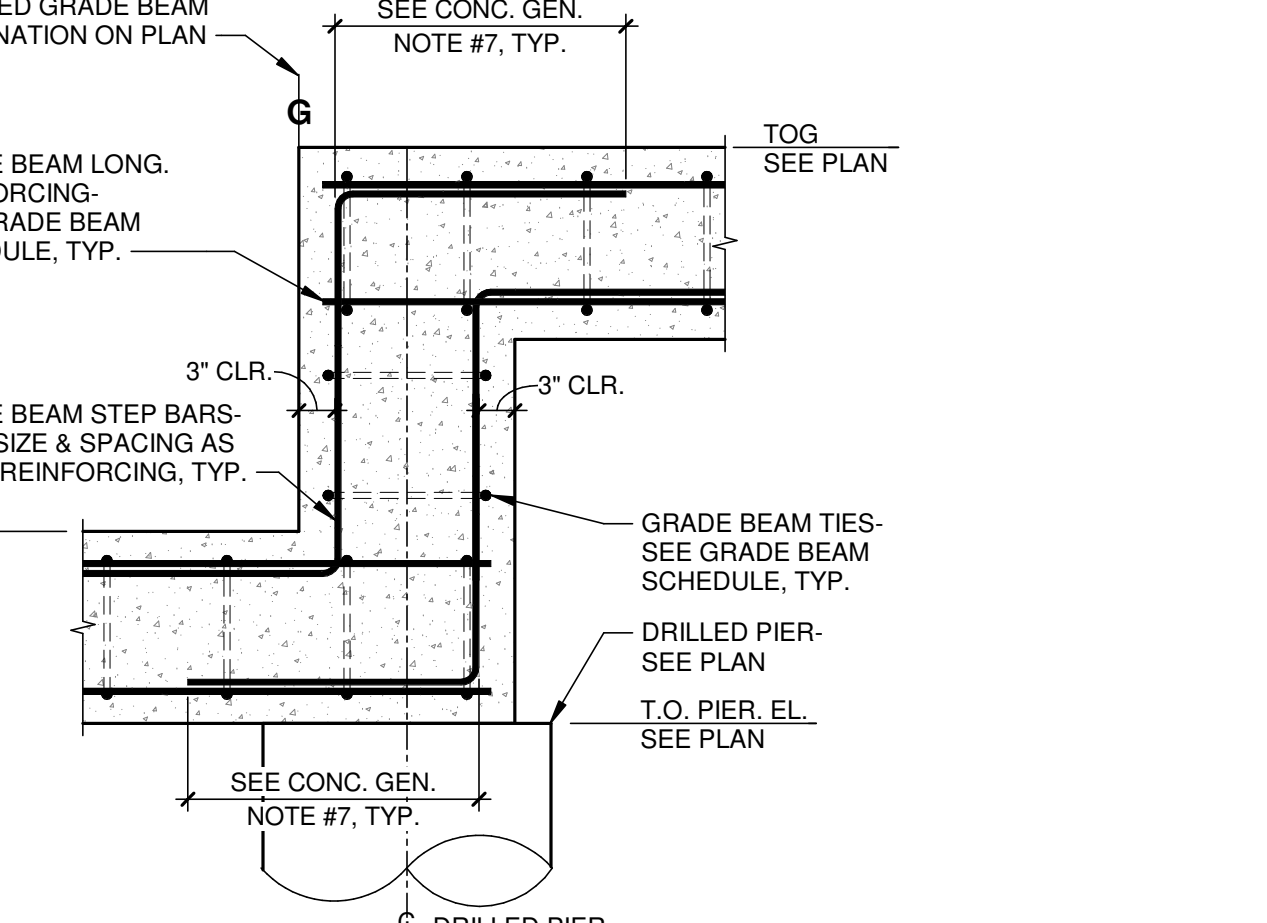
**5 TYP. DRILLED PIER DETAIL**  
NOT TO SCALE



**6 TYP. GRADE BEAM CORNER DETAILS**  
NOT TO SCALE



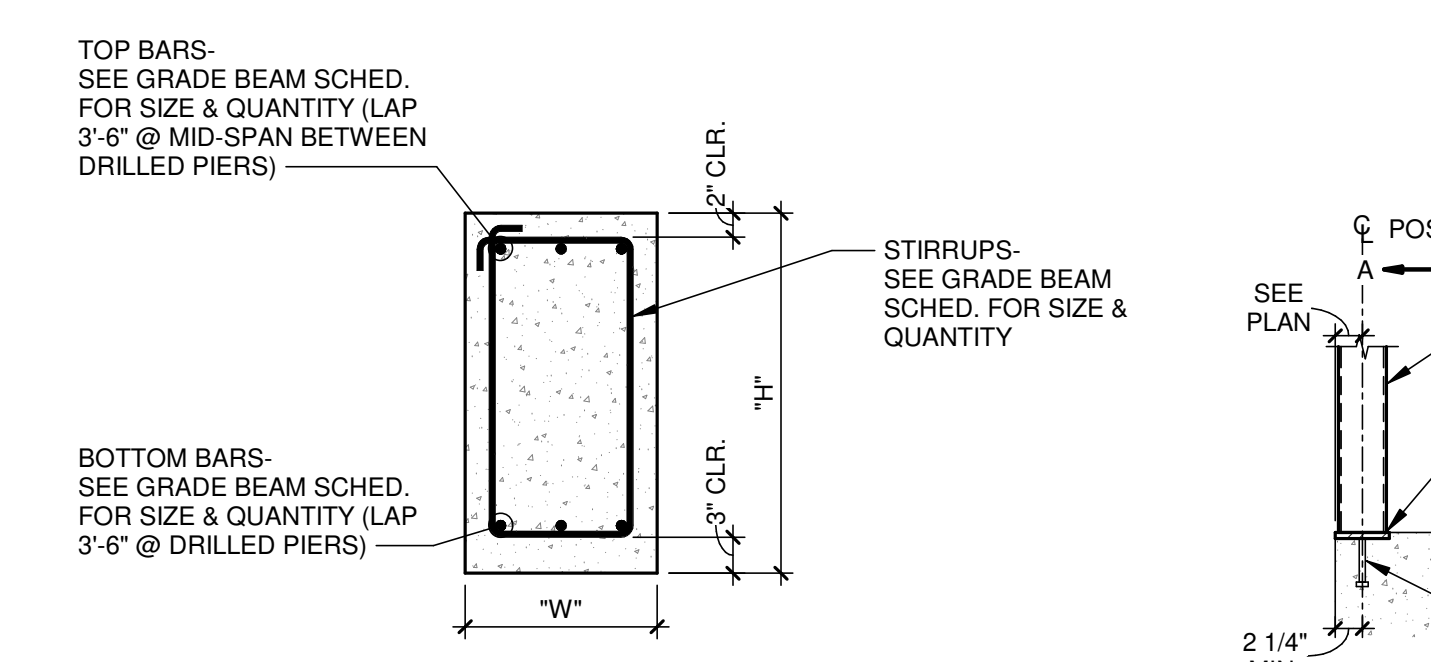
**8 TYP. CONCRETE WALL CONSTRUCTION JT. DTL.**  
NOT TO SCALE



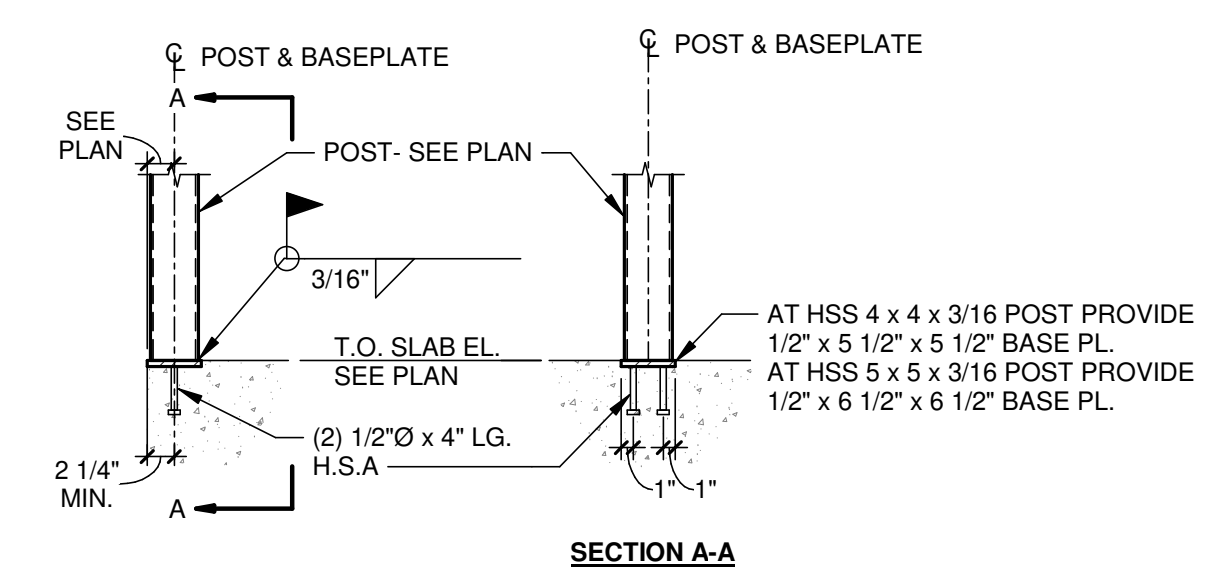
**9 TYP. STEPPED GRADE BEAM @ PIER DETAIL**  
NOT TO SCALE

**GRADE BEAM SCHEDULE**

MARK	GRADE BEAM SIZE		TOP BARS	MIDDLE BARS	BOTTOM BARS	STIRRUPS	REMARKS
	W	H					
GB1	2'-0"	2'-0"	(6) #6	----	(6) #6	#3 @ 8" O.C.	----
GB2	3'-0"	2'-0"	(9) #6	----	(6) #6	#3 @ 6" O.C.	----
GB3	3'-0"	2'-0"	(9) #6	----	(9) #6	#3 @ 6" O.C.	----
GB4	2'-0"	3'-0"	(8) #6	----	(8) #6	#3 @ 8" O.C.	----
GB5	3'-0"	3'-0"	(10) #6	----	(10) #6	#3 @ 12" O.C.	----
GB6	1'-6"	2'-6"	(6) #6	----	(6) #6	#3 @ 10" O.C.	----

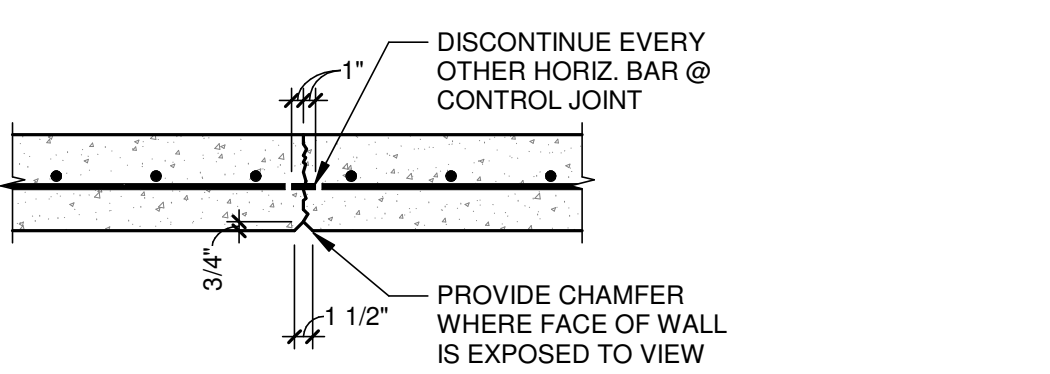


**3 TYP. GRADE BEAM DETAIL**  
NOT TO SCALE



**4 TYP. POST BASE**  
NOT TO SCALE

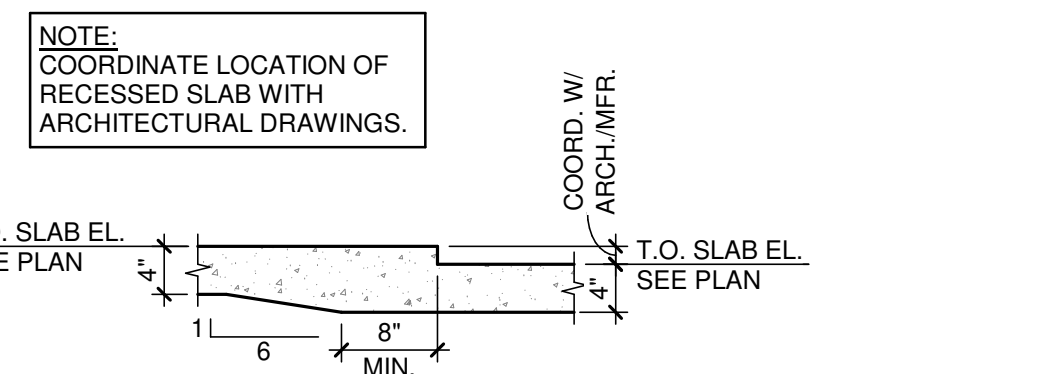
**NOTE:**  
LOCATE CONTROL JOINT AT 24'-0" O.C. MAXIMUM. COORDINATE LOCATION OF CONTROL JOINTS IN CONCRETE WALL WITH LOCATION OF CONTROL JOINTS IN VENEER AND CMU WALLS (SEE ARCHITECTURAL DRAWINGS).



**7 TYP. CONCRETE WALL CONTROL JT. DTL.**  
NOT TO SCALE



**10 TYP. CONCRETE WALL CORNER DETAIL**  
NOT TO SCALE



**11 TYP. RECESSED SLAB**  
NOT TO SCALE

**PIER CAP SCHEDULE**

MARK	SIZE		REINFORCING	REMARKS
	WIDTH	DEPTH		
PC1	2'-6"	2'-6"	(3) #4 EA. WAY TOP & BOT.	SEE NOTES
PC2	3'-6"	3'-6"	(4) #4 EA. WAY TOP & BOT.	SEE NOTES
PC3	4'-6"	4'-6"	(5) #4 EA. WAY TOP & BOT.	SEE NOTES
PC4	4'-6"	5'-0"	(5) #4 LONG. EA. WAY TOP & BOT. (6) #4 TRANS. EA. WAY TOP & BOT. #3 TIES @ 8" O.C. EA. WAY	SEE NOTES
PC5	4'-6"	4'-6"	(4) #4 LONG. EA. WAY TOP & BOT. (6) #4 TRANS. EA. WAY TOP & BOT. #3 TIES @ 8" O.C. EA. WAY	SEE NOTES
PC6	2'-6"	4'-6"	(4) #4 LONG. EA. WAY TOP & BOT. (6) #4 TRANS. EA. WAY TOP & BOT. #3 TIES @ 8" O.C. LONG	SEE NOTES

**PIER CAP SCHEDULE NOTES:**  
1. SEE FOUNDATION PLAN FOR LOCATION AND ORIENTATION.  
2. SEE FOUNDATION PLAN FOR PIER CAP DEPTH.

**PEDESTAL SCHEDULE**

MARK	PEDESTAL SIZE		VERTICAL DOWELS	TIES	REMARKS
	WIDTH	LENGTH			
P1	1'-4"	1'-4"	(6) #6 DOWELS	#3 TIES @ 12" O.C.	SEE NOTE #1
P2	1'-6"	2'-0"	(6) #6 DOWELS	#3 TIES @ 12" O.C.	SEE NOTE #1
P3	2'-0"	2'-0"	(8) #6 DOWELS	#3 TIES @ 12" O.C.	SEE NOTE #1

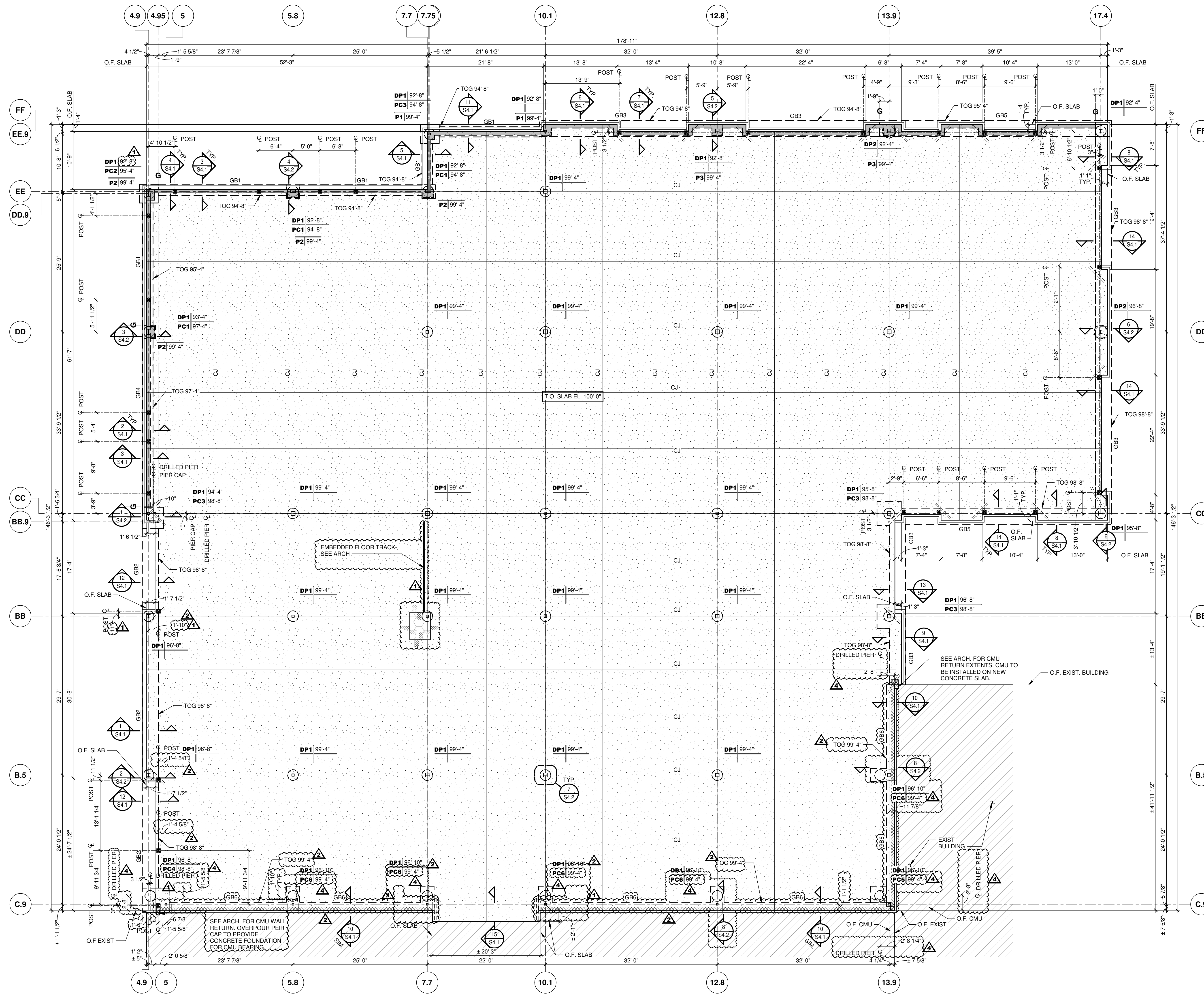
**PEDESTAL SCHEDULE NOTES:**  
1. SEE FOUNDATION PLAN FOR LOCATION AND ORIENTATION.

**Bentonville Public Library Expansion**  
 405 S Main Street  
 Bentonville, AR 72712  
 Project No. 2002007



Mark	Date	Description
1	12/15/2021	SCHEMATIC DESIGN PRICING
2	06/22/2022	DESIGN DEVELOPMENT PRICING
3	10/24/2022	90% CD PRICING
4	12/31/2022	PERMIT SET
5	01/06/2023	BID SET
6	03/24/2023	ASH-01
7	08/22/2023	ASH-07





**1 FOUNDATION PLAN**  
1/8" = 1'-0"

**LEGEND:**

- DRILLED PIER MARK (SEE DRILLED PIER SCHED.)
- TOP OF DRILLED PIER EL.
- TOP OF PIER CAP (SEE PIER CAP SCHED.)
- P1 100'-0" T.O. PEDESTAL ELEVATION
- PEDESTAL MARK (SEE PED. SCHED.)
- GB1 DENOTES GRADE BEAM (SEE GRADE BEAM SCHED.)
- TOG DENOTES TOP OF GRADE BEAM
- TOG DENOTES GRADE BEAM STEP MARK (SEE DTL. 9/S1.1)
- O.F. DENOTES OUTSIDE FACE
- ± DENOTES DIMENSION OR ELEVATION TO BE FIELD VERIFIED
- DENOTES LIMITS OF 4" SLAB (T.O. SLAB EL. 100'-0") SEE PLAN NOTES
- DENOTES LIMITS EXISTING SLAB
- DENOTES CONCRETE STEM WALL
- 100'-0" DENOTES TOP OF SLAB EL.
- DENOTES LOCATION OF POST (SEE DTL. 4/S1.1)
- DENOTES LOCATION OF RE-ENTRANT CORNER BARS (SEE CONC. GEN. NOTE #9)
- DENOTES SLAB ELEVATION CHANGE
- DENOTES LIMITS OF RECEIVED 4" CONCRETE SLAB. COORD. LOCATION & ELEV. W/ ARCH.
- DENOTES 8" CMU WALL

**PLAN NOTES:**

1. 4" S.O.G. CONSTRUCTION: 4" CONCRETE SLAB REINFORCED W/ ULTRA FIBER 500 ON 15 ML POLYETHYLENE FILM (COORD. W/ ARCH. SPECS.) OVER 4" CRUSHED STONE.
2. CENTER DRILLED PIERS AND PIER CAPS UNDER COLUMN, (U.N.O.).
3. ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 126'-15" AS ESTABLISHED BY EDG.
4. TOP OF GRADE BEAM ELEVATION 98'-8" TYP. (U.N.O.).
5. TOP OF PIER CAP ELEVATION 98'-8" TYP. (U.N.O.).
6. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
7. "CJ" DENOTES CONTROL OR CONSTRUCTION JOINT. (SEE DTL. 1/S1.1 & 2/S1.1)
8. COORDINATE LOCATION & LIMITS OF VENEER WITH ARCH. DWGS.
9. COORDINATE DOOR LOCATIONS WITH ARCH. DWGS.
10. SEE ARCH. PLUMBING DWGS. FOR EXACT LOCATIONS OF ALL FLOOR DRAINS, SLOPED & RECESSED SLABS.
11. COORDINATE LOCATIONS OF PLUMBING LINES W/ PLUMBER PRIOR TO POURING GRADE BEAMS & SLABS-ON-GRADE.
12. SEE DWG. S1.1 FOR GENERAL NOTES, SCHEDULES, & TYP. DETAILS.
13. SEE ARCH. FLOOR PATTERN PLANS FOR SLAB LOCATIONS WITH EXPOSED CONCRETE.
14. POST LOCATIONS SHOW ARE APPROXIMATE. SEE ARCH. FOR EXACT LOCATION.

**NOTES:**

1. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER IMMEDIATELY.
2. CONTRACTOR SHALL FIELD VERIFY ALL ELEVATIONS AND DIMENSIONS PRIOR TO SETTING CONCRETE REINFORCEMENT AND FORM WORK.
3. BEGINNING OF STEEL FABRICATION AND PLACEMENT OF CONCRETE CONSTITUTES CONTRACTOR ACCEPTANCE OF EXISTING CONDITIONS.

**SLAB-ON-GRADE JOINT NOTE:**  
CONTROL JOINTS AT POLISHED CONCRETE TO BE 14'-0" O.C. MAXIMUM. CONTROL JOINTS AT OTHER AREAS TO BE 16'-0" O.C. MAXIMUM.

Architecture and Interiors  
**MSRDesign**  
 510 Marquette Avenue South, Suite 200  
 Minneapolis, MN 55402 | 612.375.0336

Associate Architect  
**Hight Jackson Associates PA**  
 5201 W Village Pkwy #300  
 Rogers, AR 72758 | 479.464.4965

Civil Engineer  
**E | D | G**  
 216 West Birch Street  
 Rogers, AR 72758 | 501.378.0200

Landscape Architect  
**TEN x TEN**  
 575 SE 9th St #210  
 Minneapolis, MN 55414 | 612.440.8369

Structural Engineer  
**Tatum-Smith-Welcher Structural Engineers**  
 3100 S Market St #202  
 Rogers, AR 72758 | 479.621.6128

MEP Engineer  
**HP Engineering, Inc.**  
 5504 W. Pinnacle Point Dr. Suite 200  
 Rogers, AR 72758 | 479.899.6370

**Bentonville Public Library Expansion**  
 405 S Main Street  
 Bentonville, AR 72712

Project No. 2023027

STATE OF ARKANSAS  
 LICENSED PROFESSIONAL ENGINEER  
 Tatum Smith Welcher  
 No. 19779  
 08/22/2023

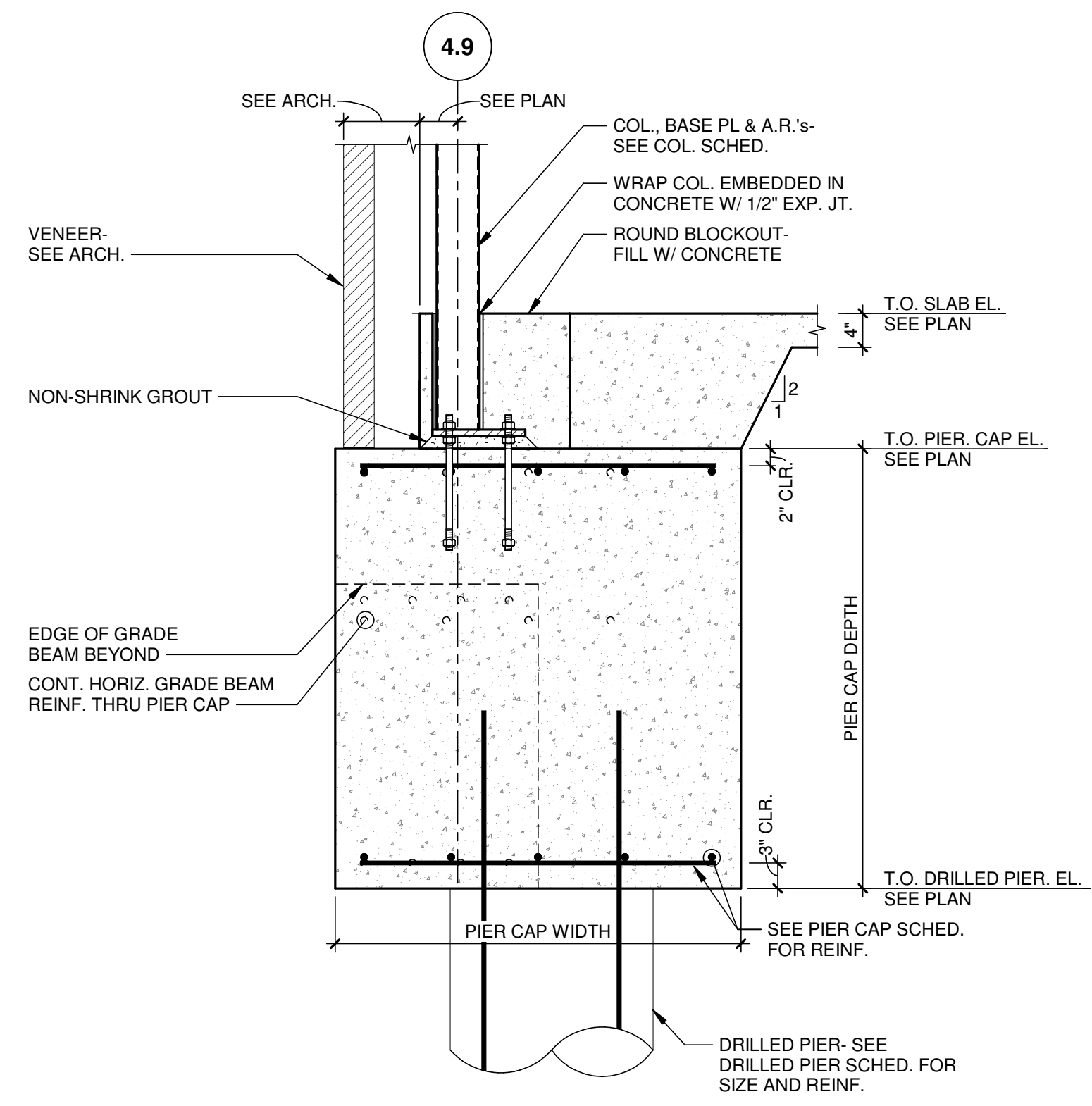
STATE OF ARKANSAS  
 LICENSED PROFESSIONAL ENGINEER  
 Hight Jackson Associates  
 No. 218  
 08/22/2023

**BID SET**

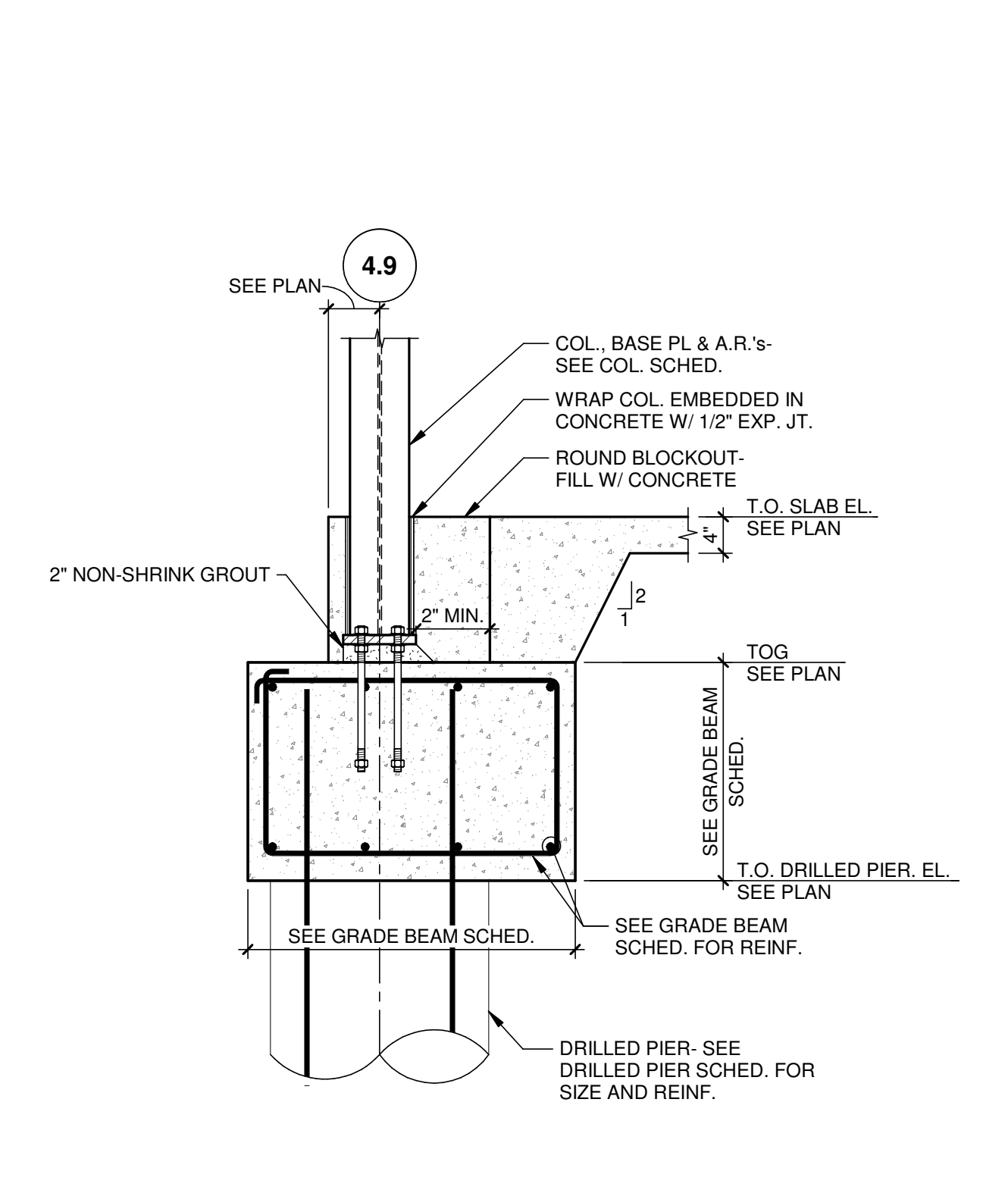
**ISSUE / REVISION**

Mark	Date	Description
	12/15/2021	SCHEMATIC DESIGN PRICING
	06/22/2022	DESIGN DEVELOPMENT PRICING
	10/24/2022	60% CD PRICING
	12/31/2022	PERMIT SET
	01/06/2023	BID SET
1	01/16/2023	BID SET - ADDENDUM 1
2	03/24/2023	AS-01
4	08/22/2023	AS-07

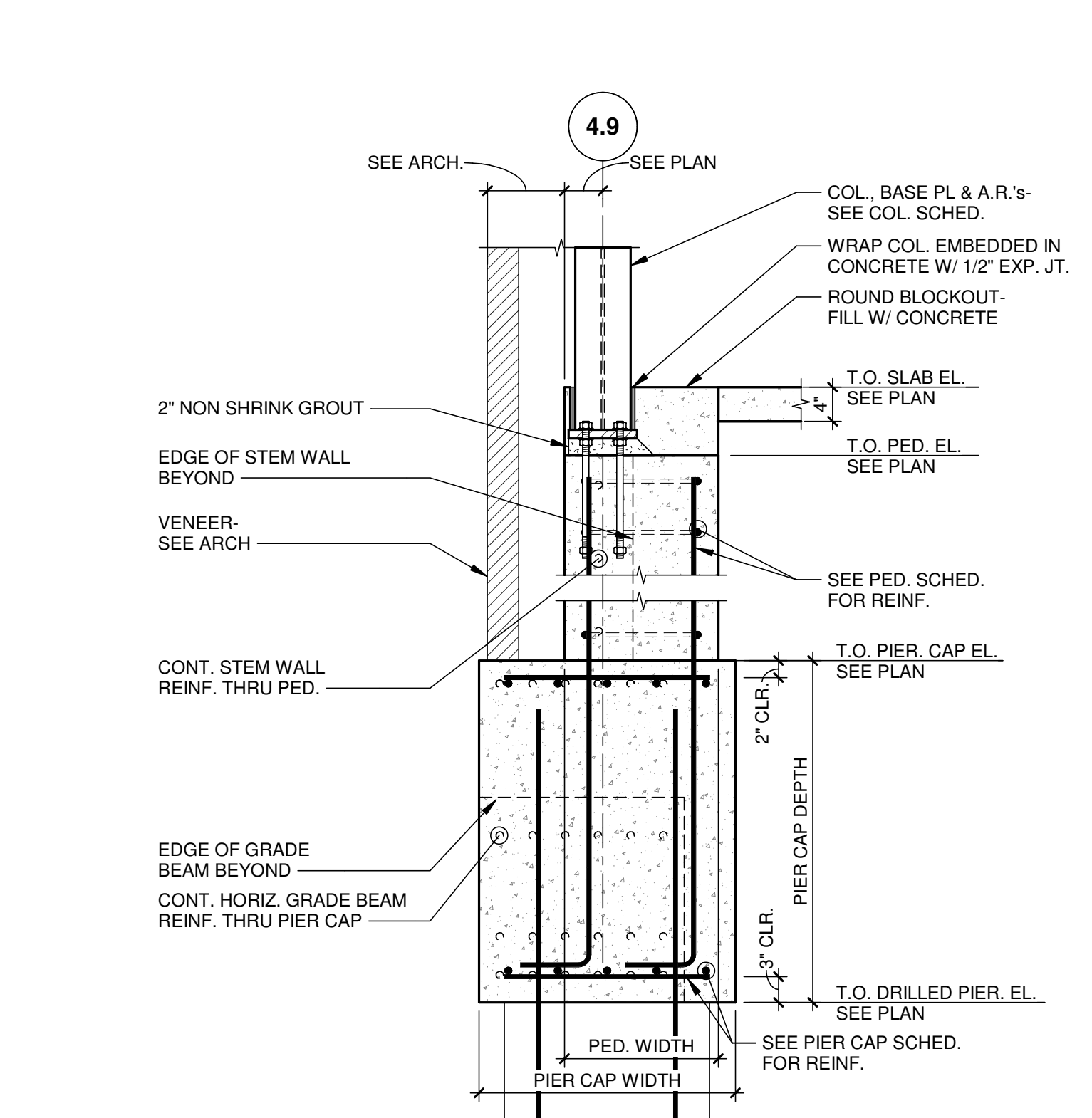




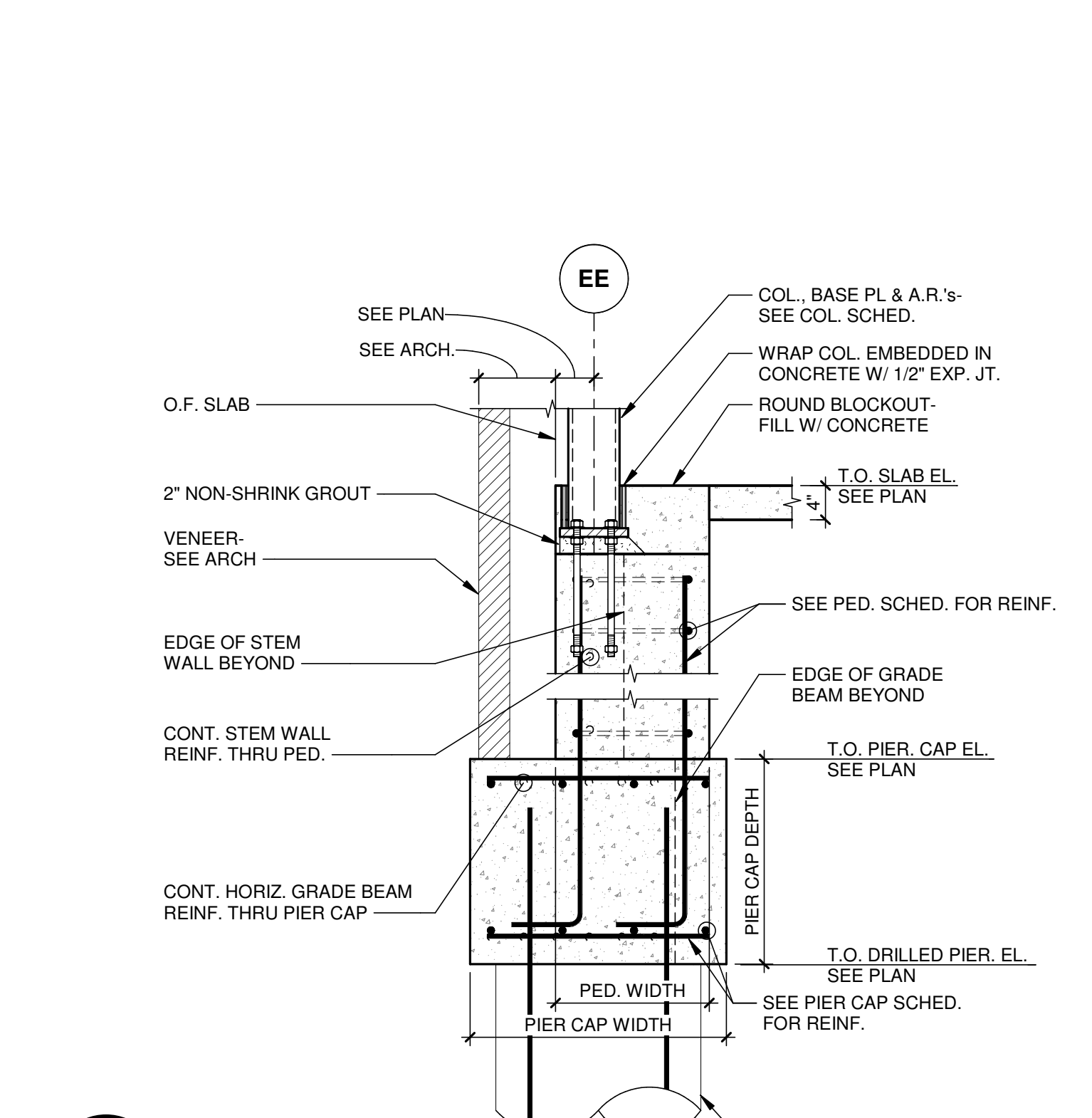
**1 SECTION**  
3/4" = 1'-0"



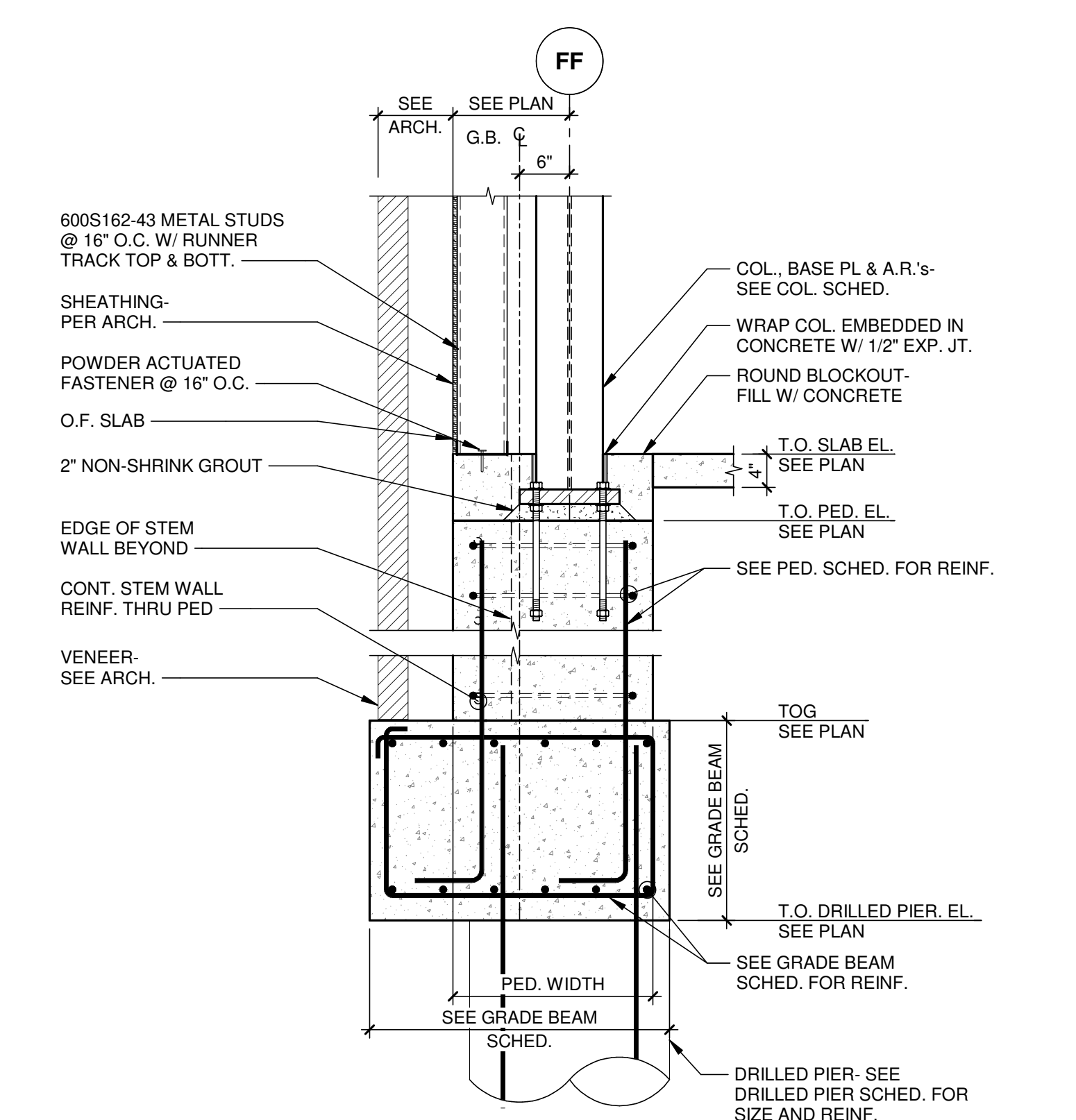
**2 SECTION**  
3/4" = 1'-0"



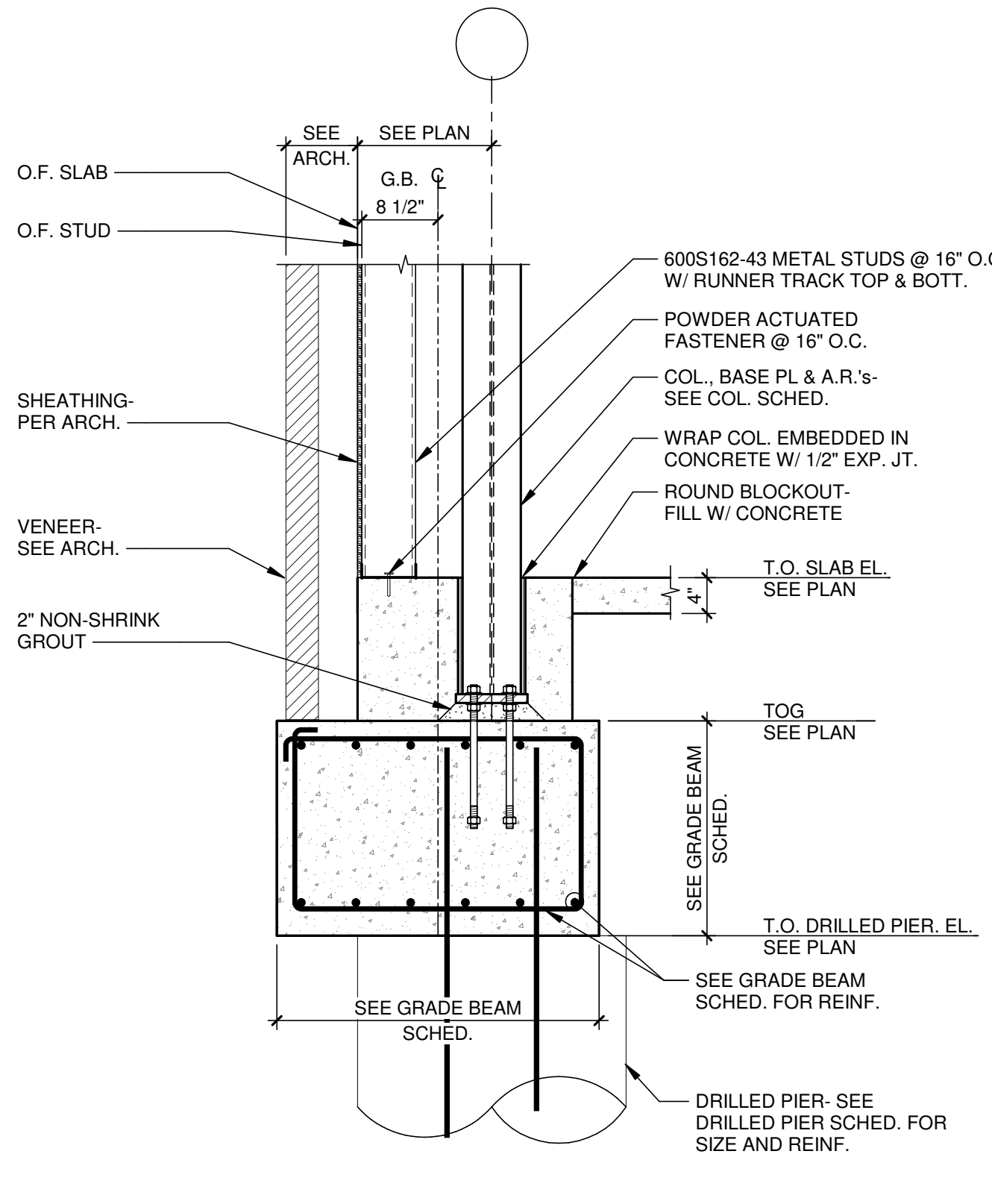
**3 SECTION**  
3/4" = 1'-0"



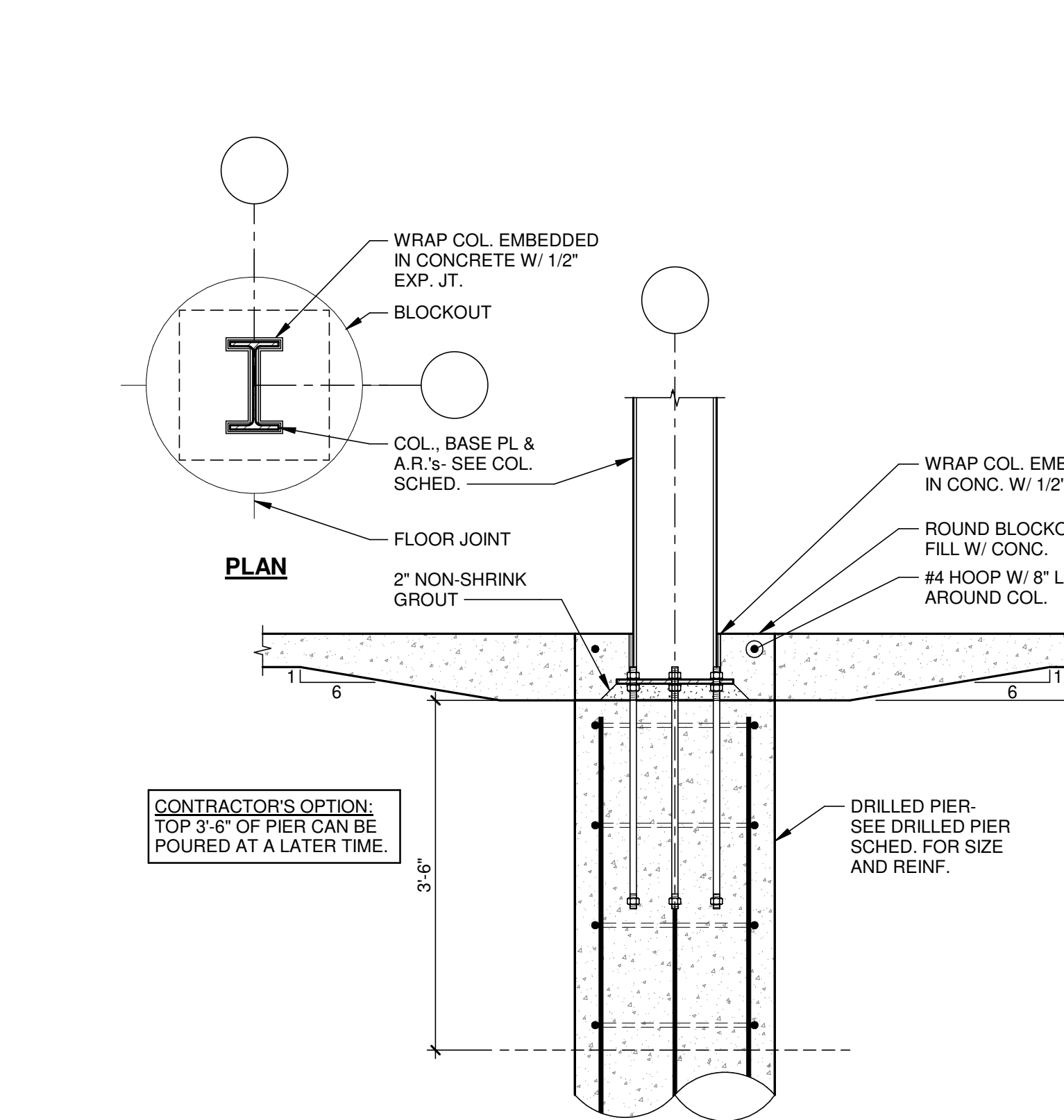
**4 SECTION**  
3/4" = 1'-0"



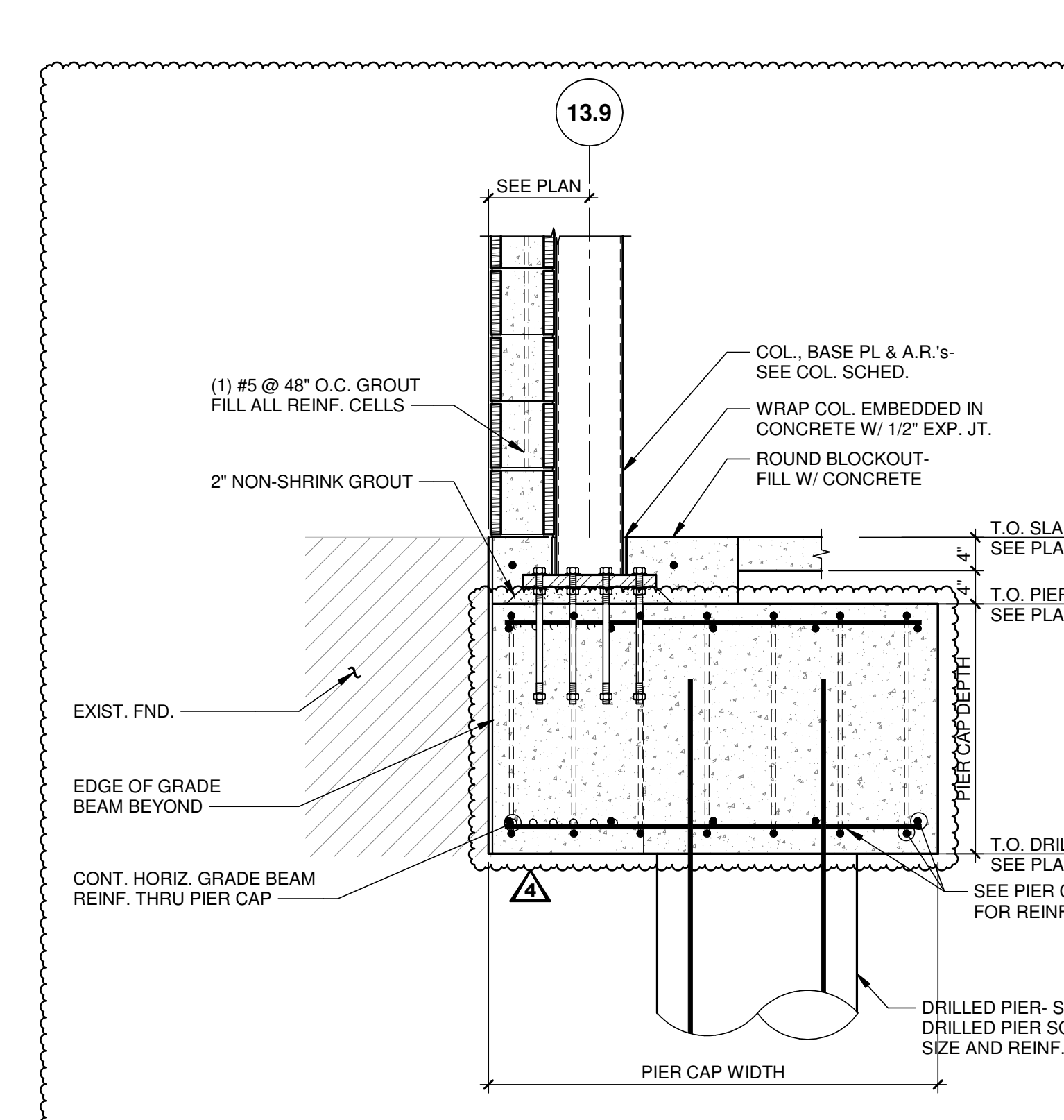
**5 SECTION**  
3/4" = 1'-0"



**6 SECTION**  
3/4" = 1'-0"



**7 SECTION**  
3/4" = 1'-0"



**8 SECTION**  
3/4" = 1'-0"

Architecture and Interiors  
**MSRDesign**  
 510 Marquette Avenue South, Suite 200  
 Minneapolis, MN 55402 | 612.375.0336

Associate Architect  
**Hight Jackson Associates PA**  
 5201 W Village Plaza #300  
 Rogers, AR 72758 | 479.464.4965

Civil Engineer  
**E | D | G**  
 216 West Birch Street  
 Rogers, AR 72758 | 501.378.0200

Landscape Architect  
**TEN x TEN**  
 575 SE 9th St #210  
 Minneapolis, MN 55414 | 612.440.8369

Structural Engineer  
**Tatum-Smith-Welcher Structural Engineers**  
 3100 S Market St #202  
 Rogers, AR 72758 | 479.621.6128

MEP Engineer  
**HP Engineering, Inc.**  
 5504 W. Pinnacle Point Dr. Suite 200  
 Rogers, AR 72758 | 479.899.6370

**Bentonville Public Library Expansion**  
 405 S Main Street  
 Bentonville, AR 72712  
 Project No. 2021037



**BID SET**

**ISSUE / REVISION**

Mark	Date	Description
1	12/10/2021	SCHEMATIC DESIGN PRICING
2	06/22/2022	DESIGN DEVELOPMENT PRICING
3	10/24/2022	60% CD PRICING
4	12/31/2022	PERMIT SET
5	01/06/2023	BID SET
6	03/24/2023	ASH-01
7	08/22/2023	ASH-07

FOUNDATION DETAILS

# S4.2

Drawing: 001-000001-Rev. 08/22/2023. Author: Tatum Smith-Welcher. Date: 08/22/2023.