

ABBREVIATION LIST

(E) or EXIST	Existing	FD	Footing Dowel	OH	Opposite Hand
/	Per	FF	Finished Floor	OPNG	Opening
@	At	FN	Finish(ed)	OPP	Opposite
AB	Anchor Bolt	FLG	Flange	OVS	Oversized
ACI	American Concrete Institute	FLR	Floor	CWS	One-Way Slab
ADDNL	Additional	FND	Foundation	PAF	Powder Actuated Fastener
AESS	Architecturally Exposed Structural Steel	FO	Face of	PCA	Portland Cement Association
AFF	Above Finish Floor	FP	Full Penetration or Fire Proofing	PD	Pier Dowel
ALT	Alternate	FRAM	Framing	PEN	Penetration
APPROX	Approximate	FS	Far Side	PERP	Perpendicular
ARCH	Architect or Architectural	FT	Foot or Feet	PL	Plate (Steel)
BT or BO	Bottom of	FTG	Footing	PLF	Pounds Per Lineal Foot
BAL	Balance	FV	Field Verify	PREFAB	Prefabricated
BG	Backgrade	GA	Gage or Gauge	PRELIM	Preliminary
BL	Brick Ledger	GALV	Galvanized	PSF	Pounds Per Square Foot
BLDG	Building	GC	General Contractor	PSI	Pounds Per Square Inch
BM	Beam	GR	Grade or Grind	QTY	Quantity
BOS	Bottom of Steel	GR MB	Grade Beam	R	Radius or Rain Load
BOT or B	Bottom	H	Slit Lateral Load	RAD	Radius
BRG	Bearing	HAS or HDAS	Headed Anchor Stud	RC	Reinforced Concrete
BSMT	Basement	HD	Headed or Holddown	RE: or REF	Refer to (Reference)
BTWN	Between	HDAR	Headed Anchor Rod	REINFORCING(d)(ment)	Reinforcing(d)(ment)
CC	Center to Center	HDS	Hot Dipped Galvanized	REQD	Required
CF	Cold Formed	HK	Hook	REQT(s)	Requirement(s)
CG	Center of Gravity	HORIZ	Horizontal	RET	Return
CIP	Cast-In-Place	HT	Height	RDO	Random Opening
CJ	Control Joint	HVAC	Heating/Ventilating and A/C	ROF	Random Oriented Fiber
CJP	Complete Joint Penetration	I.F.	Inside Face	S	South
CL	Centerline	IN	Inch	SC	Slip Critical
CLG	Ceiling	INT	Interior	SOCHED	Schedule
CLMS	Ceiling/Light/Mechanical/Superimposed Load	IS	Inside Diameter	SECT	Section
CLR	Clear	JST	Joist	SIM	Similar
CMU	Concrete Masonry Unit	JT	Joint	SL	Snow Load
COL	Column	k	Kip	SLH	Short Leg Horizontal
CONC	Concrete	L	Length or Live Load	SLV	Short Leg Vertical
CONN	Connection	LB(S)	Pound(s)	SOG	Slab on Grade
CONST	Construction	LCE	Compression Embedment	SP	Space
CONT	Continue or Continuous	LCS	Compression Lap Splice	SP @	Space at
CONTR	Contractor	LDH	Hook Development Length	SPECS	Specifications
COORD	Coordinate	LG	Length	SPRT	Support
CJS	Construction Joint	LL	Live Load	SS	Stainless Steel
CTR(D)	Center(d)	LLH	Long Leg Horizontal	STD	Standard
D or DL	Dead Load	LLV	Long Leg Vertical	STIFF	Stiffener
DAS	Deformed Anchor Stud	LOC(s)	Location(s) or Locate	STL	Steel
DBL	Double	LONG	Longitudinal	STR	Structural
DFS	Deferred Submittal	Lr	Roof Live Load	STR	Shearwall
DIA OR Ø	Diameter	LT	Light	SYM	Symmetrical
DIAG	Diagonal	LTE	Tension Embedment	T	Top or Thermal Load
DIM	Dimension	LTS	Tension Lap Splice Length	T&B	Top and Bottom
DN	Down	MACH	Machine	Ti or T.O.	Top of
DPL	Drilled Pier or Deep	MACH RM	Machine Room	THK	Thick or Thickness
DTL(s)	Detail(s)	MAS	Masonry	TL	Total Load
DWG(s)	Drawing(s)	MATL	Material	TOC	Top of Concrete
DWL(s)	Dowels(s)	MEP	Mech/Elect/Plumb	TOF	Top of Footing
E	Earthquake Load	MIN	Minimum	TOM	Top of Masonry
E-W	East-West	MISC	Miscellaneous	TOP	Topping
EA	Each	MLS	Masonry Lap Splice	TOS	Top of Steel
EC	Epoxy Coated	MNR	Manufacturer	TOW	Top of Wall
EE	Each End	MO	Masonry Opening	TRANS	Transverse
EF	Each Face	MSE	Mechanically Stabilized Earth	TYP	Typical
EJ	Expansion Joint	MTL	Metal	ULT	Ultimate
ELEV	Elevator	N	North	UNO	Unless Noted Otherwise
EMBED	Embedded	N-S	North-South	Vasd	Service Level/Nominal Design Wind Speed
ENGR	Engineer	NIC	Not in Contact	VERT	Vertical
EOR	Engineer-of-Record	NI	Non-Metallic	VFI	Verify in Field
EQ	Equal	NO OR #	Number	W	Ultimate Design Wind Speed
EQ SP	Equally Spaced	NOM	Nominal	W	Wind Load
EQUIP	Equipment	NS	Non-Shrink or Near Side	W'	With
ES	Each Side	NTS	Not to Scale	W/O	Without
EW	Each Way	NWC	Normal Weight Concrete	WD	Width or Wood
EXP ANCH	Expansion Anchor	O.F.	Outside Face	WF	Wide Flange
EXT	Exterior	OAE	Or Approved Equivalent	WI	Wind on-ice Load
Fa	Flood Load	OC	On Center	W/P	Working Point or Waterproofing
FAB	Fabricate	OD	Outside Diameter	WPS	Welding Procedure Specification
				WTT	Weight
				WWR	Welded Wire Reinforcing
				Wwt	Width x Height

GENERAL NOTES

1) GENERAL:
 1A) ENGINEER. REFERENCES ON THE STRUCTURAL DRAWINGS TO 'ENGINEER' MEAN THE STRUCTURAL ENGINEER OF RECORD. OTHER ENTITIES ARE SPECIFICALLY NOTED AS 'CONTRACTOR'S ENGINEER', 'MECHANICAL ENGINEER', ETC.
 1B) THESE NOTES SUPPLEMENT THE SPECIFICATIONS, WHICH SHALL BE REFERENCED FOR ADDITIONAL REQUIREMENTS.
 1C) UNDERGROUND UTILITIES: LOCATE EXISTING UTILITIES AND NOTIFY ARCHITECT OF EXISTING UTILITIES OR SUBGRADE CONDITIONS WHICH INTERFERE WITH UTILITIES.
 1D) STRUCTURAL ELEMENTS ARE CENTERED ON GRID LINES AND GRID LINE INTERSECTIONS UNLESS DIMENSIONED OTHERWISE.

2) USE OF DRAWINGS:
 2A) DO NOT SCALE DRAWINGS.
 2B) DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
 2C) DETAILS NOTED TYPICALLY APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.
 2D) WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES AND SPECIFICATIONS:
 - CONTACT THE ARCHITECT PRIOR TO PROCEEDING WITH CONSTRUCTION
 - THE MORE STRINGENT REQUIREMENTS SHALL GOVERN FOR BIDDING/ PRICING

3) COORDINATION:
 3A) STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO SHOP DRAWINGS AND WORK.
 3B) COORDINATE DIMENSIONS OF ALL OPENINGS, BLOCKOUTS, DEPRESSIONS, ETC., WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTALS.
 3C) SEE ARCHITECTURAL PLANS FOR INTERIOR PARTITIONS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE IN SUCH A WAY SO AS TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 OF THE FLOOR AND ROOF FRAMING. DO NOT MAKE RIGID VERTICAL AND HORIZONTAL CONNECTIONS TO THE PRIMARY STRUCTURE IN THE PLANE OF THE PARTITION.

4) SUBMITTALS AND SUBSTITUTIONS:
 4A) SUBMITTALS: REFER TO SPECIFICATIONS FOR DETAILED REQUIREMENTS.
 - IF THE CONTRACTOR REQUESTS A CHANGE FROM THE STRUCTURAL DRAWINGS, IT SHALL BE APPROVED BY THE ARCHITECT AND DESIGNED BY MARTIN/MARTIN, INC. PRIOR TO SUBMITTING SHOP DRAWINGS. VARIATION SHALL BE INDICATED ON THE SHOP DRAWINGS. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN, INC. FOR MAKING THE CHANGE.
 - CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE IN SUBMITTALS
 - ALL SHOP DRAWINGS SHALL REFERENCE THE STRUCTURAL DRAWING NUMBER AND DETAIL USED TO PREPARE THE SUBMITTAL
 - SUBMIT A STATEMENT OF RESPONSIBILITY FOR CONSTRUCTION OF THE LATERAL LOAD RESISTING SYSTEM IDENTIFIED IN THE DESIGN CRITERIA IN ACCORDANCE WITH IBC 2021 SECTION 1704
 4B) SUBSTITUTIONS: ARCHITECT'S APPROVAL SHALL BE SECURED FOR ALL SUBSTITUTIONS
 4C) NONCONFORMANCE: NOTIFY ARCHITECT OF CONDITIONS NOT CONSTRUCTED PER THE CONTRACT DOCUMENTS PRIOR TO PROCEEDING WITH CORRECTIVE WORK. SUBMIT PROPOSED REPAIR TO THE ARCHITECT FOR ACCEPTANCE. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN, INC. FOR DESIGNING THE REPAIR.
 4D) ALL SHOP DRAWINGS SHALL BE SUBMITTED IN ELECTRONIC FORMAT ONLY.

5) TEMPORARY CONDITIONS, CONSTRUCTION ENGINEERING, AND OSHA STANDARDS:
 5A) THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION AND ONLY FOR LOADS ANTICIPATED DURING THE STRUCTURE'S SERVICE LIFE.
 5B) THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. REFER TO 'LATERAL LOAD RESISTING SYSTEM DESCRIPTION' IN DESIGN CRITERIA FOR ADDITIONAL INFORMATION. CONTRACTOR SHALL PROVIDE ALL REQUIRED ENGINEERING AND OTHER MEASURES TO ACHIEVE THE MEANS, METHODS, AND SEQUENCES OF WORK WHICH MAY INCLUDE, BUT IS NOT LIMITED TO:
 - LAYOUT
 - DESIGN FOR FORMWORK, SHORING, AND RESHORING
 - DESIGN OF CONCRETE MIXES
 - ERECTION PROCEDURES WHICH ADDRESS STABILITY OF THE FRAME DURING CONSTRUCTION
 - WELD PROCEDURES
 - DESIGN OF TEMPORARY BRACING OF WALLS FOR WIND, SEISMIC, OR SOIL LOADS
 - SURVEYING TO VERIFY CONSTRUCTION TOLERANCES
 - EVALUATION OF TEMPORARY CONSTRUCTION LOADS ON STRUCTURE DUE TO EQUIPMENT AND MATERIALS
 - STRUCTURAL ENGINEERING TO RESIST ANY OTHER LOADS NOT IDENTIFIED ON DESIGN DRAWINGS
 5C) CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
 5D) FOUNDATION WALLS SHALL NOT BE BACKFILLED UNTIL THE SLABS-ON-GRADE AND UPPER SLABS ARE IN-PLACE AND REACH FULL STRENGTH UNLESS ADEQUATE BRACING IS PROVIDED. USE ONLY HAND OPERATED TOOLS FOR COMPACTION ADJACENT TO FOUNDATION WALLS AND GRADE BEAMS. GRADE BEAMS SHALL BE BACKFILLED EVENLY ON BOTH SIDES.
 5E) NOTHING SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSTRUED AS ELIMINATING THE NEED FOR THE CONTRACTOR TO COMPLY WITH ALL OSHA REQUIREMENTS. WHERE THE STRUCTURAL DRAWINGS APPEAR TO CONFLICT WITH OSHA REQUIREMENTS, THE STRUCTURAL DRAWINGS REPRESENT FINAL CONDITIONS ONLY.
 - THE CONTRACTOR SHALL ADD ALL ERECTION FRAMING NECESSARY TO COMPLY WITH OSHA.
 - THE CONTRACTOR SHALL ADD ALL NECESSARY BOLTS, ANCHOR BOLTS, PLATES, STIFFENER PLATES, STABILIZER PLATES, BRIDGING, BRACING, BEARING SEATS, COLUMN SPLICES, ETC., AS WELL AS CLOSURES FOR OPENINGS. IN ADDITION, FIELD WELD ANYTHING THAT MAY BE CONSIDERED A TRIP HAZARD, SUCH AS SHEAR STUDS, AFTER PROTECTIVE DECKING IS INSTALLED.
 - WASHERS OR RINGS MAY BE WELDED TO COLUMNS TO PROVIDE FOR SAFETY CABLES. HOLES IN COLUMNS FOR SAFETY CABLES SHALL BE SHOP INSTALLED AND SHALL BE INDICATED ON SHOP DRAWINGS. ADJUST COLUMN SPICE LOCATIONS OR ADD COLUMN SPLICES AS NECESSARY TO COMPLY WITH OSHA REQUIREMENTS. SUBMIT PROPOSED LOADINGS.

DESIGN CRITERIA

1) CODES AND STANDARDS:
 1A) GENERAL DESIGN - INTERNATIONAL BUILDING CODE 2021
 1B) LOADS
 - ASCE 7-16 'MINIMUM DESIGN LOAD FOR BUILDINGS AND OTHER STRUCTURES' WHERE INDICATED ON DRAWINGS INDIVIDUAL UNFACTORED LOAD COMPONENTS (D, L, Lr, R, S, H, F, Fa, E, W, Wt) ARE AS DEFINED AND DETERMINED BY THE BUILDING CODES AND STANDARDS. INDICATED LOAD COMPONENTS SHALL BE COMBINED USING THE LOAD COMBINATIONS OF THE BUILDING CODE FOR SPECIALTY DESIGN BY OTHERS.
 1C) CONCRETE
 - ACI 308-16 'SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS'
 - ACI 318-19 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE'
 1D) MASONRY
 - TMS 402-2016 'BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES'
 - TMS 602-16 'SPECIFICATION FOR MASONRY STRUCTURES'
 1E) STEEL
 - ANSII/AISC 360-16 'SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS' LOAD AND RESISTANCE FACTOR DESIGN

2) SEISMIC LOADS
 - SEISMIC DESIGN CATEGORY = B
 - RISK CATEGORY = III
 - EARTHQUAKE IMPORTANCE FACTOR, $I_e = 1.25$
 - MAPPED SPECTRAL RESPONSE ACCELERATION, $S_s = 15.30 \text{ %g}$
 - MAPPED SPECTRAL RESPONSE ACCELERATION, $S_1 = 9.90 \text{ %g}$
 - DESIGN SPECTRAL RESPONSE COEFFICIENT, $S_{ds} = 0.133$
 - DESIGN SPECTRAL RESPONSE COEFFICIENT, $S_{d1} = 0.089$
 - SOIL SITE CLASS = C
 - BASIC STRUCTURAL SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
 - STRUCTURAL SEISMIC LATERAL SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
 - RESPONSE MODIFICATION FACTOR, $R = 3$
 - SEISMIC RESPONSE COEFFICIENT, $C_s = 0.0553$
 - SYSTEM OVERSTRENGTH FACTOR, $\Omega = 3$
 - DEFLECTION AMPLIFICATION FACTOR, $C_d = 3$
 - BUILDING DESIGN BASE SHEAR EAST-WEST DIRECTION = 1850 K
 - BUILDING DESIGN BASE SHEAR NORTH-SOUTH DIRECTION = 1850 K
 - SEISMIC ANALYSIS TYPE: EQUIVALENT LATERAL-FORCE PROCEDURE

3) WIND LOADS
 - RISK CATEGORY = III
 - BASIC ULTIMATE WIND SPEED, $V_{ult} = 115 \text{ mph}$
 - BASIC NOMINAL WIND SPEED, $V_{asd} = 89 \text{ mph}$
 - EXPOSURE CATEGORY = C
 - INTERNAL PRESSURE COEFFICIENT, $G_{p1} = +/- 0.18$
 - TOPOGRAPHIC FACTOR, $K_{zt} = 1.0$
 - GROUND ELEVATION FACTOR, $K_e = 0.9558$

4) DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING AND ELEMENTS DESIGNED BY THE CONTRACTOR
 ALL LISTED COMPONENT AND CLADDING WIND PRESSURES ARE INCLUDED FOR REFERENCE ONLY. FINAL CALCULATIONS SHALL BE COMPLETED BY CONTRACTOR

4B) PRESSURES LISTED BELOW ARE ULTIMATE

4C) SEE WALL CORNER AND SPECIAL ROOF ZONES DIAGRAM

4D) COMPONENT AND CLADDING SURFACE PRESSURES (PSF)

WALLS PRESSURES	10 SF	100 SF	200 SF	500 SF	500 SF
WALLS AREA					
WALLS INTERIOR NEG (ZONE 4)	-38.8	-33.6	-32	-29.9	
WALLS CORNER NEG (ZONE 5)	-7.8	-7.3	-4.1	-2.9	
WALLS POSITIVE (ZONE 4 & 5)	35.9	30.6	29	26.9	

ROOF PRESSURES

ROOF AREA	10 SF	100 SF	200 SF	500 SF	500 SF
ROOF INTERIOR NEG (ZONE 1)	-2.4	-4.7	-44.6	-39.2	
ROOF INTERIOR NEG (ZONE 1)	-35.9	-35.9	-30.9	-24.3	
ROOF NEGATIVE (ZONE 2)	-82.3	-64.8	-59.5	-52.5	
EAVES, RAKES, RIDGES					
ROOF CORNERS NEG (ZONE 3)	-82.3	-64.8	-59.5	-26.9	
ROOF POSITIVE (ZONE 1)	16	16	16	16	
ROOF POSITIVE (ZONE 2 & 3)	35.9	30.6	29.0	26.9	
ROOF NEGATIVE OVERHANG (ZONE 1&1)	-56.4	-53.1	-44.5	-33.2	
ROOF NEGATIVE OVERHANG (ZONE 2)	-76.4	-52.9	-45.9	-36.5	
ROOF NEGATIVE OVERHANG (ZONE 3)	-72.4	-52.9	-45.9	-36.5	

PARAPET PRESSURES

PARAPET OP = 33.5 psf	10 SF	100 SF	200 SF	500 SF	500 SF
SOLID PARAPET PRESSURE					
PARAPET CASE A: ZONE 2:	107.2	84.1	77.2	68	
ZONE 3:	107.2	84.1	77.2	68	
PARAPET CASE B: INTERIOR ZONE:	-63.3	-52.7	-49.5	-45.2	
CORNER ZONE:	-72.4	-56.4	-51.6	-45.2	
PARAPET CASE A = PRESSURE TOWARDS BUILDING (POS)					
PARAPET CASE B = PRESSURE AWAY FROM BLDG (NEG)					
PARAPET CAP UP/FT PRESSURES					
ZONE 2:	-83.7	PSF			
ZONE 3:	-83.7	PSF			

5) LATERAL LOAD RESISTING SYSTEM DESCRIPTION:
 - CONCRETE SLAB ON METAL DECK DIAPHRAGM SPANNING TO A COMBINATION OF CONCRETE SHEAR WALLS, STEEL BRACED FRAMES AND STEEL MOMENT FRAMES WHICH TRANSFER LATERAL FORCES INTO SOIL THROUGH DRILLED PIERS

6) GRAVITY LOADS
 6A) SEE LOAD MAP SHEET FOR SUPERIMPOSED DEAD LOAD AND LIVE LOADS USED IN DESIGN

6B) DRIFTING, SLIDING AND UNBALANCED SNOW
 - GROUND SNOW LOAD = 15.0 psf
 - SNOW EXPOSURE FACTOR, $C_e = 1.0$
 - SNOW LOAD IMPORTANCE FACTOR, $I_s = 1.1$
 - THERMAL FACTOR, $C_t = 1.0$
 - RAIN ON SNOW LOAD = 5.0 psf
 - FLAT ROOF SNOW LOAD = 16.5 psf
 - UNIFORM ROOF SNOW PLUS RAIN LOAD = 16.6 psf

7) RAIN LOADS:
 7A) DESIGN RAIN INTENSITY = 3.6 INCHES PER HOUR
 7B) DESIGN RAIN ROOF PRESSURE = 20.8 psf

8) FIRE RESISTANCE, CONDITIONS OF RESTRAINT:
 8A) FOR DETERMINING FIRE-RESISTANCE RATINGS PER IBC SECTION 703, ALL STEEL FLOOR AND ROOF CONSTRUCTION SUPPORTING CONCRETE SLABS IS ASSUMED TO BE RESTRAINED. ALL OTHER STEEL FRAMING IS ASSUMED TO BE UNRESTRAINED.

STRUCTURAL DRAWING LIST

SHEET NUMBER	NOTES	SHEET TITLE
S001	NOTES	
S002	NOTES	
S003	QUALITY ASSURANCE	
S004	LOAD MAPS	
S100	OVERALL PLAN - UNDERGROUND PARKING	
S100A	UNDERGROUND PARKING - AREA A	
S100B	UNDERGROUND PARKING - AREA B	
S101	OVERALL PLAN - 1ST FLOOR	
S101A	STRUCTURAL PLAN - 1ST FLOOR - AREA A	
S101B	STRUCTURAL PLAN - 1ST FLOOR - AREA B	
S102	OVERALL PLAN - 2ND FLOOR	
S102A	STRUCTURAL PLAN - 2ND FLOOR - AREA A	
S102B	STRUCTURAL PLAN - 2ND FLOOR - AREA B	
S102C	GALLERY SEATING	
S102D	MAIN STAIR	
S103	OVERALL PLAN - 3RD FLOOR	
S103A	STRUCTURAL PLAN - 3RD FLOOR - AREA A	
S104	OVERALL PLAN - 4TH FLOOR	
S104A	STRUCTURAL PLAN - 4TH FLOOR - AREA A	
S105	OVERALL PLAN - 5TH FLOOR	
S105A	STRUCTURAL PLAN - 5TH FLOOR - AREA A	
S106	SLAB REINFORCING - 1ST AND 2ND FLOORS	
S107	SLAB REINFORCING - 3RD AND 4TH FLOORS	
S200	GRAPHICAL COLUMN SCHEDULE	
S201	LOAD TRUSS ELEVATIONS	
S202	STEEL TRUSS ELEVATIONS	
S203	STEEL TRUSS ELEVATIONS	
S204	CORE WALL MAT FOUNDATION PLANS	
S207	STAIR CORE 1 WALL ELEVATIONS	
S208	ELEVATOR CORE 1 WALL ELEVATIONS	
S209	STAIR CORE 2 WALL ELEVATIONS	
S210	ELEVATOR CORE 2 WALL ELEVATIONS	
S211	STAIR CORE 3 WALL ELEVATIONS	
S212	ELEVATOR CORE 3 WALL ELEVATIONS	
S213	TYPICAL WALL ELEVATION	
S218	BRACED FRAME ELEVATIONS	
S219	BRACED FRAME ELEVATIONS	
S220	BRACED FRAME ELEVATIONS	
S221	EXTERIOR WALL ELEVATIONS	
S222	EXTERIOR WALL ELEVATIONS	
S223	EXTERIOR WALL ELEVATIONS	
S300	DRILLED PIER DETAILS	
S301	FOUNDATION WALL DETAILS	
S302	TYPICAL CONCRETE CORE DETAILS	
S310	TYPICAL SOG DETAILS	
S380	CONCRETE SUPPORTING STEEL	
S381	CONCRETE SUPPORTING STEEL	
S390	TYPICAL CONNECTION DETAILS	
S400	MASONRY PARTITION DETAILS	
S500	TYPICAL CONNECTION DETAILS	
S501	TYPICAL CONNECTION DETAILS	
S502	TYPICAL STEEL DETAILS	
S503	BRACED FRAME DETAILS	
S504	TRUSS DETAILS	
S505	TRUSS DETAILS	
S507	TYPICAL STEEL DETAILS	
S508	WF BEAM STEP DETAILS	
S509	STEEL DETAILS	
S510	STEEL DETAILS	
S531	TYPICAL SLAB ON METAL DECK DETAILS	
S540	EXTERIOR WALL DETAILS	
S541	EXTERIOR WALL DETAILS	
S542	EXTERIOR WALL DETAILS	
S543	EXTERIOR WALL DETAILS	

DEFERRED SUBMITTALS

1) GENERAL:
 1A) THE FOLLOWING PORTIONS OF THE STRUCTURAL DESIGN WILL NOT BE SUBMITTED AT THE TIME OF PERMIT APPLICATION. WHEN RECEIVED AND REVIEWED, THESE DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR:
 - METAL STAIRS
 - PRECAST TERRAZZO STAIR TREADS
 - FENCING
 - CURTAIN WALL/STREETFRONT SYSTEMS
 - ARCHITECTURAL METAL CLADDING PANEL
 - LIGHT GAGE METAL STUDS (NON LOAD-BEARING, EXTERIOR AND ATYPICAL INTERIOR STUDS REQUIRING ENGINEERING DESIGN)
 - METAL RAILINGS
 - TEMPORARY EXCAVATION SHORING
 - SKYLIGHTS
 - ANCHORAGE, BRACING AND ATTACHMENT OF REQUIRED ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, FIRE SPRINKLER, MEDICAL EQUIPMENT, AND OTHER EQUIPMENT AND SYSTEMS
 - PRECAST WALL PANELS AND CONNECTIONS TO PRIMARY STRUCTURE
 - HELICAL PILES
 - MSE WALLS
 - STEEL TRUSS CONNECTIONS
 - STEEL CONNECTIONS THAT EXCEED CAPACITY AND GEOMETRY SHOWN ON STRUCTURAL DRAWINGS

1B) CONNECTION OF DEFERRED SUBMITTAL ITEMS TO PRIMARY STRUCTURE BY DEFERRED SUBMITTAL SUPPLIER. DEFERRED SUBMITTAL SUPPLIER TO PROVIDE CONNECTIONS AND FRAMING ARRANGEMENT TO AVOID LOADING WHICH EXCEEDS THE CAPACITY OF THE ELEMENT BEING ATTACHED TO REFERENCE LOAD MAPS FOR MECHANICAL, ELECTRICAL, PLUMBING AND FIRE SPRINKLER LOAD ALLOWANCES.

1C) ALL DEFERRED SUBMITTALS TO BE ATTACHED TO PRIMARY STRUCTURE WITH A PINNED CONNECTION. MOMENT CONNECTIONS TO PRIMARY STRUCTURE NOT PERMITTED UNLESS NOTED ON DRAWINGS OR APPROVED BY ENGINEER IN WRITING PRIOR TO SUBMITTAL OF DRAWINGS OR CALCULATIONS.

1D) LOADING AND LOCATION FOR ATTACHMENT OF DEFERRED SUBMITTAL ITEMS ARE NOTED ON DRAWINGS AND ARE NOT TO BE RE-LOCATED OR INCREASED WITHOUT WRITTEN APPROVAL.

1E) GC / METAL STUD FRAMING DESIGNER / CLADDING DESIGNER COORDINATION:
 - METAL STUD FRAMING AND FRAMING ATTACHMENT IS DESIGNED FOR THE TRIBUTARY WIND AND GRAVITY LOAD OF THE STUD SPACING.
 - CLADDING SUPPLIER TO DESIGN CLADDING TO ATTACH AT EACH STUD. CLADDING ATTACHMENT SPACING WHICH EXCEEDS THE STUD SPACING IS NOT ACCEPTABLE WITHOUT APPROVAL FROM THE METAL STUD SUPPLIER DESIGNER AND THE PROJECT EOR.
 - IF THE CLADDING SUPPLIER DOES NOT WANT OR CANNOT ATTACH TO EACH STUD THE LOADS FROM THE CLADDING SUPPLIER MUST BE PROVIDED TO THE METAL STUD FRAMING SUPPLIER. THE METAL STUD FRAMING SUPPLIER WILL NEED TO INCORPORATE THESE LOADS INTO THE METAL STUD FRAMING DESIGN
 - GC TO COORDINATE BETWEEN METAL STUD FRAMING SUPPLIER AND CLADDING SUPPLIER AS REQUIRED.

1F) FLOOR FRAMING AND EDGE ANGLE ARE DESIGNED TO SUPPORT ONE LEVEL OF CURTAIN WALL OR METAL STUD WALL FRAMING. SUPPORTING MULTIPLE LEVELS OF CURTAIN WALL OR METAL STUD WALL FROM ONE FLOOR LEVEL IS NOT PERMITTED.

1G) WALLS, GRADE BEAMS AND THE UNDERSIDE OF CONCRETE

POST-INSTALLED ANCHOR NOTES

1) PERSONNEL REQUIREMENTS:
 1A) THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. SUBMIT DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS HAVE PASSED THE TRAINING COURSE PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
 1B) PERSONNEL WHO WILL INSTALL HORIZONTAL OR UPWARDLY INCLINED ADHESIVE ANCHORS IN CONCRETE THAT SUPPORT SUSTAINED TENSION LOADS SHALL BE CERTIFIED BY THE ANCHOR'S ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. THESE ANCHORS ARE DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALL OUT. SUBMIT DOCUMENTED CONFIRMATION THAT PERSONNEL HAVE PASSED THE TRAINING COURSE PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
2) INSTALLATION REQUIREMENTS:
 2A) ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED ACCORDING TO MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND PER MANUFACTURER'S ON-SITE TRAINING.
 2B) ALL ADHESIVE ANCHORS AND ADHESIVE ANCHORED REINFORCEMENT DESIGNS ARE FOR INSTALLATION IN THE FOLLOWING CONDITIONS, UNLESS NOTED OTHERWISE. WRITTEN APPROVAL MUST BE RECEIVED FROM ENGINEER PRIOR TO INSTALLATION IN ALTERNATE CONDITIONS.
 - DRY CONCRETE, UNLESS NOTED OTHERWISE.
 - CONCRETE TEMPERATURE AT TIME OF INSTALLATION THROUGH CURE TIME MUST BE WITHIN THE TEMPERATURE RANGE SPECIFIED IN MANUFACTURER'S PRINTED INSTALLATION INSTRUCTION FOR ADHESIVE GEL AND CURE TIMES.
 - ANCHOR HOLES TO BE HAMMER DRILLED AND CLEANED.
 - CONCRETE MUST BE AT LEAST 21 DAYS OLD BEFORE INSTALLATION OF ANCHORS.
 - HOLES TO BE CLEANED AND PREPARED IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND EVALUATION REPORT PRIOR TO ADHESIVE INJECTION.
 2C) THE POSITION OF EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE SHALL BE LOCATED PRIOR TO INSTALLING POST INSTALLED ANCHORS OR REINFORCEMENT. EXISTING REINFORCEMENT SHALL BE LOCATED USING A SCANNER, GPR, X-RAY, CHIPPING OR OTHER MEANS. DO NOT DAMAGE OR CUT EXISTING REINFORCEMENT.

3) SUBSTITUTION REQUESTS:
 3A) SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS AND PRODUCT DATA DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS IN COMPLIANCE WITH THE RELEVANT BUILDING CODES, LOAD RESISTANCE, INSTALLATION CATEGORY, CREEP APPROVAL, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE OF THE SPECIFIED PRODUCT.

POST-INSTALLED ANCHOR TABLE - HILTI					
ANCHOR TYPE	PRODUCT	Fy (KSI)	Fu (KSI)	COMMENT	
ADHESIVE (IN CONCRETE)	[HILTI HIT-HY 200 V3]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	
ADHESIVE (IN CONCRETE W/12" EMBEDMENT)	[HILTI HIT-RE 500 V3]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	
ADHESIVE (IN GROUTED OR HOLLOW MASONRY)	[HILTI HIT-HY 270]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	
ADHESIVE ANCHOR RODS	-	36 MIN	58 MIN	THREADED ROD, UNGREASED	
EXPANSION ANCHORS (IN CONCRETE)	[HILTI KWIK BOLT TZ2]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	
EXPANSION ANCHORS (IN GROUTED MASONRY)	[HILTI KWIK BOLT TZ2]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	
SCREW ANCHORS	[HILTI KWIK HUS-EZ]	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS	

FOUNDATION NOTES

1) DESIGN CRITERIA:
 THE GEOTECHNICAL REPORT AND SUBSEQUENT ADDENDUMS #1 AND #2 PREPARED BY MCLELLAND CONSULTING ENGINEERS, INC., PROJECT NUMBER 21-0942, DATED 12/1/2022, 08/05/2022, AND 04/20/2023 PROVIDED CRITERIA FOR THE FOUNDATION DESIGN FOR THE PROJECT.
2) DRILLED PIERS:
 - PIER DESIGN CRITERIA:
 - MAXIMUM END BEARING PRESSURE = 150 KSF
 - MAXIMUM END BEARING PRESSURE = 200 KSF (AT PIERS A/9, A/11.4, C/9, C/11, E/9, E/10.9, F/10.9, AND G/10.9)
 - MAXIMUM SIDE SHEAR FOR LENGTH OF PENETRATION INTO OVERBURDEN FOR GRAVITY LOADS = 0.5 KSF (IGNORE WITHIN TOP 2 FEET OF TOP OF PIER)
 - MAXIMUM SIDE SHEAR FOR LENGTH OF PENETRATION INTO BEDROCK FOR UPLIFT LOADS = 5 KSF
 - MAXIMUM SIDE SHEAR FOR LENGTH OF PENETRATION INTO BEDROCK FOR UPLIFT LOADS = 10 KSF (AT PIERS A/9, A/11.4, C/9, C/11, E/9, E/10.9, F/10.9, AND G/10.9)
 - FROST DEPTH TO BOTTOM OF FOUNDATION = 24 IN
 - MINIMUM PENETRATION INTO BEDROCK = 1 PIER DIAMETER (NOT LESS THAN 24 INCHES)
 - MINIMUM PIER SPACING = 1 PIER DIAMETER CLEAR
 - MINIMUM PIER LENGTH TO DIAMETER RATIO = 3
 - DRILL PROBE HOLES AT EACH PIER LOCATION BEYOND BOTTOM OF PIER TO VERIFY COMPETENCY OF END-BEARING MATERIAL PER GEOTECHNICAL REPORT. AT A MINIMUM, PROBE HOLES SHALL EXTEND THE GREATER OF TWICE THE PIER DIAMETER OR FIVE FEET INTO CONSISTENT ROCK.
3) SITE RETAINING WALLS:
 3A) EQUIVALENT FLUID PRESSURES USED FOR WALL DESIGN:
 - "ACTIVE" CONDITION = 60 PCF
 - "AT REST" CONDITION = 82 PCF
 - "PASSIVE" CONDITION = 167 PCF
 - LATERAL PRESSURE DUE TO SURCHARGE = 100 PSF
 - ALLOWABLE BEARING PRESSURE = 2000 PSF
 - ULTIMATE COEFFICIENT OF FRICTION TO RESIST LATERAL LOADS = 0.35

WALL DESIGN BASED ON SITE SOILS ADJACENT TO FOUNDATION WALLS. REFER TO GEOTECHNICAL REPORT FOR REQUIREMENTS.

SOIL INPUT DATA FOR LP1E				
SOIL STRATA LP1E MODEL	SOFT CLAY (MATLOCK)	STIFF CLAY W/ FREE WATER (RESE)	STRONG ROCK (VUGGY LIMESTONE)	WEAK ROCK (RESE)
EFFECTIVE UNIT WEIGHT (pcf)	115	120	160	153
UN-DRAINED COHESION, c (psf)	3.0	7.5	-	-
STRAIN FACTOR, C _α (in/in)	0.02	0.007	-	-
YOUNG'S MODULUS, E _s (psi)	-	-	-	4.061e+6
UNIAXIAL COMPRESSIVE STRENGTH (psi)	-	-	8,300	5,500
ROCK QUALITY DESIGNATION, RQD (%)	-	-	-	50
WEAK ROCK PARAMETER, K _α	-	-	-	0.007

WOOD NOTES

1) FRAMING LUMBER:
 1A) DRY (19% MAXIMUM MOISTURE CONTENT AT THE TIME OF INSTALLATION), SOUTHERN PINE WITH MINIMUM DESIGN VALUES BASED ON THE 2018 NDS. SEE "FRAMING LUMBER TABLE" FOR MINIMUM GRADES.
2) SHEATHING:
 2A) WOOD STRUCTURAL PANELS (WSP)
 - WOOD STRUCTURAL PANELS SHALL BE APA RATED SHEATHING CONFORMING TO U.S. DEPARTMENT OF COMMERCE STANDARD PS 2-10. ALL WOOD PANELS SHALL BE EXPOSURE 1.
3) BLOCKING AND BRIDGING:
 3A) PROVIDE 1" X 4" SIMPSON NCA/CA CROSS BRIDGING AT 8' O.C. MAXIMUM SPACING FOR ALL SOLID SAWN WOOD JOISTS AND RAFTERS. PROVIDE FULL HEIGHT SOLID BLOCKING (MINIMUM WIDTH TO MATCH WIDTH OF FRAMING) BETWEEN ALL FRAMING MEMBERS (SOLID SAWN JOISTS AND RAFTERS, FABRICATED JOISTS AND RAFTERS AND TRUSSES) AT SUPPORTS.
4) NAILING:
 4A) UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE BOX NAILS COMMON NAILS. MINIMUM NAILING SHALL BE IN ACCORDANCE WITH THE TYPICAL WOOD CONNECTION SCHEDULE AND IBC 2021 TABLE 2304.10.1
 4B) WHERE COMMON NAILS ARE SPECIFIED, BOX NAILS OF EQUAL LENGTH MAY BE SUBSTITUTED PROVIDED ONE BOX NAIL IS ADDED FOR EVERY THREE COMMON NAILS SPECIFIED.
5) METAL CONNECTORS:
 5A) FRAMING CONNECTORS SHALL CONFORM TO IBC 2021 SECTION 2303.5 FRAMING CONNECTOR DESIGNATIONS ARE THOSE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, SAN LEANDRO, CALIFORNIA. OTHER MANUFACTURER'S PRODUCTS MAY BE USED IF APPROVED BY THE ENGINEER. FURNISH NAILS AND/OR BOLTS OF DIAMETER, LENGTH, AND NUMBER SPECIFIED BY THE MANUFACTURER FOR EACH CONNECTOR.
 5B) ALL CONNECTOR HOLES SHALL BE FILLED WITH PROPER NAILS/BOLTS INCLUDING OPTIONAL NAIL LOCATIONS FOR UPLIFT. ALL BOLT HOLES SHALL BE DRILLED INTO FRAMING MEMBERS. MAXIMUM HOLE DIAMETER IS 1/16" LARGER THAN THE BOLT DIAMETER.
6) OPENINGS:
 6A) OPENINGS, POCKETS, ETC., SHALL NOT BE PLACED IN BEAMS, JOISTS, RAFTERS, STUDS, POSTS, COLUMNS, TIMBER AND OTHER STRUCTURAL MEMBERS UNLESS DETAILED ON THE STRUCTURAL DRAWINGS.

FRAMING LUMBER SCHEDULE - SOUTHERN PINE (SP)				
TYPE OF USE	GRADE	Fb (PSI)	Fv (PSI)	E (PSI)
EXPOSED FRAMING	NO. 1	1,500	175	1,600,000
DECKING	SEE LANDSCAPE	-	-	-

APA RATED SHEATHING		APA RATED STURD-I-FLOOR	
PANEL SPAN RATING	PANEL THICKNESS	PANEL SPAN RATING	PANEL THICKNESS
48/24	23/32", 3/4"	24/16	23/32", 3/4"

GEOFOAM NOTES

1) GENERAL
 1A) ALL GEOFOAM MATERIAL SHALL CONFORM WITH ASTM D6817.
2) PLACEMENT
 2A) PLACE GEOFOAM PER MANUFACTURER'S RECOMMENDATIONS. ALL BLOCKS SHALL BE PLACED TIGHT TO EACH OTHER WITH NO GAPS.
 2B) SEE ARCHITECTURAL DRAWINGS FOR GEOFOAM LAYOUT AND THICKNESSES.

GEOFOAM SCHEDULE				
TYPE	DENSITY (PCF)	COMPRESSIVE RESISTANCE MIN. @ 1% DEFORMATION (PSI)	MAX DEPTH (FT.)	USE
EPS15	0.9	3.6	12	BELOW SOIL/PLANTING
EPS29	1.8	10.9	12	BELOW CONCRETE/STONE/BRITTLE FINISHES

STEEL NOTES

1) CONNECTIONS:
 1A) PROVIDE CONNECTIONS AS SHOWN IN THE STEEL BEAM CONNECTION SCHEDULES AND DETAILS HEREIN. REFER TO SPECIFICATION FOR ALTERNATIVES AND CONNECTIONS NOT SHOWN.
2) WELDING REQUIREMENTS:
 2A) WELDERS: HAVE IN POSSESSION CURRENT EVIDENCE OF PASSING THE APPROPRIATE AWS. QUALIFICATION TESTS.
 2B) MINIMUM WELDS: AISC SPECIFICATION, NOT LESS THAN 3/16" FILLET, CONTINUOUS UNLESS OTHERWISE NOTED.
 2C) WELD SIZES AND LENGTHS CALLED FOR ON THE DRAWINGS ARE THE NET EFFECTIVE REQUIRED. INCREASE WELD SIZE IF GAPS EXIST AT THE FAYING SURFACE.
 2D) WELD SIZES SHALL BE AS SHOWN UNLESS A GREATER SIZE IS REQUIRED BY ANSIAISC 360-16 TABLES J2.3 AND J2.4.
 2E) ALL GROOVE WELDS SHALL BE COMPLETE PENETRATION UNLESS NOTED.
 2F) FIELD WELDING SYMBOLS INDICATE SEQUENCE CONSIDERED DURING DESIGN. THE CONTRACTOR SHALL REQUEST APPROVAL FROM THE ENGINEER TO MODIFY WELD INSTALLATION LOCATION INDICATED ON THE DOCUMENTS.
 - FROM SHOP TO FIELD
 - FROM FIELD TO SHOP
 2G) DEFORMED ANCHOR STUDS (DAS) AND HEADED ANCHOR STUDS (HAS / HDAS) SHALL BE SHOP OR FIELD WELDED AT CONTRACTOR'S OPTION UNLESS NOTED OTHERWISE.
3) COMPOSITE GRAVITY FRAMING:
 3A) COMPOSITE BEAMS ARE DESIGNED ASSUMING STUDS ARE INSTALLED IN THE WEAK POSITION (R_p = 0.6). SEE TYPICAL METAL DECK DETAILS FOR PLACEMENT REQUIREMENTS.
 3B) COMPOSITE GIRDERS ARE DESIGNED ASSUMING STUDS ARE WELDED THROUGH THE METAL DECK AND/OR METAL DECKING/SHEET STEEL COVERS MORE THAN HALF OF THE TOP FLANGE (R_p = 0.75). SEE TYPICAL METAL DECK DETAILS FOR PLACEMENT REQUIREMENTS.
4) CAMBER:
 4A) FABRICATE BEAMS SUCH THAT ROLLING OR FABRICATION INDUCED CAMBER IS UP AFTER ERECTION.
 4B) CAMBER SHOWN IS BASED ON THE COMPUTED DEFLECTION OF THE BEAM DUE TO SELF WEIGHT OF CONCRETE PLACED. DESIGN IS BASED ON THE THEORETICAL CONCRETE THICKNESS PLUS 1/2" THICKNESS FOR DECK LEVELING AND 1/2" THICKNESS FOR BEAM LEVELING. INCLUDE QUANTITY OF ADDED CONCRETE DUE TO DECK AND BEAM DEFLECTION IN BID.
5) STRUCTURAL STEEL INSTALLATION:
 5A) UNLESS INDICATED OTHERWISE, SNUG TIGHTEN ALL JOINTS AS DEFINED BY AISC CONNECTIONS AS INDICATED BELOW SHALL BE PRETENSIONED PER TABLE J3.1 OF ANSIAISC 360-16
 - WHERE NOTED ON THE DRAWINGS AS "PT"
 5B) CONNECTIONS NOTED ON THE DRAWINGS AS "SC" SHALL MEET THE FOLLOWING REQUIREMENTS:
 - FAYING SURFACES SHALL BE CLASS A PER AISC
 - BOLTS SHALL BE PRETENSIONED PER TABLE J3.1 OF ANSIAISC 360-16
6) METAL DECK:
 6A) SEE METAL DECK SCHEDULE "FOR MATERIALS, PROFILE, AND CONNECTIONS TO STRUCTURE.
 6B) QUALITY CONTROL AND QUALITY ASSURANCE FOR STEEL DECK INSTALLATION SHALL BE IN ACCORDANCE WITH SDI QA/QC-2011, "STANDARD FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR THE INSTALLATION OF STEEL DECK" AS MODIFIED BY TABLE C-1 CONTAINED IN THE COMMENTARY TO THAT STANDARD.
 6C) DECK DESIGN IS IN ACCORDANCE WITH STEEL DECK INSTITUTE (SDI) FLOOR DECK DESIGN MANUAL (2014), SDI ROOF DECK DESIGN MANUAL (2013), AND SDI DIAPHRAGM DESIGN MANUAL, 4TH EDITION (2015)
 6D) PLACE CONCRETE ON METAL DECK IN ACCORDANCE WITH SDI FLOOR DECK DESIGN MANUAL (2014) TO LIMIT CONSTRUCTION LOADS TO ALLOWABLE MAGNITUDES.
 6E) REINFORCE OPENINGS IN METAL ROOF DECK AND FLOOR DECK SUPPORTING CONCRETE FILL IN ACCORDANCE WITH TYPICAL DECK OPENING DETAILS.
 6F) INSTALL DECK OVER 4 SUPPORTS (3 SPAN CONTINUOUS) UNLESS NOTED OTHERWISE. DO NOT INSTALL DECK AS SINGLE SPAN UNLESS SPECIFICALLY SHOWN ON DRAWINGS.
 6G) PROVIDE DECK ATTACHMENTS AS NOTED ON DRAWINGS.
 6H) HANGERS: SEE TYPICAL METAL DECK DETAILS FOR ALLOWABLE HANGER LOADS, SPACING AND ATTACHMENT.
7) STRUCTURAL COLD FORMED METAL FRAMING:
 7A) COLD FORMED METAL FRAMING IS A PERFORMANCE SPECIFIED ITEM DESIGNED BY THE CONTRACTOR. PROVIDE STUD DEPTH INDICATED ON THE DRAWINGS. DO NOT EXCEED MAXIMUM SPACING INDICATED. VARY FLANGE WIDTH, GAGE, YIELD STRENGTH, BRIDGING, STUD SPACING, ETC. AS REQUIRED TO SATISFY PERFORMANCE CRITERIA IN THE CONTRACT DOCUMENTS. MINIMUM STUD GAGE SPECIFIED IS REQUIRED FOR ATTACHMENT OF OTHER MATERIALS TO STUDS. DO NOT BASE BIDS ON MINIMUM GAGE OR MAXIMUM GAGE SPECIFIED.
 7B) REFER TO DETAILS FOR MINIMUM CONNECTIONS AND OTHER REQUIREMENTS. DEVELOP FORCES NOTED. DO NOT IMPOSE FORCES ON THE BUILDING STRUCTURE IN DIRECTIONS OR AT LOCATIONS OTHER THAN THAT SHOWN ON THE STRUCTURAL DRAWINGS. DO NOT IMPOSE FORCES LARGER THAN SPECIFIED. CONNECTIONS TO CONCRETE SHALL NOT USE PAFs TO RESIST TENSION LOADS.
8) STEEL GRATING
 8A) STEEL GRATING TYPE AND SIZE AS INDICATED ON DRAWINGS
 8B) INSTALLATION
 - INSTALL GRATING PER INSTALLATION GUIDELINES OF NAAMM MBG 531 "METAL BAR GRATING MANUAL".
 - FIT EXPOSED CONNECTIONS ACCURATELY TO FORM TIGHT JOINTS.
 - USE MANUFACTURER'S STANDARD ANCHOR CLIPS AND HOLD-DOWN FASTENERS FOR BOLTED CONNECTIONS
 8C) SUBMITTALS
 - SUBMIT SHOP DRAWINGS AND PRODUCT DATA FOR GRATING, FASTENERS, AND FINISH.

STEEL MATERIAL TABLE				
STEEL ELEMENT	ASTM/TYPE	Fy (KSI)	Fu (KSI)	COMMENTS
ANCHOR RODS	F1554 GR 55	55	75	WELDABLE, HEAVY HEX HEADED
ANCHOR RODS IN MASONRY	F1554 GR 36, F1554 GR 55, OR A307 GRADE A/C	36	58	WELDABLE, STD HEX HEAD
COLD THREADED BAR	A722	150	-	FOR USE AT TRUSS TENSION PIERS
ANGLE	A529	50	65	
3/4"Ø BOLTS, TYPICAL	F3125 - TYPE A325 OR F1552 TYP	-	120	BOLTS ARE 3/4"Ø UNLESS NOTED OTHERWISE
1"Ø BOLTS	TYPE A490 OR F2280 WHERE SPECIFIED	-	150	1"Ø WHERE SPECIFIED, USE TENSION-CONTROLLED WHERE POSSIBLE
1 1/4" Ø BOLTS	A490	-	200	AS REQUIRED FOR DELEGATED DESIGN
COLD-FORMED STUDS/PLATE, 33 AND 43 MIL	A1003	33	-	-
COLD-FORMED STUDS/PLATE, 54 MIL AND HEAVIER	A1003	50	-	-
COLD-FORMED TRACK, ALL THICKNESSES	A1003	33	-	-
DAS	A1064	70	80	-
HAS	A108	51	65	STUDS ARE 3/4"Ø UNO
OTHER SHAPES	A36	36	58	-
PIPE	A53 GR B	35	60	-
PLATES	A572, GR 50	50	65	-
PLATES AT TRUSS CONNECTIONS	A572, GR 65	65	80	FOR TRUSS CONNECTIONS
RECT HSS	A500 GR C	50	62	-
ROUND HSS	A500 GR C	46	62	-
STEEL GRATING	-	-	-	PER NAAMM MBG 531, "METAL BAR GRATING MANUAL"
WELDING ELECTRODES, THICKNESS OF THINNER PART > 0.1 INCHES (12 GA)	E70 OR E80	-	-	PER AWS
WELDING ELECTRODES, THICKNESS OF THINNER PART ≤ 0.1 INCHES (12 GA)	E60 OR E70	-	-	PER AWS
WF, WT	A992	50	65	-
WF TRUSSES	A913	65	80	FOR TRUSS MEMBERS AND SUPPORTING COLUMNS

METAL GAUGE CONVERSION	
GAUGE	MINIMUM THICKNESS (MILS")
22	27
20	33
18	43
16	54
14	68
12	97

NOTES:
 * 1 MIL = 1/1000"

CONCRETE NOTES

1) GENERAL:
 1A) ALL WORK SHALL CONFORM WITH ACI 301-10, UNLESS NOTED OTHERWISE IN DRAWINGS OR PROJECT SPECIFICATIONS.
 1B) DETAIL BARS IN ACCORDANCE WITH THE DRAWINGS, PROJECT SPECIFICATIONS, AND ACI PUBLICATION SP-66 (2004) "ACI DETAILING MANUAL"
2) REINFORCING MATERIALS:
 2A) SEE REINFORCING MATERIAL TABLE
3) REINFORCING FABRICATION:
 3A) SPLICES:
 - NO SPLICING OF REINFORCEMENT PERMITTED EXCEPT AS NOTED ON DRAWINGS. MAKE BARS CONTINUOUS AROUND CORNERS WHERE DETAIL NOT PROVIDED. WHERE PERMITTED, SPLICES MAY BE MADE BY CONTACT LAPS OR MECHANICAL CONNECTORS.
 - SEE LAP SPLICE SCHEDULE FOR LAP LENGTHS
 - SPLICE CONTINUOUS TOP AND BOTTOM BARS IN WALLS, BEAMS, AND GRADE BEAMS L.T.S UNLESS NOTED OTHERWISE.
 - SPLICE TOP BARS AT MIDSPAN AND BOTTOM BARS OVER SUPPORT UNLESS NOTED OTHERWISE.
 3B) MISCELLANEOUS REINFORCING REQUIREMENTS:
 - PROVIDE ADDITIONAL BARS OR STIRRUPS REQUIRED TO SECURE REINFORCING IN PLACE DURING CONCRETE PLACEMENT.
 - MAKE ALL REINFORCING BAR BENDS IN THE FABRICATOR'S SHOP UNLESS NOTED.
 - NO WELDING OF REINFORCING PERMITTED UNLESS NOTED ON DRAWINGS. WHERE PERMITTED, PERFORM WELDING IN ACCORDANCE WITH AWS D1.4:2011.
 - PROVIDE ADDED REINFORCING TO TRIM ALL OPENINGS, NOTCHES, AND REINFRANT CORNERS AS NOTED IN TYPICAL DETAILS.
4) STRUCTURAL CONCRETE MIX REQUIREMENTS:
 4A) SEE "CONCRETE MIX TABLE"
5) SLAB-ON-GRADE:
 5A) VERIFY ALKALINITY OF CONCRETE SURFACE, SLAB VAPOR TRANSMISSION, AND SLAB FLATNESS/LEVELNESS ARE COMPATIBLE WITH FLOORING SYSTEM AND ADHESIVES PRIOR TO INSTALLING FLOORING.
 5B) TAKE PRECAUTIONS TO MINIMIZE SLAB CURLING. GRIND SLAB OR USE LEVELING COMPOUND IF FLOOR FLATNESS AND LEVELNESS VALUES ARE NOT ACCEPTABLE TO THE ARCHITECT.
6) NON-SHRINK GROUT:
 6A) CONFORM TO ASTM C1107
 6B) ACHIEVE 8,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
7) PLACING REINFORCEMENT:
 7A) REINFORCEMENT PROTECTION:
 - SEE REBAR COVER TABLE
 - SEE ACI 117-10 FOR REINFORCEMENT PLACING TOLERANCES
 7B) PROVIDE ACCESSORIES NECESSARY TO PROPERLY SUPPORT REINFORCEMENT AND WELDED WIRE REINFORCEMENT AT POSITIONS SHOWN ON PLANS. ALL REINFORCING, DOWELS, BOLTS, AND EMBEDDED PLATES SHALL BE SET AND TIED IN PLACE BEFORE THE CONCRETE IS POURED. "STABBING" INTO PREVIOUSLY PLACED CONCRETE IS NOT PERMITTED.
8) CONSTRUCTION/CONTROL JOINTS:
 8A) SUBMIT DRAWINGS SHOWING CONSTRUCTION AND CONTROL JOINT LOCATIONS ALONG WITH THE SEQUENCE OF POURS. CONSTRUCTION JOINT LOCATIONS AND CASTING SEQUENCE SHALL BE ARRANGED TO MINIMIZE THE EFFECTS OF ELASTIC AND LONG-TERM SHORTENING/SHRINKAGE.
 8B) CONSTRUCTION JOINTS IN SLABS-ON-GRADE AND STRUCTURAL SLABS SHALL BE LOCATED TO ACCOMMODATE THE MAXIMUM LENGTH AND AREA THE CONTRACTOR CAN REASONABLY POUR. FINISH AND JOIN IN THE SAME DAY, BUT SHALL NOT EXCEED 150 FEET WITH A MAXIMUM AREA OF 15,000 SQUARE FEET UNLESS APPROVED BY THE ENGINEER.
 8C) CONCRETE CONSTRUCTION JOINT SURFACE SHALL BE CLEANED AND ALL LAITANCE AND LOOSE MATERIAL REMOVED PRIOR TO SECOND CONCRETE PLACEMENT.
9) MODIFICATIONS TO HARDENED OR EXISTING CONCRETE
 9A) UNLESS NOTED ON THE STRUCTURAL DOCUMENTS MODIFICATIONS AS LISTED BELOW SHALL NOT BE MADE TO HARDENED OR EXISTING CONCRETE WITHOUT APPROVAL OF THE ARCHITECT:
 - SAW CUTTING
 - CORING
 - CHIPPING
 9B) DO NOT CUT OR DAMAGE ANY REINFORCING WITHOUT APPROVAL OF THE ARCHITECT
10) SLEEVES, OPENINGS, AND EMBEDDED PIPE/CONDUITS:
 10A) GENERAL
 - REFER TO TYPICAL DETAILS FOR REQUIREMENTS FOR CONDUIT AND PIPE EMBEDDED IN WALLS AND SLABS
 - REFER TO TYPICAL DETAILS FOR SPACING AND LAYOUT LIMITATIONS FOR SLEEVES AND OPENINGS
 - FORM OPENINGS AND PROVIDE SLEEVES BEFORE PLACING CONCRETE. CORING OF CONCRETE IS NOT PERMITTED
 - AT COMPOSITE SLABS DO NOT CUT DECK FOR AT LEAST 7 DAYS AFTER CONCRETE PLACEMENT
 10B) REINFORCING
 - REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS AT SLEEVES, OPENINGS OR CONDUIT
 - DO NOT CUT REINFORCING WHICH MAY CONFLICT
11) MASS CONCRETE:
 11A) ALL STRUCTURAL CONCRETE DRILLED PIER WITH A DIAMETER OF 6'-0" AND GREATER ARE CONSIDERED MASS CONCRETE ELEMENTS. SEE THERMAL CONTROL PLAN IN SPECIFICATIONS FOR ADDITIONAL INFORMATION.

CONCRETE MIX TABLE							
CONC MIX TYPE	INTENDED USE	28 DAY STRENGTH Fc (KSI)	CONC WEIGHT	MAX W/C RATIO, INCLUDING FLY ASH	MAX AGGREGATE SIZE (IN), NOTE a	TOTAL AIR CONTENT (%), NOTE b	OTHER REQTS, NOTE c
1	DRILLED PIERS < 60" Ø	5	NWC	-	1	-	DP
2	DRILLED PIERS ≥ 60" Ø	6	NWC	-	1	-	DP
3	SLABS ON DECK	3.5	NWC	0.50	3/4	NP	-
4	GRADE BMS, BASEMENT WALLS, EXTERIOR WALLS, PILASTERS AND COLUMN WRAPS	5	NWC	0.45	1	6	-
5	CORE WALLS, TIE BMS, MAT SLAB	6	NWC	0.50	3/4	-	-
6	EXTERIOR SLAB ON GRADE	5	NWC	0.40	3/4	6	-
7	SLAB ON GRADE, TOPPING SLABS	4.5	NWC	0.45	1	NP	-

CONCRETE MIX TABLE NOTES:
 PROPORTIONS OF MATERIALS IN CONCRETE MIX SHALL BE ESTABLISHED TO:
 - PROVIDE THE MINIMUM COMPRESSIVE STRENGTH AS INDICATED IN THE MIX TABLE. DO NOT EXCEED THE MAXIMUM WATER-CEMENT RATIO NOTED.
 - PROVIDE WORKABILITY AND CONSISTENCY TO PERMIT CONCRETE TO BE WORKED READILY INTO FORMS AND AROUND REINFORCEMENT UNDER CONDITIONS OF PLACEMENT TO BE EMPLOYED, WITHOUT SEGREGATION OR EXCESSIVE BLEEDING. CONTRACTOR SHALL SELECT APPROPRIATE SLUMP. USE ADMIXTURES AS REQUIRED TO OBTAIN DESIRED RESULTS.
 USE TYPE II / III PORTLAND CEMENT UNLESS NOTED OTHERWISE. FOR CONCRETE MIXES USED ON FLOORS MINIMUM CEMENTITIOUS CONTENT SHALL BE 540 POUNDS PER CUBIC YARD.
 FOR CONCRETE PLACED BY PUMPING PROVIDE CONCRETE MIX FLOWABILITY TO FACILITATE PUMPING. ENTRAINED AIR MAY BE USED TO FACILITATE PUMPING SUBJECT TO THE PROVISIONS OF NOTE b BELOW.
 a. FOR THE MAXIMUM COARSE AGGREGATE SIZE INDICATED, USE THE FOLLOWING AGGREGATE SIZE NUMBERS PER ASTM C33:
 1": #57 AGGREGATE
 b. WHERE AIR CONTENT IS INDICATED IN THE MIX TABLE, PROVIDE AIR ENTRAINING ADMIXTURE. TOTAL AIR CONTENT LIMITS INCLUDE BOTH ENTRAINED AND ENTRAPPED AIR +/- 1 1/2%. "NP" IN COLUMN INDICATES ADDITION OF ENTRAINED AIR IS NOT PERMITTED EXCEPT WHERE CONTRACTOR CAN DEMONSTRATE THAT SLABS WITH ENTRAINED AIR WILL HAVE A FINISH ACCEPTABLE TO THE ARCHITECT WITHOUT BLISTERS. AIR CONTENT NOTED IS BASED ON 3/4" AGGREGATE. IF 3/8" AGGREGATE IS USED, INCREASE AIR CONTENT BY 1 1/2%.
 c. ABBREVIATIONS FOR OTHER REQUIREMENTS AS FOLLOWS:
 DP = PROVIDE 5 1/2" MINIMUM SLUMP. FOR DRILLED PIERS CASED DURING CONSTRUCTION PROVIDE CONCRETE MIX WITH FLOWABILITY TO PERMIT ARCHING WHEN CASING IS PULLED. ENTRAINED AIR MAY BE USED TO FACILITATE CASING REMOVAL.

REINFORCING MATERIAL TABLE				
REINF ELEMENT	ASTM	Fy (KSI)	Fu (KSI)	COMMENTS
TYP REINFORCING	A615	60	90	-
WELDED & FIELD BENT REINF	A706	60	80	-
WELDED WIRE REINFORCING, SMOOTH	A1064	65	75	-
EPOXY COATING OF REINFORCING	A775 OR A934	-	-	-

MASONRY NOTES

1) DEFINITIONS:
 1A) SEE ARCHITECTURAL DRAWINGS FOR LOCATION, THICKNESS AND EXTENT OF MASONRY PARTITIONS. SEE DETAILS ON THE STRUCTURAL DRAWINGS FOR GENERAL MASONRY PARTITION REQUIREMENTS.
2) DESIGN STRENGTH:
 2A) DEVELOP 2000 PSI COMPRESSIVE STRENGTH (f_m) IN 28 DAYS.
3) STEEL REINFORCING:
 - PRIMARY REINFORCING: ASTM A615, 60 KSI
 - HORIZONTAL JOINT REINFORCING: ASTM A951, PREFABRICATED, LADDER TYPE
3) SPLICES:
 3A) SEE MASONRY LAP SPLICE SCHEDULE FOR LAP LENGTHS.
4) INSTALLATION REQUIREMENTS:
 4A) GROUT SOLID ALL CELLS CONTAINING REINFORCING, EMBEDDED ITEMS, AND ALL OTHER CELLS NOTED ON THE CONTRACT DOCUMENTS.



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LANDSCAPE
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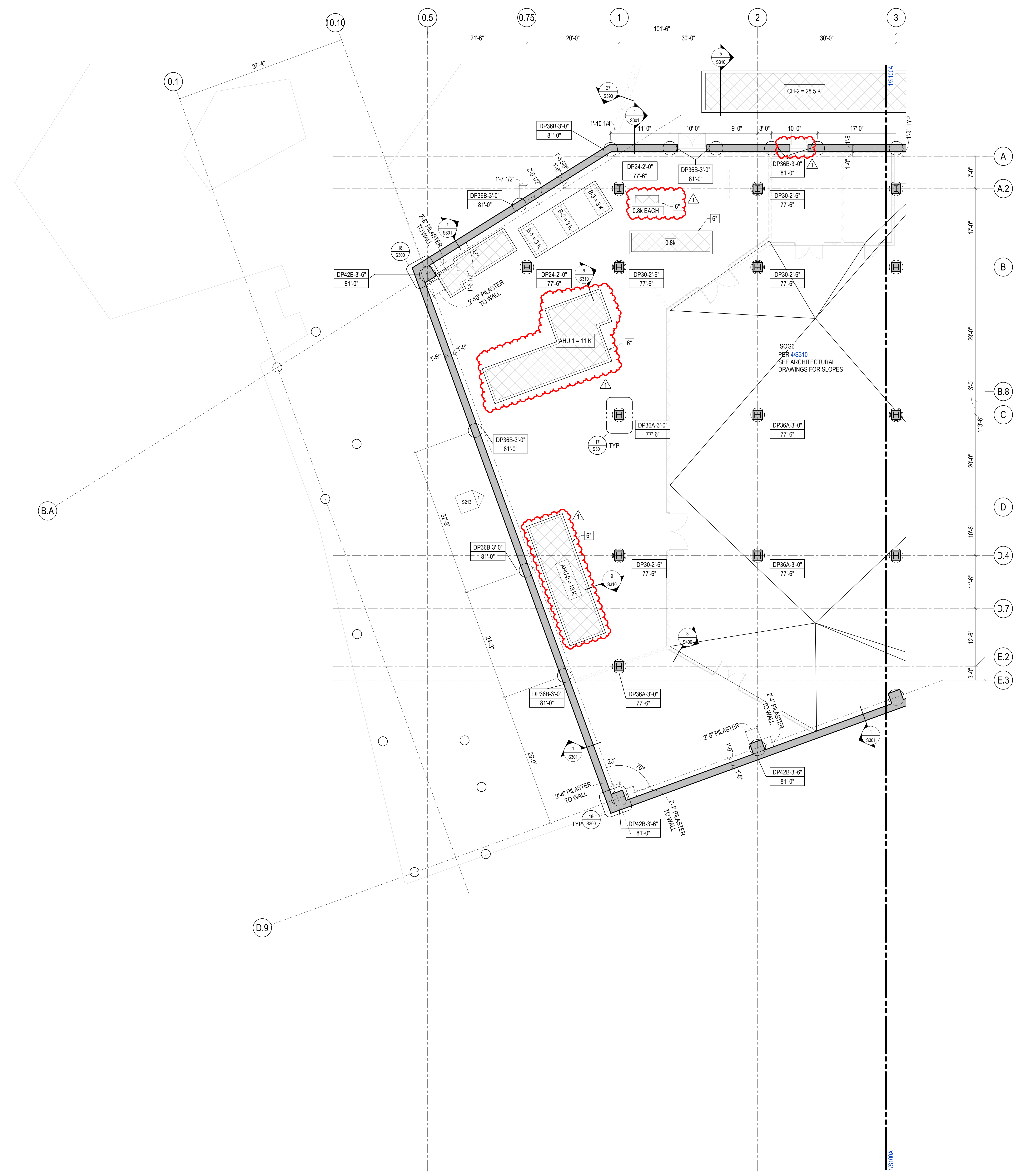
STRUCTURAL
 MartinMartin Consulting Engineers
 806 SOUTH WALTON BLVD. STE 177
 BENTONVILLE, AR 72712
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MEFP - LOW VOLTAGE
 Henderson Engineers
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NUMBER	DATE	DESCRIPTION
1	06.29.2023	ACCENDAZI2



STRUCTURAL PLAN - UNDERGROUND PARKING - AREA B

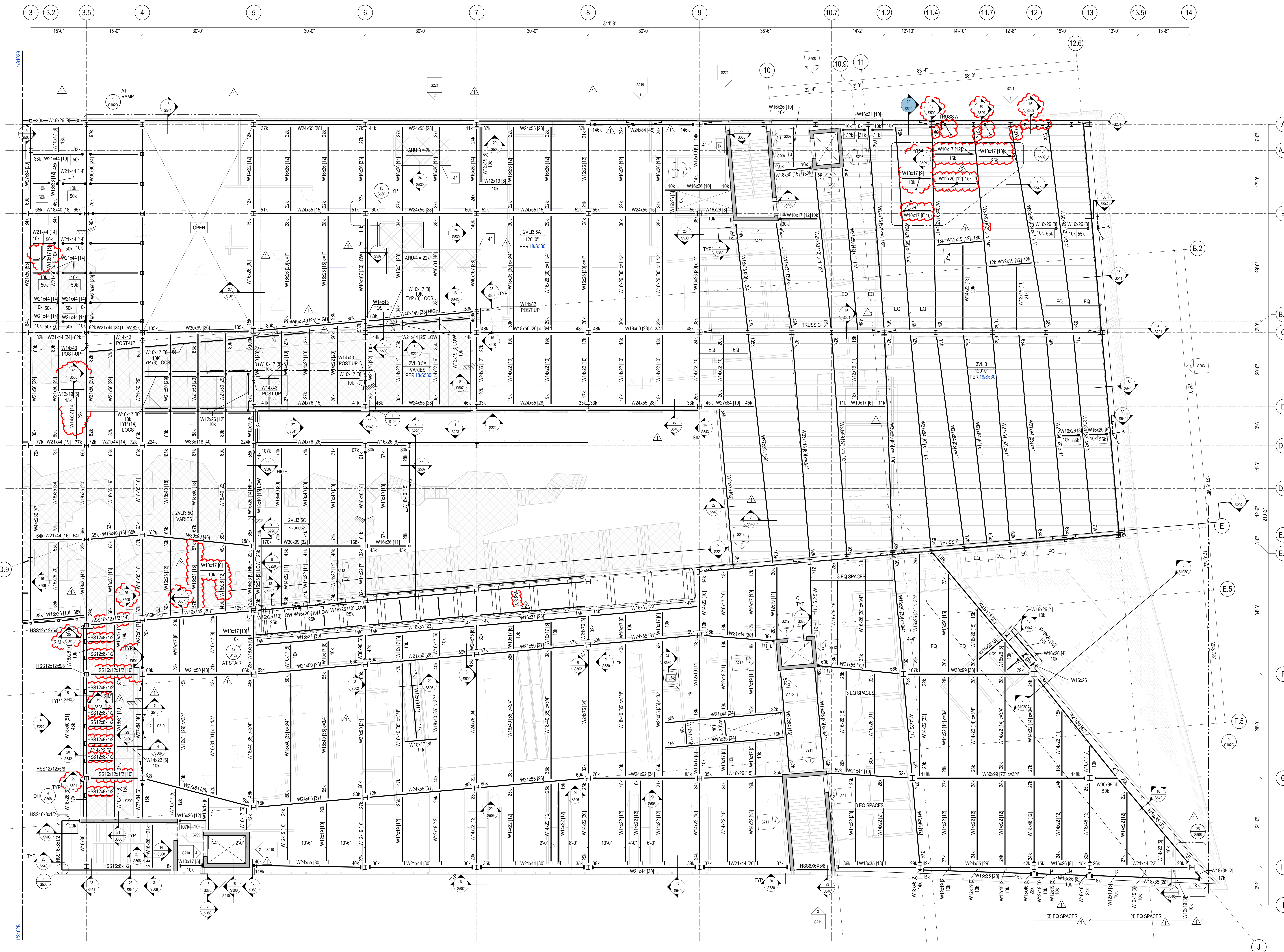
1/8" = 1'-0"

MM JOB #: AS21194LSJ21
PRINCIPAL: BENDON WILCOX
DESIGNER: ETHAN WICKS, CHRISTINA CHILDRESS, CALEB CHENUT
LEAD REVIT TECH: BRAD WELLS, CAM
FILE PATH: \\P:\Projects\2021\2021-02-24\194LSJ21\194LSJ21 - Under-Halls_SOW.dwg - S201.dwg
PROJECT MANAGER: BAILEIGH FISHER



REVISIONS

NUMBER	DATE	DESCRIPTION
1	01/12/23	ACCENDAS 1
2	06/29/23	ACCENDAS 2



STRUCTURAL PLAN - 2ND FLOOR - AREA A

1/8" = 1'-0"

DESIGNERS: ETWANKOVS, CHRISTINA; CALDERESCHI, LEAD; REITZ, TONY; BRAWWELLS, AMY; PROJECT MANAGER: BAILEIGH FISHER; DATE: 02/24/2023; FILE PATH: \\server\hous\2023\18889\18889-1\18889-1.dwg; USER: HANS.SCHWAB; SIZE: 41

ASST. PRINCIPAL: BENJAMIN W. BENDISY; PRINCIPAL: POLK STANLEY WILCOX; PROJECT MANAGER: BAILEIGH FISHER

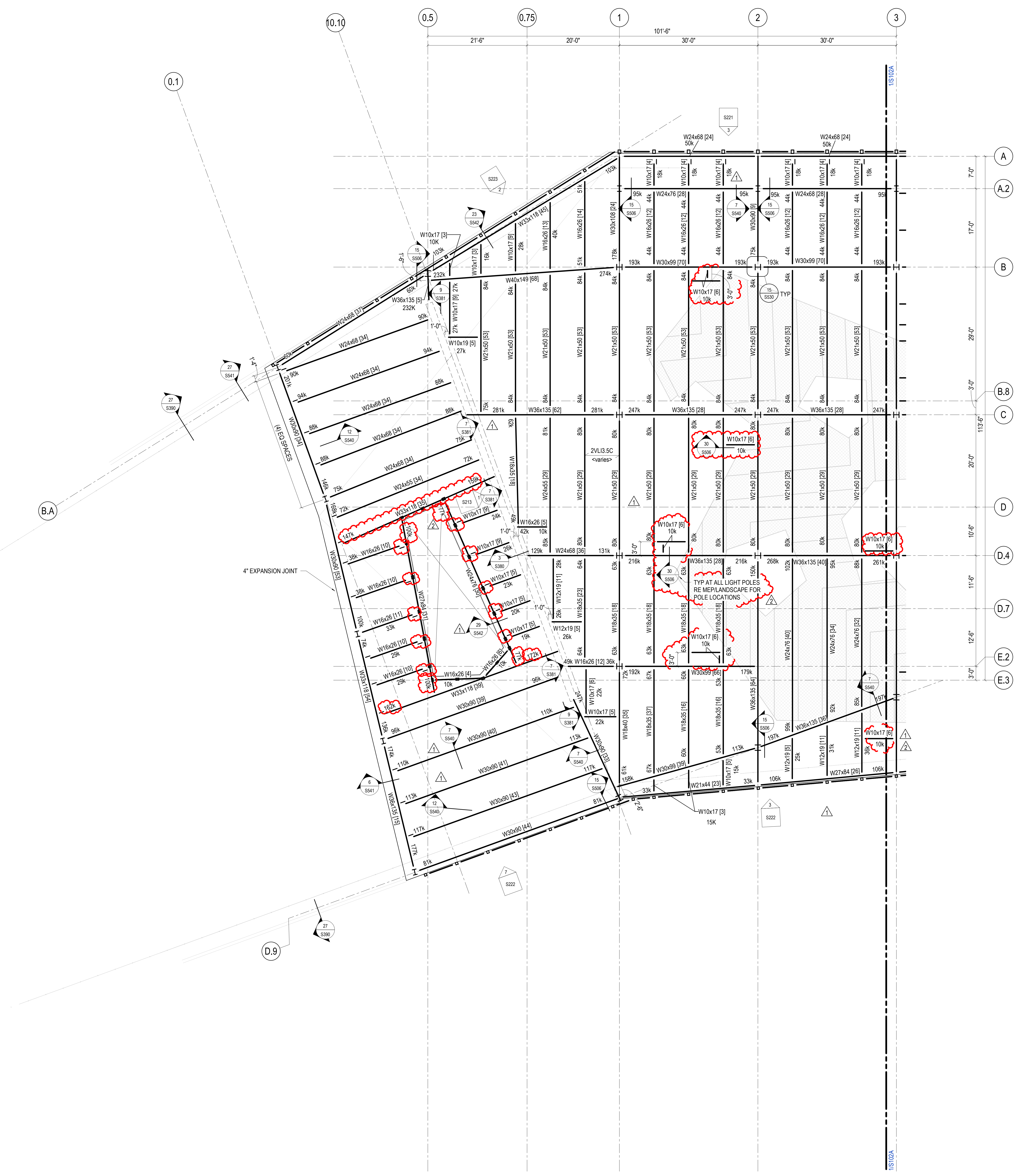


REVISIONS		
NUMBER	DATE	DESCRIPTION
1	03.13.2023	ADDENDUM 1
2	06.29.2023	ADDENDUM 2

MM JOB #: AS211818.01
PRINCIPAL: BENJAMIN W. DEWITT
PROJECT MANAGER: BAILEIGH FISHER
DESIGNERS: ETHAN WICKS, CRISTINA CHILDS, CALEB CHENUT
LEAD REVIT TECH: BRAD WELLS, AM
FILE PATH: \\awsom\docs\985a\2023\1818\1818.rvt - W:\H:\H:\S:\M\ - S22.rvt

STRUCTURAL PLAN - 2ND FLOOR - AREA B

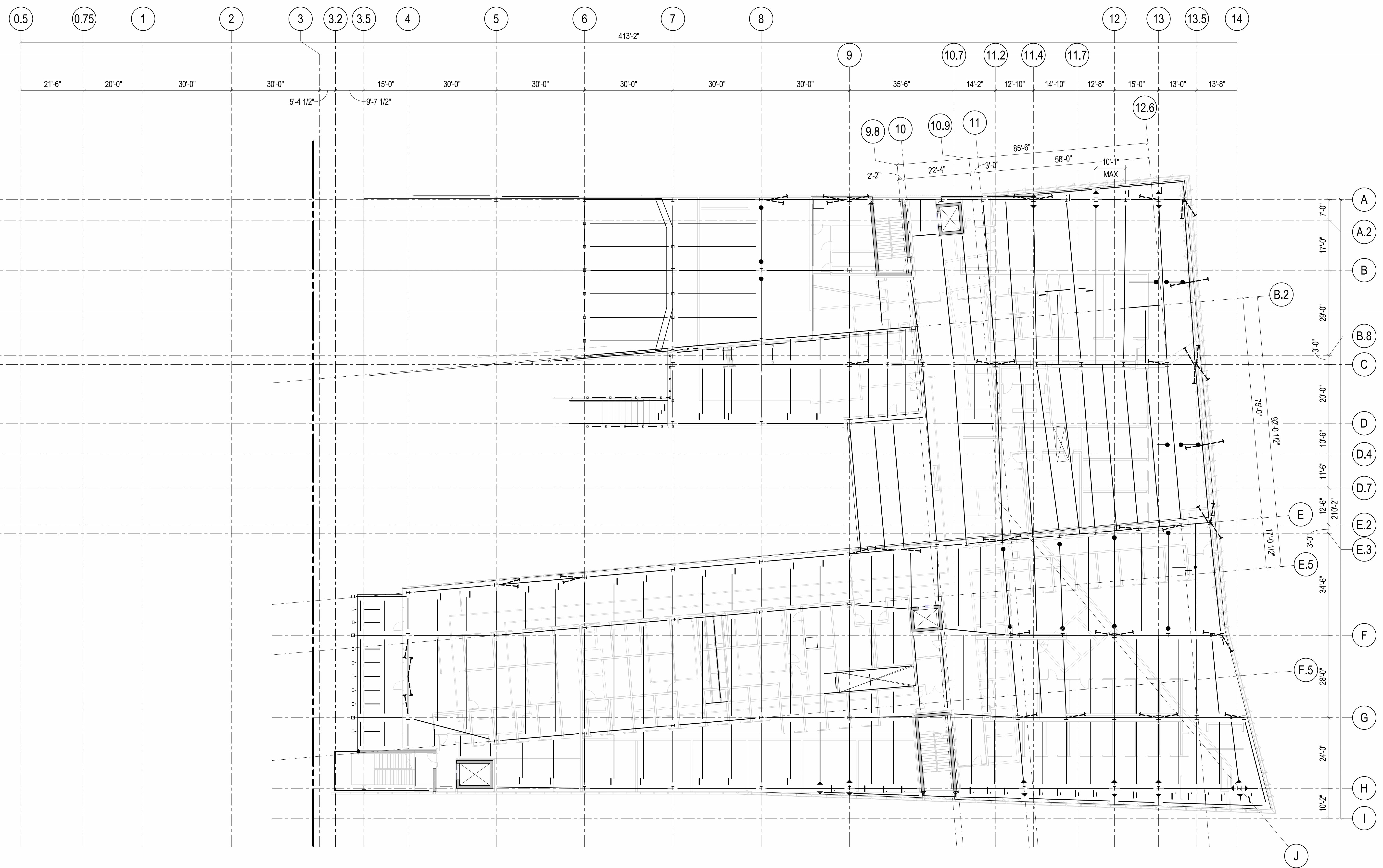
1/8" = 1'-0"



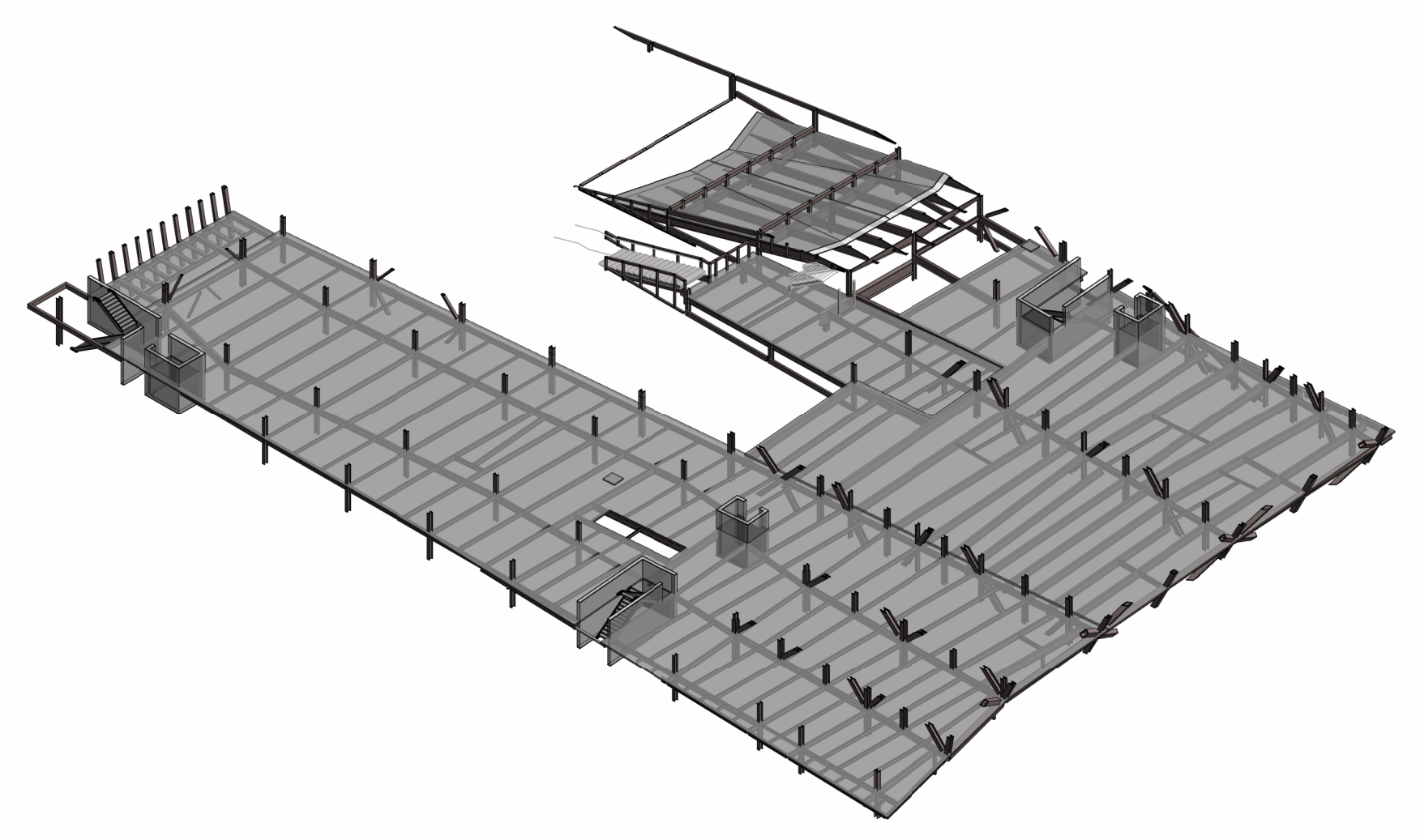
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0.5
0.75
1
2
3
A
A.2
B
B.8
C
D
D.4
D.7
E.2
E.3
D.9

COMPOSITE STEEL FRAMING NOTES

- 1. GENERAL:
 - A. SEE 50 SERIES SHEETS FOR GENERAL NOTES, SYMBOLS AND ABBREVIATIONS.
 - B. SEE 55 SERIES SHEETS FOR TYPICAL STEEL DETAILS.
- 2. COLUMNS/PILASTER:
 - A. ALL COLUMNS/PILASTERS ARE CENTERED ON THE INTERSECTION OF GRIDS BELOW THE SUPPORTED COLUMN UNLESS DIMENSIONED OTHERWISE ON PLAN.
- 3. STEEL BEAMS:
 - A. STEEL BEAMS SHALL BE EQUALLY SPACED BETWEEN GRID LINES/COLUMNS/GIRDERS UNLESS DIMENSIONED OTHERWISE.
 - B. TOP OF STEEL BEAMS SHALL EQUAL BOTTOM OF METAL DECK ELEVATION. SEE PLAN FOR TOP OF CONCRETE ELEVATION AND SLAB THICKNESS TO DETERMINE BOTTOM OF METAL DECK ELEVATION.
 - C. REQUIRED BEAM END CONNECTION CAPACITY IN KIPS NOTED ON PLAN THUS: XXX. IF TWO SYMBOLS ARE SHOWN THEY DENOTE THE REQUIRED CONNECTION CAPACITY AT THE CORRESPONDING BEAM END. IF ONLY ONE SYMBOL IS SHOWN IT DENOTES THE REQUIRED CONNECTION CAPACITY AT BOTH ENDS OF THE BEAM. DETAIL CONNECTIONS FOR REQUIRED CONNECTION CAPACITY PER DETAIL 1/SS500 FOR ALL TYPICAL SHEAR CONNECTIONS.
 - D. PLACE NUMBER OF SHEAR STUDS INDICATED ON PLAN THUS: [XX] PER DETAIL 18/SS30. ALL SHEAR STUDS ARE 3/4".
- 4. METAL DECK:
 - A. SEE SHEETS S530 FOR TYPICAL METAL DECK DETAILS.
 - B. SEE DETAIL 28/SS30 FOR DECK SUPPORT FRAMING REQUIRED AT DECK PENETRATIONS LARGER THAN 10" IN EITHER DIRECTION.
- 5. CONCRETE SLAB-ON-METAL DECK:
 - A. TOP OF CONCRETE SLAB NOTED ON PLAN.
 - B. LOCATE SLAB CONSTRUCTION JOINTS PER DETAIL 4/SS30. SUBMIT LOCATIONS OF SLAB CONSTRUCTION JOINTS FOR REVIEW 3 WEEKS (MINIMUM) PRIOR TO PLACEMENT OF CONCRETE. SPACE JOINTS AND POUR SEQUENCES TO MINIMIZE SHRINKAGE CRACKS. SEE "CONCRETE NOTES" FOR JOINTING REQUIREMENTS AT SLAB-ON-DECK.
 - C. SEE DETAIL 26/SS30 FOR ADDITIONAL REINFORCING REQUIRED AT SLAB PENETRATIONS/OPENINGS/REENTRANT CORNERS.
 - D. SEE ARCHITECTURAL DRAWINGS FOR SLAB EDGE DIMENSIONS.
 - E. SEE 30/SS50 FOR TREE TIE DOWNS. COORD LOCATIONS WITH LANDSCAPE.
- 6. MECHANICAL AND ELECTRICAL EQUIPMENT:
 - A. SEE 4/SS30 FOR REQUIREMENTS AT MECHANICAL AND ELECTRICAL EQUIPMENT PADS.
 - B. CONTRACTOR TO VERIFY ALL EQUIPMENT WEIGHTS, SIZES, LOCATIONS, AND OPENINGS REQUIRED WITH MECHANICAL CONTRACTOR. CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CHANGES IN THE WEIGHTS OR LOCATIONS SHOWN ON THE DRAWINGS. SUCH CHANGES IN CONDITIONS SHALL BE SUBJECT TO STRUCTURAL ENGINEER REVIEW. RE: MECHANICAL AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL OPENINGS NOT SHOWN.
 - C. MECHANICAL EQUIPMENT WEIGHTS, IN KIPS, NOTED ON PLAN THUS: XXX.
 - D. EQUIPMENT TO BE PLACED TO BEAR ON TWO BEAMS MINIMUM.



OVERALL PLAN - 3RD FLOOR
1/16" = 1'-0"

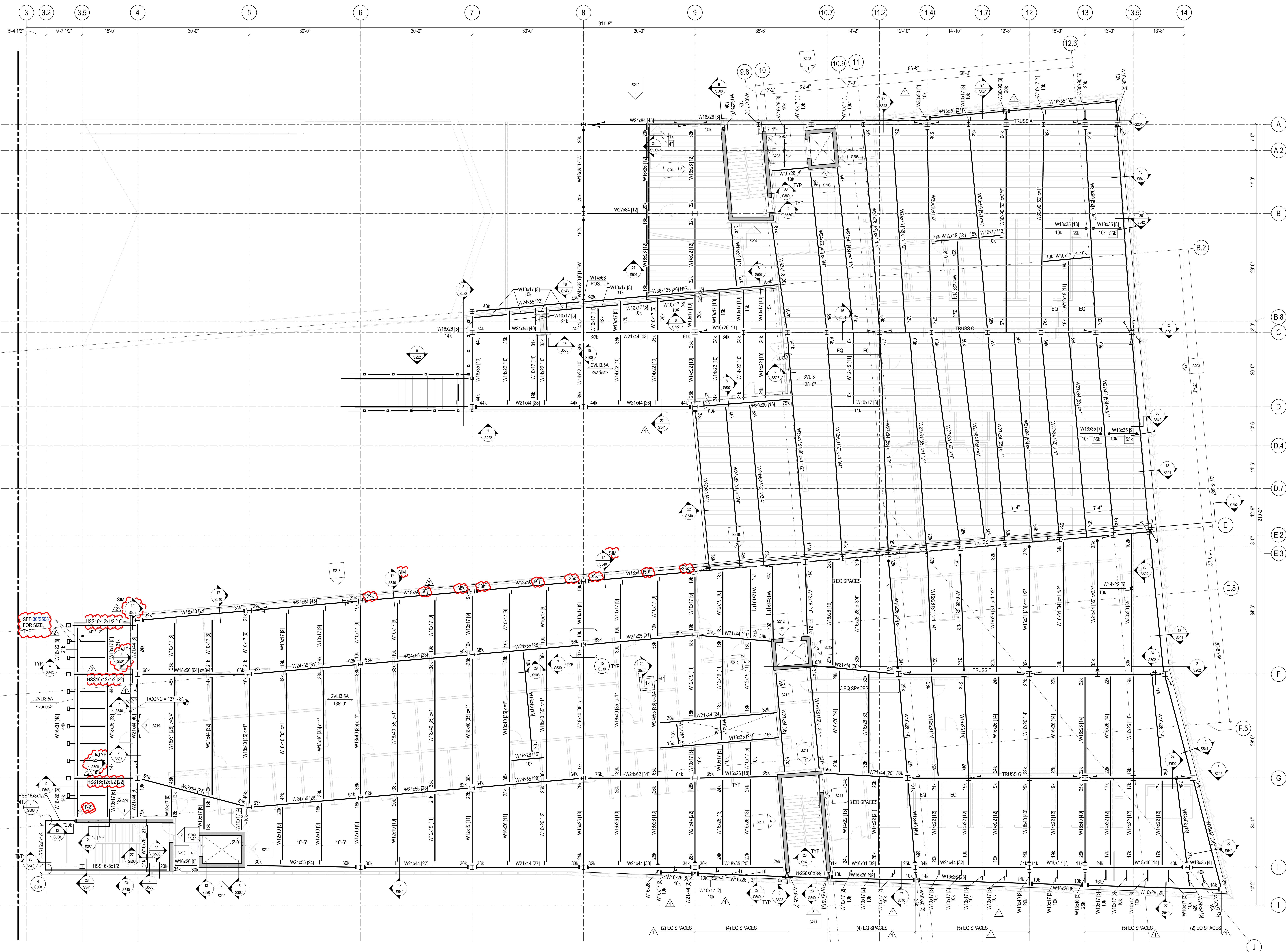


ISOMETRIC - 3RD FLOOR
NO SCALE



REVISIONS		
NUMBER	DATE	DESCRIPTION
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MM JOB #: AS21194LSJ21
PRINCIPAL: BENDOWNEY
PROJECT MANAGER: BAILEIGH FISHER
DESIGNERS: ETHAN WICKS, CRISTINA CHILDRIS, CALEB CHENUT
LEAD REVIT TECH: BRAY WELLS
FILE PATH: Autodesk Docs:985A.2023/1948/1948/AS21194LSJ21 - Whole House SCHMM - S22.rvt



STRUCTURAL PLAN - 3RD FLOOR - AREA A

1/8" = 1'-0"

DESIGNER: ETIAN WICKS, CHRISTINA CHILDRESS, CALEB CHENUIT
 LEAD REVIT TECH: BRAYNIEE AM
 PROJECT MANAGER: BAILEIGH FISHER

NO. 001: ASI, 1843.01
 BENDONNEY
 PRINCIPAL

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 224 SOUTH MICHIGAN AVENUE
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 P: 312.390.4121

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 238 W. 23RD ST., SUITE 802
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FOOD SERVICE: **JMC HOSPITALITY**
 698 SKYLINE DR., SUITE 8210
 THE WOODLANDS, TX 77380
 P: 681.41.2222

WATER FEATURES: **OTL**
 2150 S. TOWNE CENTER, SUITE 100
 ANAHEIM, CA 92806
 P: 714.637.4747

IRRIGATION: **WC3 DESIGN**
 11A ROBERTSON MANOR BLVD.
 WOODBRIDGE, PA 19388
 P: 844.231.7042

PSW Job Number: 993A

CERTIFICATE OF AUTHORIZATION
 MartinMartin Inc.
 No. 3183
 ARKANSAS

STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
 No. 18889
 MARTIN W. WILCOX

2.24.2023

AWSOM
 Bentonville, AR

Issue Date: 02.24.2023

NUMBER	DATE	DESCRIPTION
1	03.13.2023	ADDENDUM 1
2	06.09.2023	ADDENDUM 2

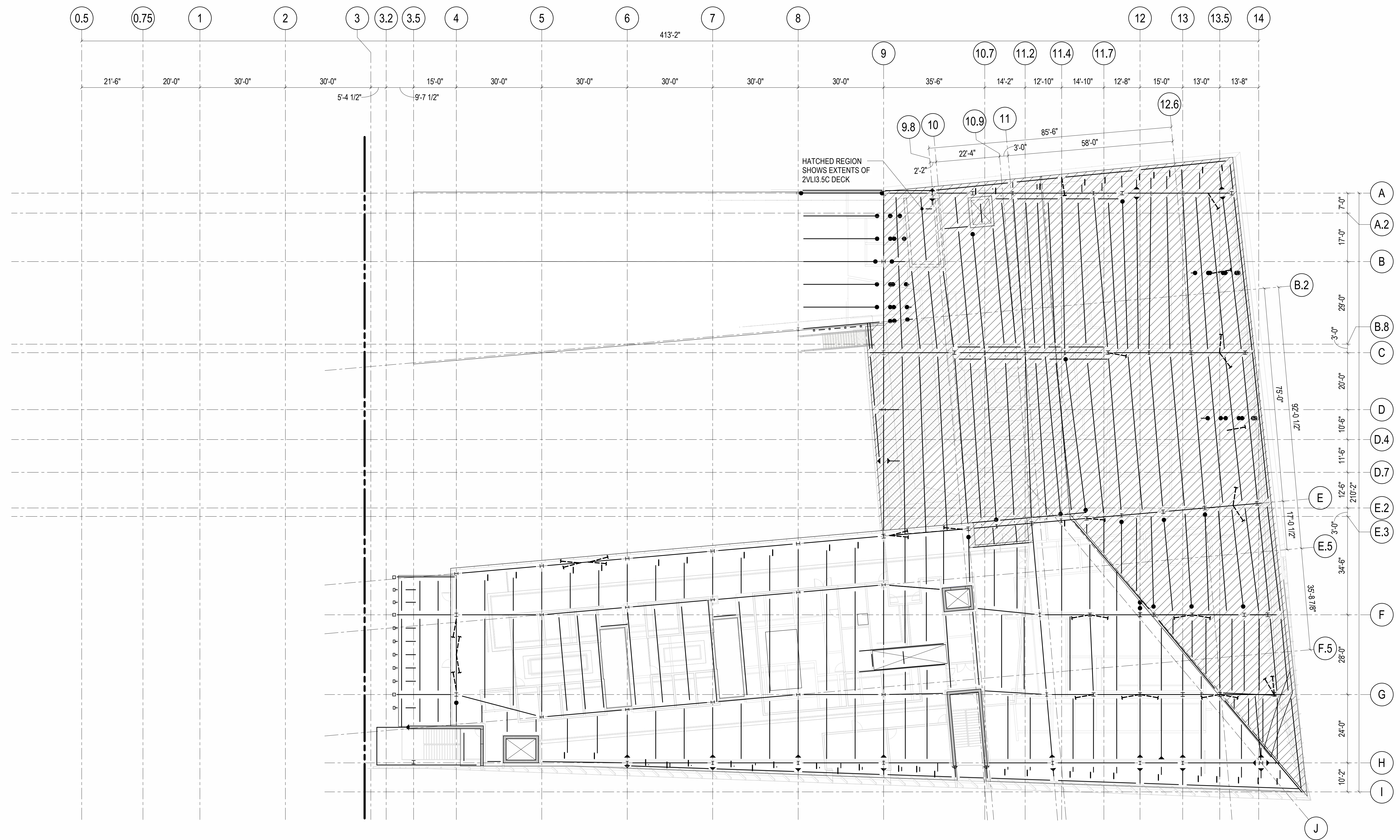
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 STRUCTURAL PLAN - 3RD FLOOR - AREA A

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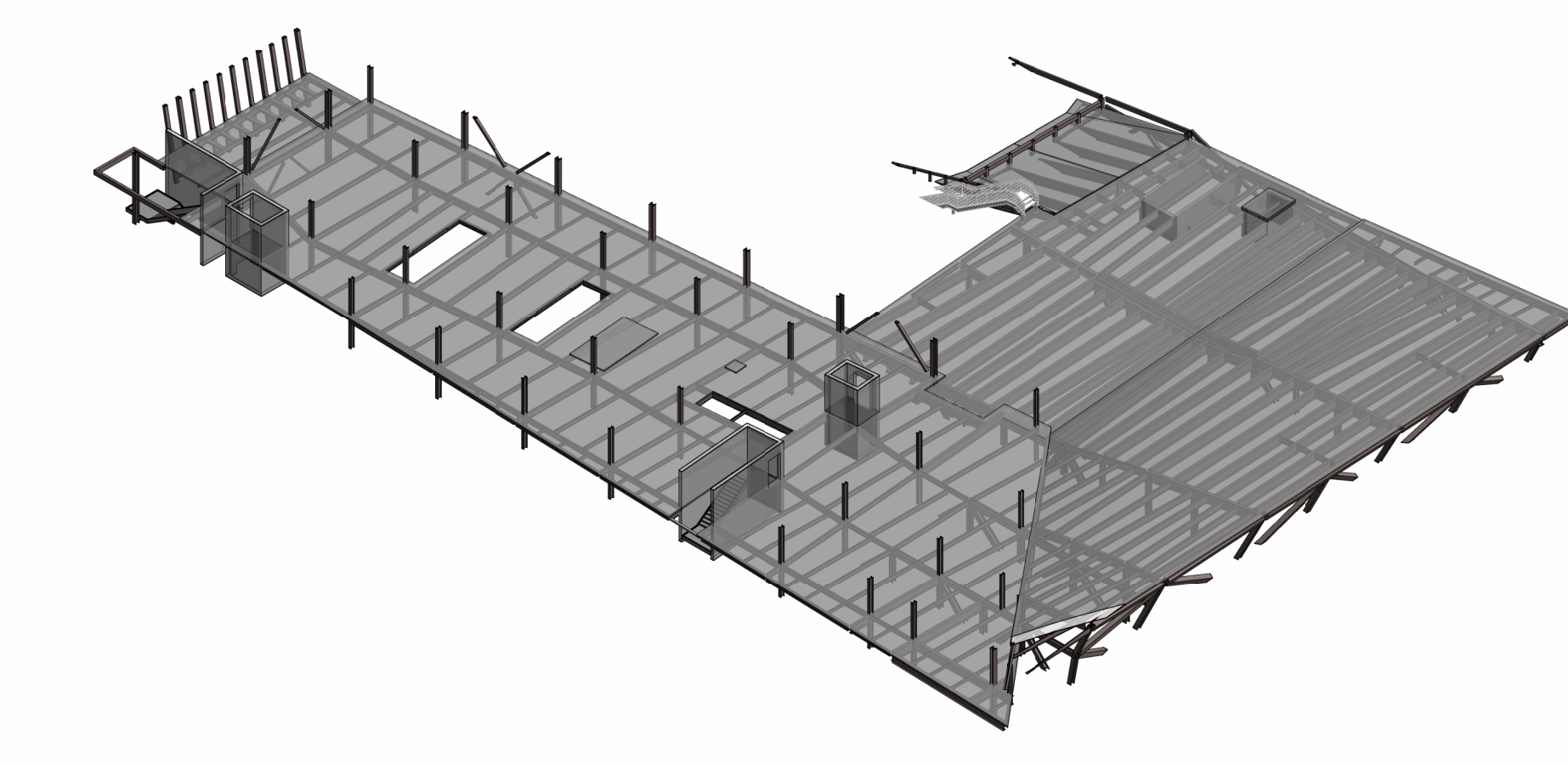
S103A

COMPOSITE STEEL FRAMING NOTES

1. GENERAL:
 - A. SEE S0 SERIES SHEETS FOR GENERAL NOTES, SYMBOLS AND ABBREVIATIONS.
 - B. SEE S6 SERIES SHEETS FOR TYPICAL STEEL DETAILS.
2. COLUMNS/PILASTERS:
 - A. ALL COLUMNS/PILASTERS ARE CENTERED ON THE INTERSECTION OF GRIDS BELOW THE SUPPORTED COLUMN UNLESS DIMENSIONED OTHERWISE ON PLAN.
3. STEEL BEAMS:
 - A. STEEL BEAMS SHALL BE EQUALLY SPACED BETWEEN GRIDLINES/COLUMNS/GIRDERS UNLESS DIMENSIONED OTHERWISE.
 - B. TOP OF STEEL BEAMS SHALL EQUAL BOTTOM OF METAL DECK ELEVATION. SEE PLAN FOR TOP OF CONCRETE ELEVATION AND SLAB THICKNESS TO DETERMINE BOTTOM OF METAL DECK ELEVATION.
 - C. REQUIRED BEAM END CONNECTION CAPACITY IN KIPS NOTED ON PLAN THUS: XXX. IF TWO SYMBOLS ARE SHOWN THEY DENOTE THE REQUIRED CONNECTION CAPACITY AT THE CORRESPONDING BEAM END. IF ONLY ONE SYMBOL IS SHOWN IT DENOTES THE REQUIRED CONNECTION CAPACITY AT BOTH ENDS OF THE BEAM. DETAIL CONNECTIONS FOR REQUIRED CONNECTION CAPACITY PER DETAIL 1/S500 FOR ALL TYPICAL SHEAR CONNECTIONS.
 - D. PLACE NUMBER OF SHEAR STUDS INDICATED ON PLAN THUS: (XX) PER DETAIL 18/S530. ALL SHEAR STUDS ARE 3/4"Ø.
4. METAL DECK:
 - A. SEE SHEETS S530 FOR TYPICAL METAL DECK DETAILS.
 - B. SEE DETAIL 28/S530 FOR DECK SUPPORT FRAMING REQUIRED AT DECK PENETRATIONS LARGER THAN 10" IN EITHER DIRECTION.
5. CONCRETE SLAB-ON-METAL DECK:
 - A. TOP OF CONCRETE SLAB NOTED ON PLAN.
 - B. LOCATE SLAB CONSTRUCTION JOINTS PER DETAIL 4/S530. SUBMIT LOCATIONS OF SLAB CONSTRUCTION JOINTS FOR REVIEW 3 WEEKS (MINIMUM) PRIOR TO PLACEMENT OF CONCRETE. SPACE JOINTS AND POUR SEQUENCES TO MINIMIZE SHRINKAGE CRACKS. SEE "CONCRETE NOTES" FOR JOINTING REQUIREMENTS AT SLAB-ON-DECK.
 - C. SEE DETAIL 28/S530 FOR ADDITIONAL REINFORCING REQUIRED AT SLAB PENETRATIONS/OPENINGS/REENTRANT CORNERS.
 - D. SEE ARCHITECTURAL DRAWINGS FOR SLAB EDGE DIMENSIONS.
 - E. SEE 30/S500 FOR TREE TIE DOWNS. COORD LOCATIONS WITH LANDSCAPE.
6. MECHANICAL AND ELECTRICAL EQUIPMENT:
 - A. SEE 24/S530 FOR REQUIREMENTS AT MECHANICAL AND ELECTRICAL EQUIPMENT PADS.
 - B. CONTRACTOR TO VERIFY ALL EQUIPMENT WEIGHTS, SIZES, LOCATIONS, AND OPENINGS REQUIRED WITH MECHANICAL CONTRACTOR. CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CHANGES IN THE WEIGHTS OR LOCATIONS SHOWN ON THE DRAWINGS. SUCH CHANGES IN CONDITIONS SHALL BE SUBJECT TO STRUCTURAL ENGINEER REVIEW. RE: MECHANICAL AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL OPENINGS NOT SHOWN.
 - C. MECHANICAL EQUIPMENT WEIGHTS, IN KIPS, NOTED ON PLAN THUS: XXX.
 - D. EQUIPMENT TO BE PLACED TO BEAR ON TWO BEAMS MINIMUM.



OVERALL PLAN - 4TH FLOOR
1/16" = 1'-0"



ISOMETRIC - 4TH FLOOR
NO SCALE

DESIGNERS: ETHAN WICKS, CRISTINA CHILDRIS, CALEB CHENUT
 LEAD REVIT TECH: BRAY WELLS
 ARCHITECT: JAMES W. FISHER, INC.
 PROJECT MANAGER: BAILEIGH FISHER

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PSW Job Number:
985A

2/24/2023

AWSOM
Bentonville, AR

Issue Date:
02.24.2023

NUMBER	DATE	DESCRIPTION
1	06.29.2023	ACCENDIA.02

Contents:
OVERALL PLAN - 4TH FLOOR

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S104



REVISIONS	
NUMBER	DATE
1	03/15/2023 ACCORDIAN 1
2	06/09/2023 ACCORDIAN 2



STRUCTURAL PLAN - 4TH FLOOR - AREA A
1/8" = 1'-0"

DESIGNERS: ETWANKO, CHRISTINA; CALDERON, CALEB
LEAD REVIT TECH: BRANWELL, TAM
PROJECT MANAGER: BAILEIGH FISHER
ASST. PRINCIPAL: ASST. PRINCIPAL
PRINCIPAL: PRINCIPAL
FILE PATH: FILE PATH



2/24/2023

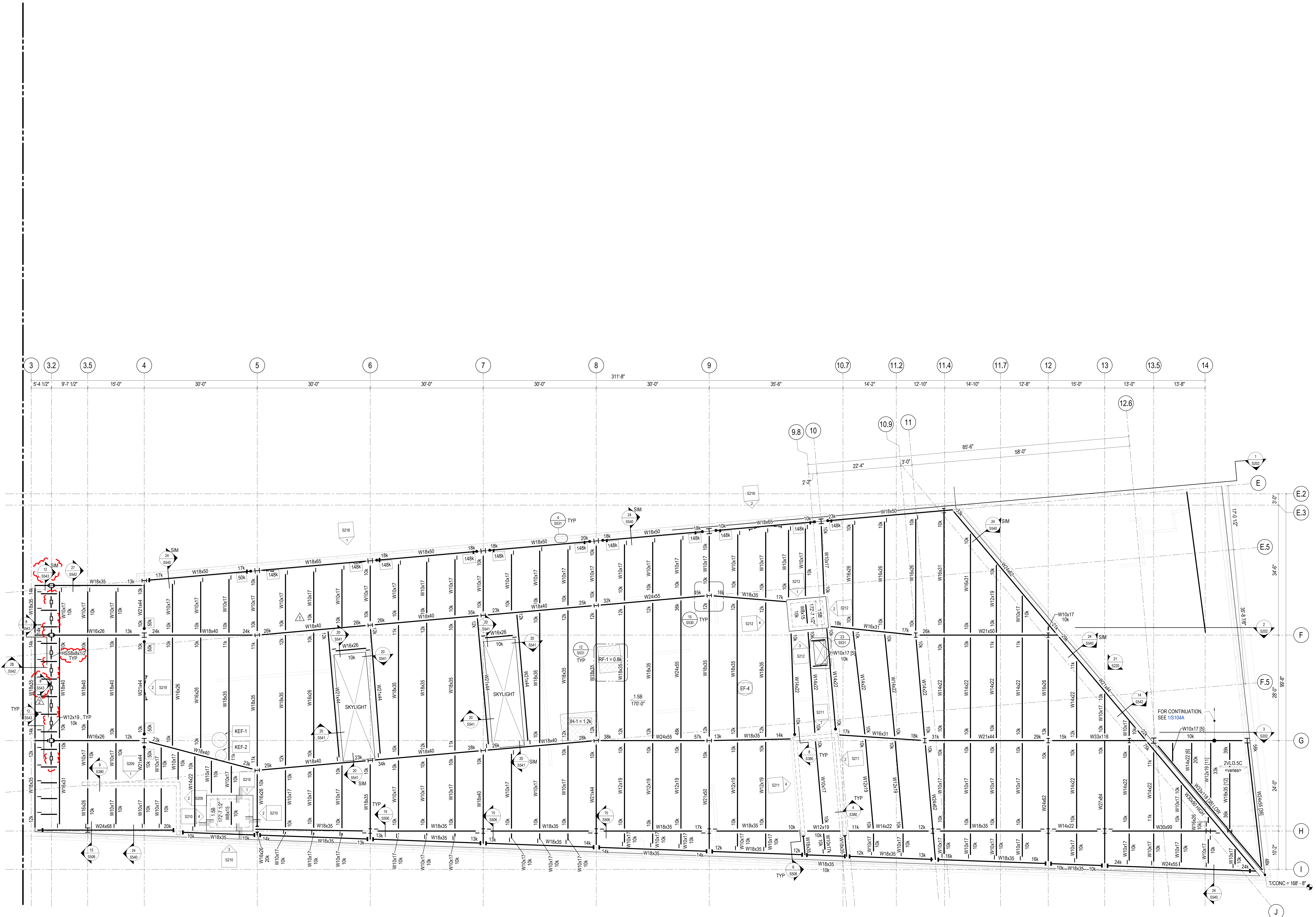
AWSOM
 Bentonville, AR

Issue Date:
02.24.2023

REVISIONS	
NUMBER	DATE
1	03/13/2023 ACCORDIAN 1
2	06/29/2023 ACCORDIAN 2

Contents:
STRUCTURAL PLAN - 5TH FLOOR - AREA A

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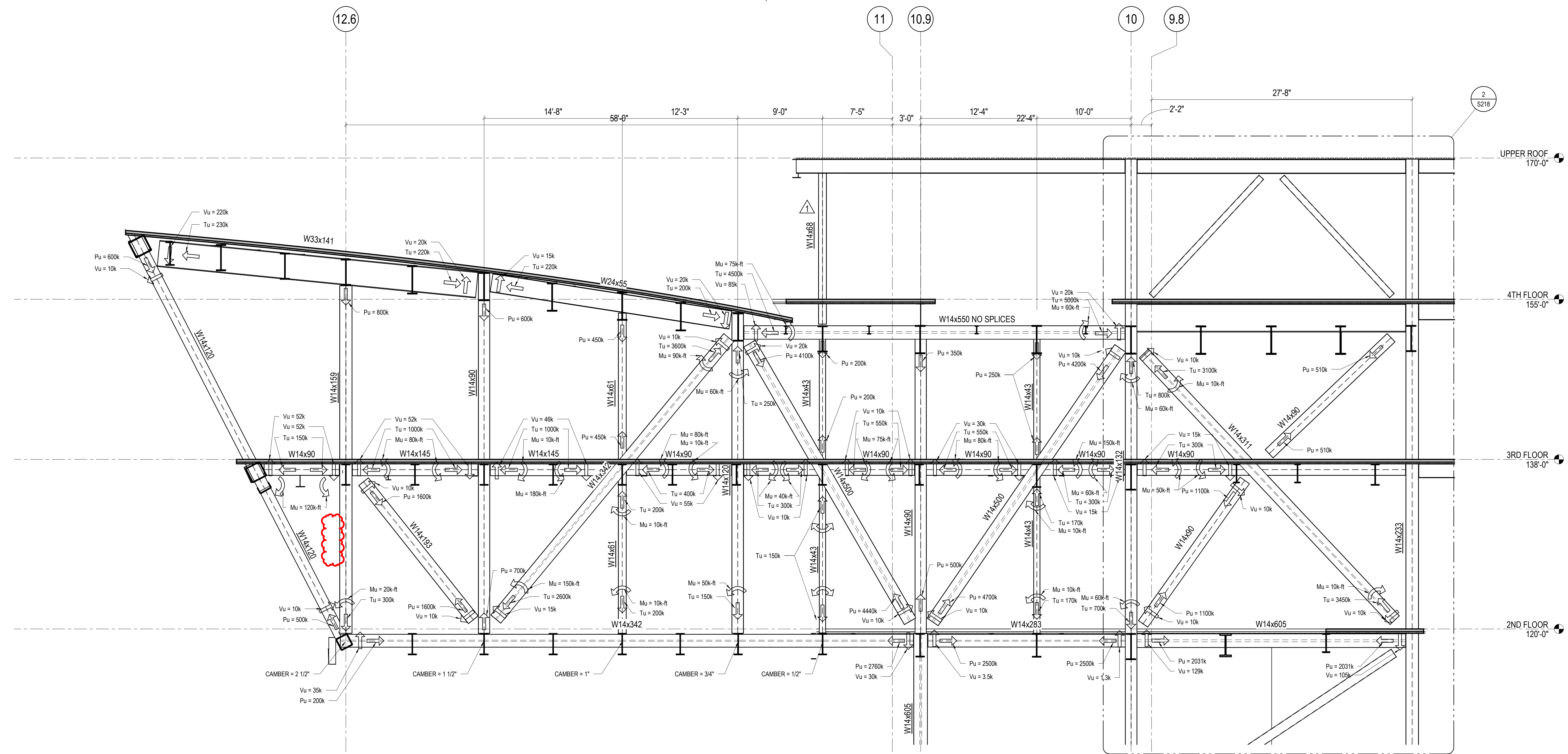


DESIGNERS: ETHAN WICKS, CHRISTINA CHILDRESS, CALEB CHENET
 LEAD REVIT: TONY BRAUNWELLS, JAM
 PROJECT MANAGER: BAILEIGH FISHER

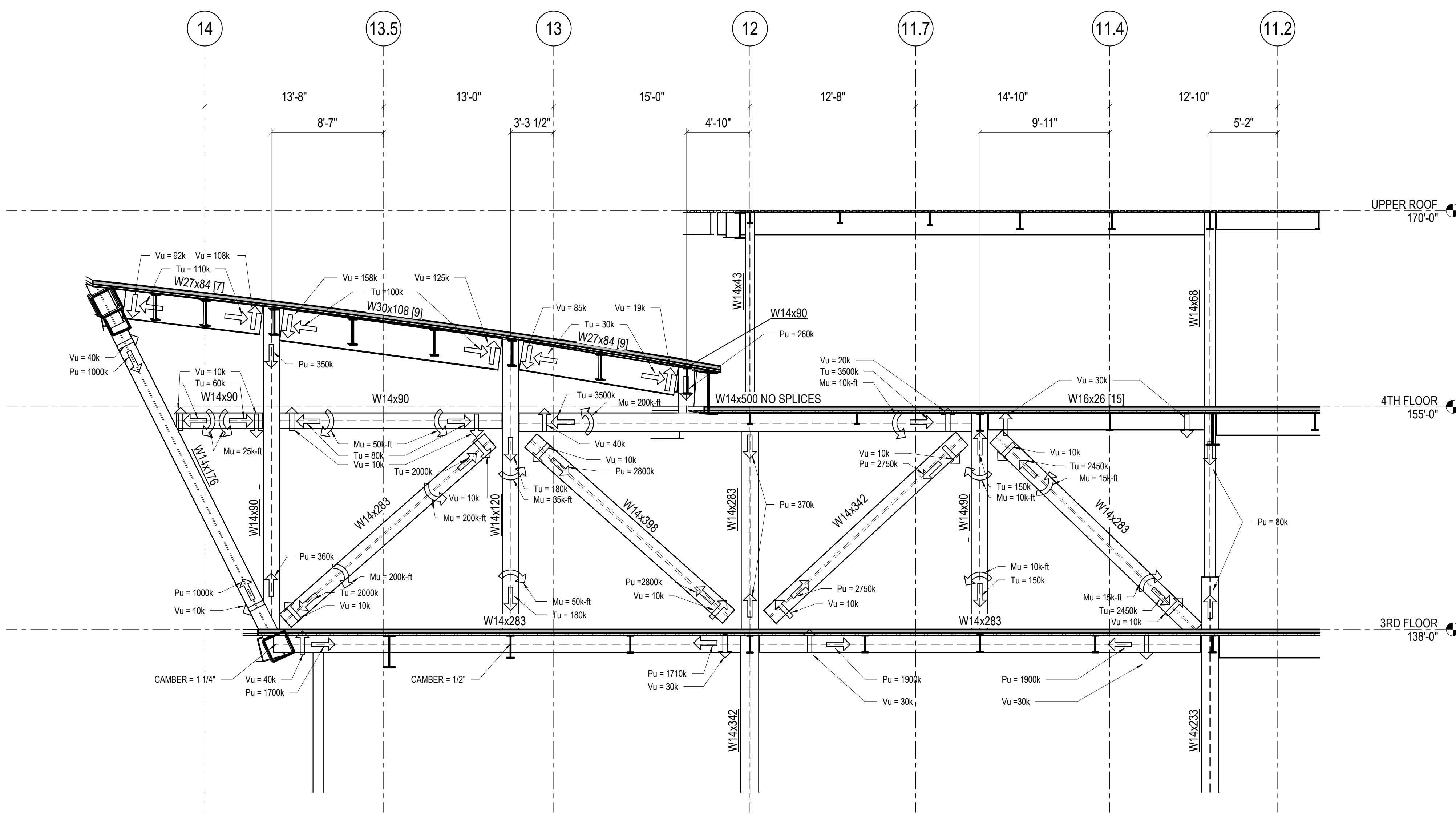
ASST: 1818.571
 BENTONVILLE
 PROJECT MANAGER: BAILEIGH FISHER

STRUCTURAL PLAN - UPPER ROOF - AREA A
 1/8" = 1'-0"

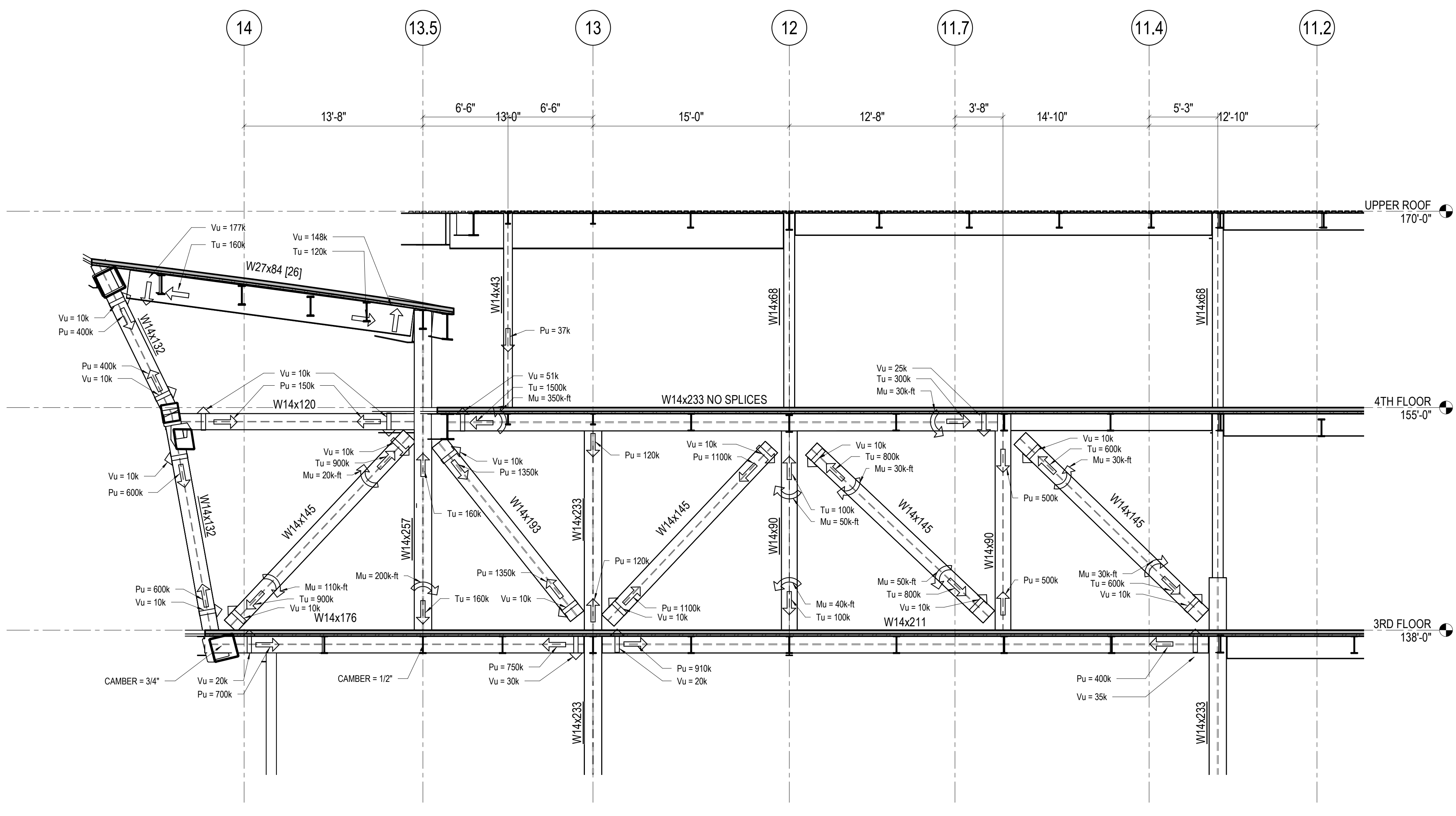
- 1) DEFERRED SUBMITTALS:
 - 1A) TRUSS CONNECTIONS ARE A DEFERRED SUBMITTAL. SEE CONCEPTUAL DETAILS FOR TRUSS CONNECTION TYPES.
 - 1B) TRUSS MEMBER AND SUPPORTING COLUMN SPLICE CONNECTION LOCATIONS SHALL BE AS SHOWN IN THESE DRAWINGS.
 - 1C) CHORD WF SPLICES SHALL BE BOLTED SPLICE CONNECTIONS.
- 2) ERECTION:
 - 2A) THE OVERALL STRUCTURE, INCLUDING TRUSSES, IS DESIGNED TO FUNCTION AS A COMPLETED SYSTEM OF STEEL FRAMEWORKS AND CONCRETE DIAPHRAGMS. THE CONTRACTOR'S ENGINEER SHALL EVALUATE THE STABILITY OF THE PARTIALLY COMPLETED STRUCTURE THROUGHOUT THE CONSTRUCTION PROCESS AND PROVIDE TEMPORARY WORKS AS REQUIRED TO SUPPORT CONSTRUCTION ACTIVITIES AND SEQUENCING.
 - 2B) CONTRACTOR IS RESPONSIBLE FOR ERECTION ENGINEERING, TEMPORARY STABILITY INCLUDING LATERAL STABILITY, AND ANY TEMPORARY SHORING SYSTEMS. SHORING SYSTEMS SHALL BE DESIGNED FOR ALL DEAD, LIVE, CONSTRUCTION AND SUPERIMPOSED LOADS FROM THE TRUSSES.
 - 2C) SHORING AND BRACING SYSTEMS USED DURING CONSTRUCTION SHALL BE SIGNED AND SEALED BY A LICENSED DESIGN PROFESSIONAL IN THE STATE OF ARKANSAS.
- 3) CAMBER:
 - 3A) SEE TRUSS ELEVATIONS FOR CAMBER PROVIDED AT PANEL POINT LOCATIONS.
- 4) TRUSS MEMBER MATERIALS:
 - 4A) TRUSS WIDE FLANGE MEMBERS AND SUPPORTING COLUMNS SHALL BE A913 GRADE 65
 - 4B) TRUSS HSS MEMBERS SHALL BE A500 GRADE C
- 5) CONNECTION DETAILS:
 - 5A) REFER TO TRUSS ELEVATIONS ON S201 FOR TYPICAL CONNECTION DETAIL REFERENCES



1 TRUSS E - ELEVATION
3/16" = 1'-0"



2 TRUSS F - ELEVATION
3/16" = 1'-0"



3 TRUSS G - ELEVATION
3/16" = 1'-0"

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479.444.0633 office
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FAVETTEVILLE, AR 72703
P: 479.443.2377

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8340 LEXA DRIVE, STE 300
LEXINGTON, MS 38214
P: 913.660.8191

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224 SOUTH MICHIGAN AVENUE
CHICAGO, IL 60604
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SIGNAGE - WAYFINDING
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FOOD SERVICE
JMC HOSPITALITY
8955 SKYWAY DR., SUITE 8210
THE WOODLANDS, TX 77380
P: 689.841.2222

WATER FEATURES
OTL
2150 S. TOWNE CENTER, SUITE 100
ANNARICA, CA 92009
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IRRIGATION
WC3 DESIGN
11A ROBINSON MANOR BLVD.
MOORESBURG, VA 24155
P: 844.231.7042

PSW Job Number:
995A

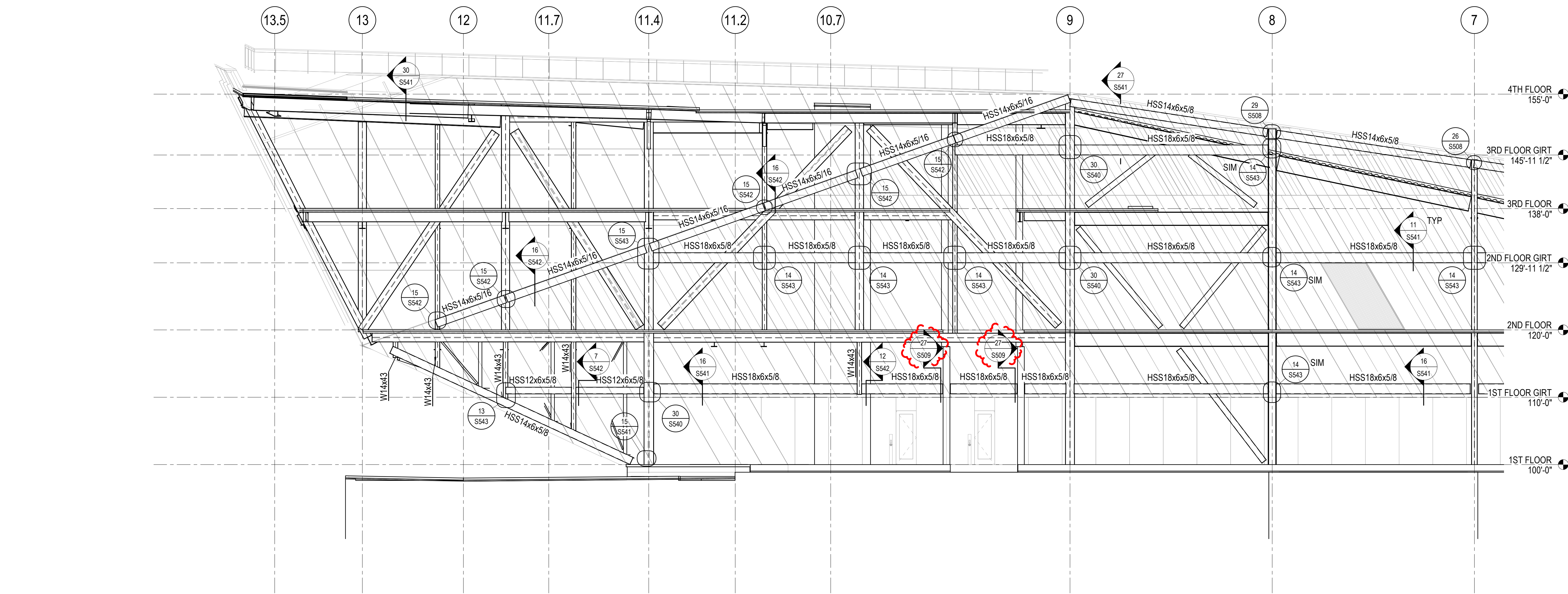


Issue Date:
02.24.2023

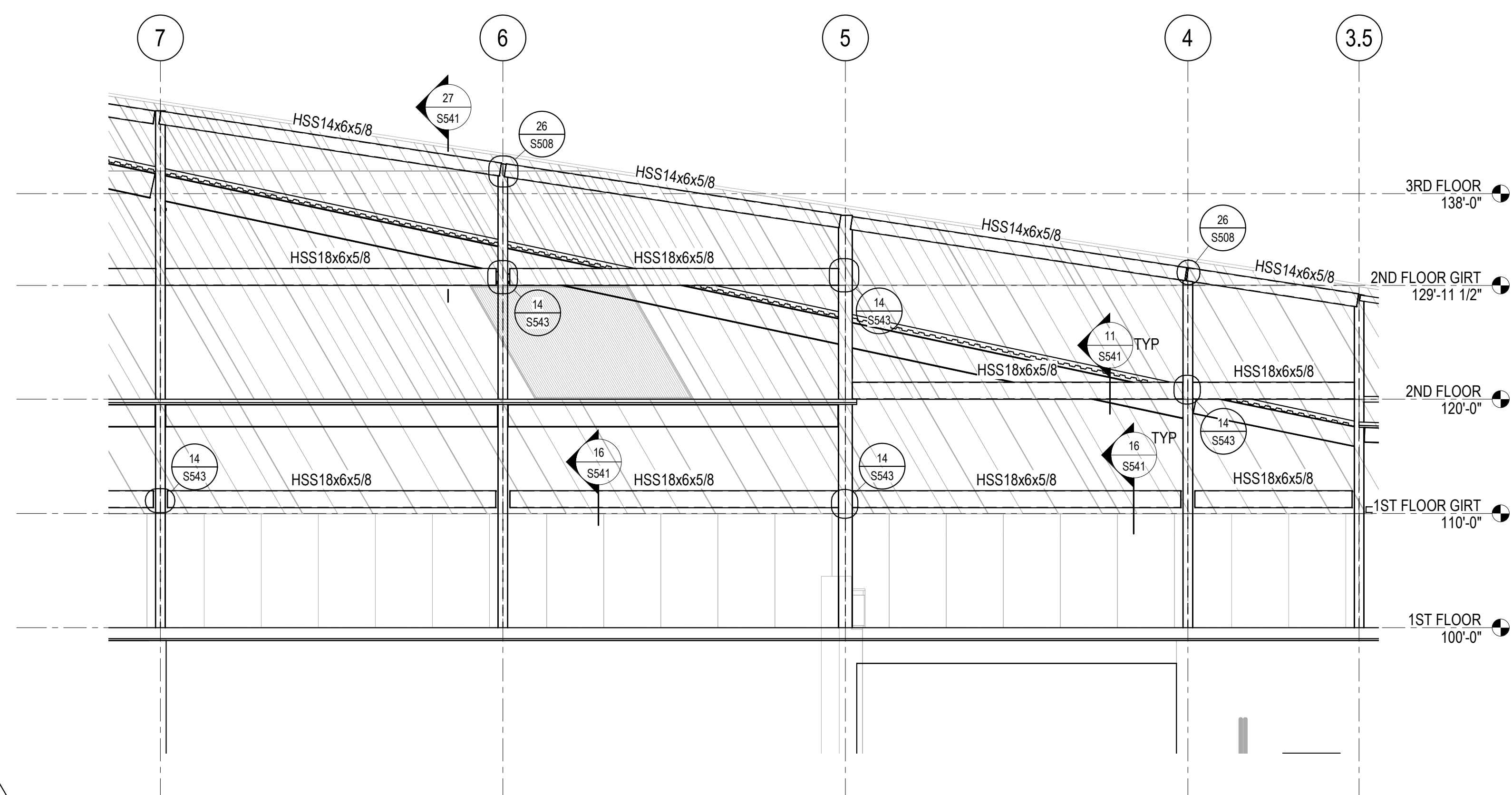
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NUMBER	DATE / DESCRIPTION
1	03.13.2023 ACCENDUM 1
2	06.29.2023 ACCENDUM 2

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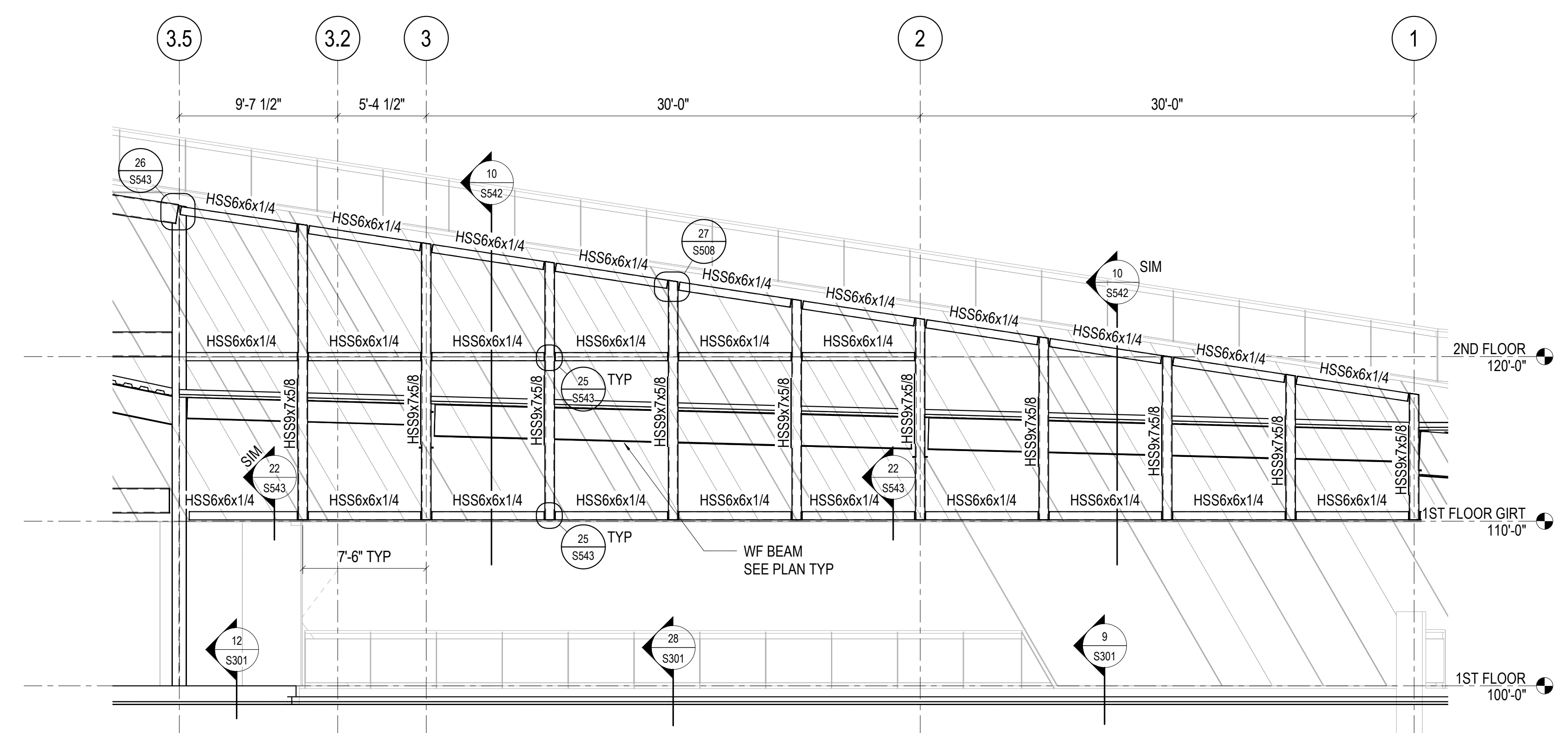
DESIGNERS: ETHAN WICKS, CRISTINA CHILDRIS, CALEB CHESNUT
 LEAD DESIGNER: TERRY BRANNWELL, P.E.
 PROJECT MANAGER: BAILEIGH FISHER
 ASST. 1818.521
 BENTONVILLE
 PROJECT MANAGER: BAILEIGH FISHER



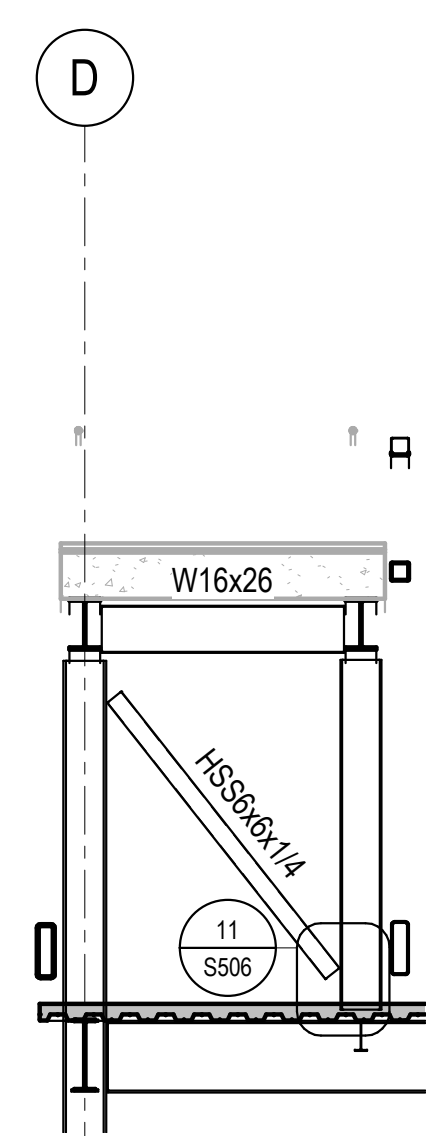
1 NORTH PRECAST EXTERIOR WALL GIRTS - GRID 7-13
1/8" = 1'-0"



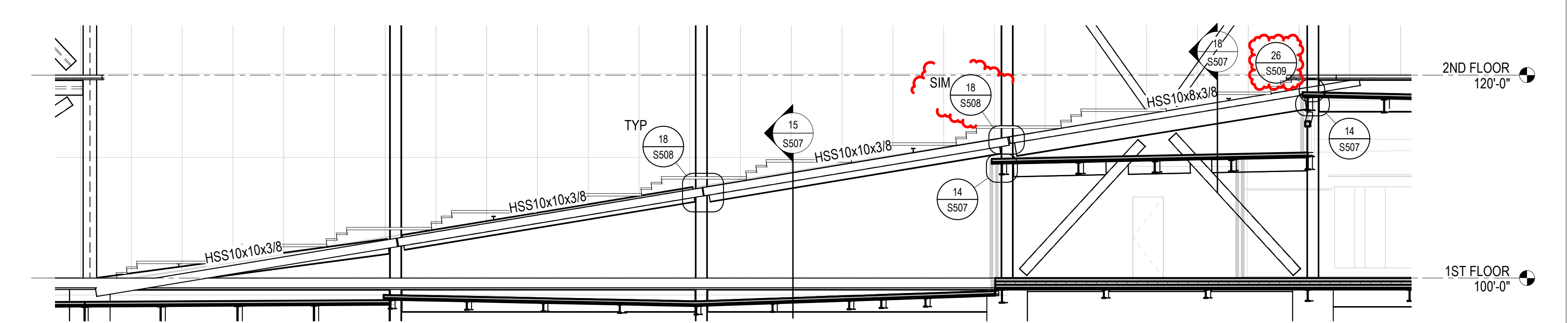
2 NORTH PRECAST EXTERIOR WALL GIRTS - GRID 3.5-7
1/8" = 1'-0"



3 NORTH PRECAST EXTERIOR WALL GIRTS - GRID 1-3.5
3/16" = 1'-0"



4 PLAZA STAIR BRACE FRAME
3/16" = 1'-0"

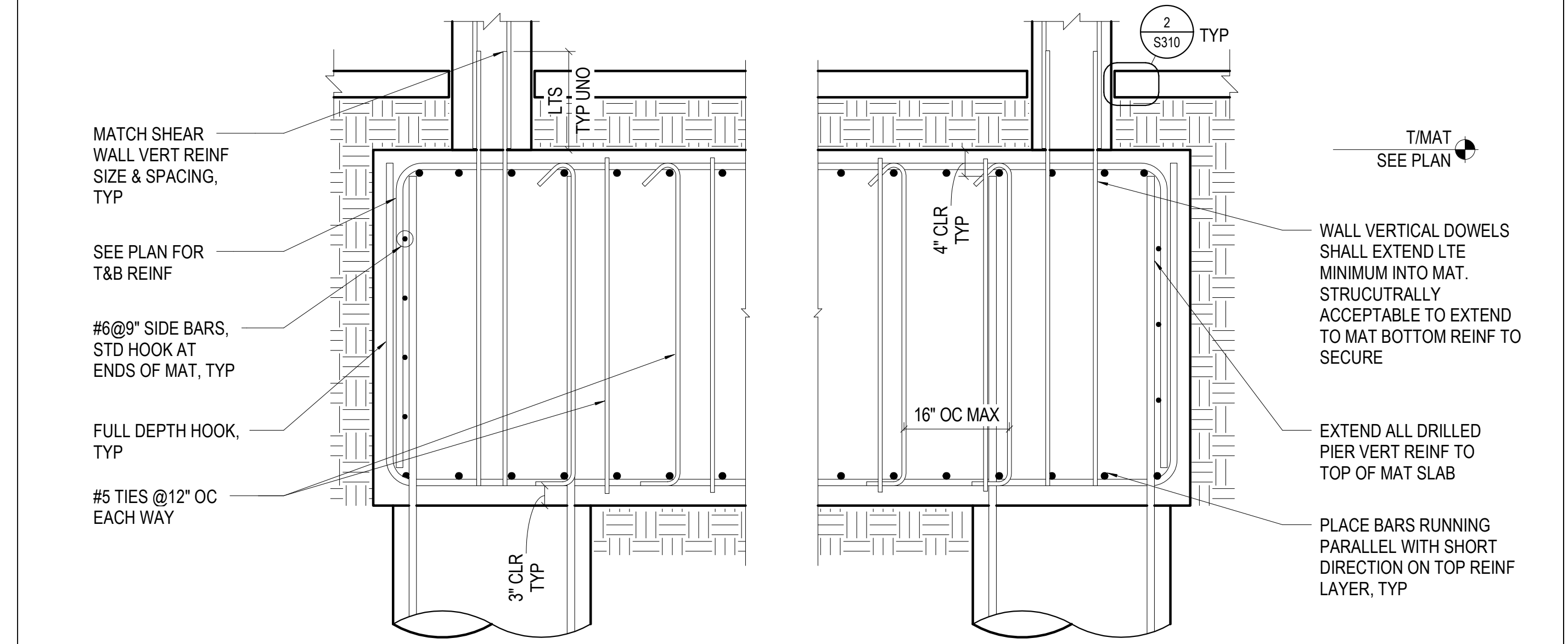


5 EXTERIOR WALL GRID E
1/8" = 1'-0"

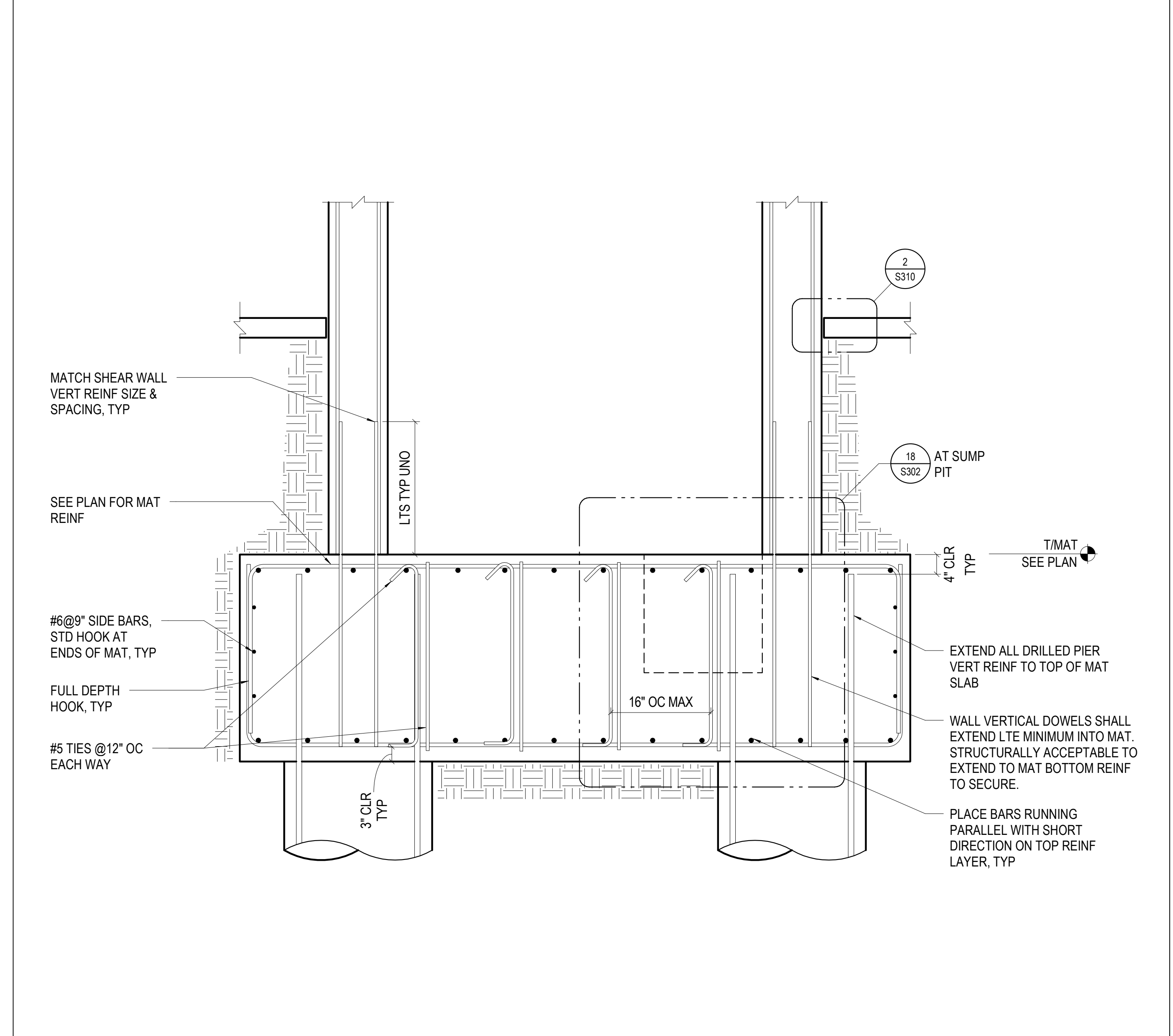
- PRECAST SUPPORT NOTES:**
1. SEE ELEVATIONS AND DETAILS FOR SUPPORT OF PRECAST PANELS.
 2. FINAL PANEL LAYOUT INCLUDING PANEL JOINTS AND LOADING CONDITIONS TO THE PRIMARY STRUCTURE IS BY THE CONTRACTOR.
 3. PRECAST PANELS MAY SUPPORT OTHER ARCHITECTURAL FINISHES SUCH AS COLD FORMED METAL STUDS AND/OR CURTAIN WALL. CONTRACTOR COORDINATE LOADS WITH PRECASTER.
 4. ALL PRECAST DEAD AND WIND LOADS AND LOCATIONS SHALL BE SUBMITTED PRIOR TO STEEL SHOP DRAWINGS, LOADS, LOADING LOCATIONS, AND PANEL SIZES THAT SUBMITTED FROM THE STRUCTURAL DRAWINGS MAY REQUIRE ALTERNATE STEEL SIZES AND DETAILS.
 5. PRECAST SUPPORT DETAILS INCLUDING GIRTS, SLAB EDGES, AND SPANDREL BEAMS ARE FOR PRICING ONLY. FINAL DESIGN IS PENDING PRECAST POINT LOAD LOCATIONS.



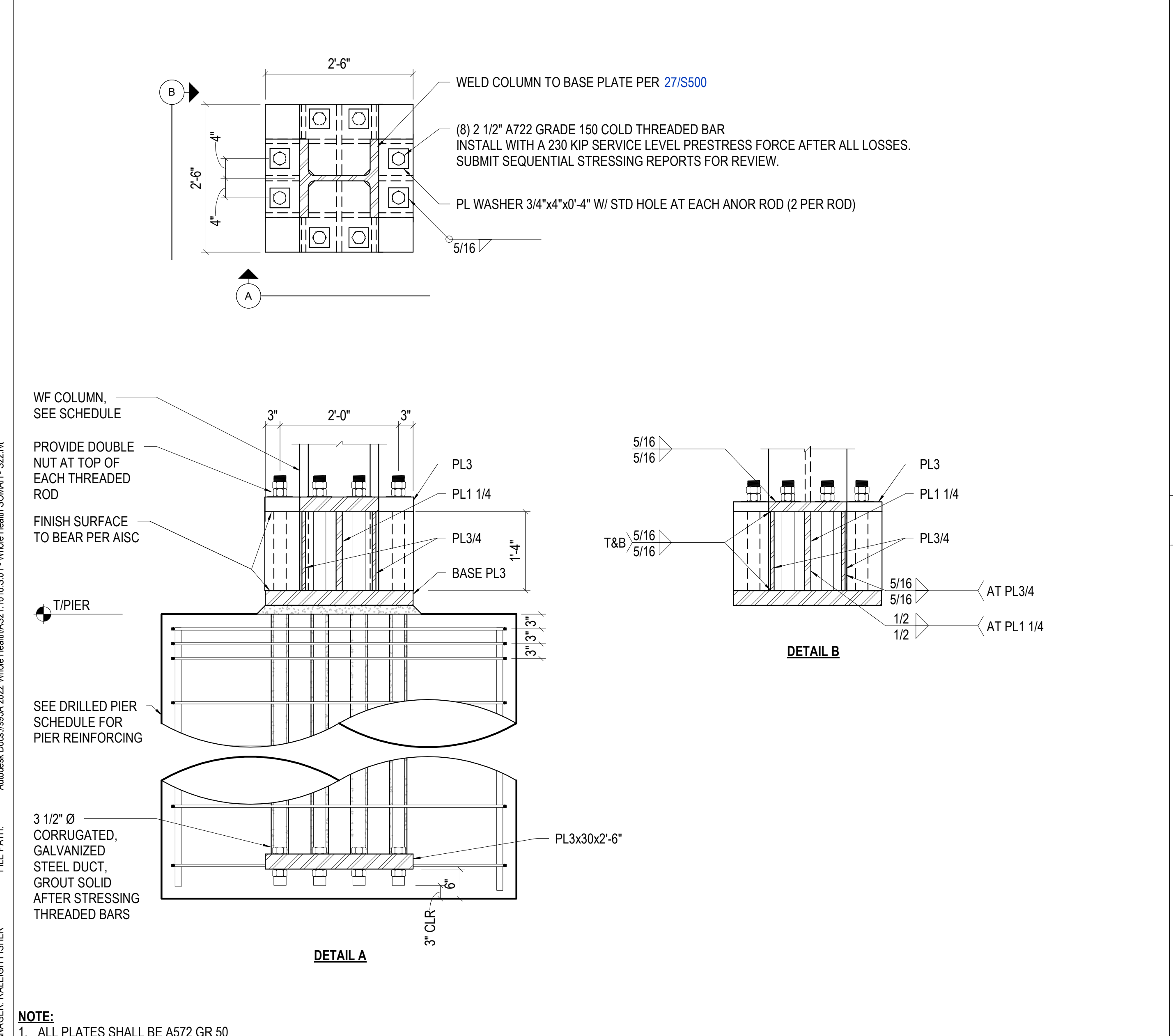
REVISIONS		
NUMBER	DATE	DESCRIPTION
1	03.13.2023	ACCENDIAM 1
2	06.29.2023	ACCENDIAM 2



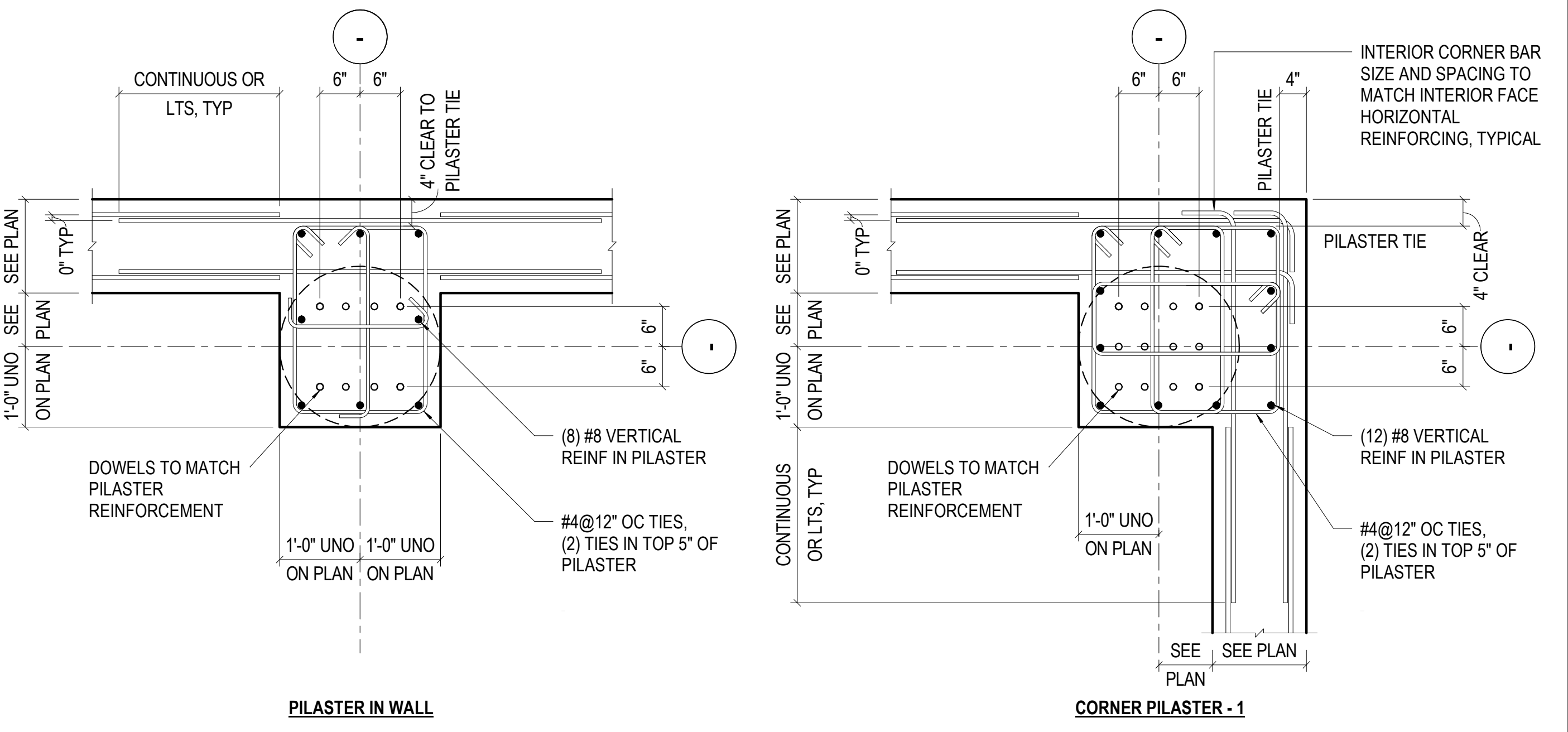
26 3/4" = 1'-0" TYP STAIR CORE MAT



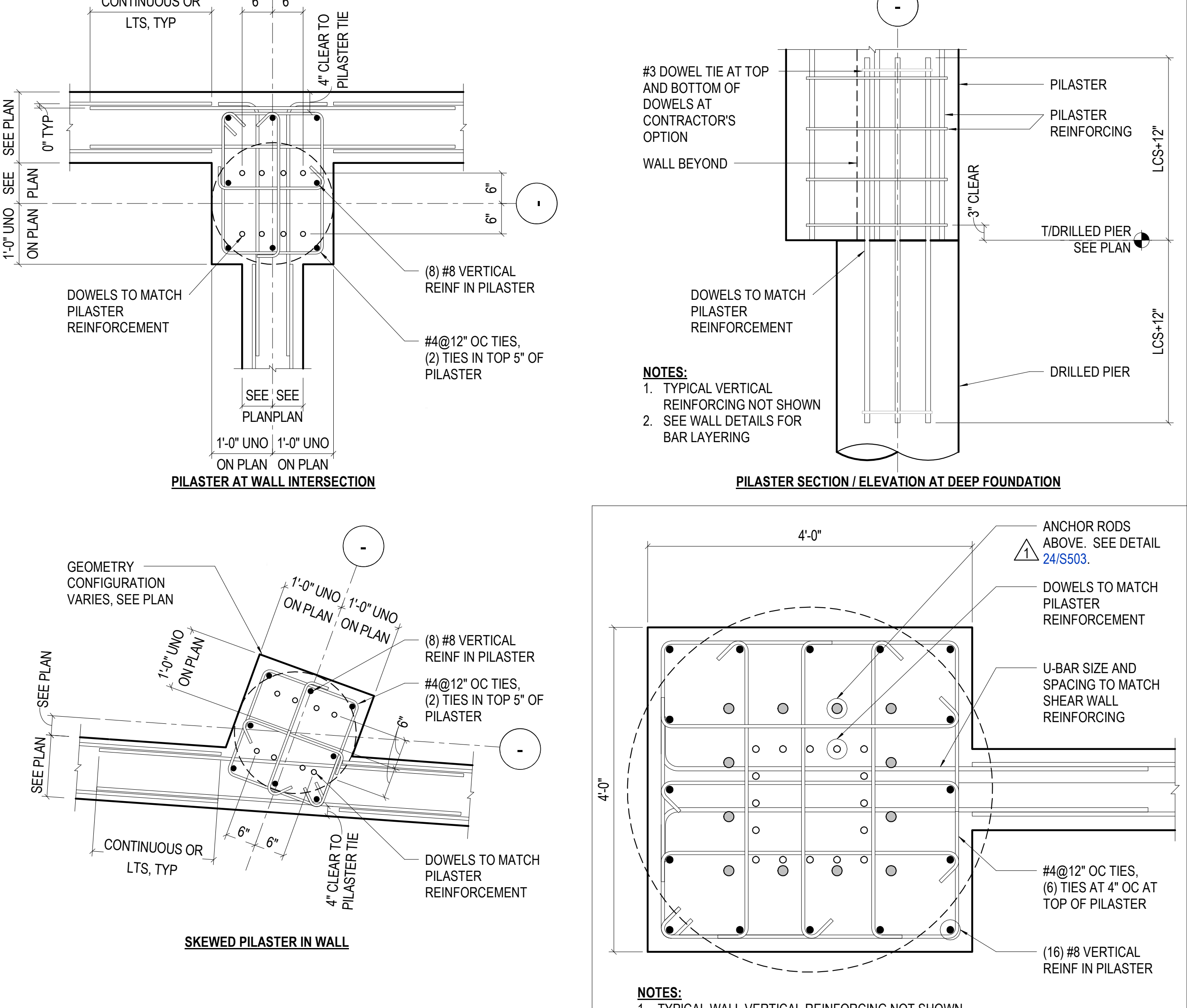
28 3/4" = 1'-0" TYP ELEV CORE MAT FOUNDATION



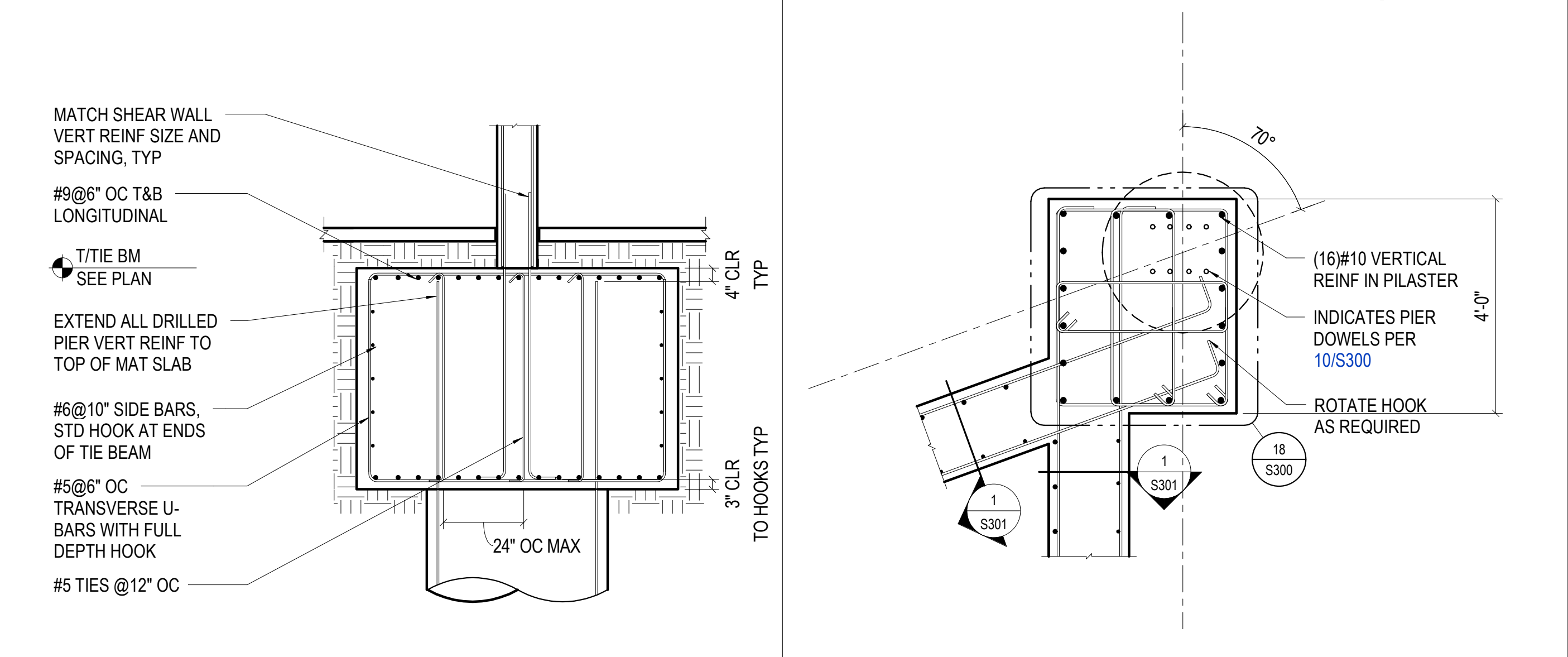
30 3/4" = 1'-0" TRUSS TENSION PIER



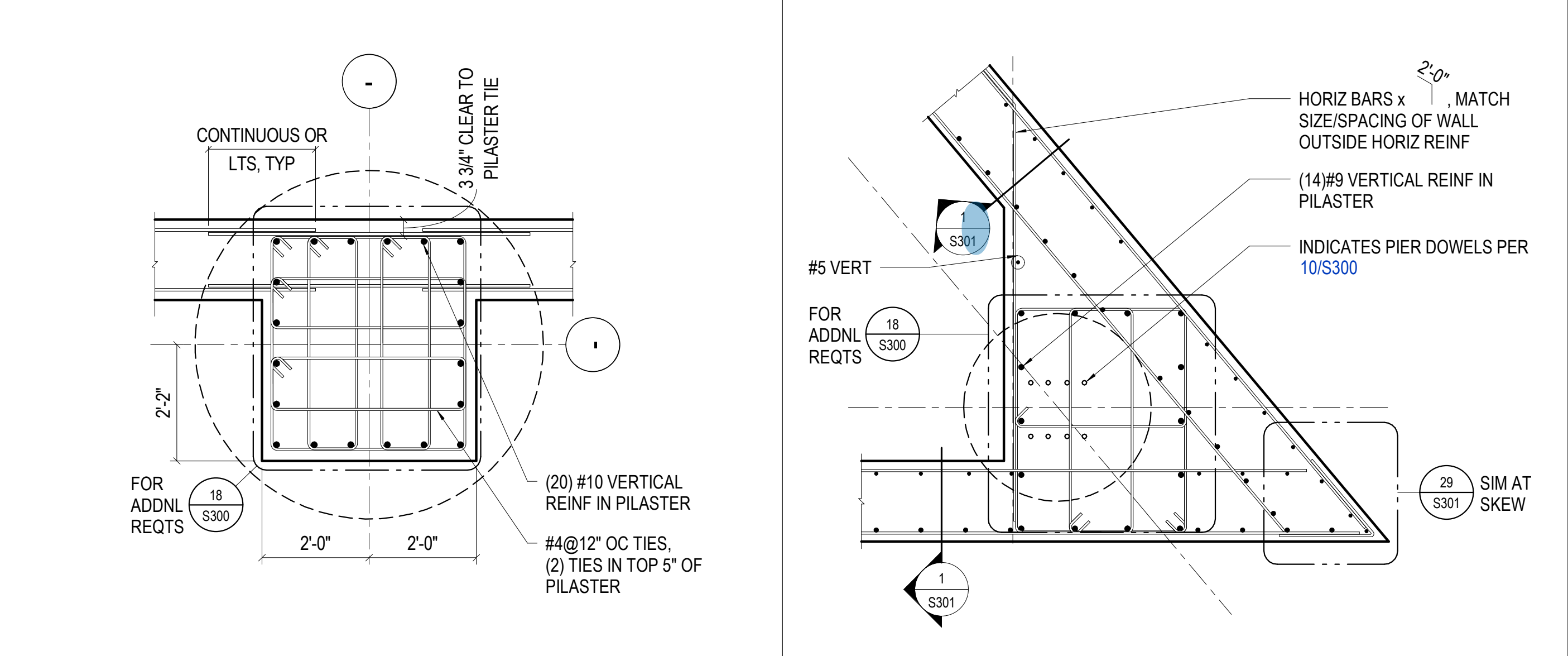
18 NO SCALE PILASTER DETAILS AT EXTERIOR WALLS AT DRILLED PIERS



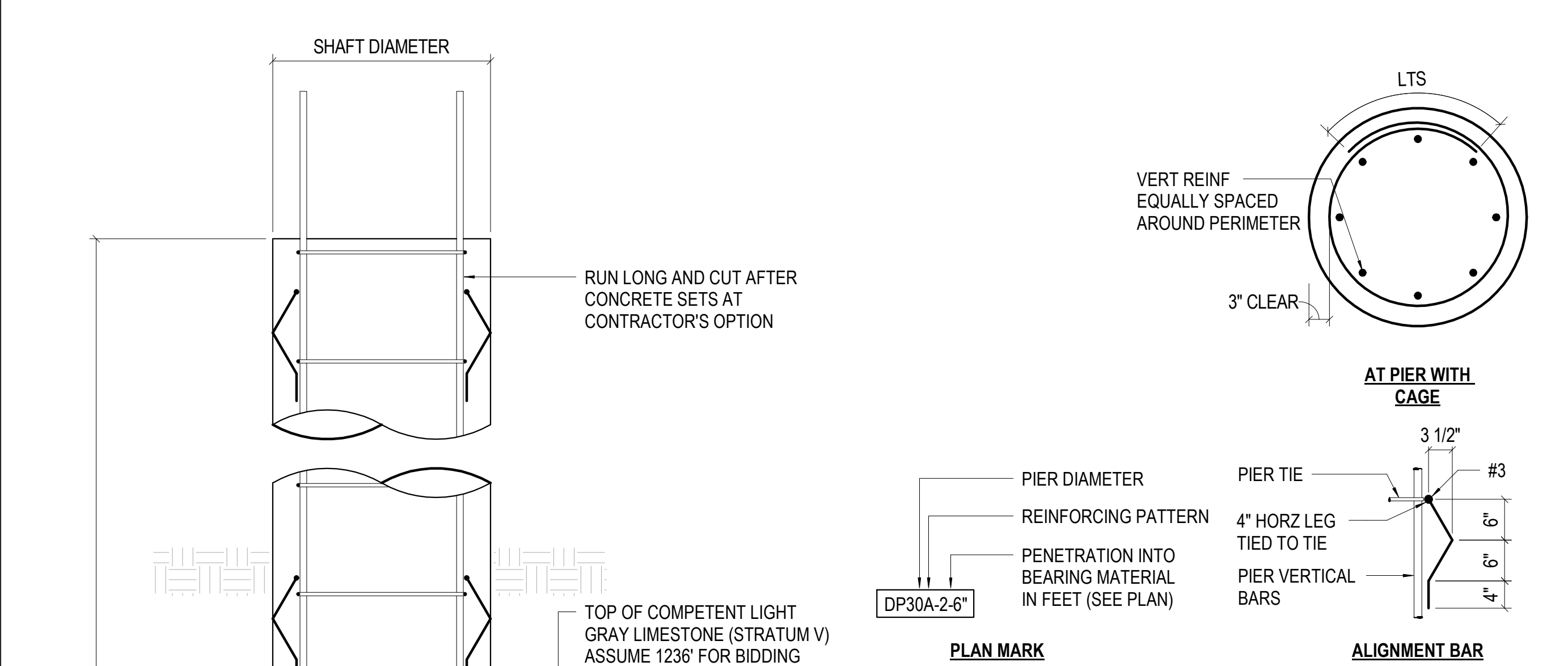
13 1" = 1'-0" PILASTER AT INTERIOR WALL



19 3/8" = 1'-0" TIE BEAM WEST



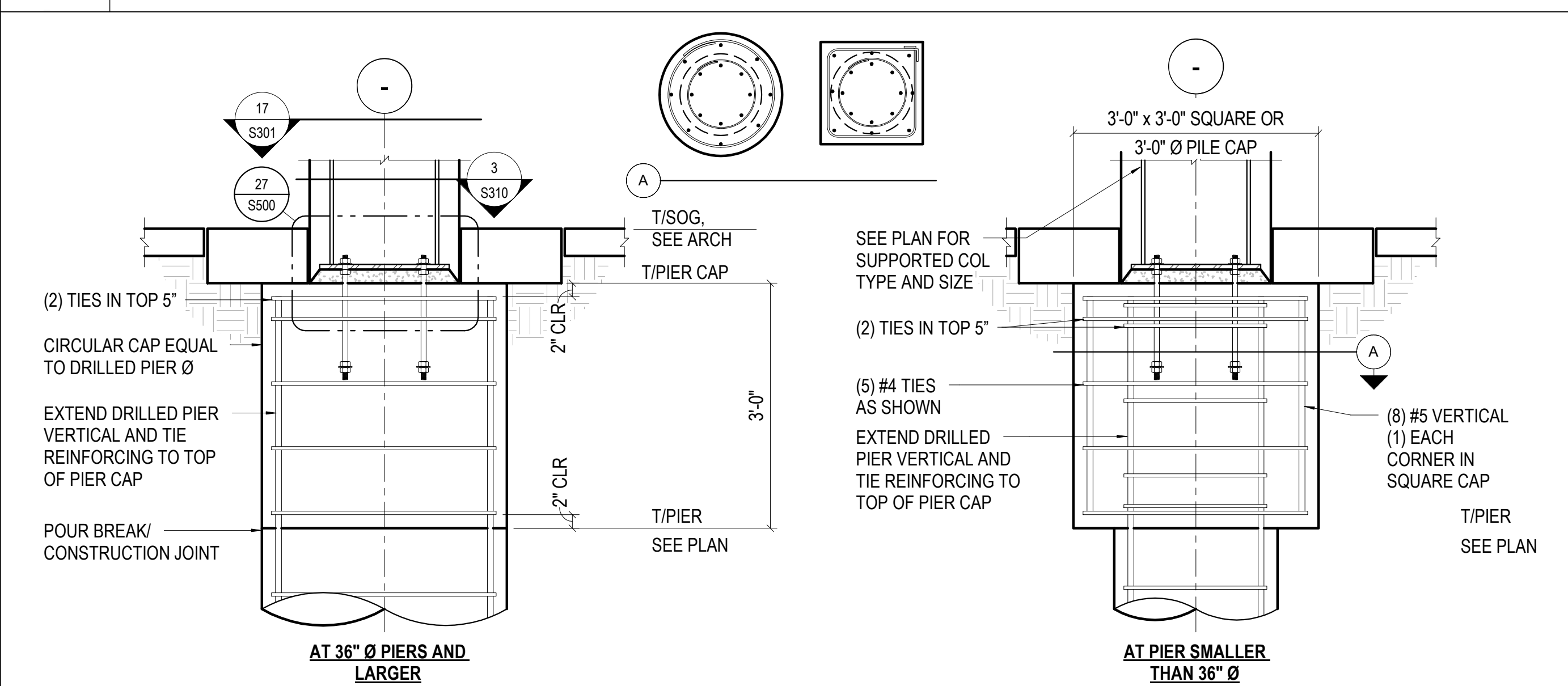
20 1/2" = 1'-0" PILASTER AT GRIDS 11.4/A



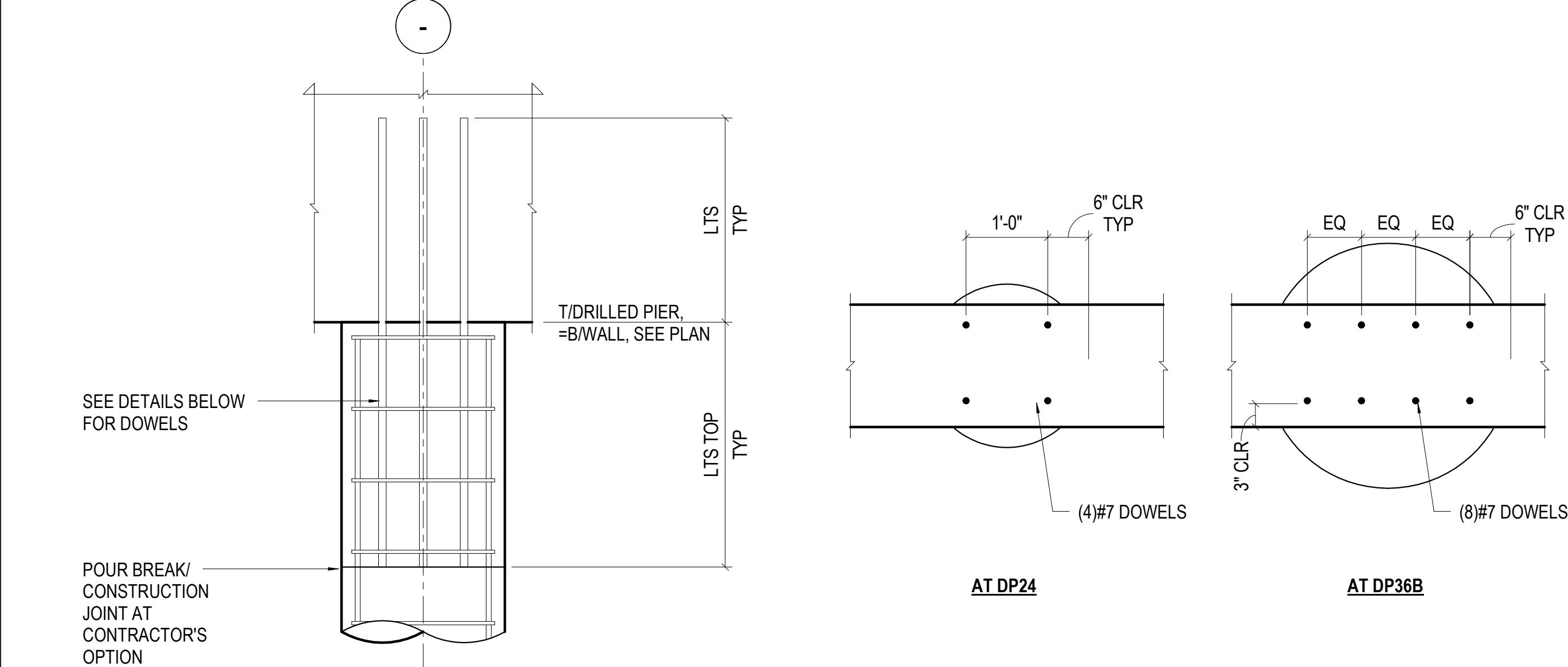
7 3/4" = 1'-0" DRILLED PIER SCHEDULE

TYPE	SHAFT DIA.	REINFORCEMENT	
		VERTICAL	TIES
DP24	24"	(6)#6	#4@18"
DP30	30"	(5)#7	#4@18"
DP36A	36"	(7)#8	#4@18"
DP36B	36"	(2)#9	#4@8"
DP36C	36"	(14)#9	#4@18"
DP42A	42"	(12)#10	#4@18"
DP42B	42"	(32)#10	#5@8"
DP42C	42"	(20)#10	#5@8"
DP48A	48"	(8)#10	#5@8"
DP48B	48"	(32)#10	#5@8"
DP48C	48"	(24)#10	#5@8"
DP54	54"	(44)#10	(2) #5@8"
DP60	60"	(12)#10	#5@8"
DP60B	60"	(18)#10	#5@8"
DP78	78"	(20)#10	#5@8"
DP84	84"	(25)#10	#5@8"
DP96	96"	(30)#10	#5@8"

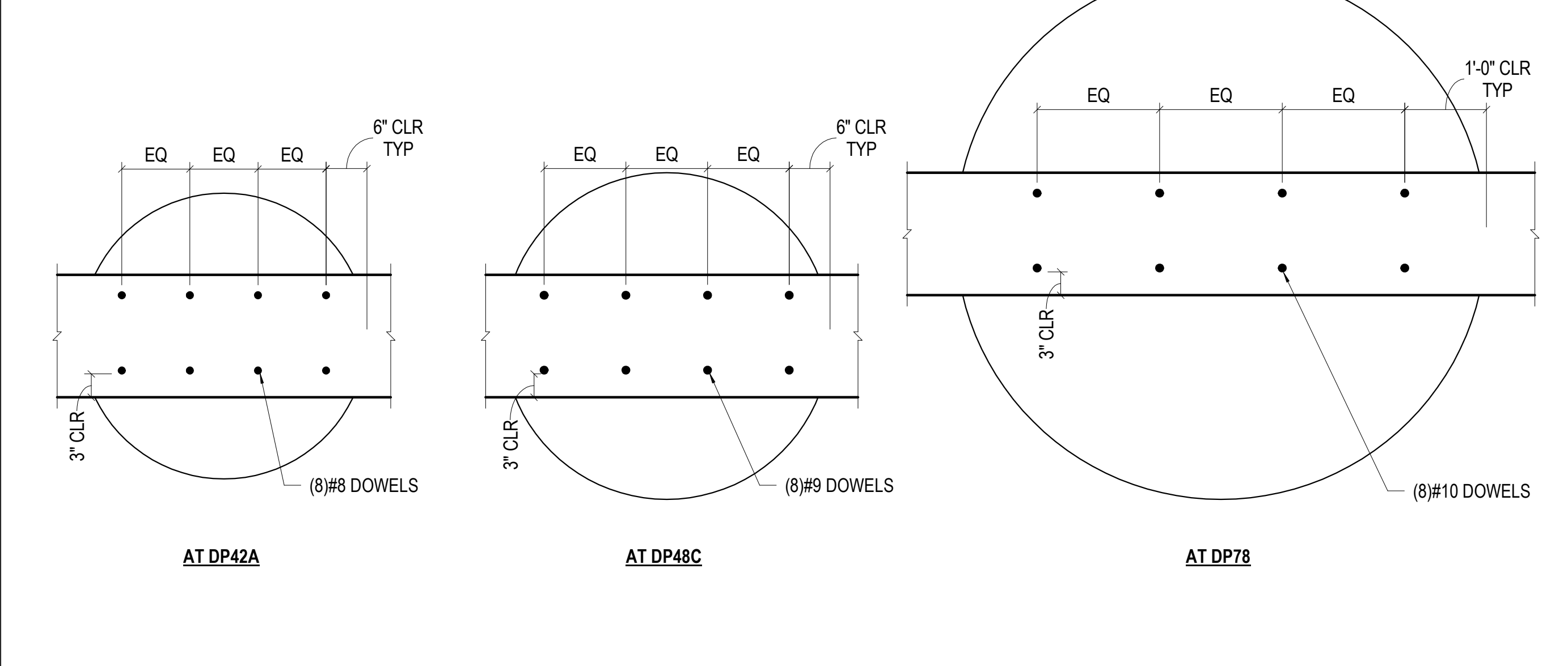
8 3/4" = 1'-0" TYPICAL DRILLED PIER AT INTERIOR GRAVITY COLUMN



14 1/2" = 1'-0" PILASTER AT GRIDS 3.5/E.3



15 1/2" = 1'-0" PILASTER AT GRIDS 13/H



10 3/4" = 1'-0" DRILLED PIER DOWELS AT WALL/GRADE BEAM/PILASTER

PSW Job Number:
983A

STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
No. 18889
Martin Wilcox
2/24/2023

AWSOM
Bentonville, AR

Issue Date:
02.24.2023

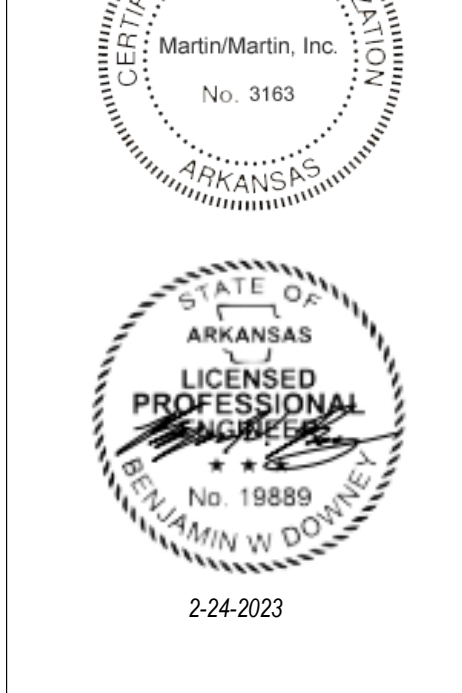
NUMBER	DATE	DESCRIPTION
1	03.11.2023	ACCENDIA.1
2	06.09.2023	ACCENDIA.2

Consents:
DRILLED PIER DETAILS

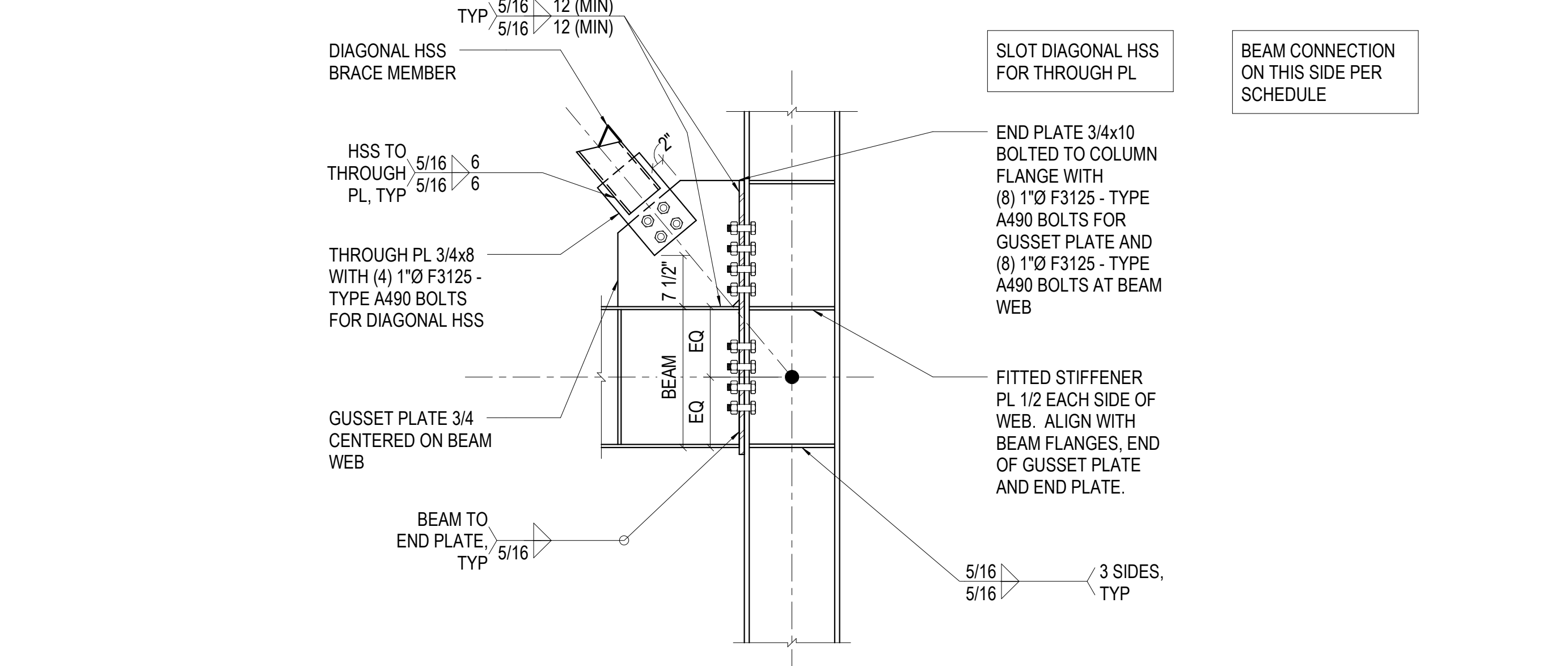
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S300

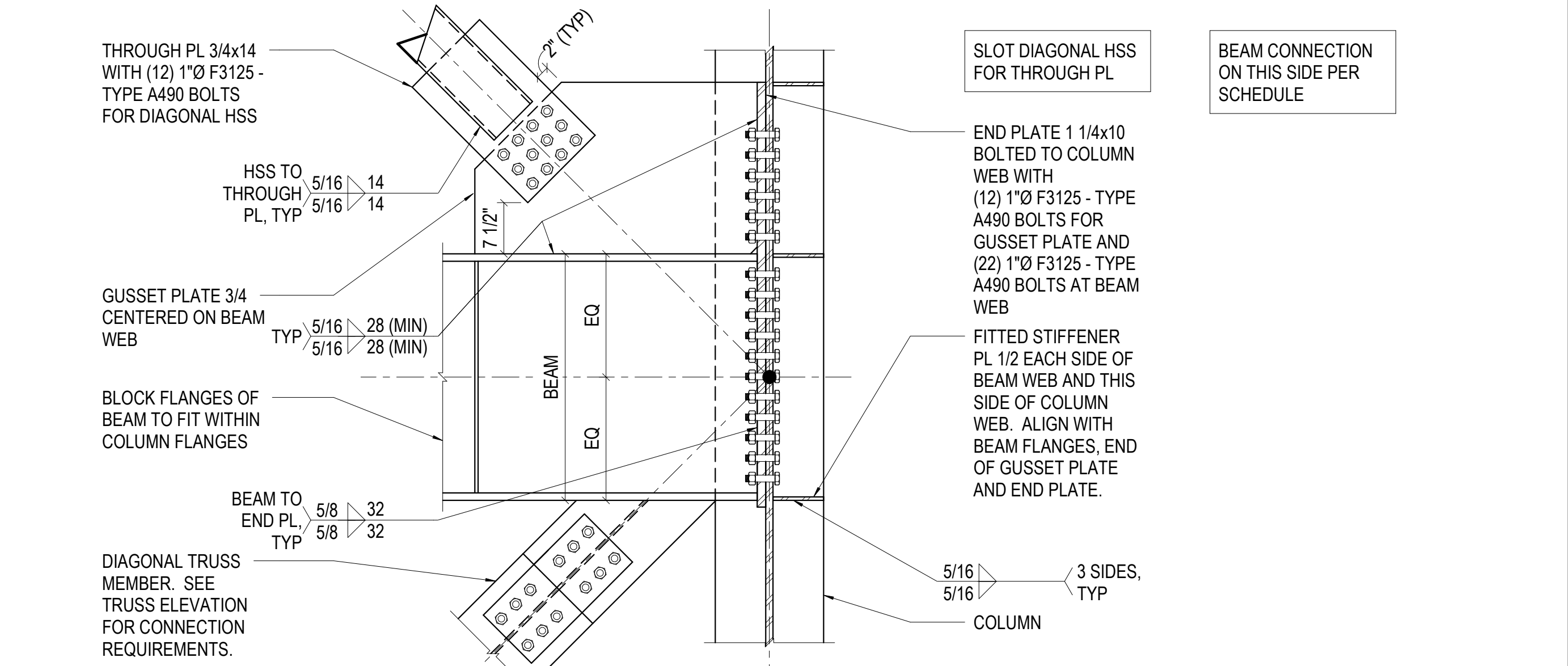
PROJECT MANAGER BAILEIGH FISHER
 LEAD DESIGNER TEEN BRAUNWELLS, AIA
 DESIGNER ETHAN WOODS, CHRISTINA CHILDRESS, CALEB CHENUT
 ASST. 1881331
 BENTONVILLE
 PROJECT MANAGER BAILEIGH FISHER



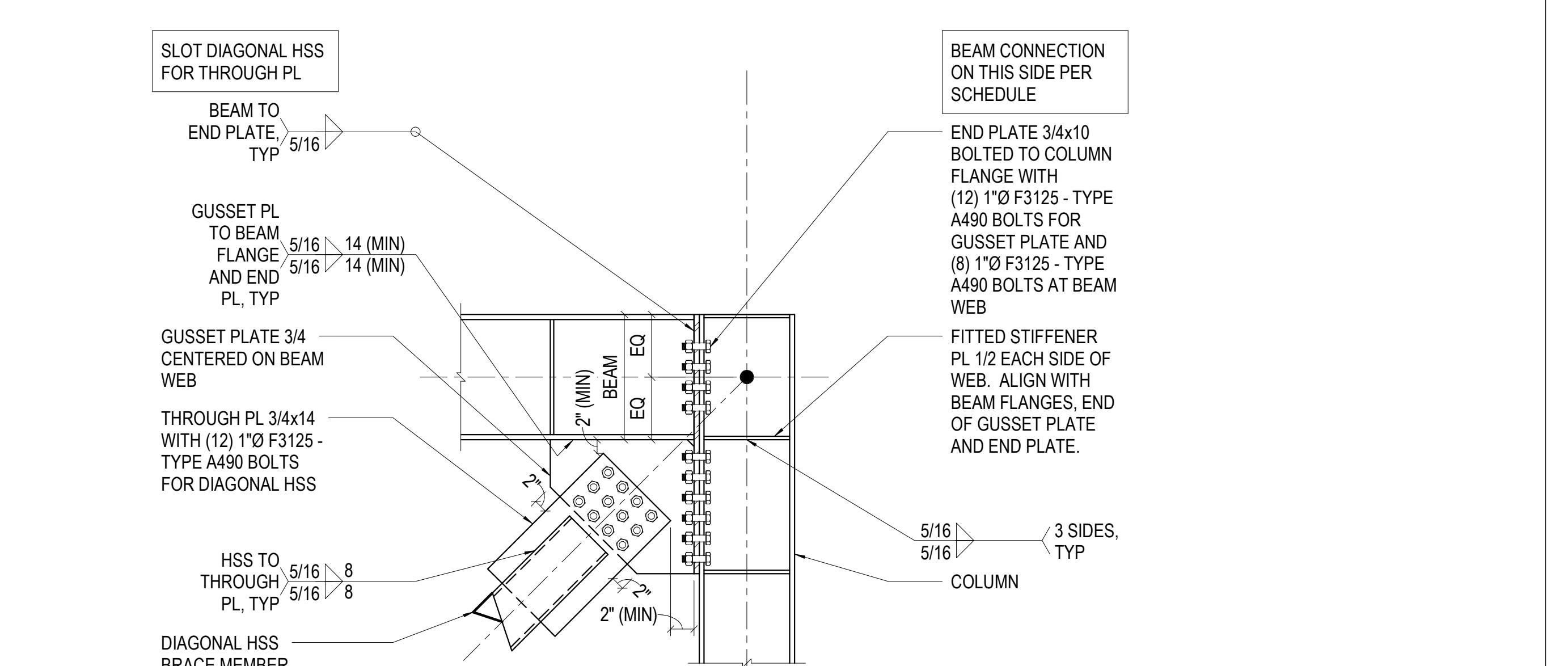
REVISIONS	
1	NUMBER DATE REVISION
1	03.13.2023 ACCORDIAN 1
2	06.29.2023 ACCORDIAN 2



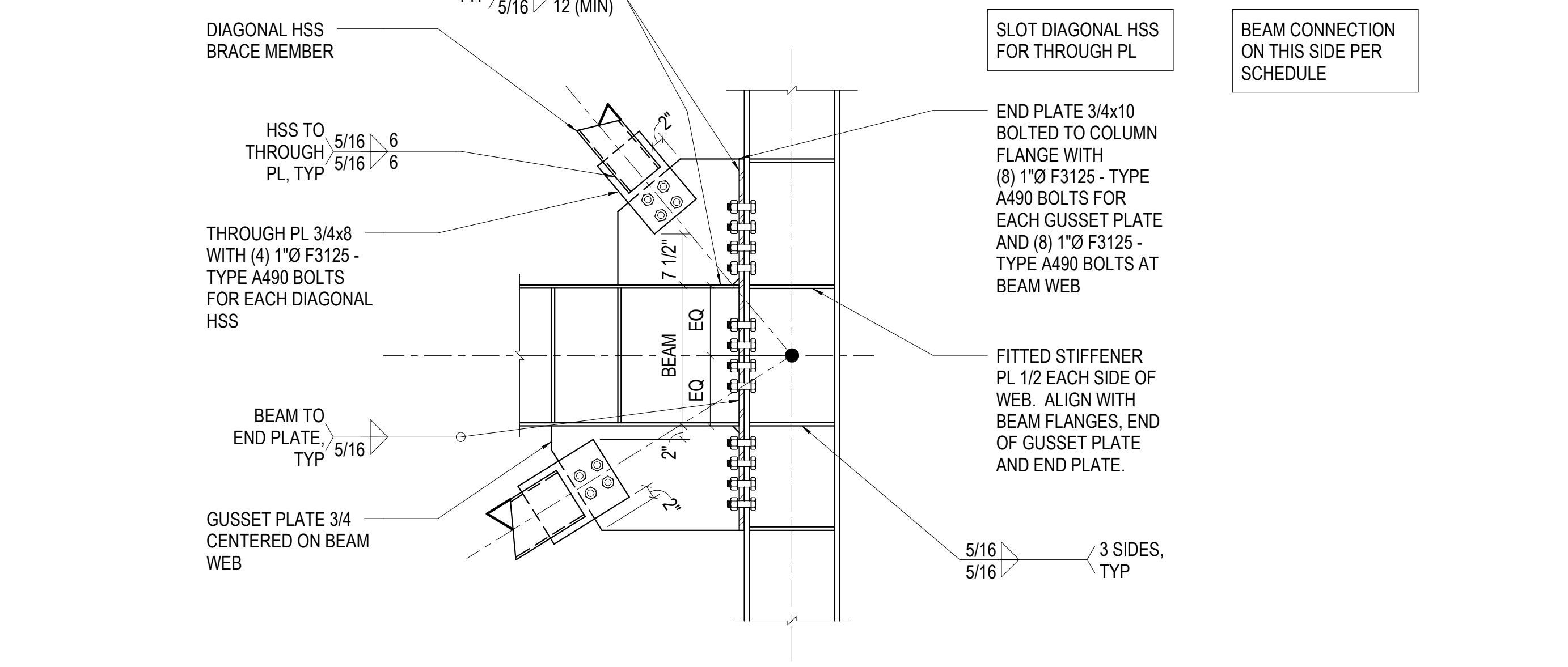
26 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO COLUMN FLANGE - DIAGONAL BRACE ABOVE STEEL BEAM



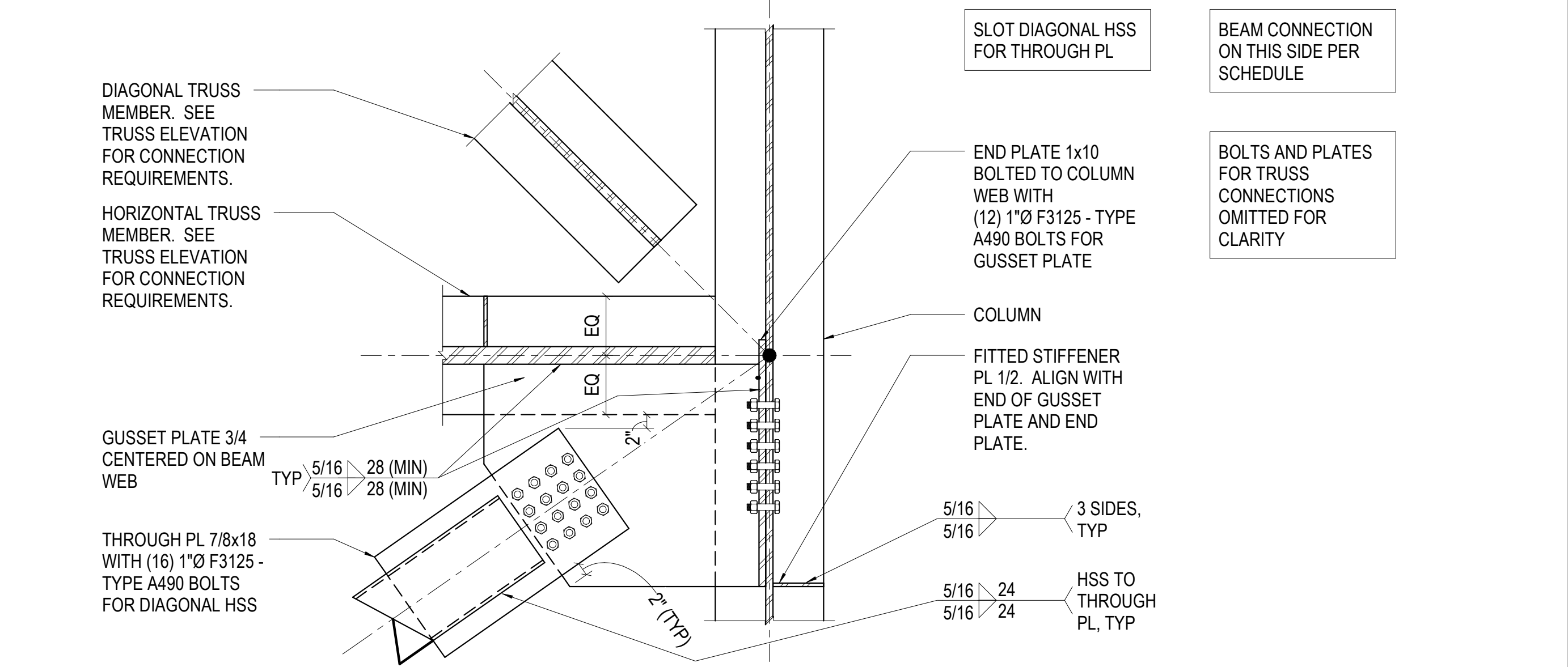
16 3/4" = 1'-0" BRACED FRAME ON GRID E - DIAGONAL CONNECTION TO COLUMN WEB - DIAGONAL BRACE ABOVE TRUSS



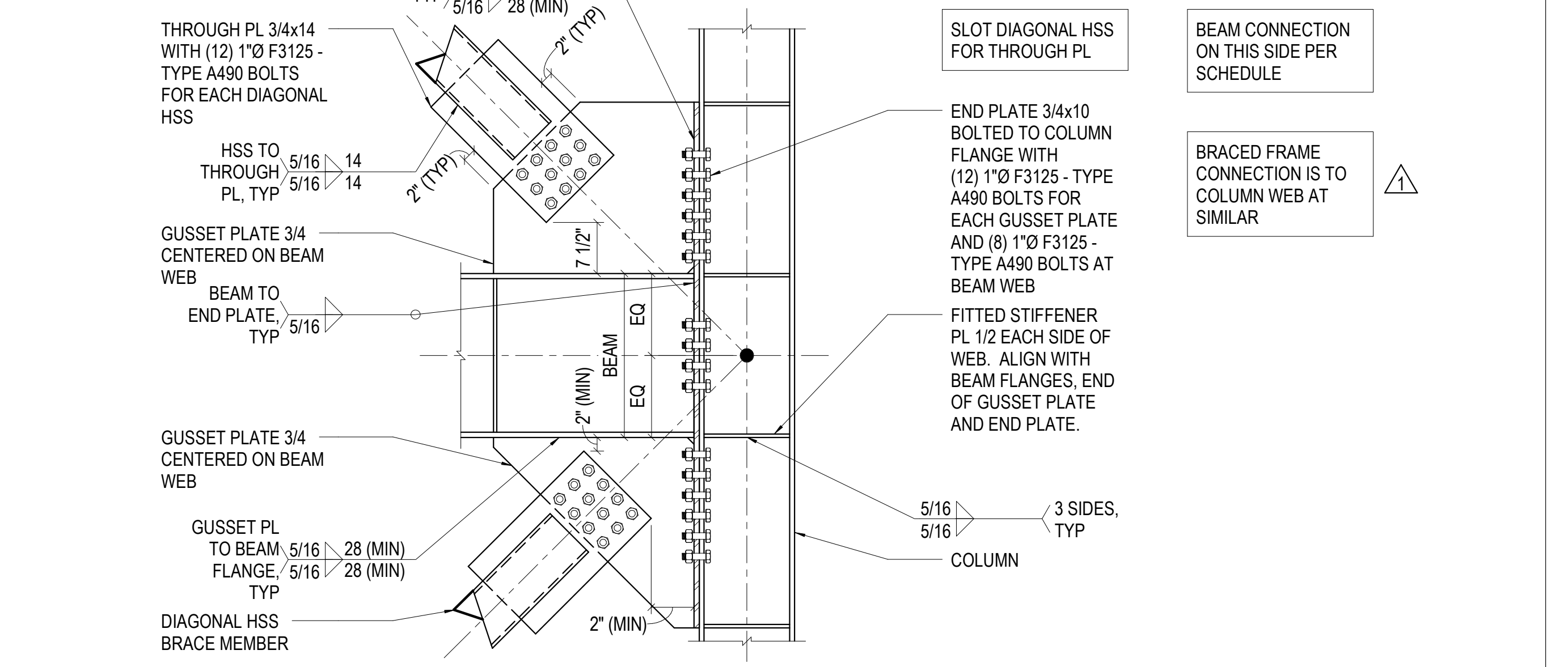
6 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO COLUMN FLANGE - ROOF LEVEL



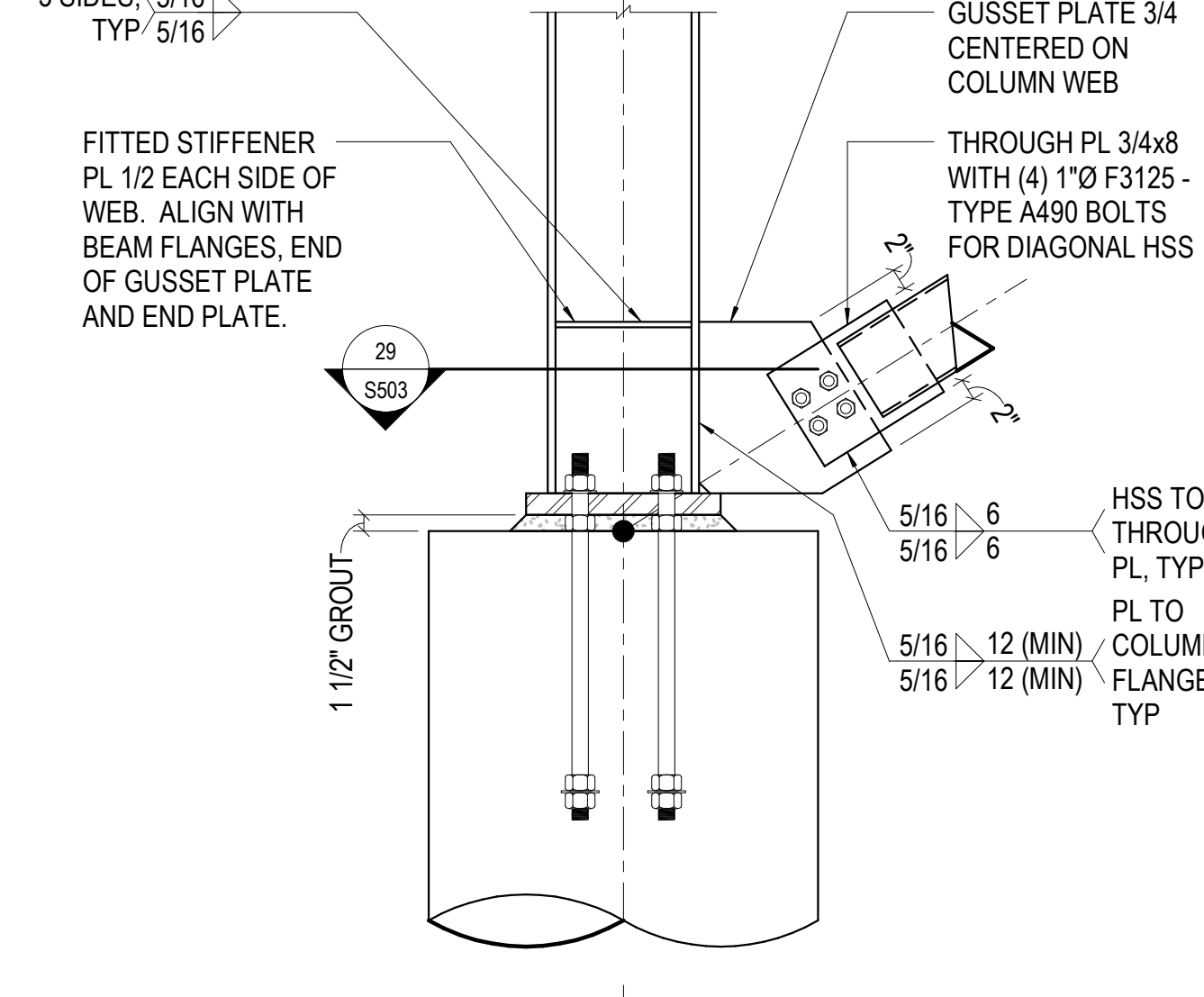
27 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO COLUMN FLANGE - DIAGONAL BRACE ABOVE AND BELOW STEEL BEAM



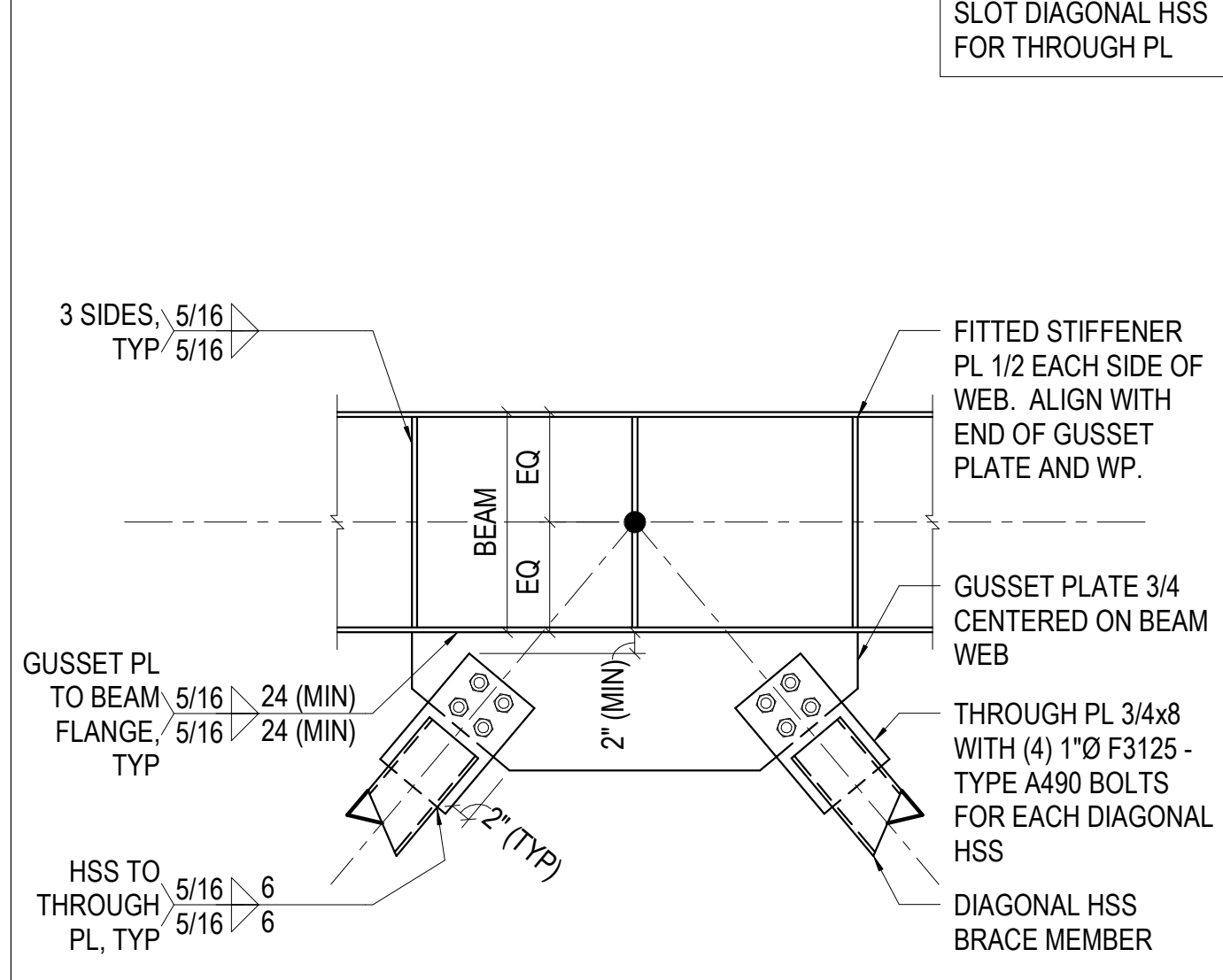
17 3/4" = 1'-0" BRACED FRAME ON GRID E - DIAGONAL CONNECTION TO COLUMN WEB - DIAGONAL BRACE BELOW TRUSS



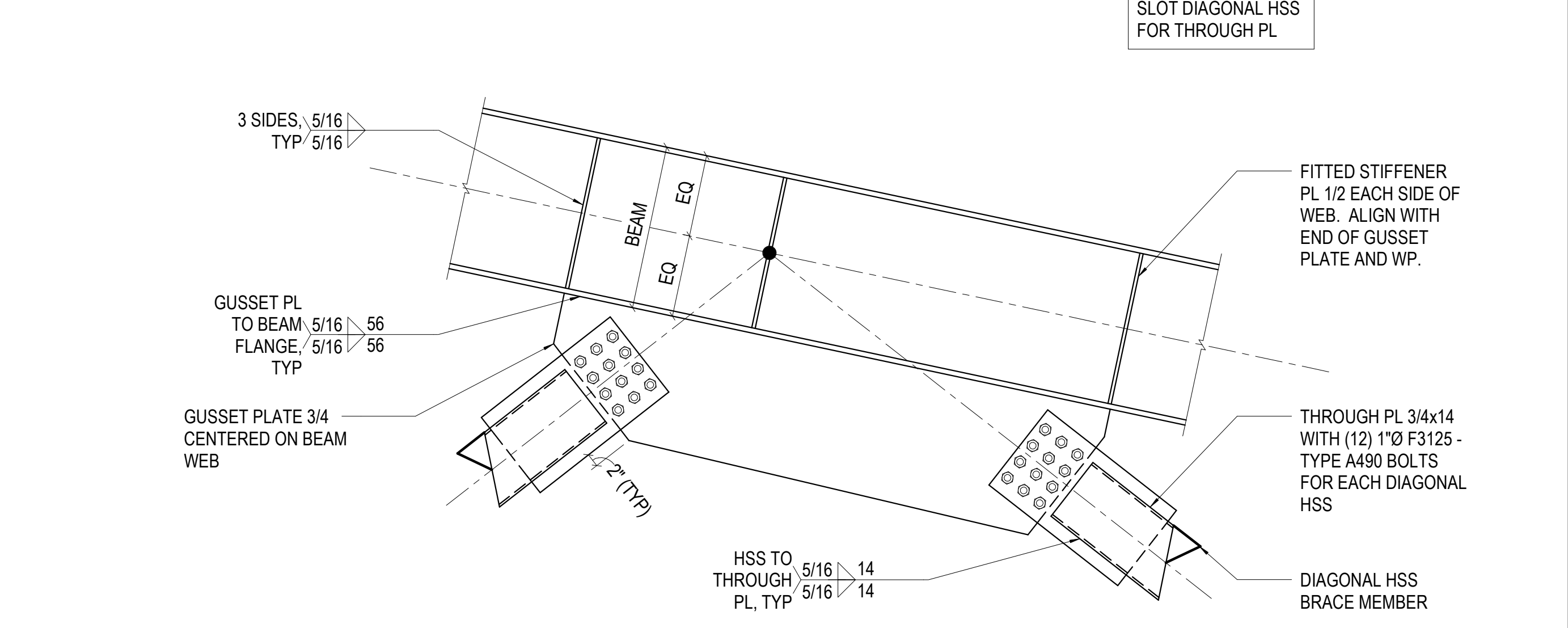
7 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO COLUMN FLANGE - FLOOR LEVELS



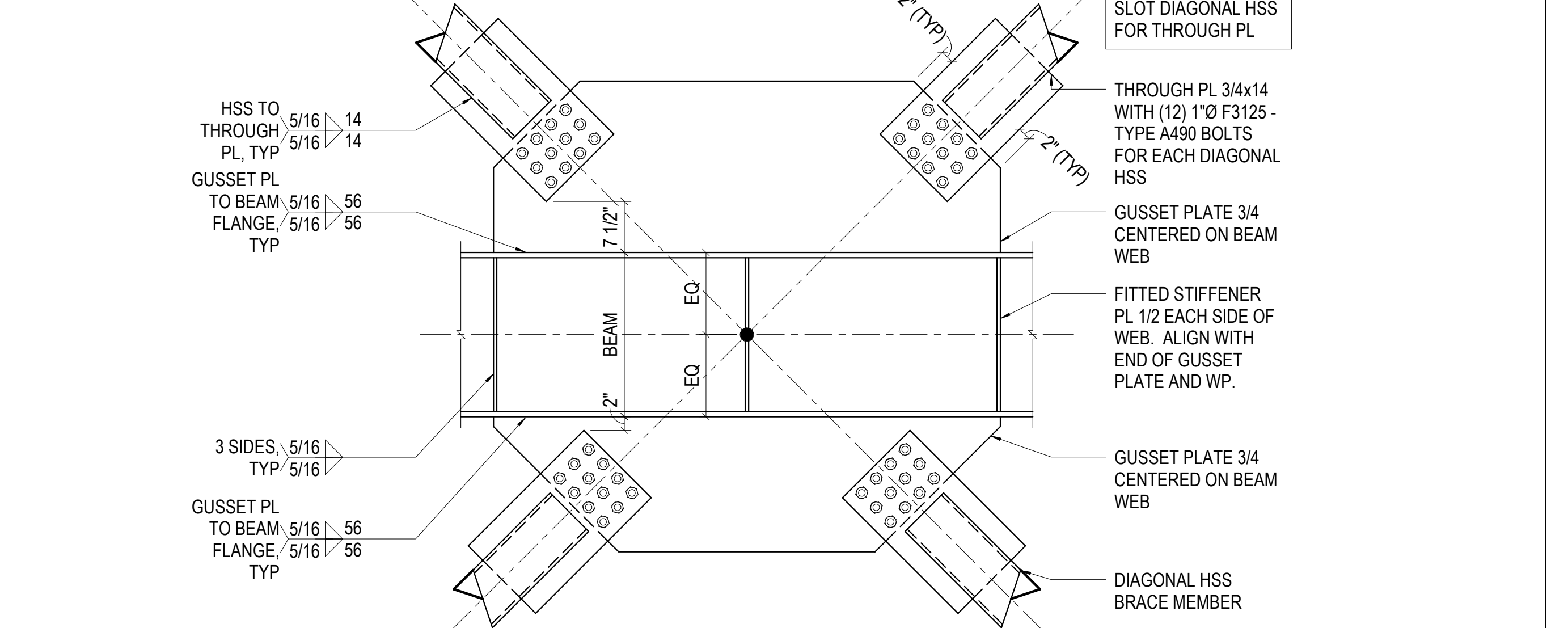
28 3/4" = 1'-0" DIAGONAL CONNECTION TO CONCRETE AT GRID 4/G



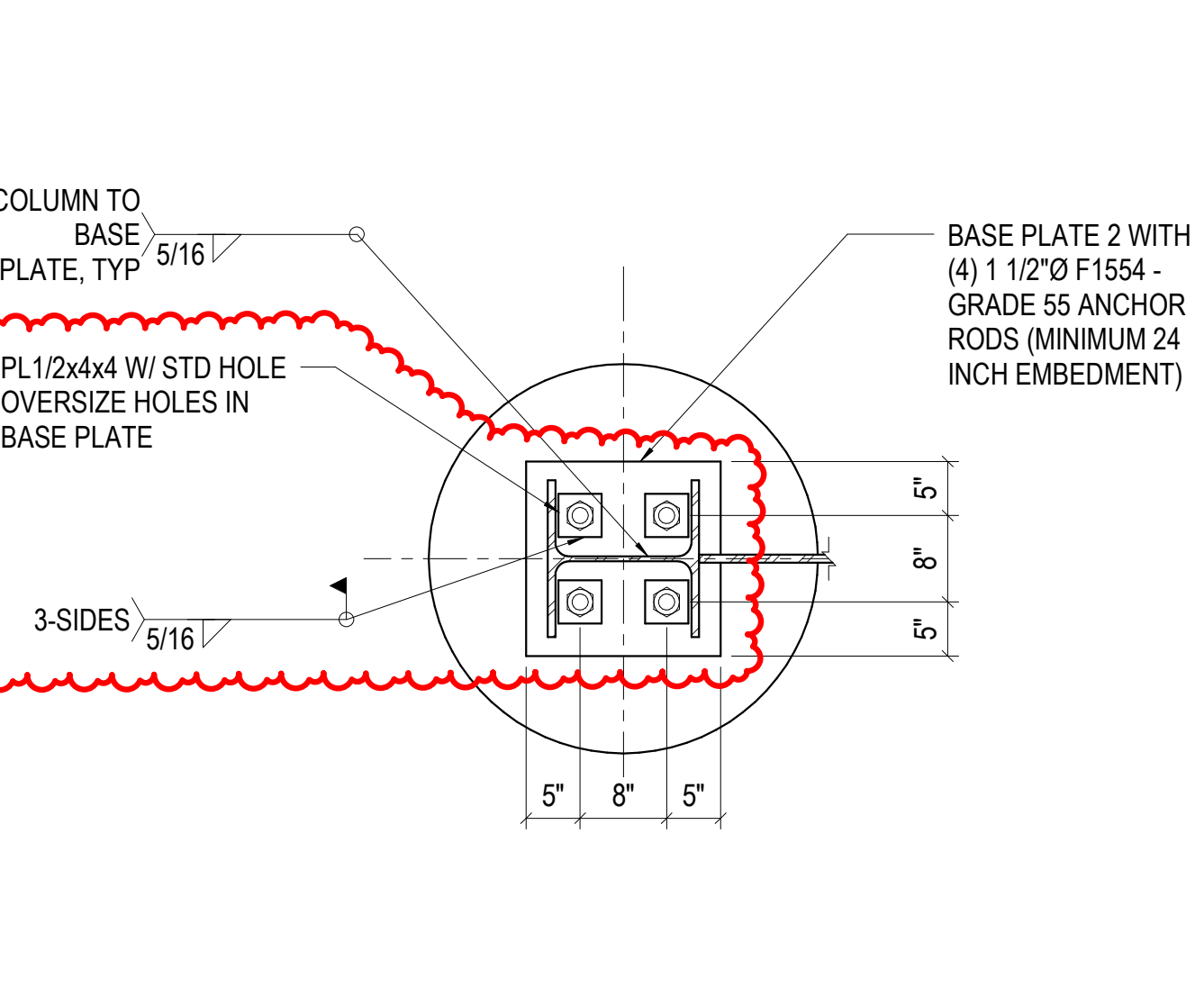
23 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION BELOW STEEL BEAM



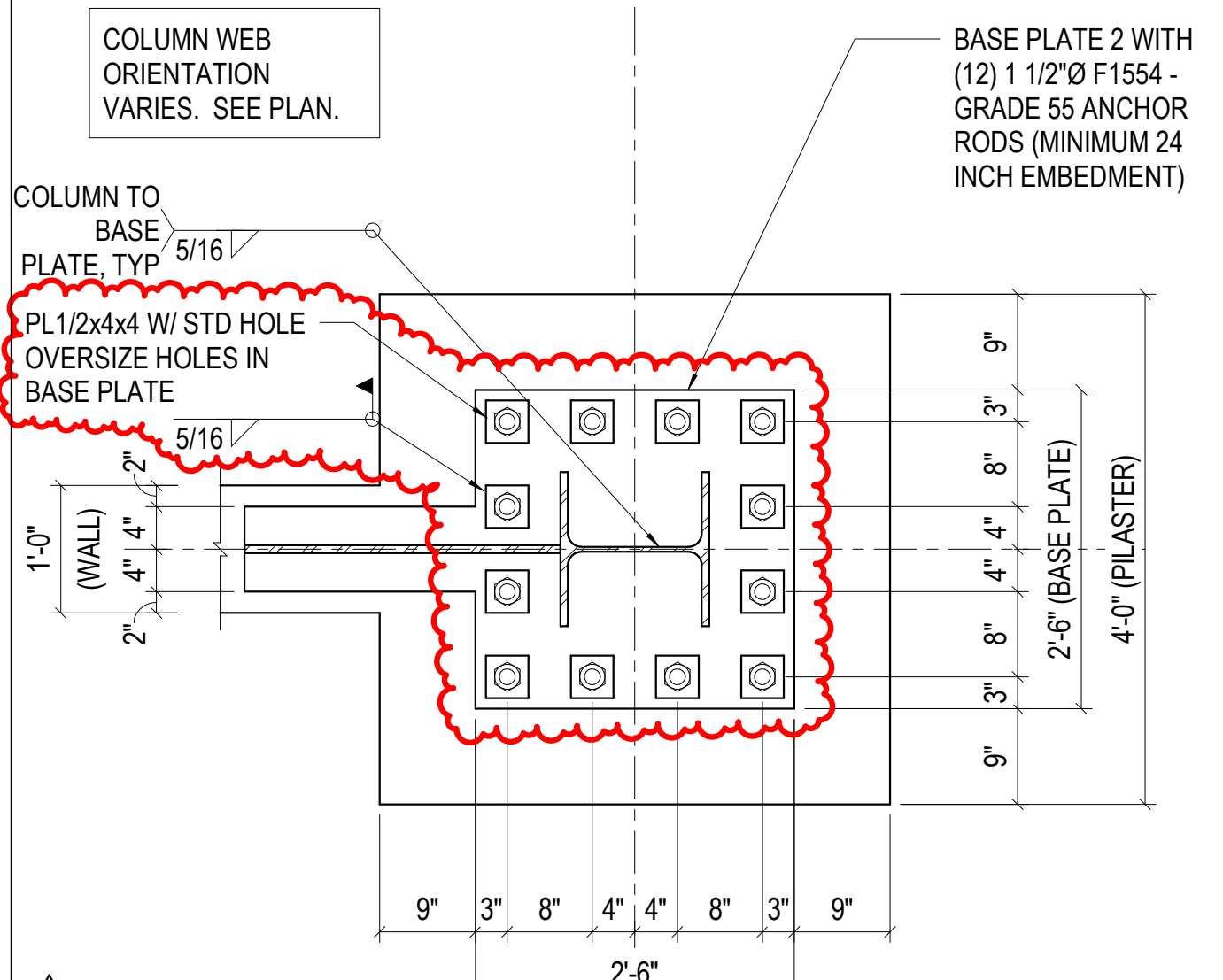
18 3/4" = 1'-0" BRACED FRAME ON GRID A - DIAGONAL CONNECTION TO STEEL BEAM - DIAGONAL BRACES ABOVE AND BELOW STEEL BEAM



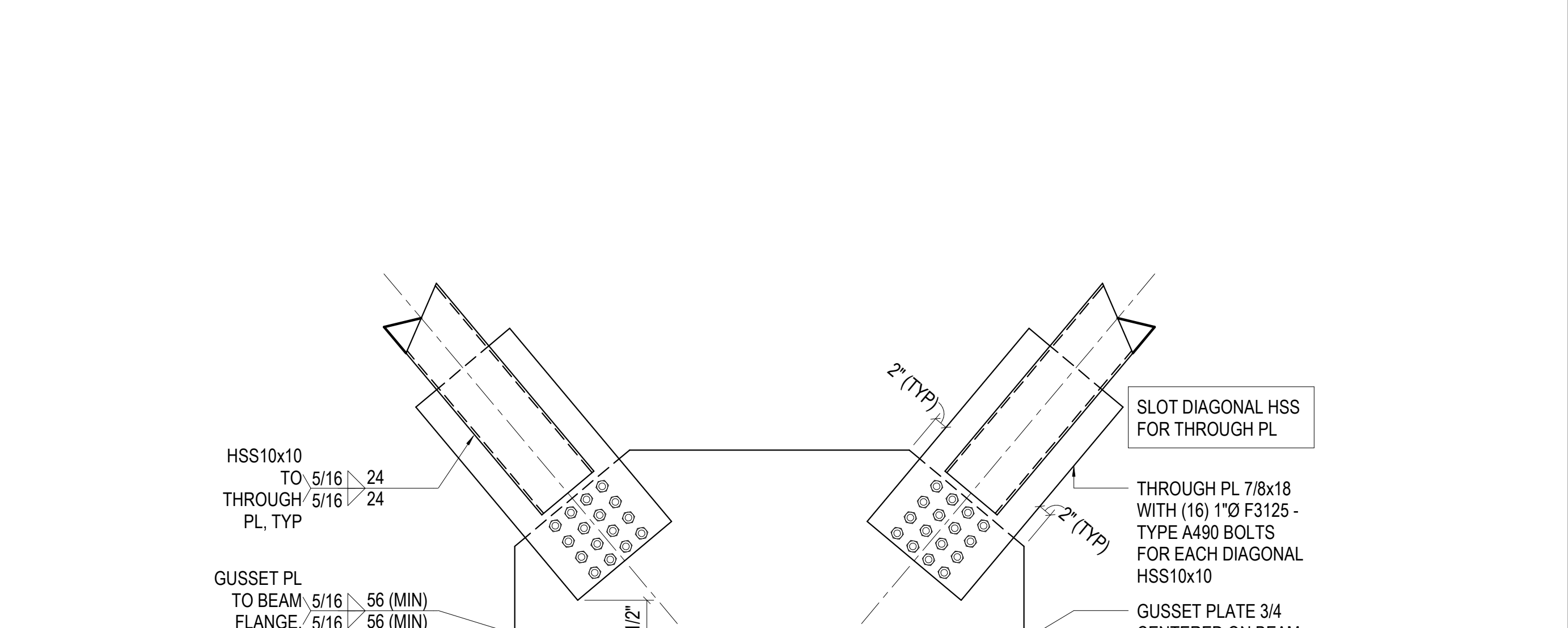
8 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO STEEL BEAM - (2) DIAGONAL BRACES ABOVE AND BELOW STEEL BEAM



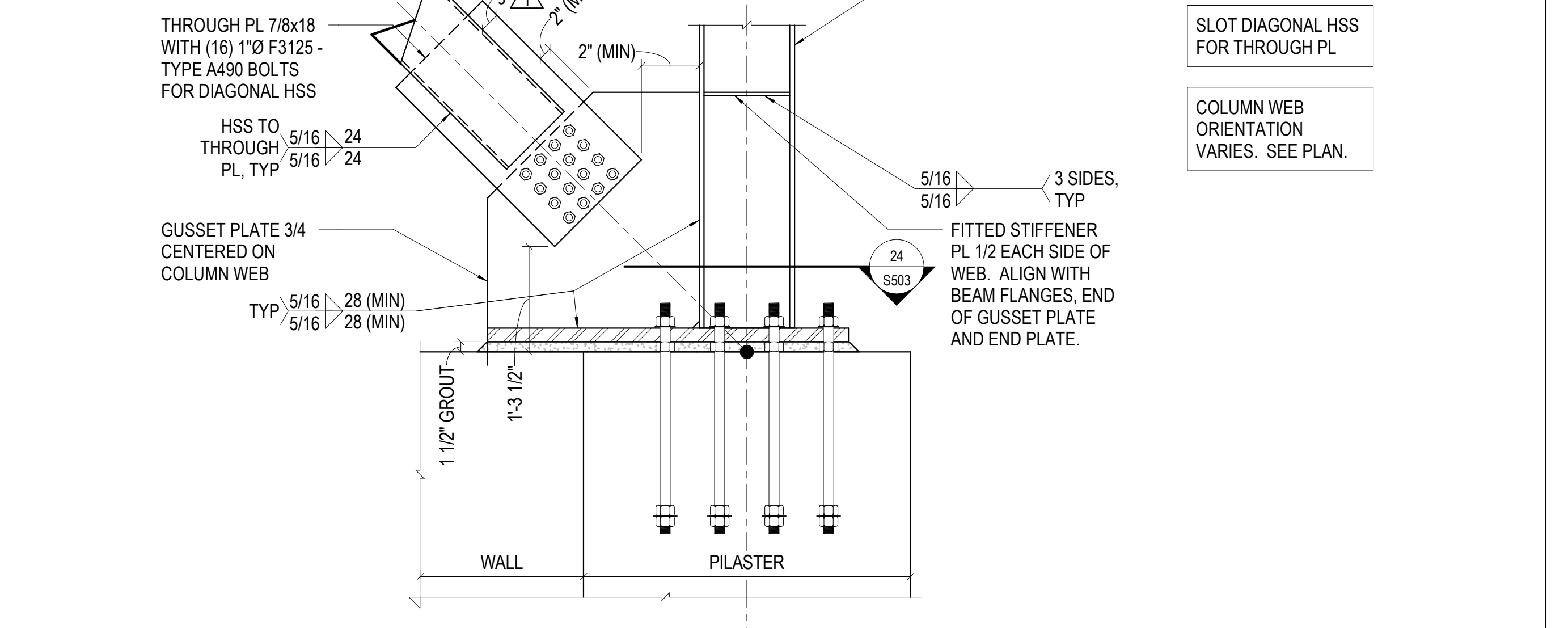
29 3/4" = 1'-0" BRACED FRAME COLUMN BASE PLATE AT GRID 4/G



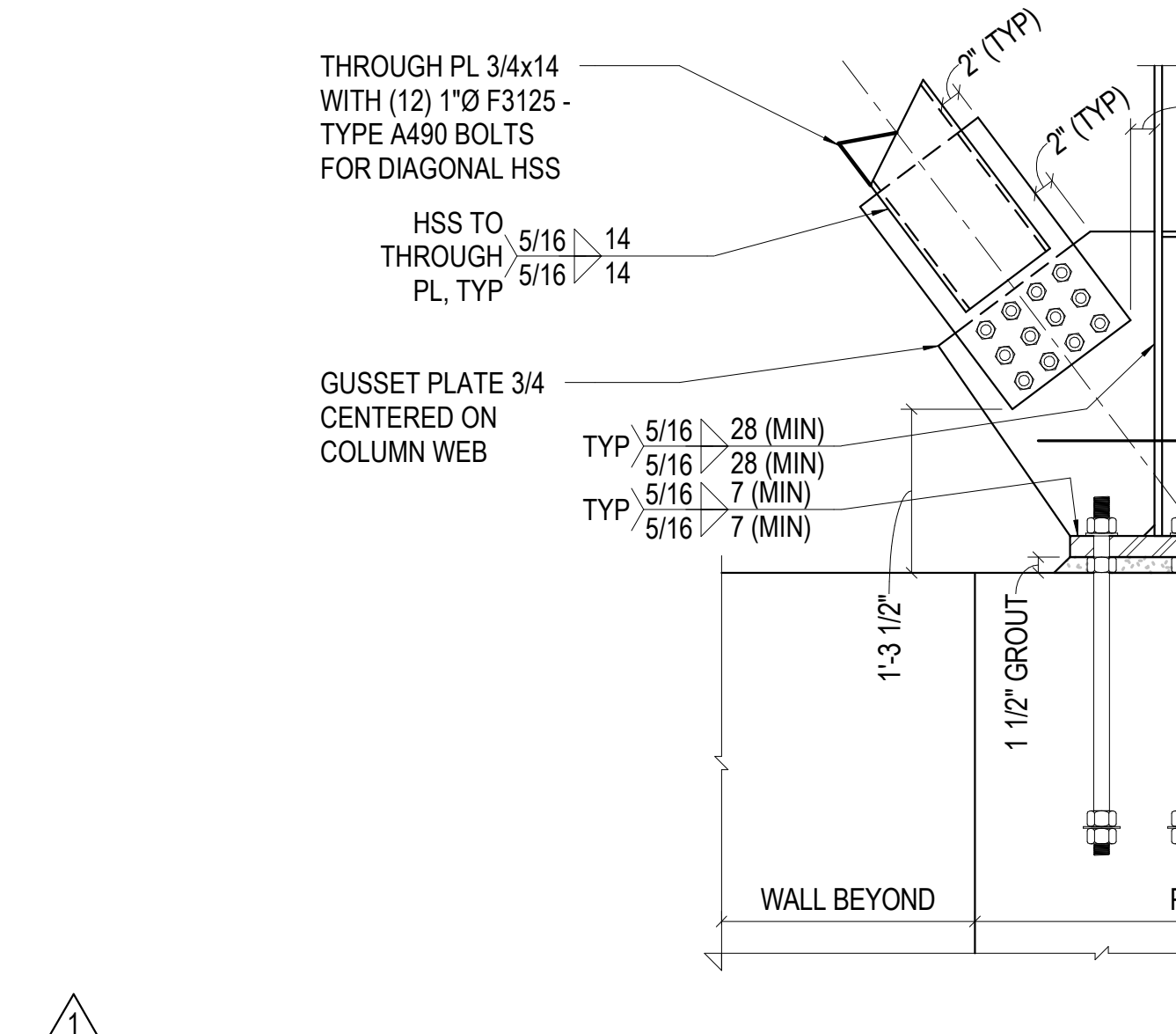
24 3/4" = 1'-0" TYPICAL BRACED FRAME COLUMN BASE PLATE



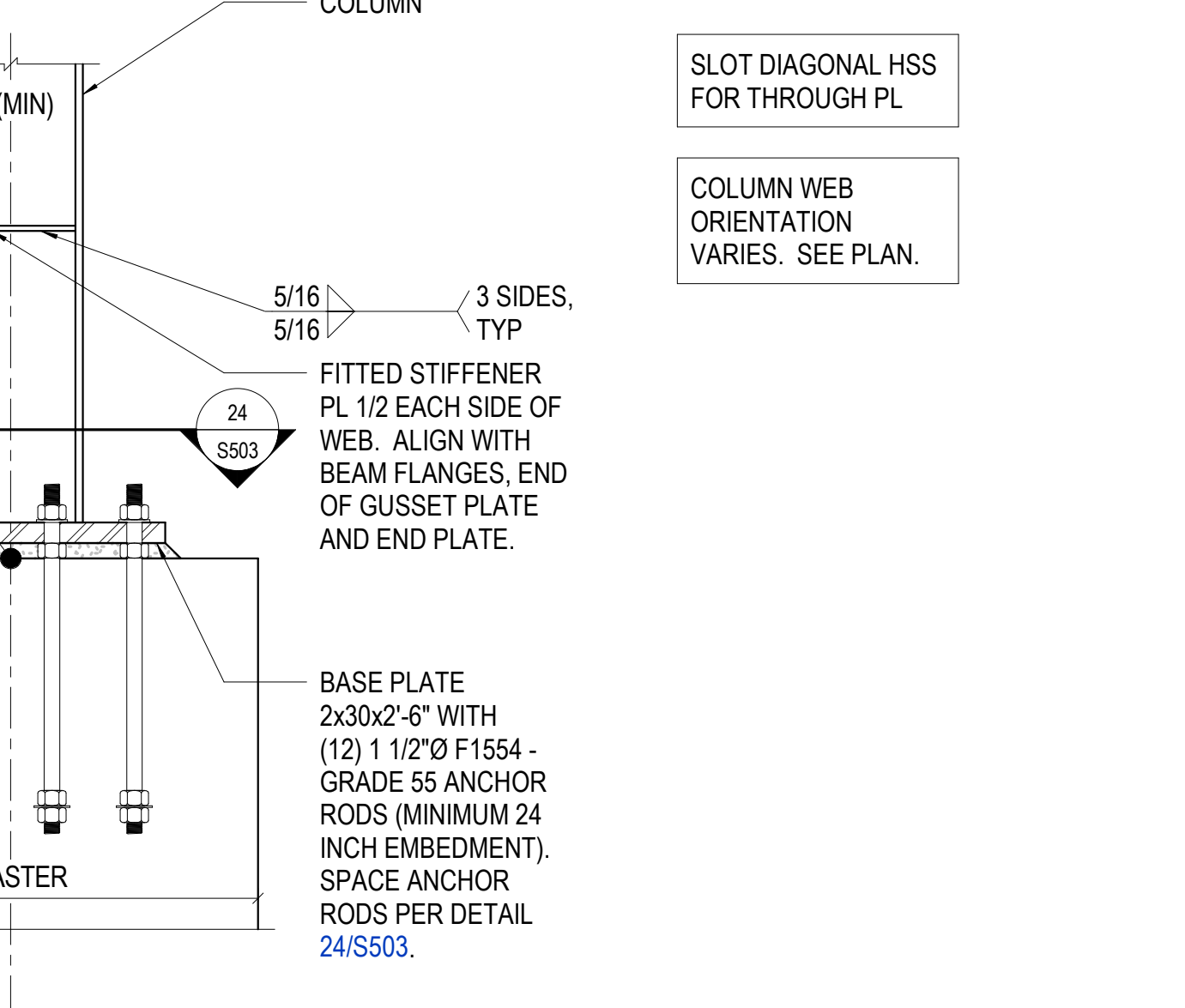
20 3/4" = 1'-0" BRACED FRAME ON GRID A - DIAGONAL CONNECTION TO STEEL BEAM - DIAGONAL BRACES ABOVE AND BELOW STEEL BEAM



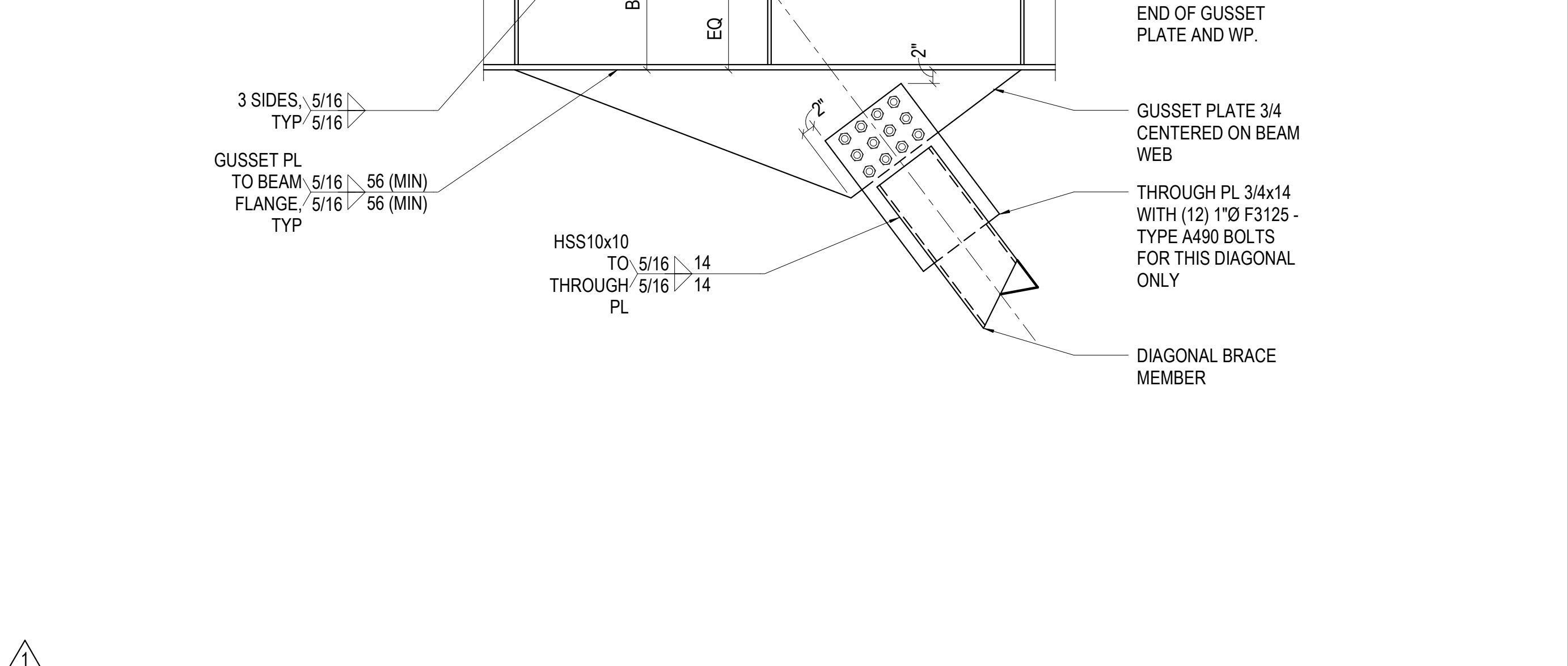
9 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO CONCRETE



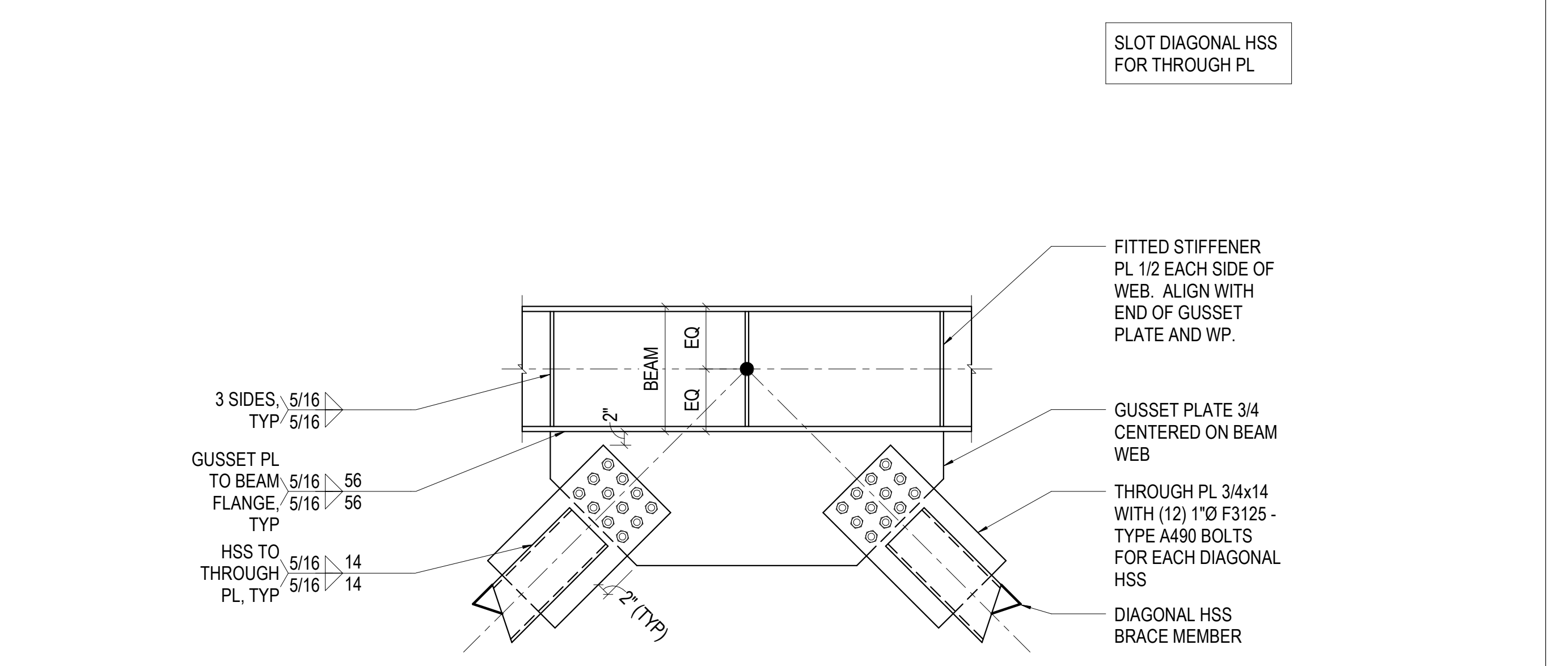
30 3/4" = 1'-0" BRACED FRAME DIAGONAL CONNECTION TO CONCRETE AT GRID 8/A



10 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO STEEL BEAM - (2) DIAGONAL BRACES BELOW STEEL BEAM

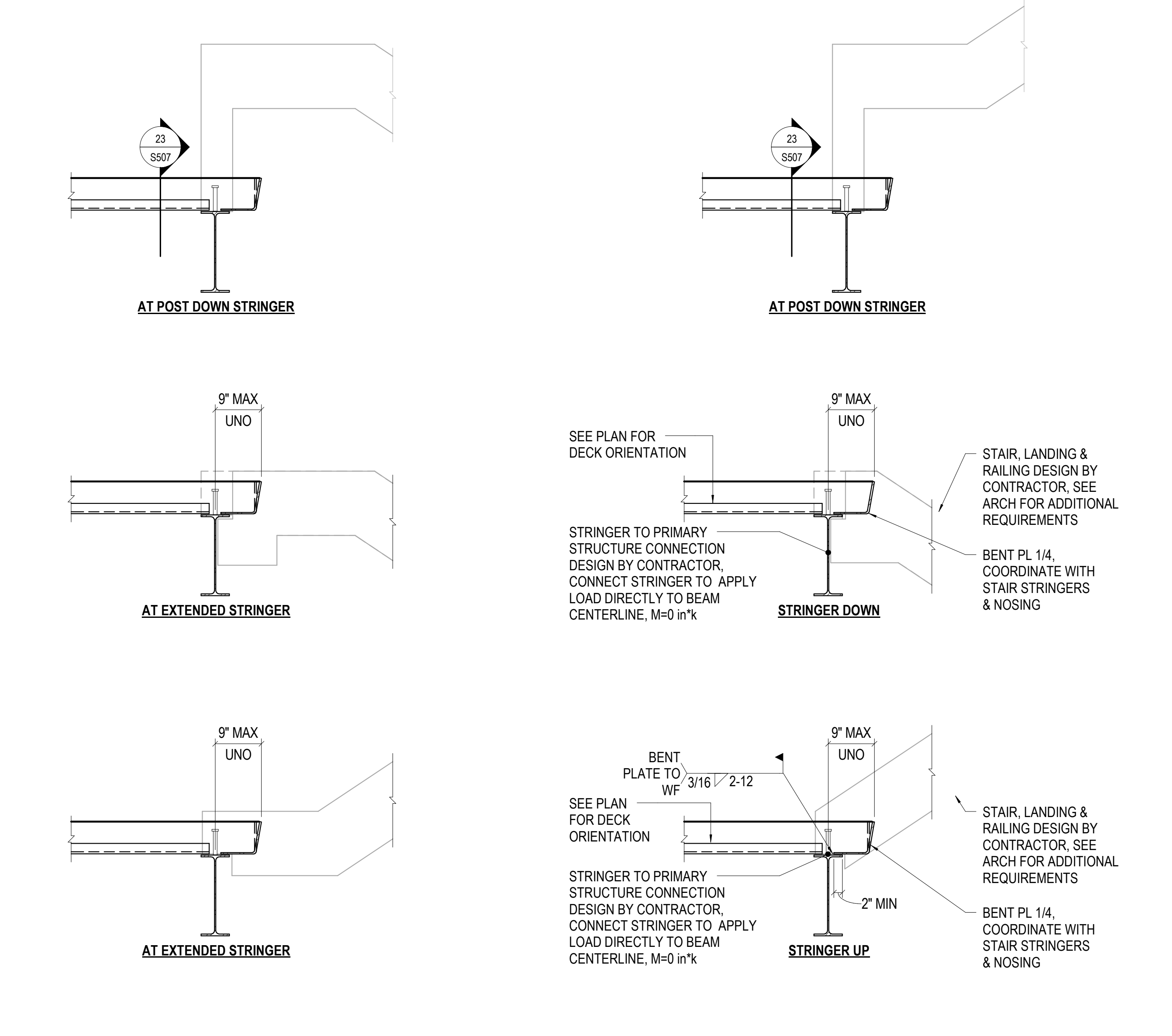


20 3/4" = 1'-0" BRACED FRAME ON GRID A - DIAGONAL CONNECTION TO STEEL BEAM - DIAGONAL BRACES ABOVE AND BELOW STEEL BEAM

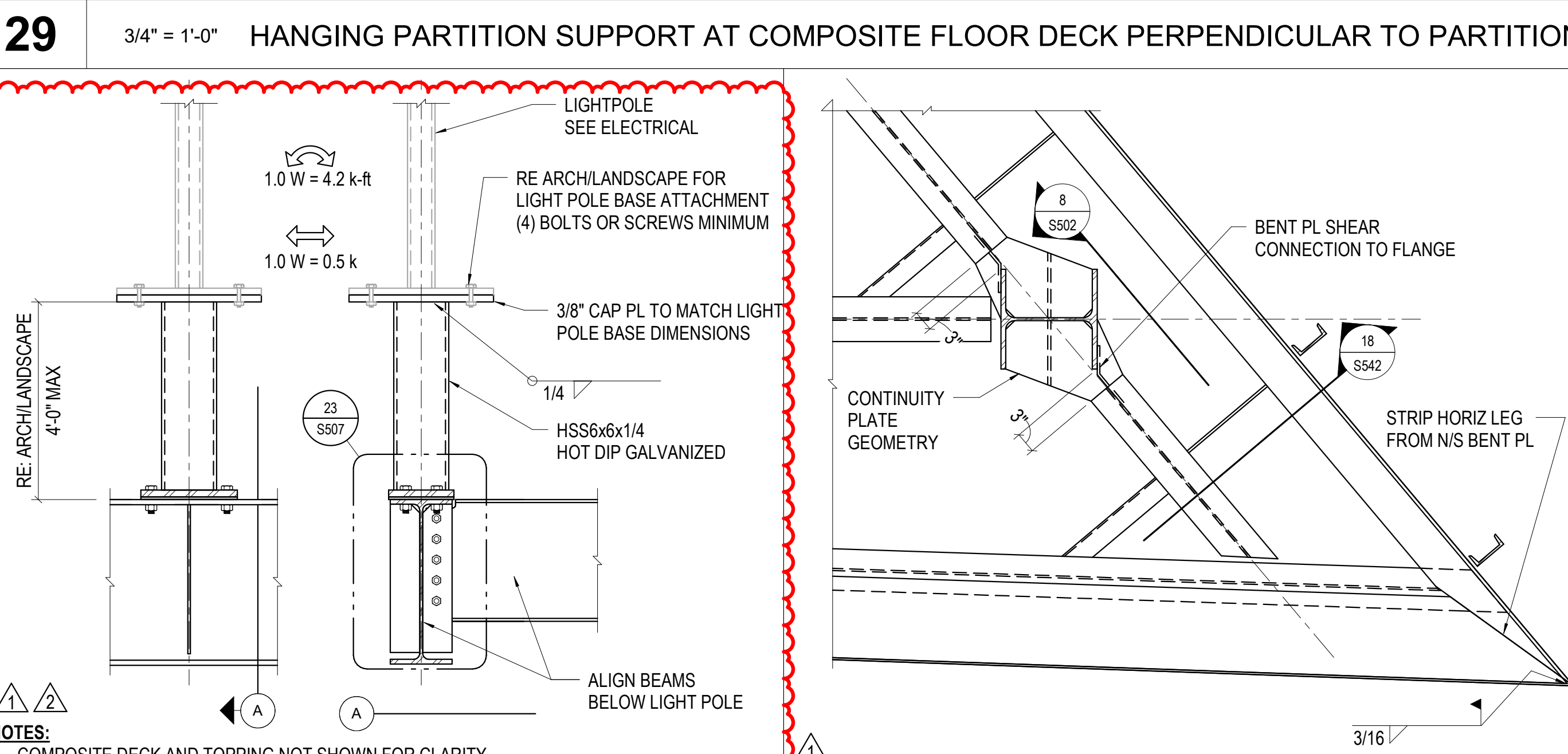
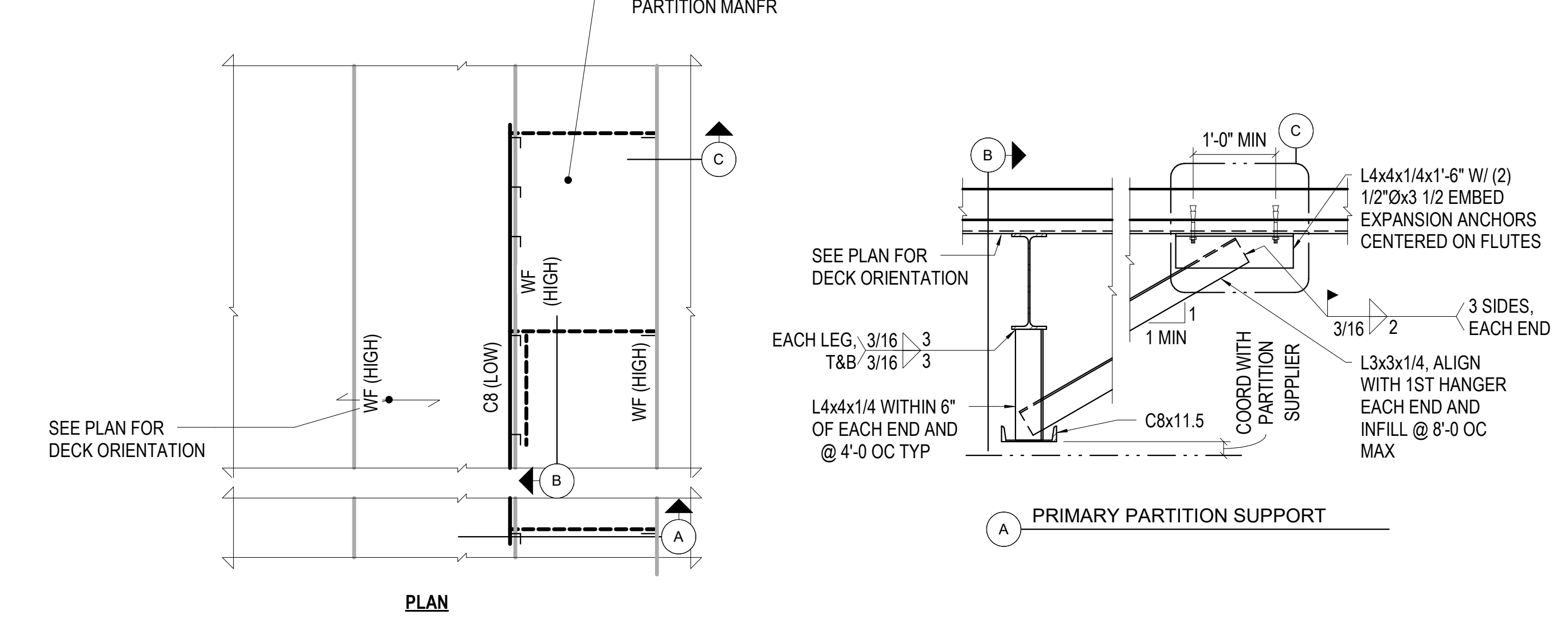
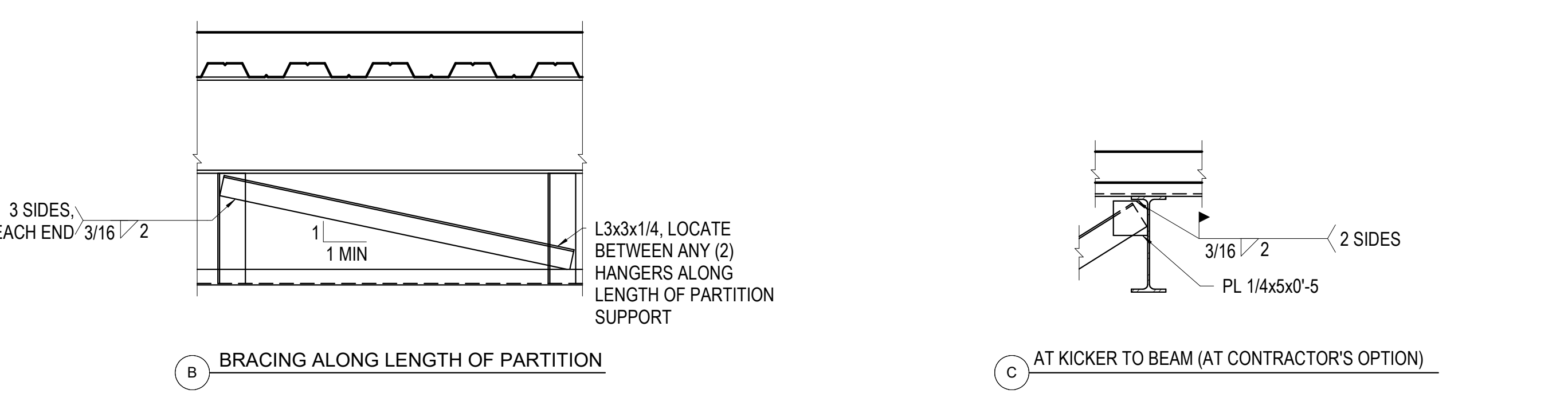


10 3/4" = 1'-0" TYPICAL BRACED FRAME DIAGONAL CONNECTION TO STEEL BEAM - (2) DIAGONAL BRACES BELOW STEEL BEAM

DESIGNERS: ETHAN WICKS, CHRISTINA CHILDS, CALEB CHENET
LEAD REVIT: TONY BRAYWELL, SAM SUTHERLAND
FILE PATH: \\AWSON\Users\STW\My Documents\0224-0044-HH-NH-NST-18-R-01 - Whole House SOWM - S24.rvt
ASB: 16.8.21
REVISION: 03/13/23
PROJECT MANAGER: BAILEIGH FISHER

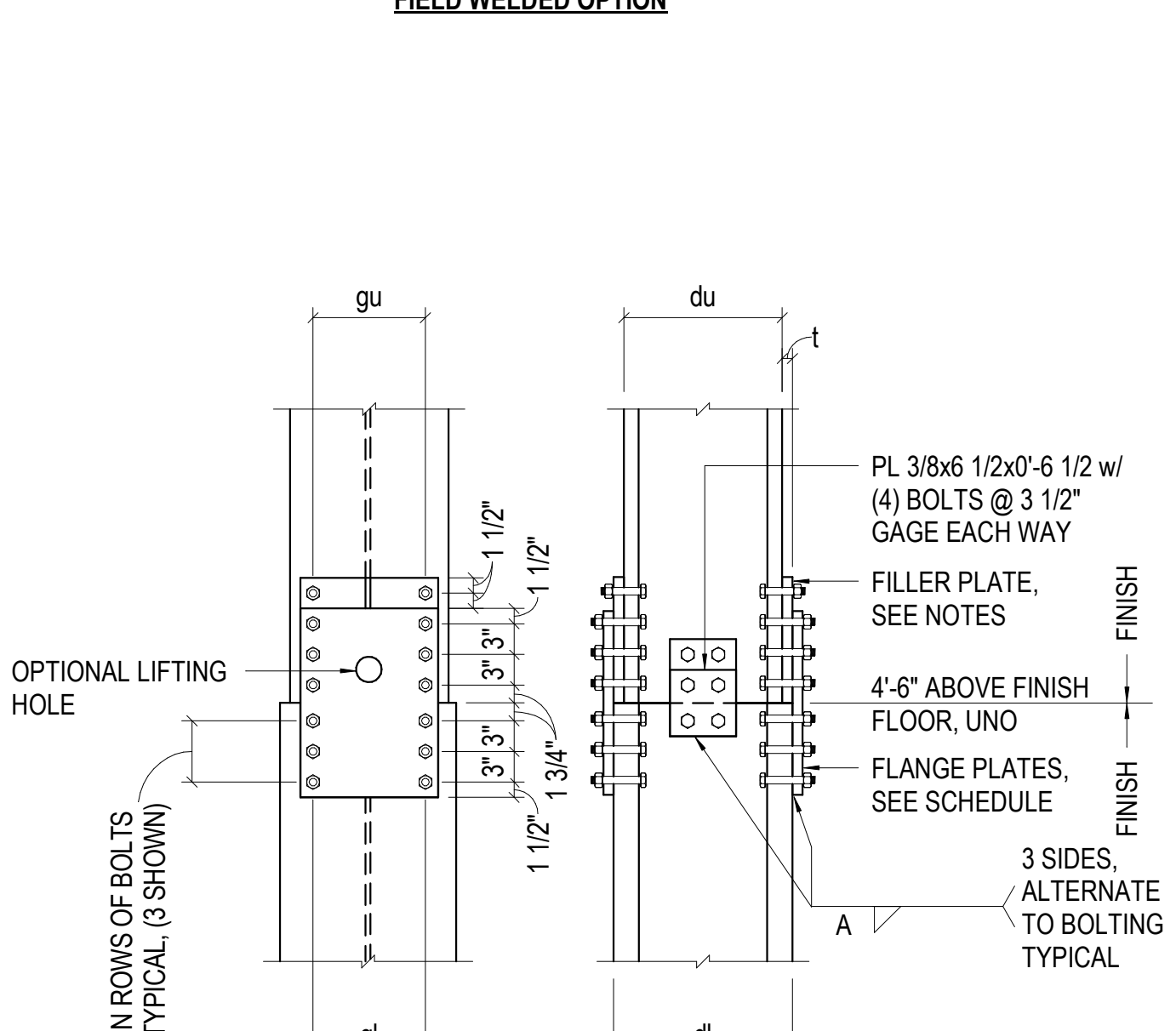
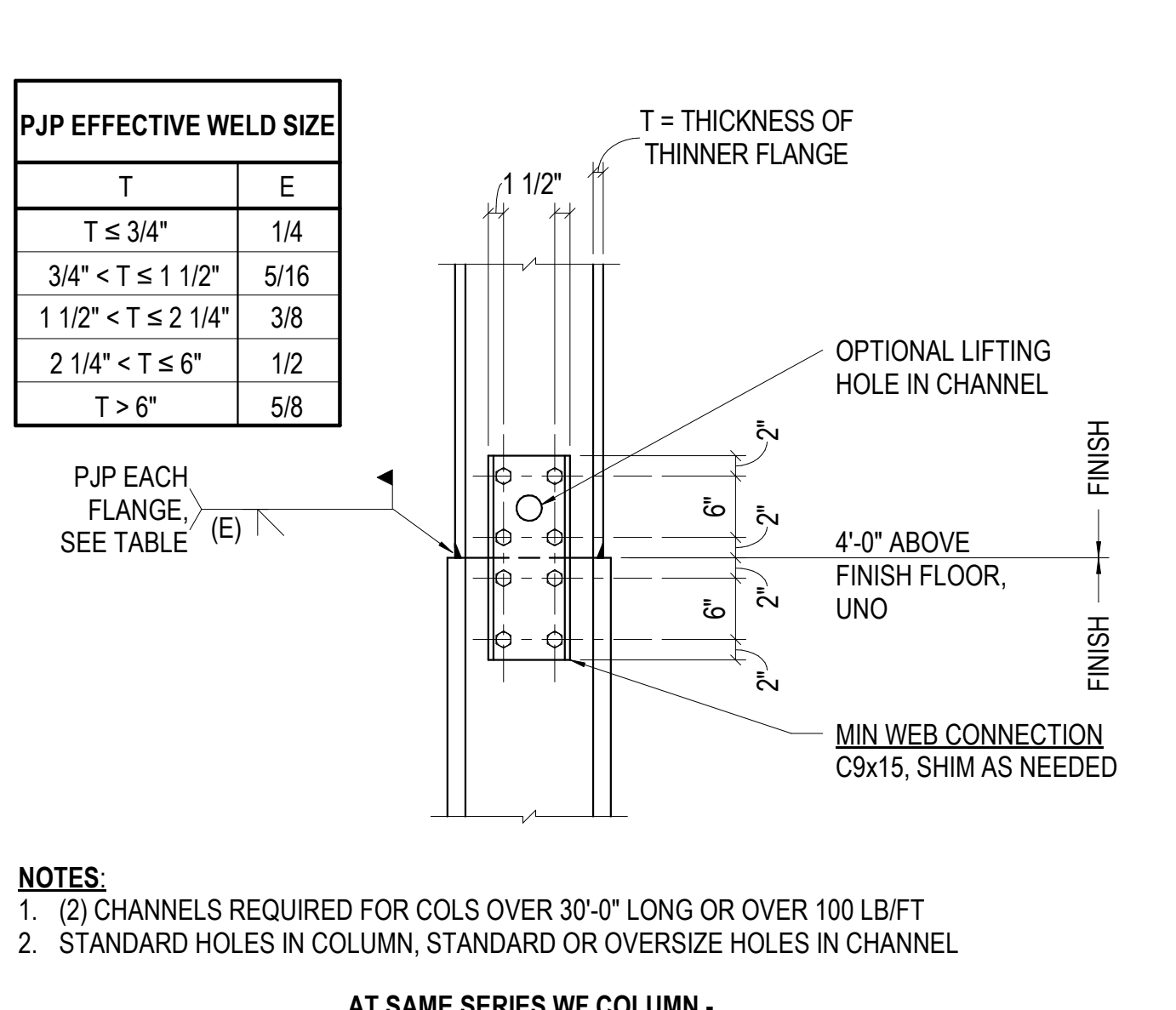


27 3/4" = 1'-0" PERFORMANCE SPECIFIED STAIR TO COMPOSITE FRAMING

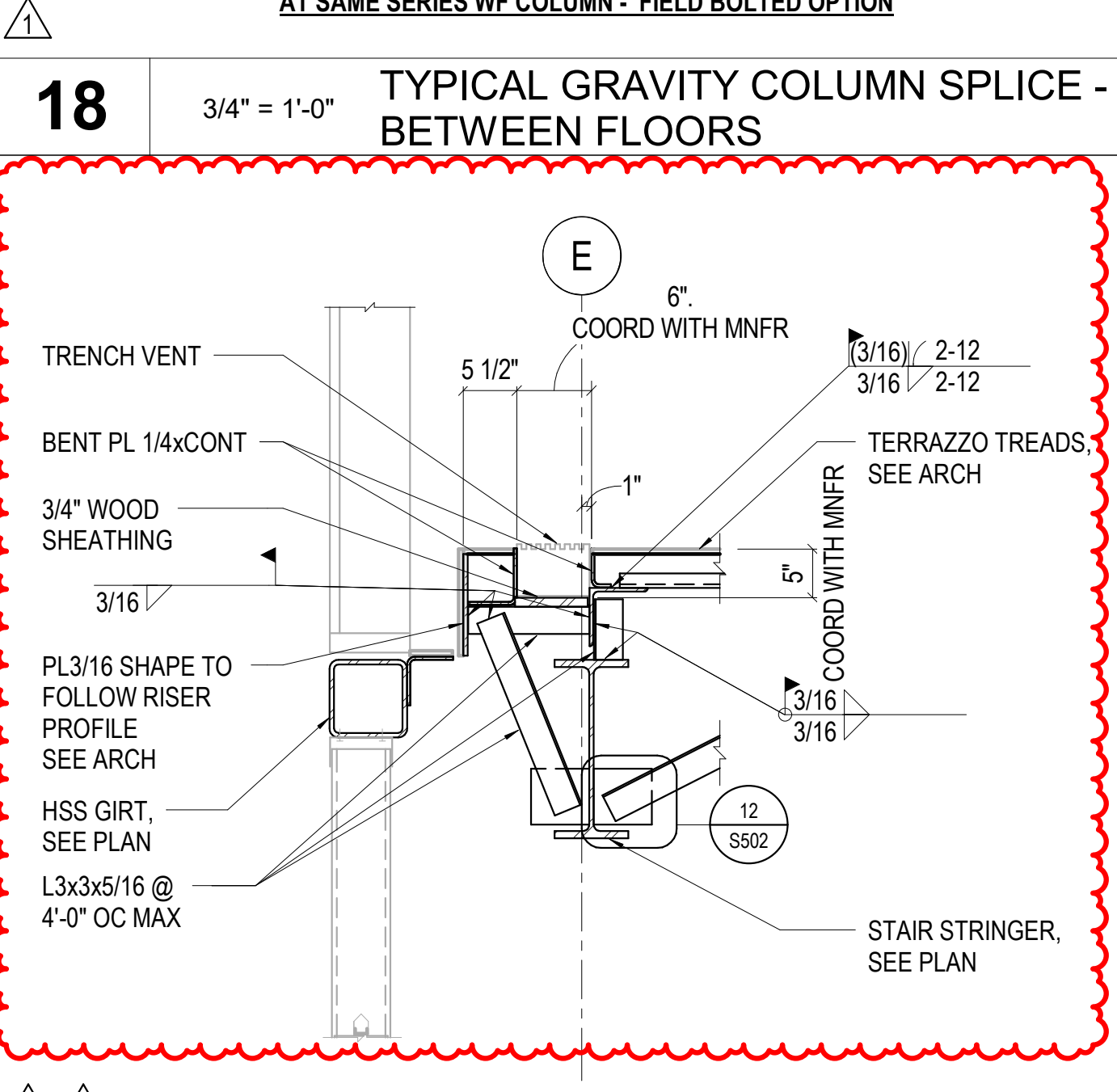


30 3/4" = 1'-0" LIGHT POLE SUPPORT AT ROOF

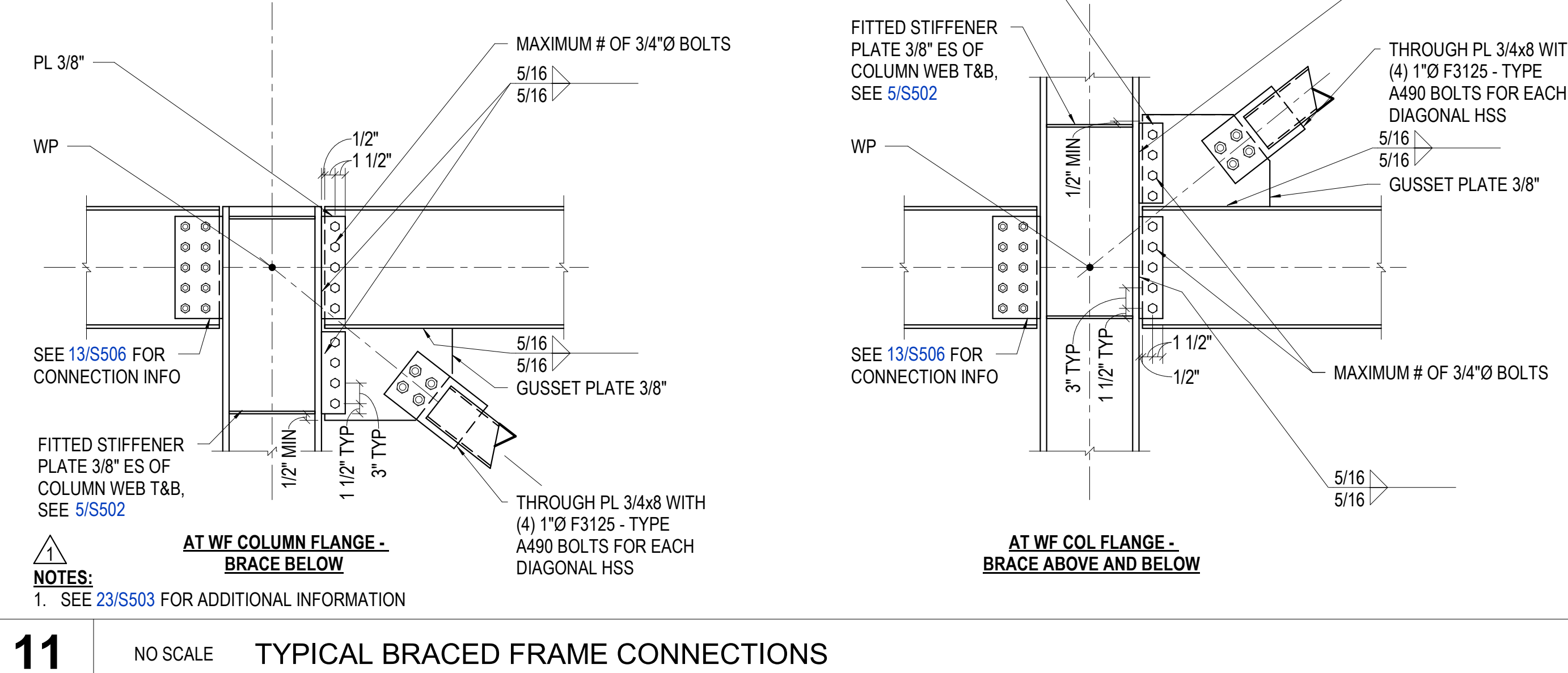
25 3/4" = 1'-0" LEVEL 2 CORNER



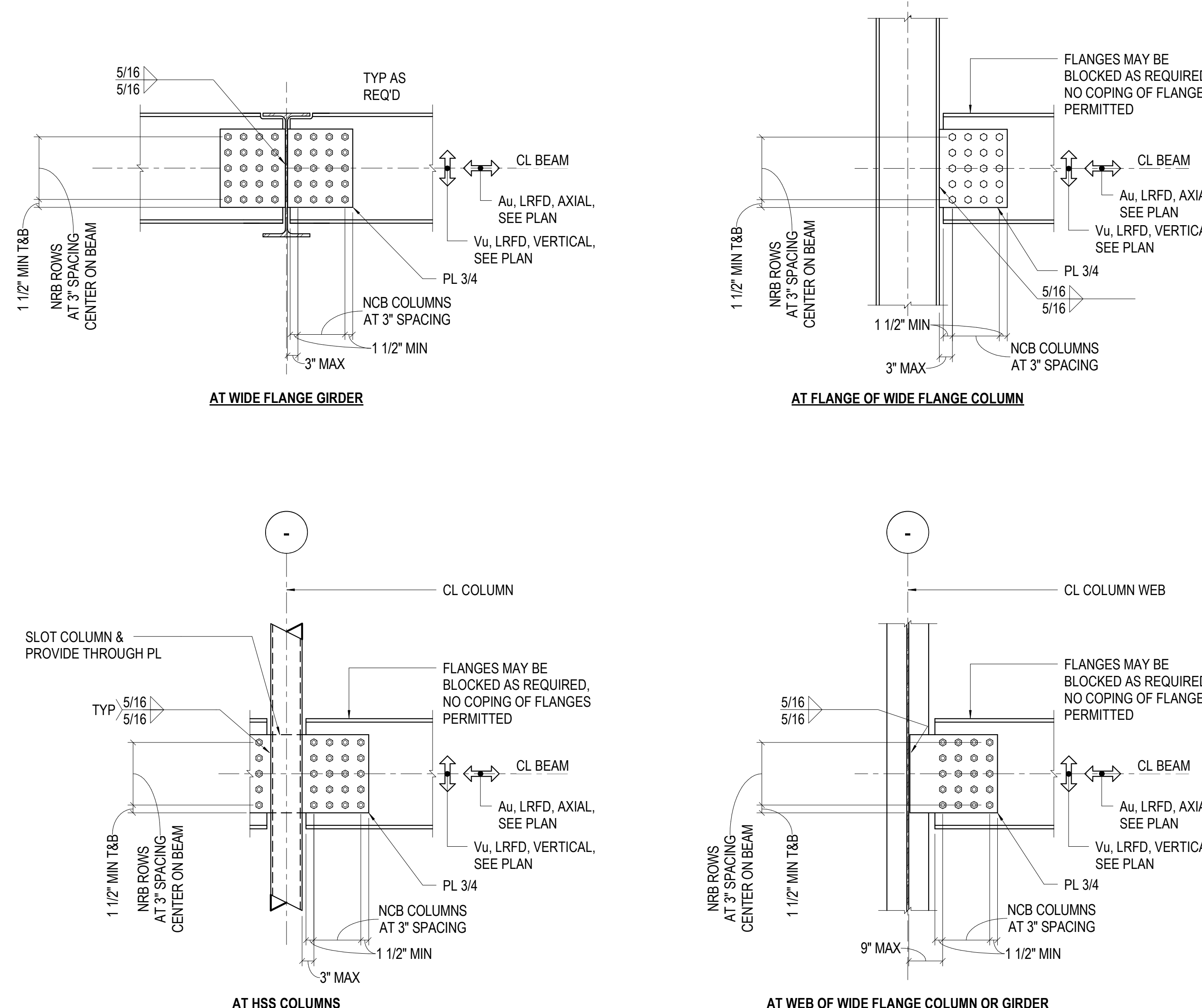
COLUMN SIZE	GAGE, gu OR gl	OPTIONAL WELD, A (IN)	FLANGE PLATES		
			ROWS OF BOLTS, N	WIDTH (IN)	LENGTH (IN)
W14x45 TO 730	13 1/2"	1/2	3	16"	1'-6 1/2
W14x257 TO 426	11 1/2"	1/2	3	14"	1'-6 1/2
W14x145 TO 233	11 1/2"	3/8	3	14"	1'-0 1/2
W14x90 TO 132	11 1/2"	5/16	2	14"	1'-0 1/2
W14x43 TO 82	5 1/2"	5/16	2	8"	1'-0 1/2



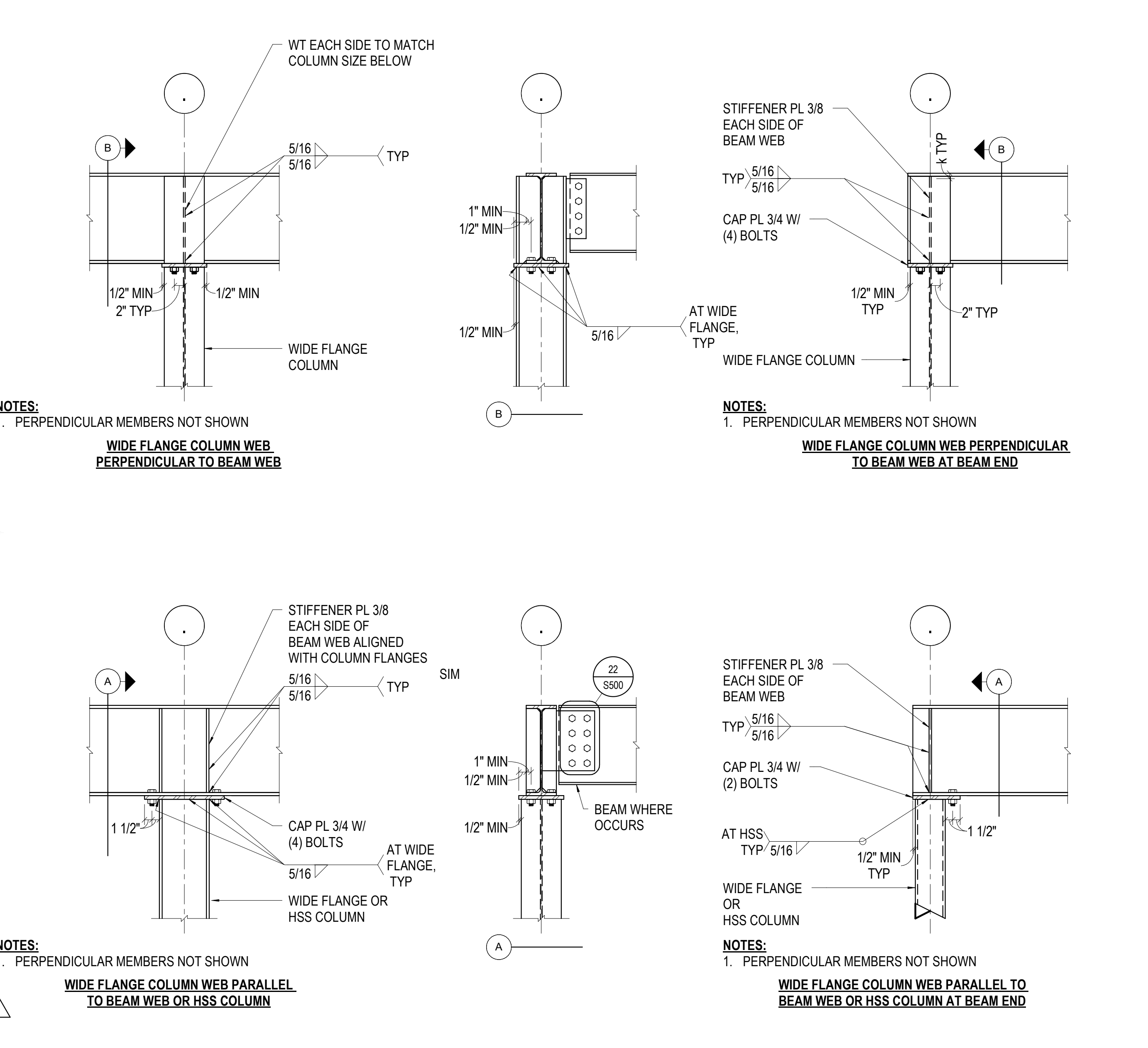
19 3/4" = 1'-0" MONUMENTAL STAIR TRENCH VENT DETAIL



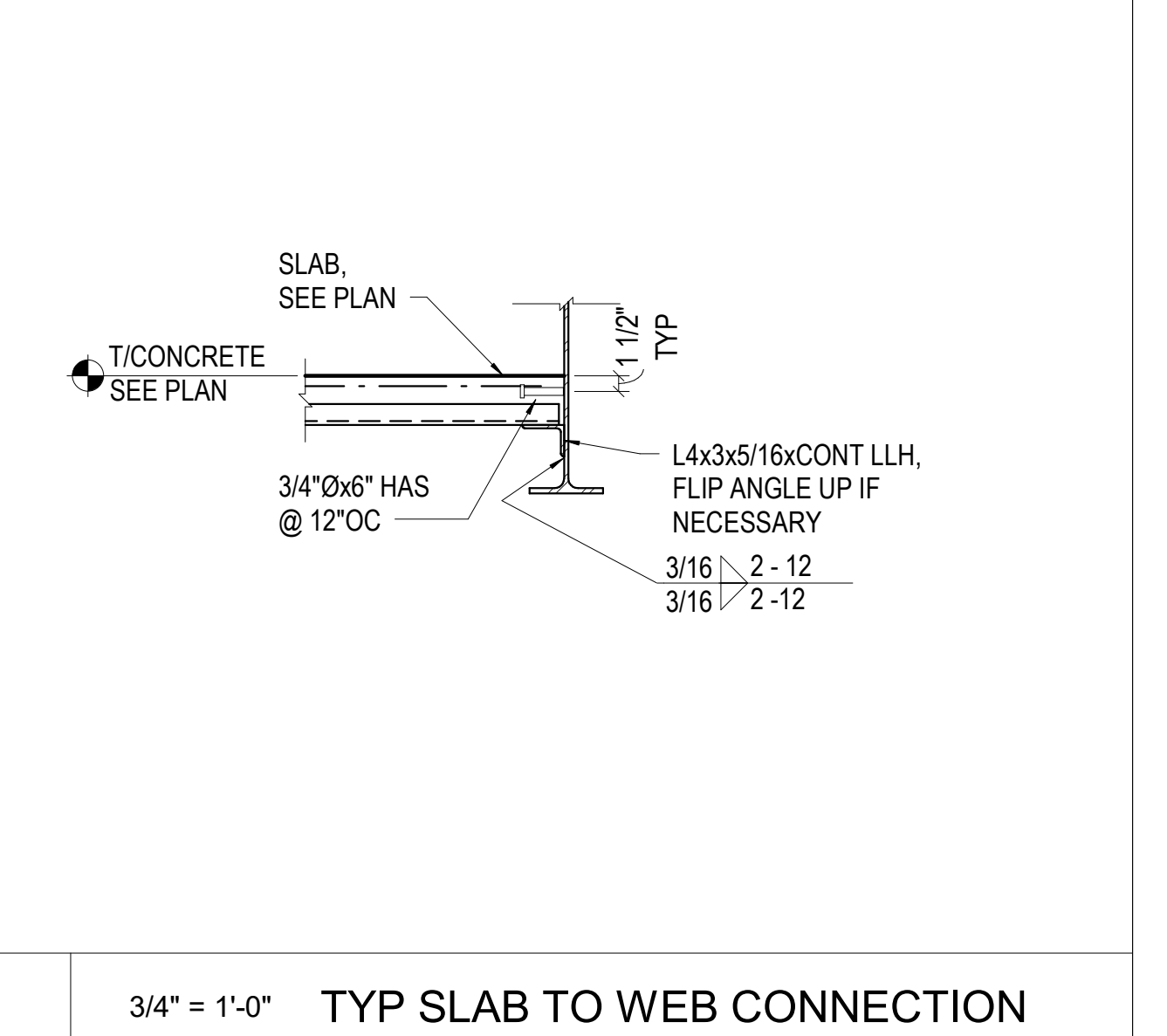
11 NO SCALE TYPICAL BRACED FRAME CONNECTIONS



13 NO SCALE TYPICAL BEAM DRAG CONNECTION

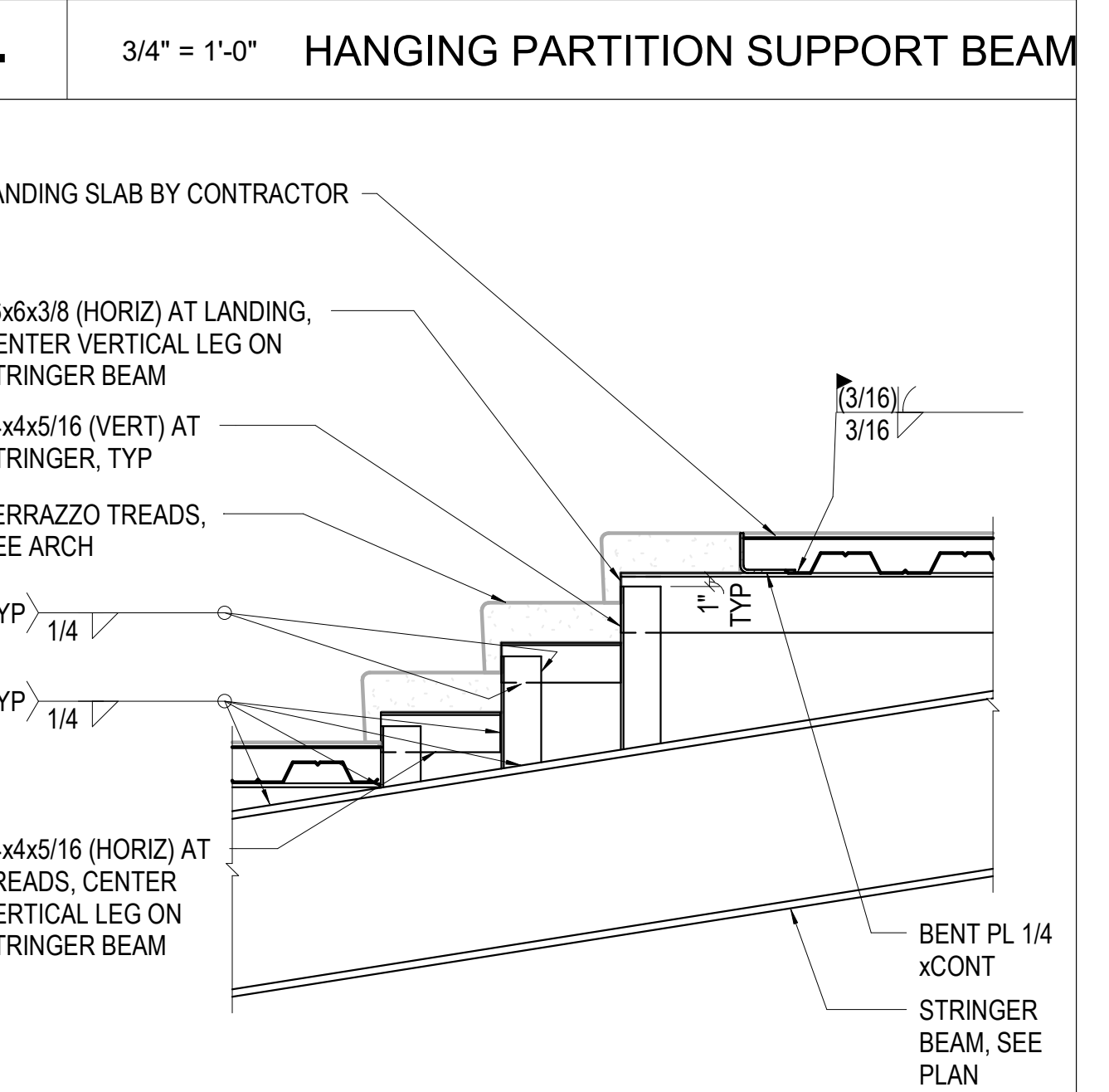
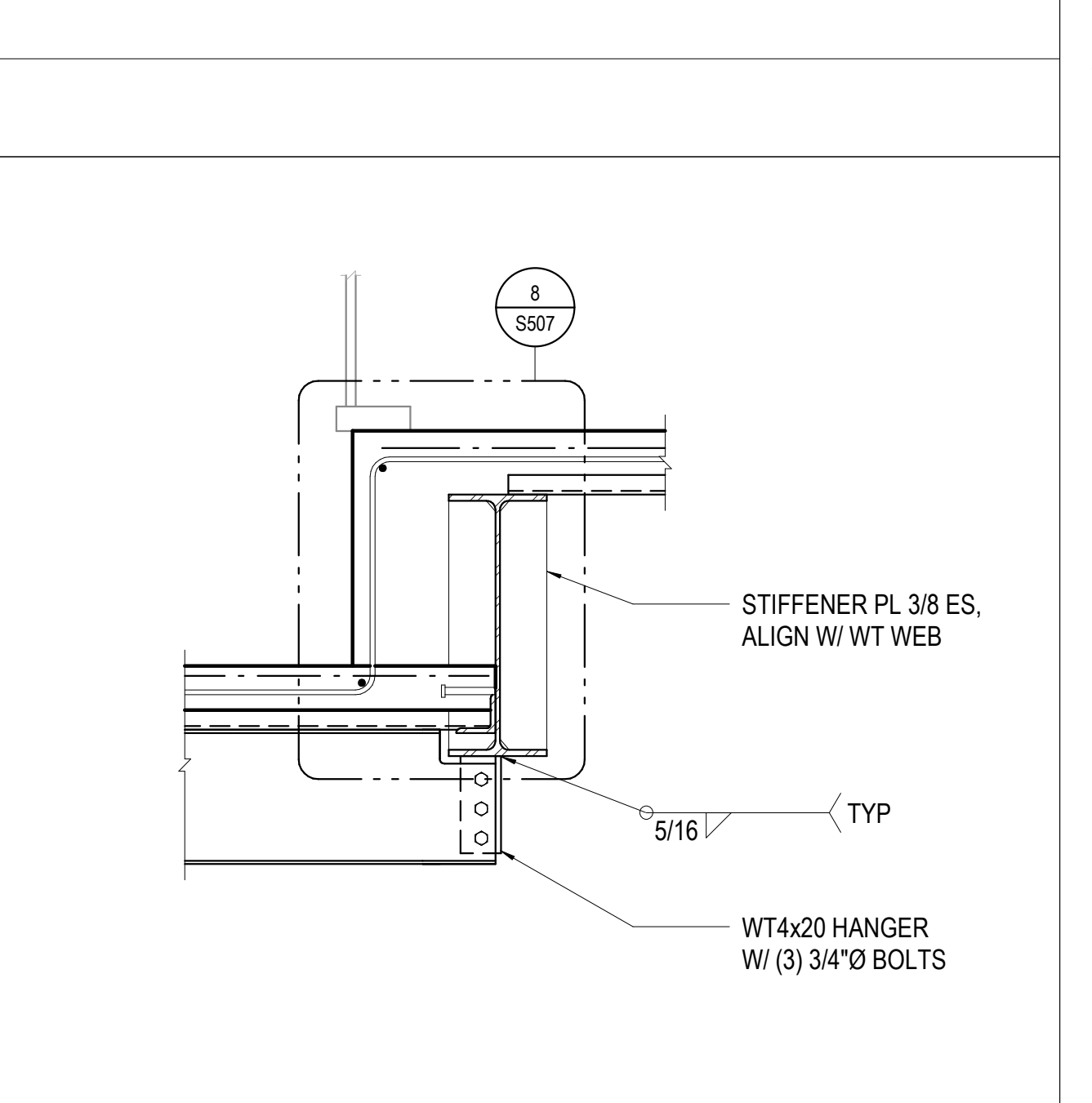


15 3/4" = 1'-0" BEAM OVER COLUMN



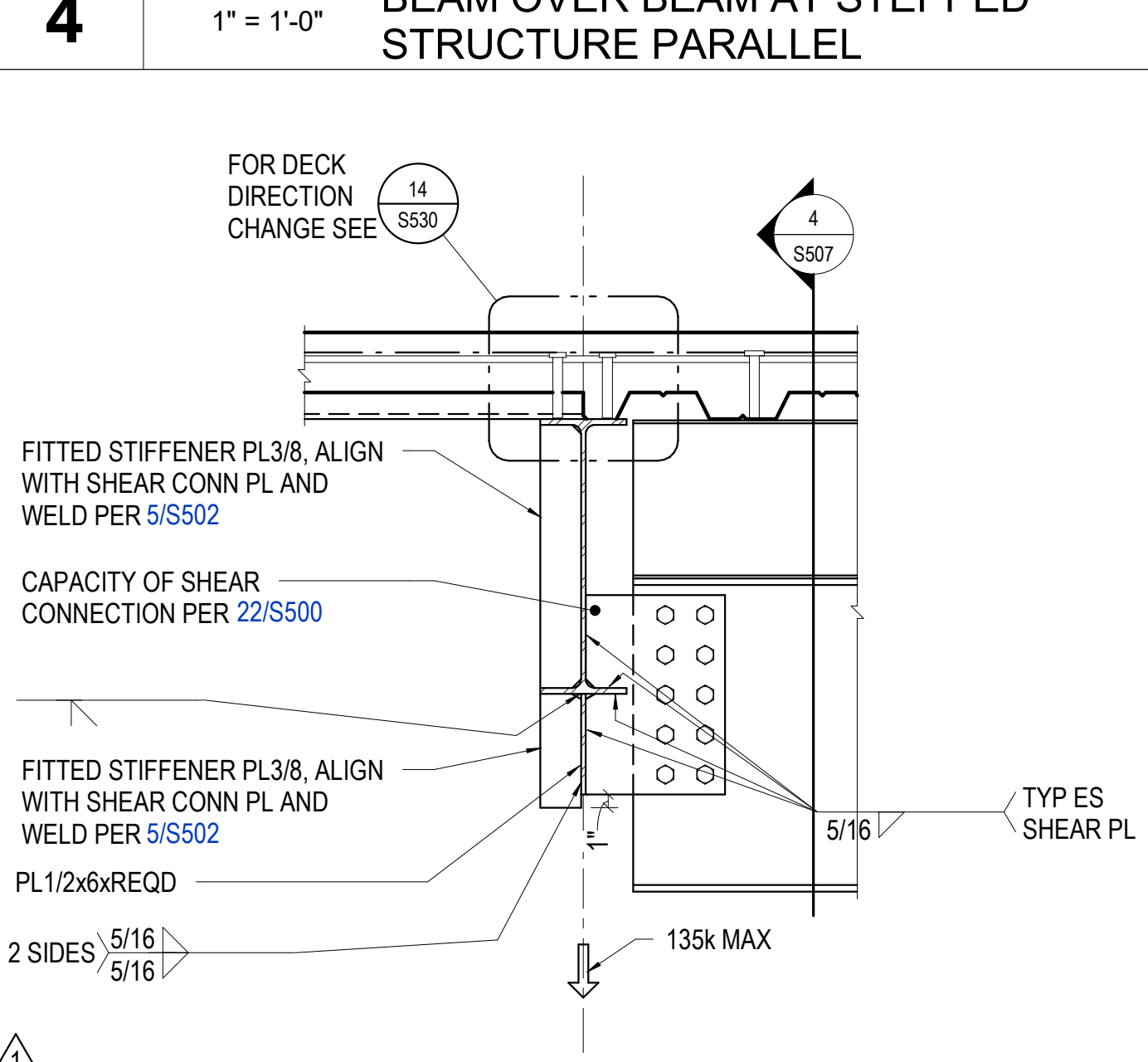
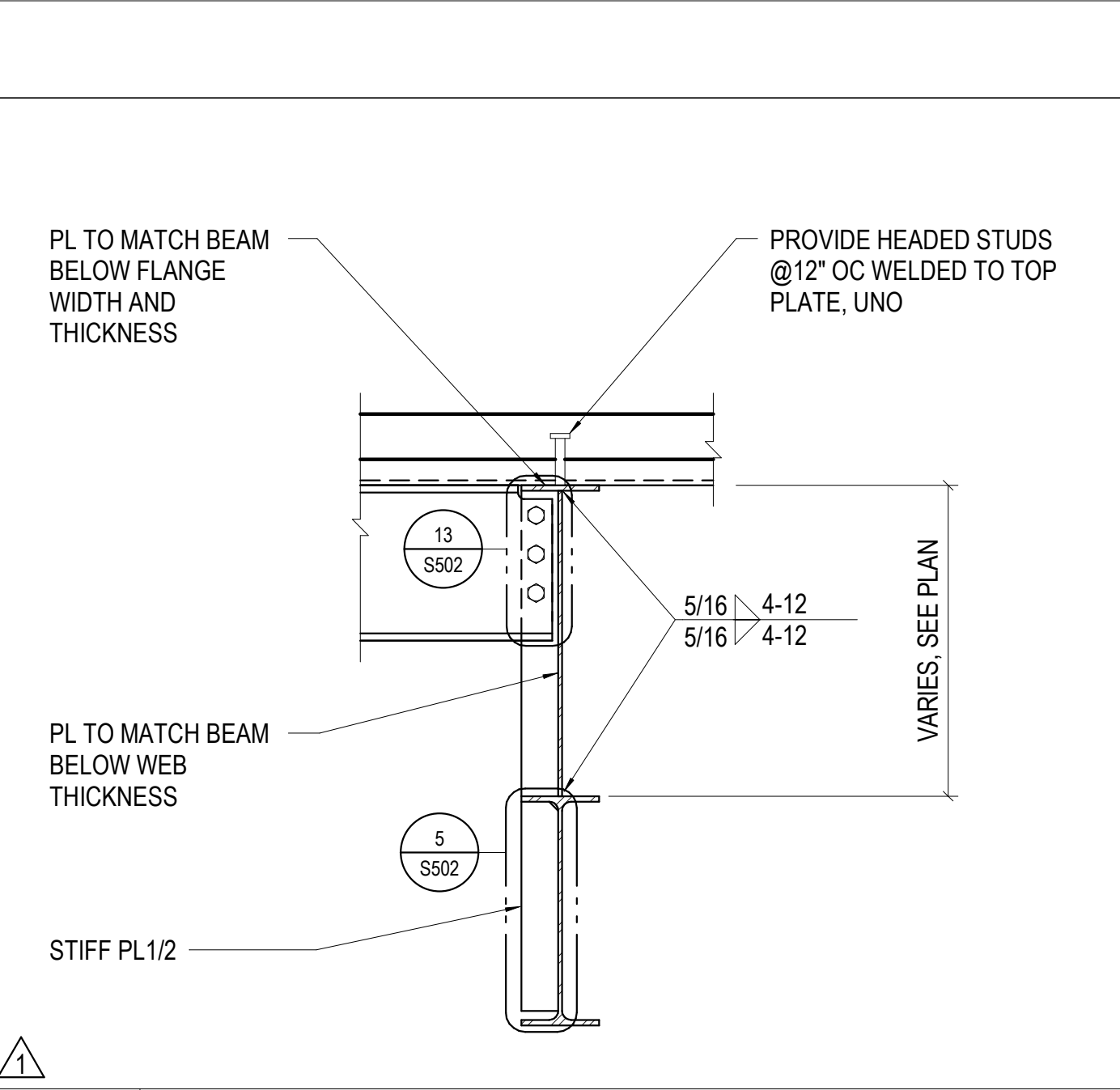
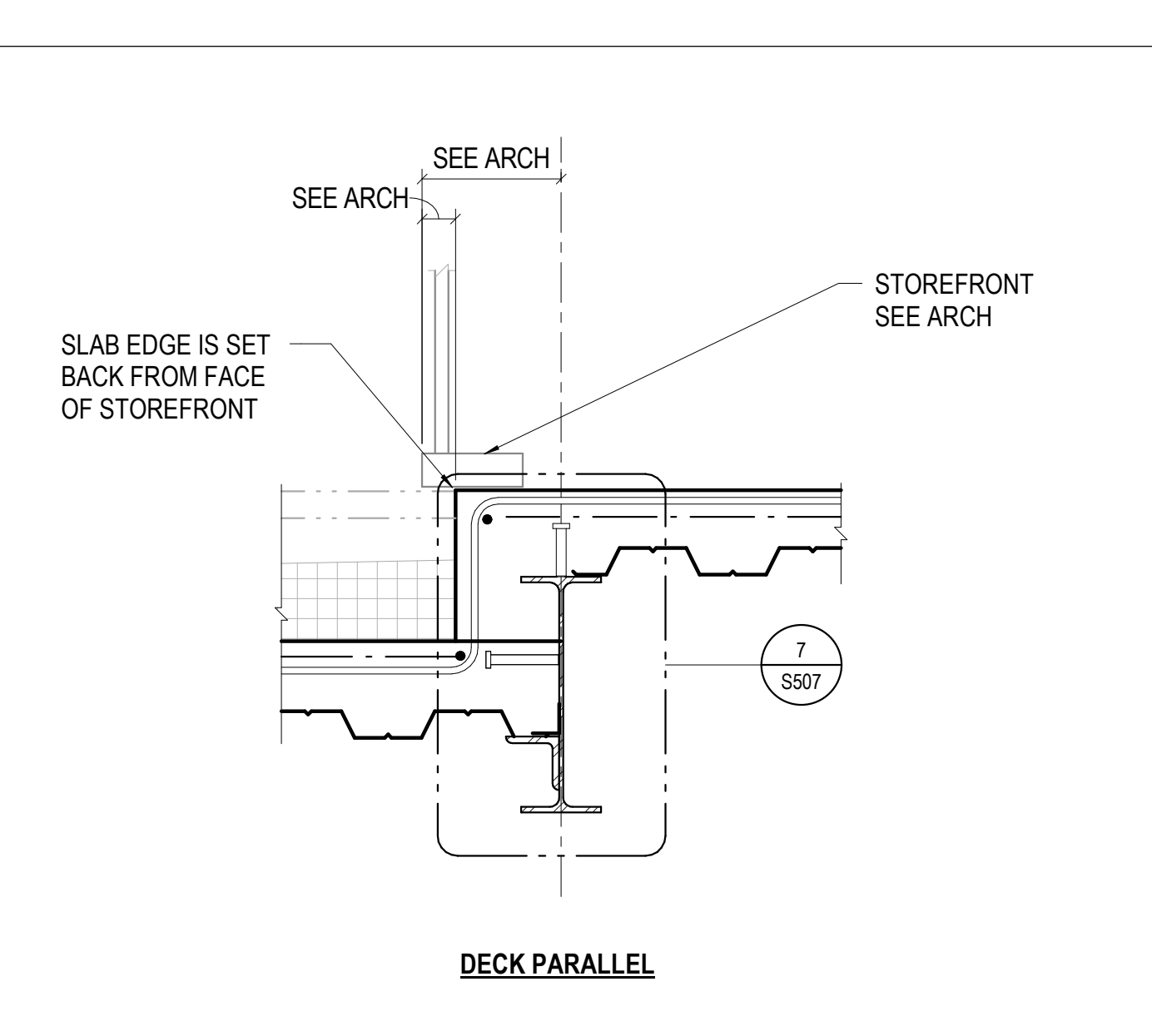
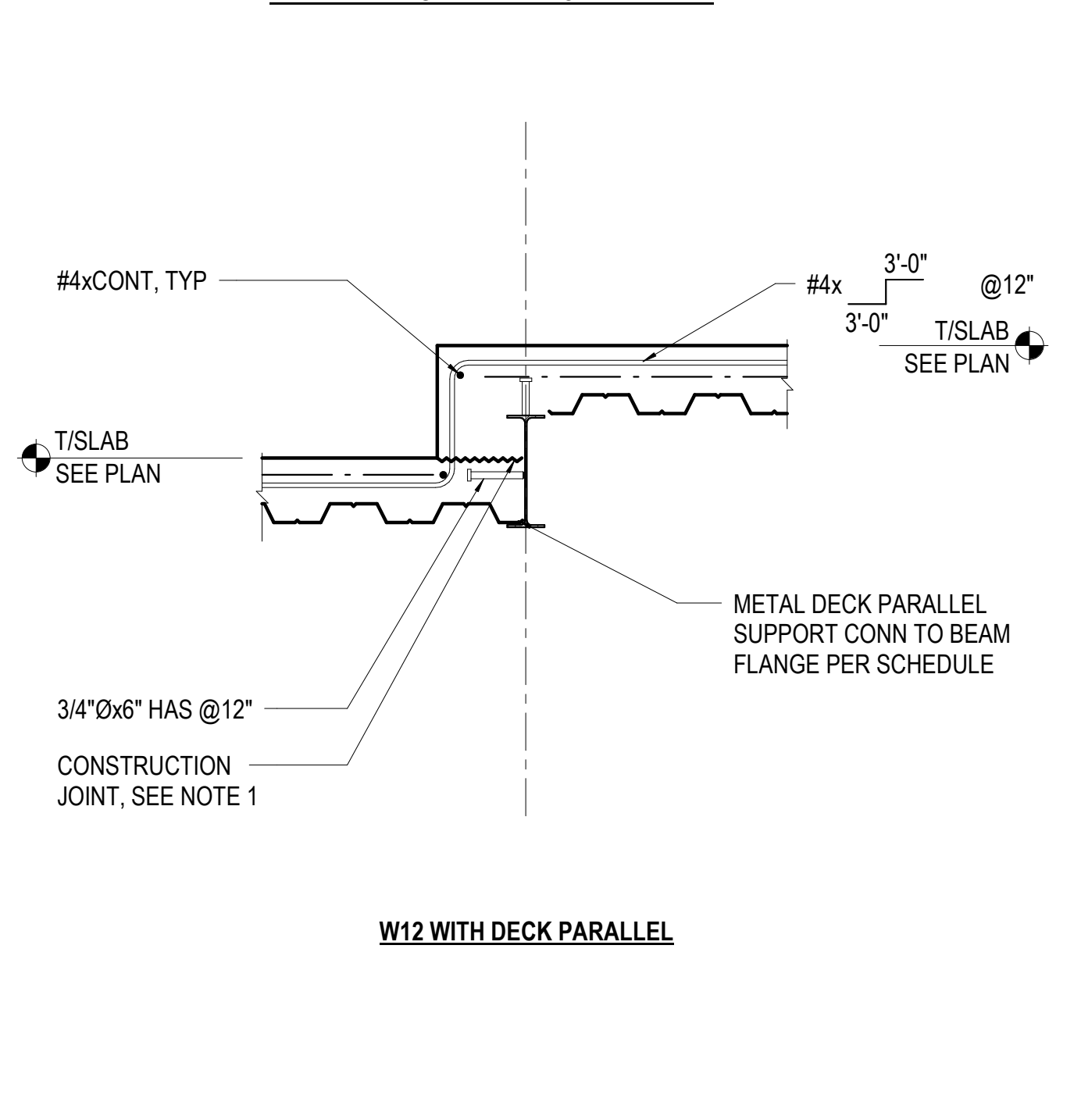
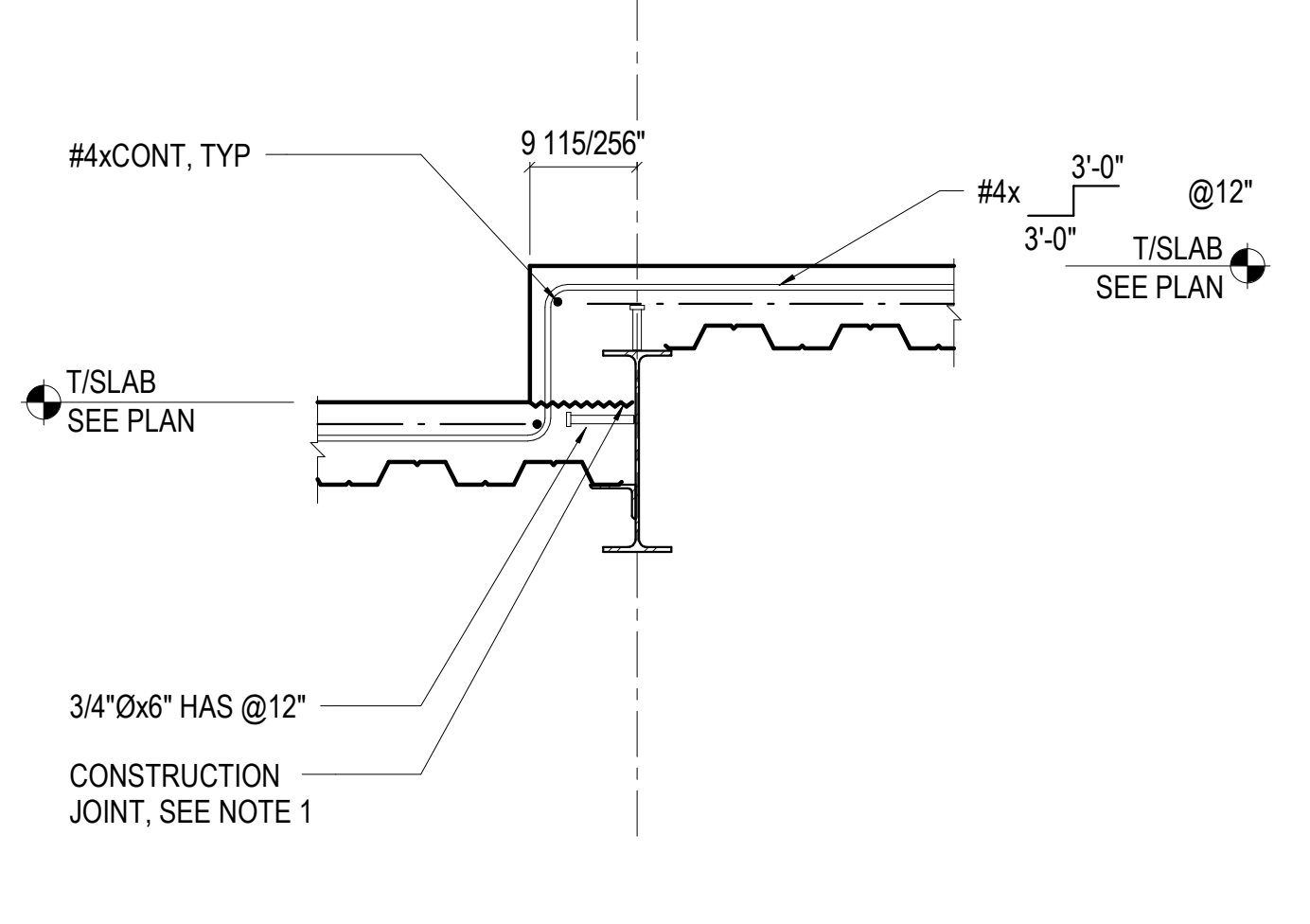
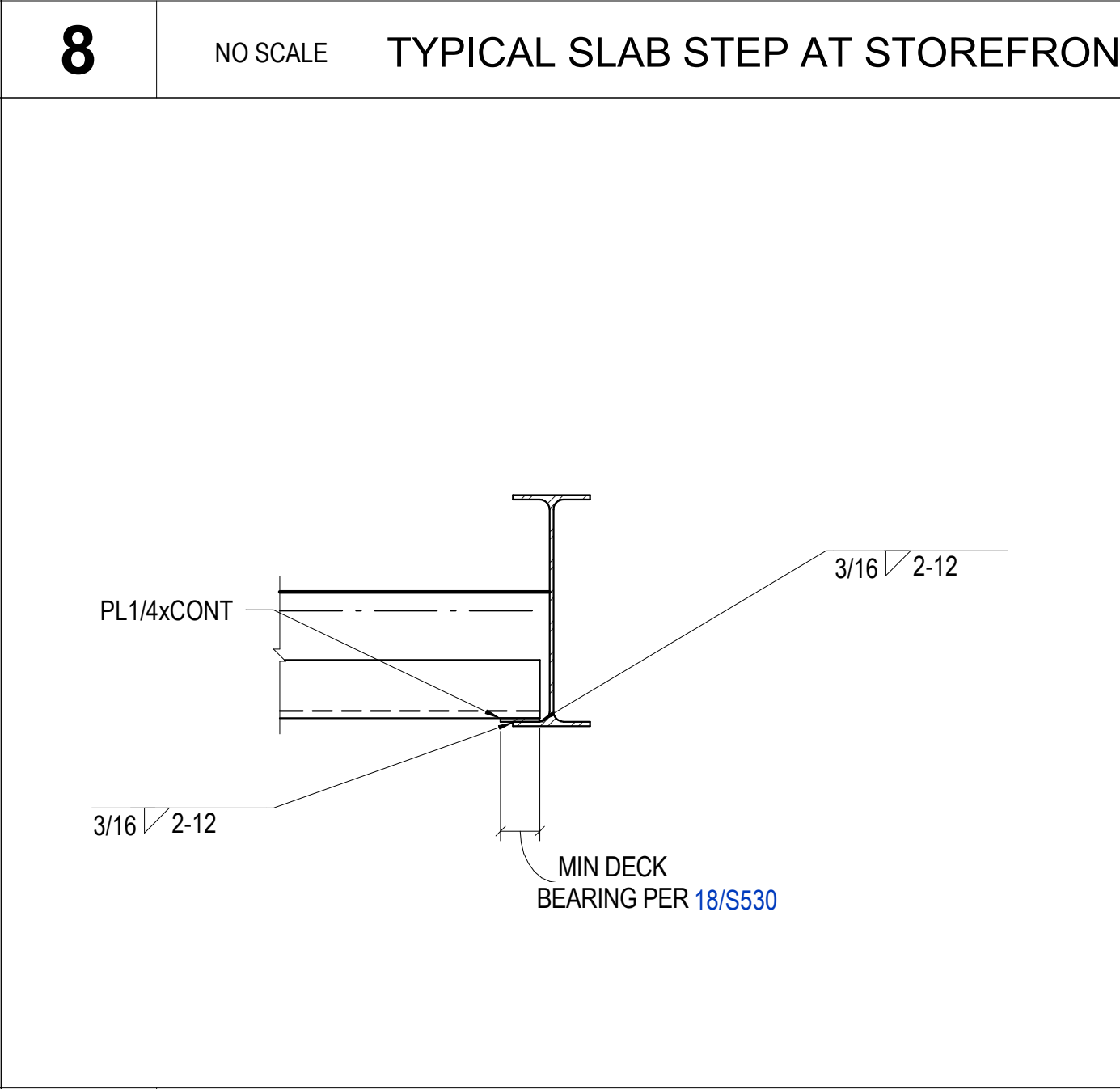
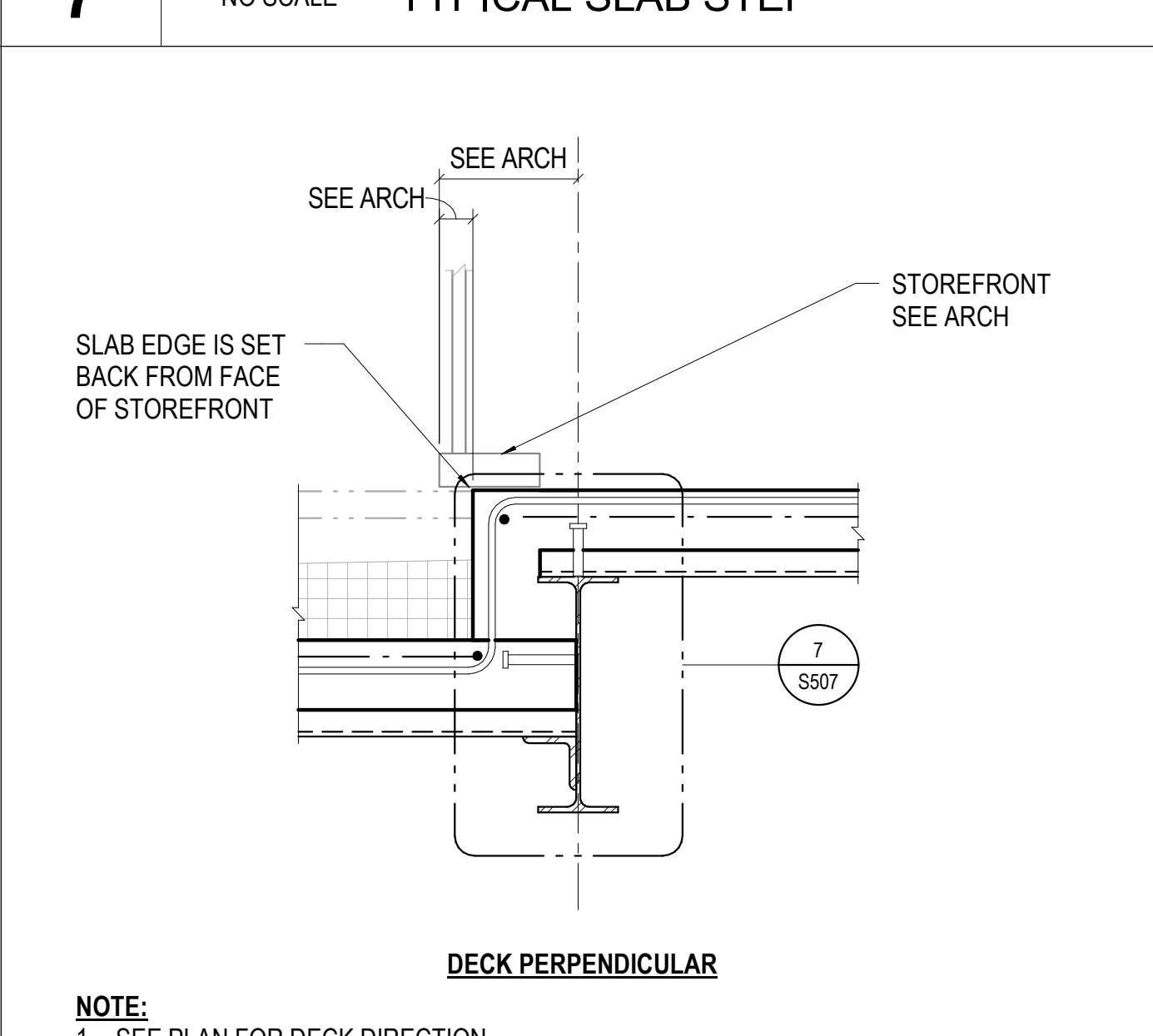
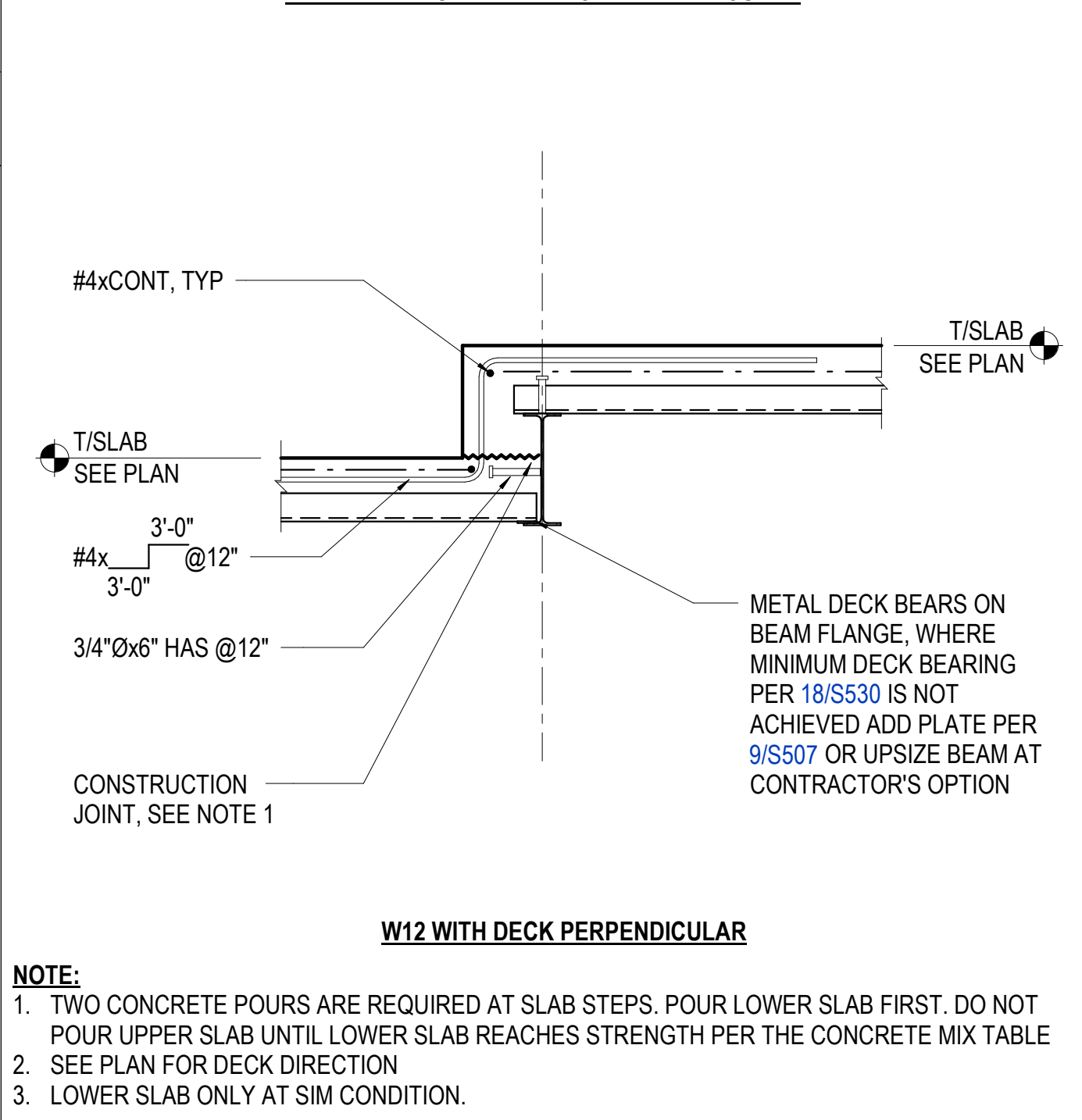
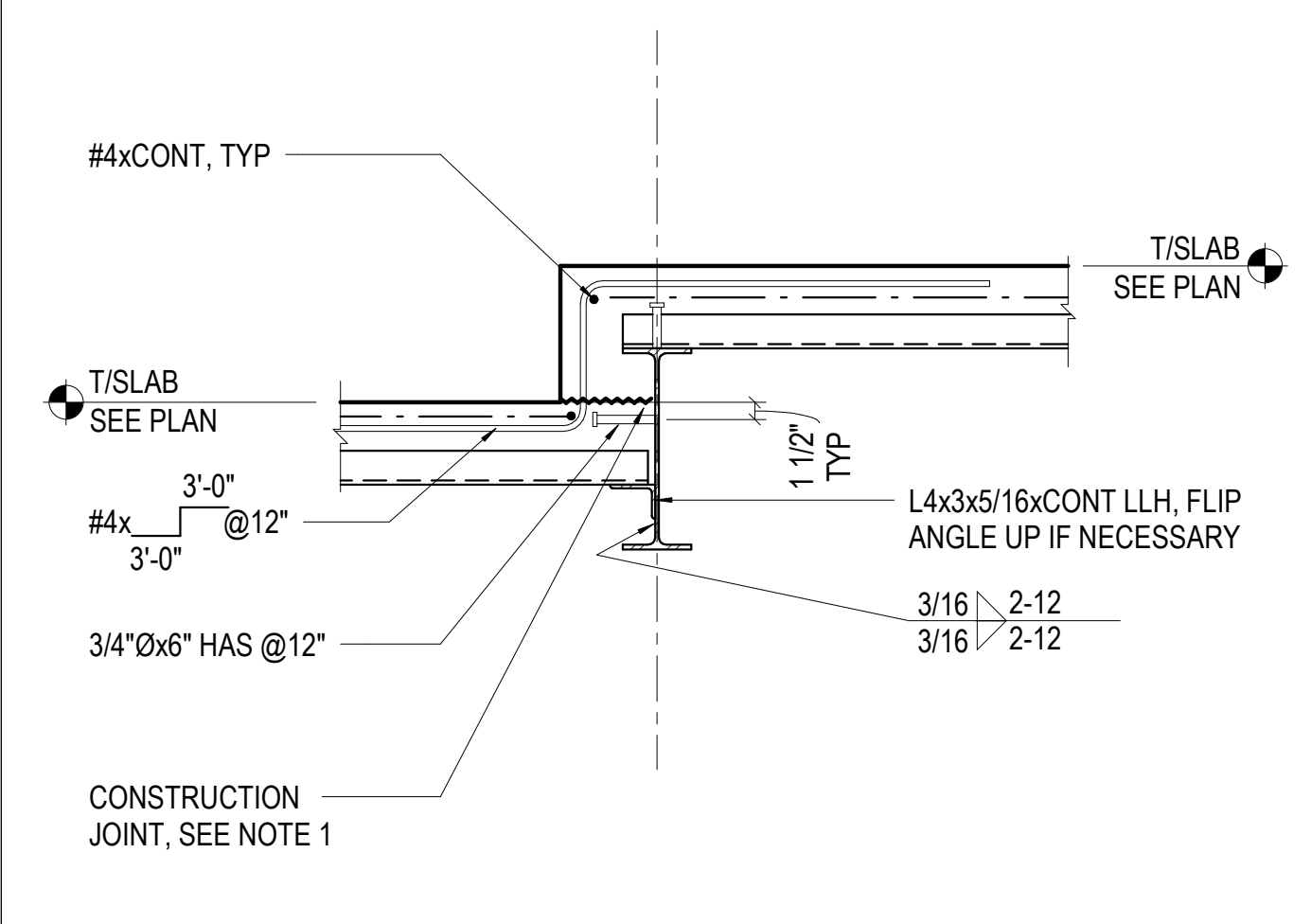
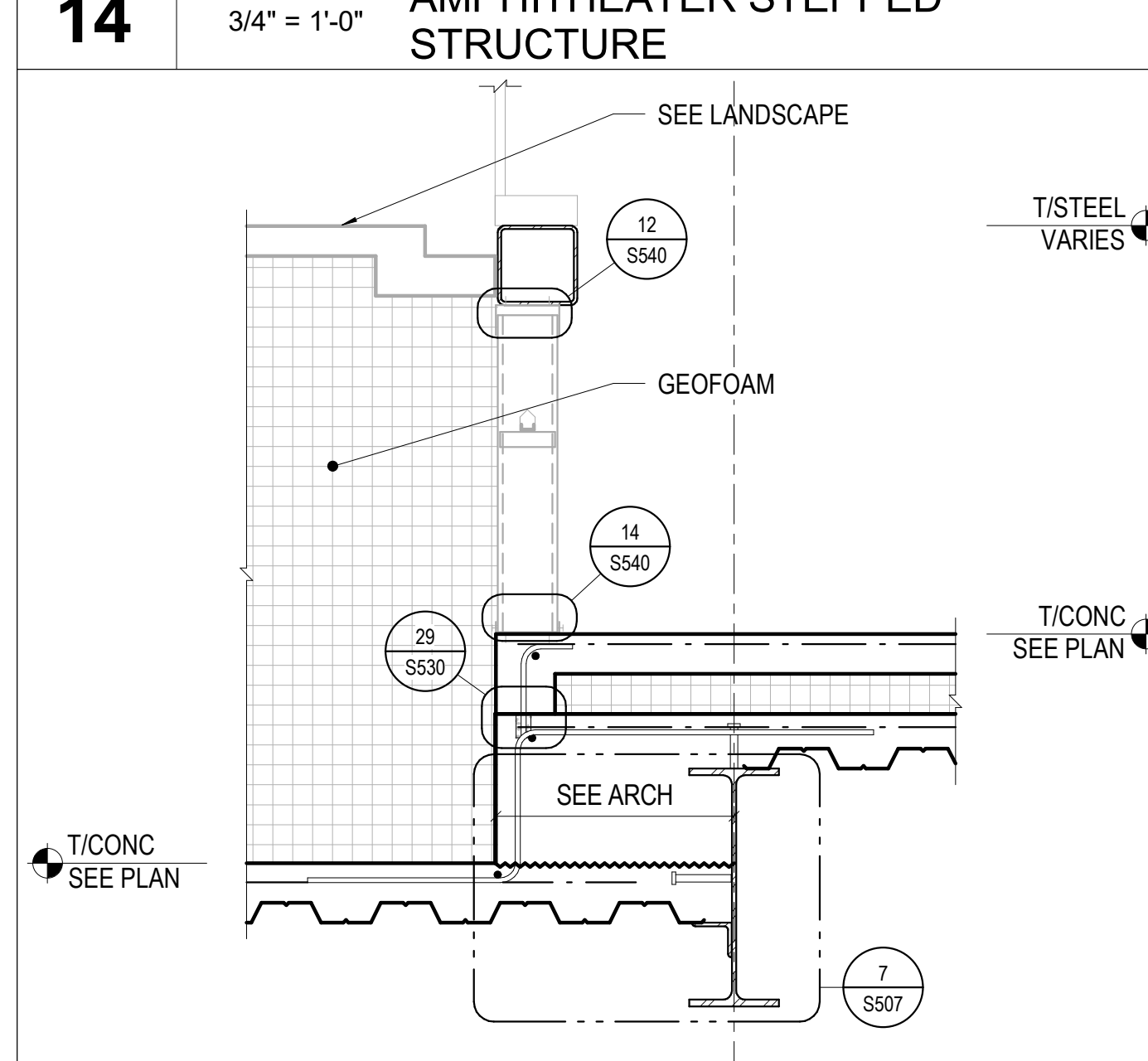
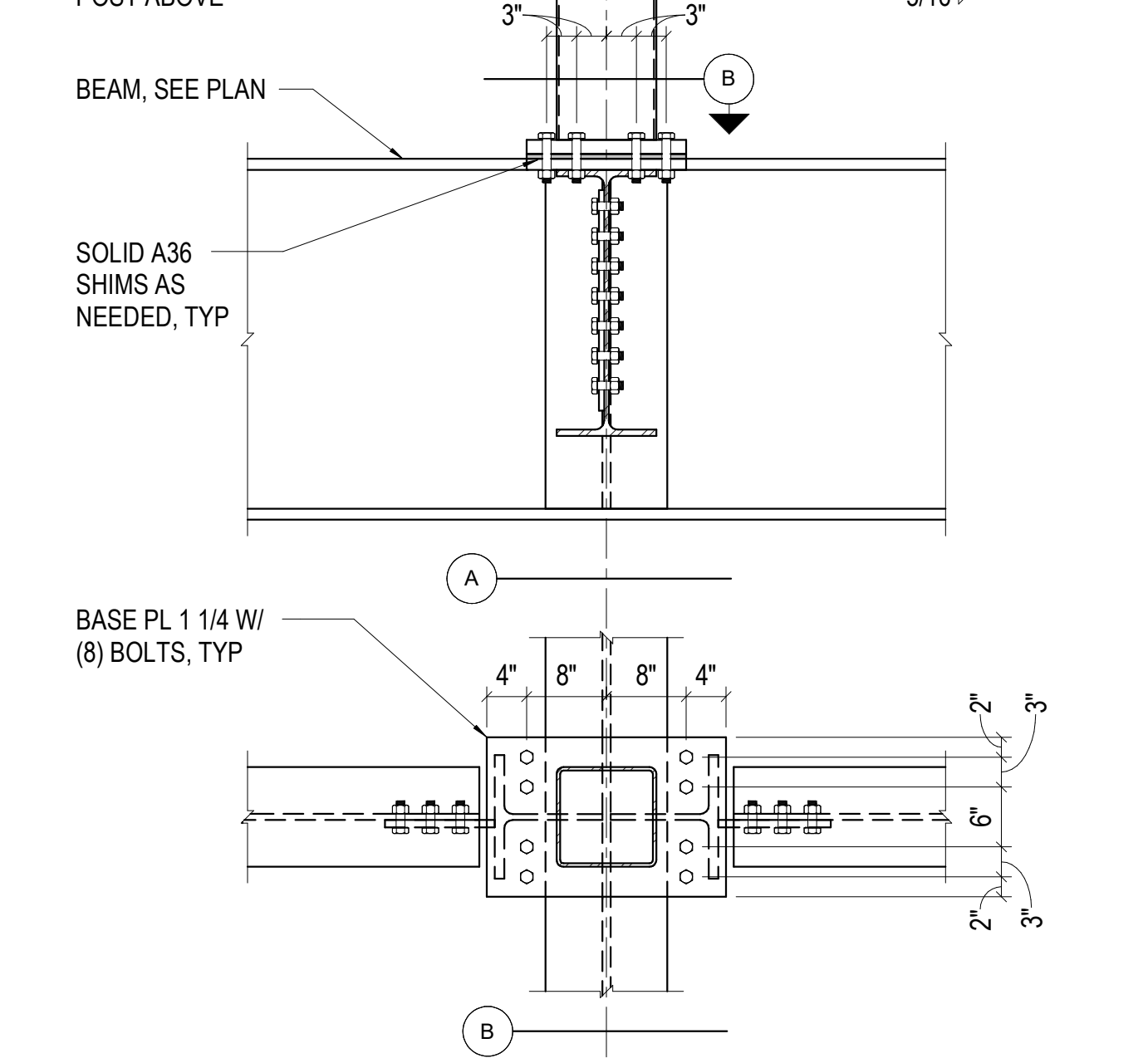
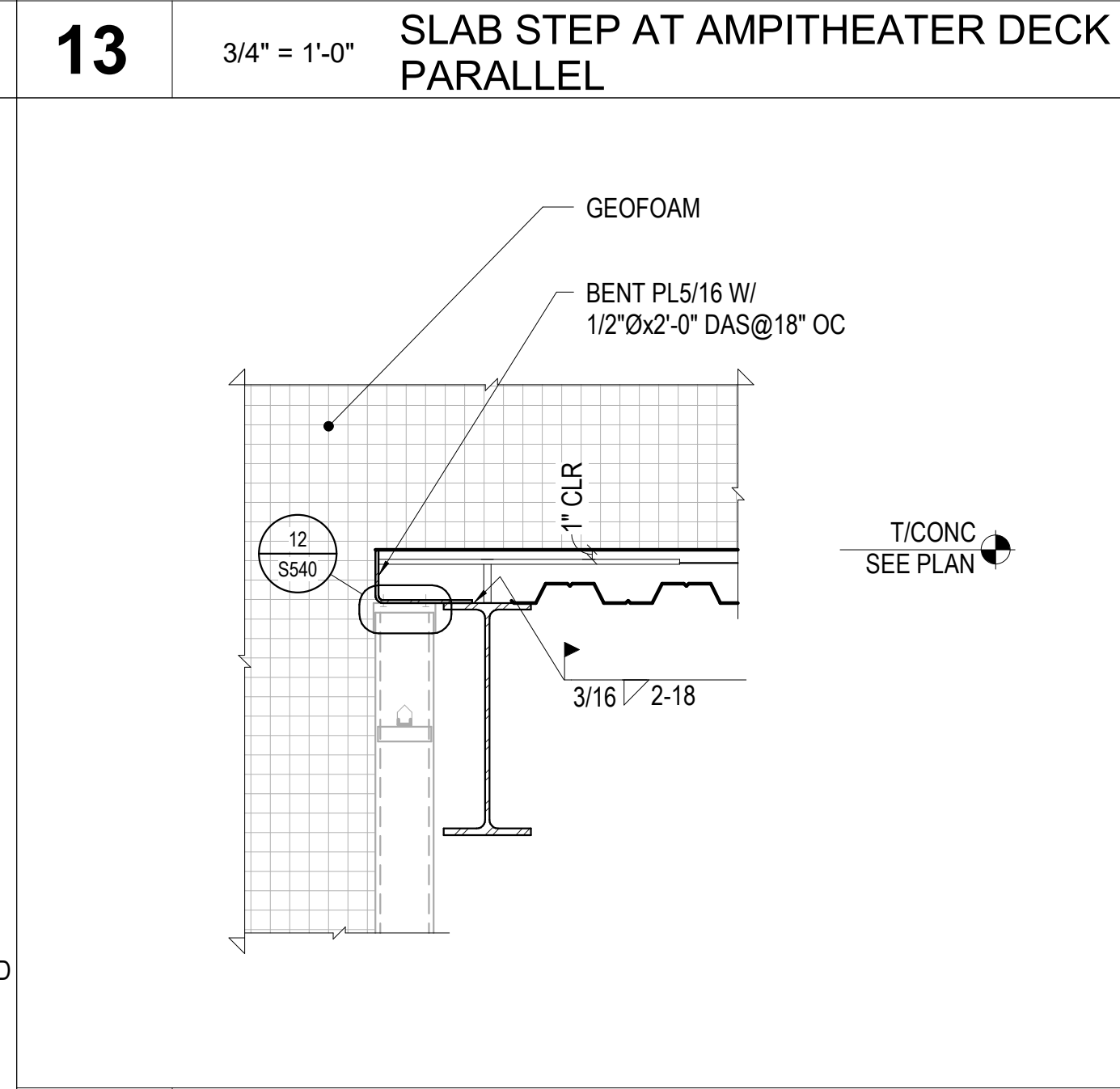
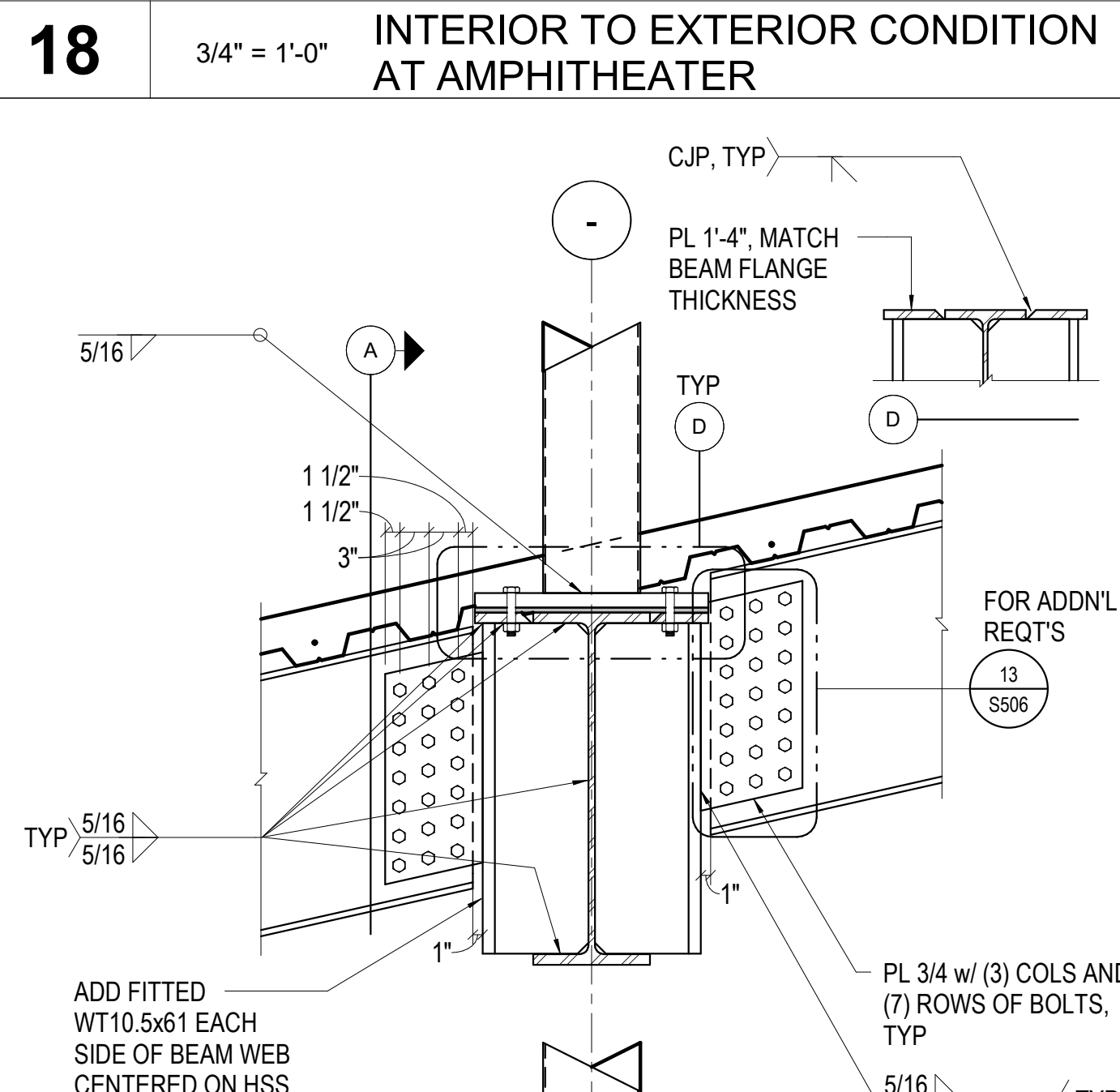
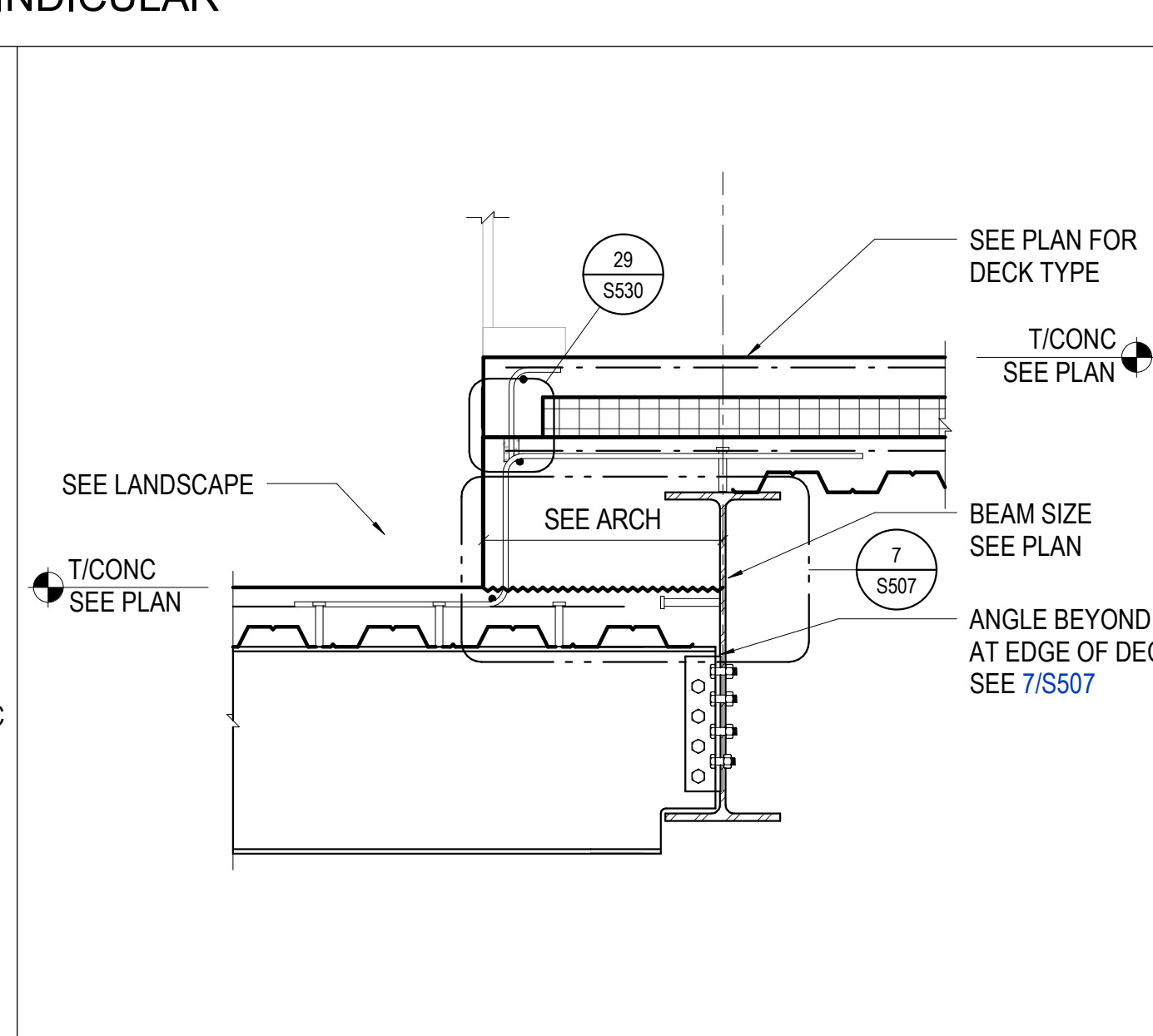
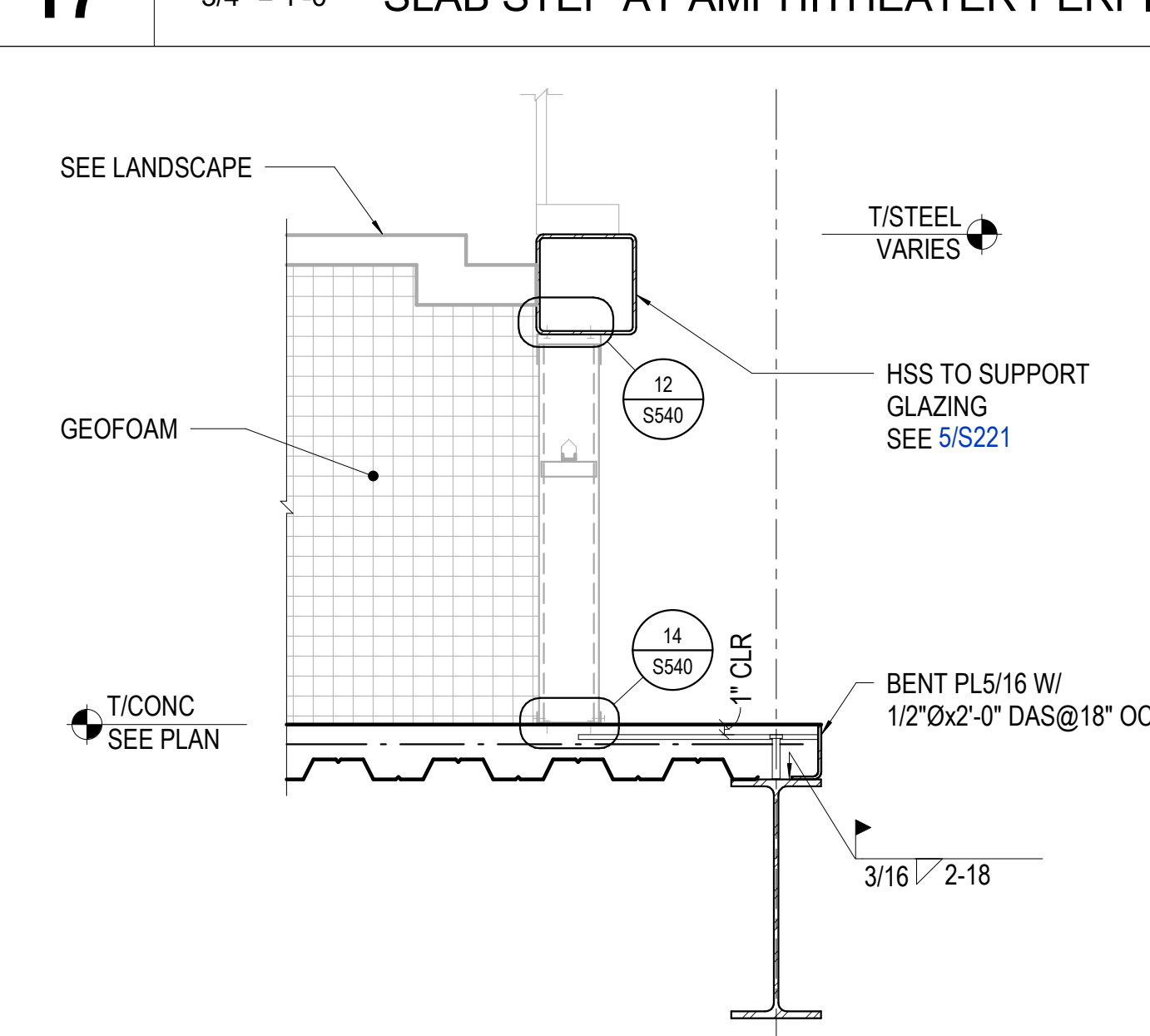
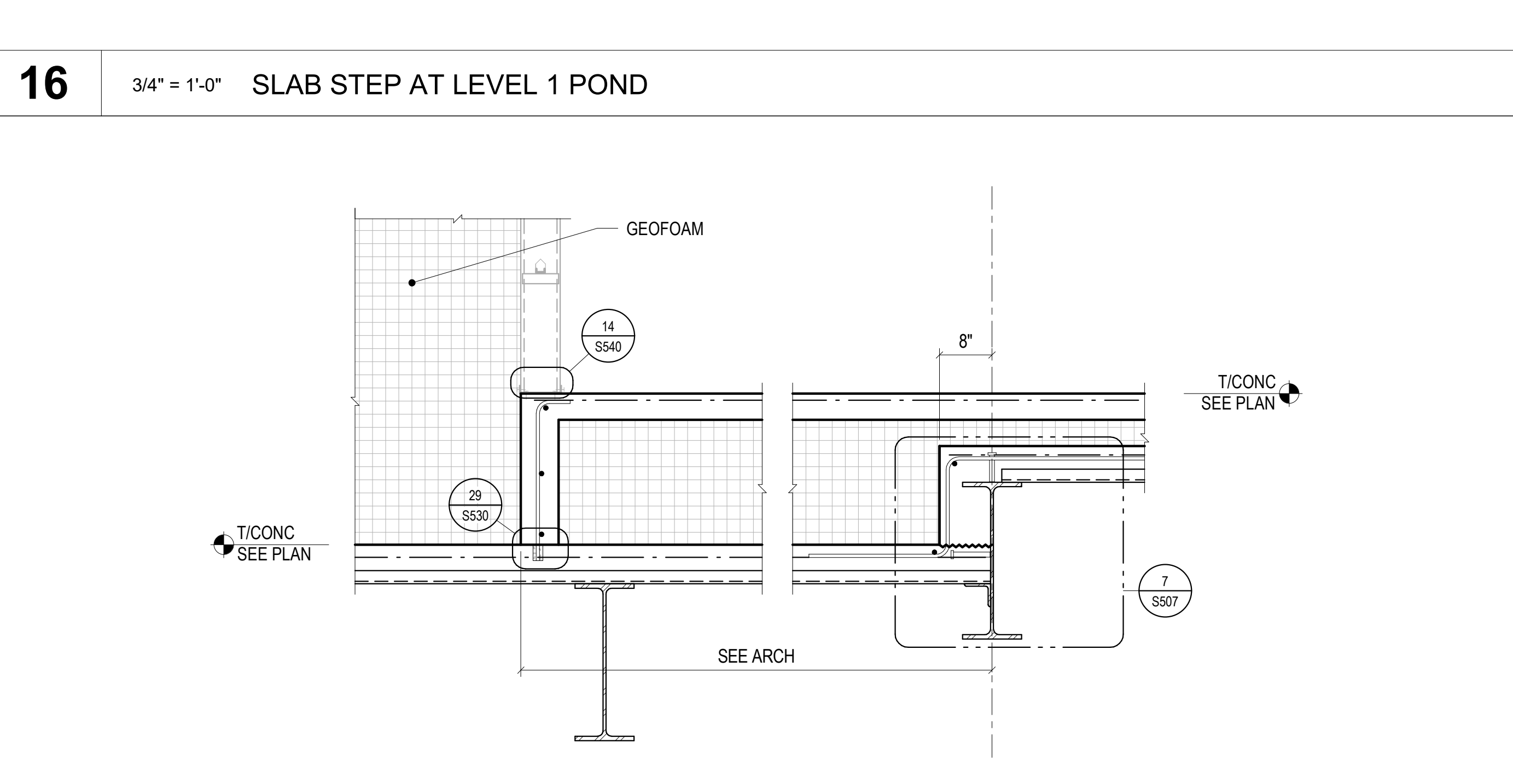
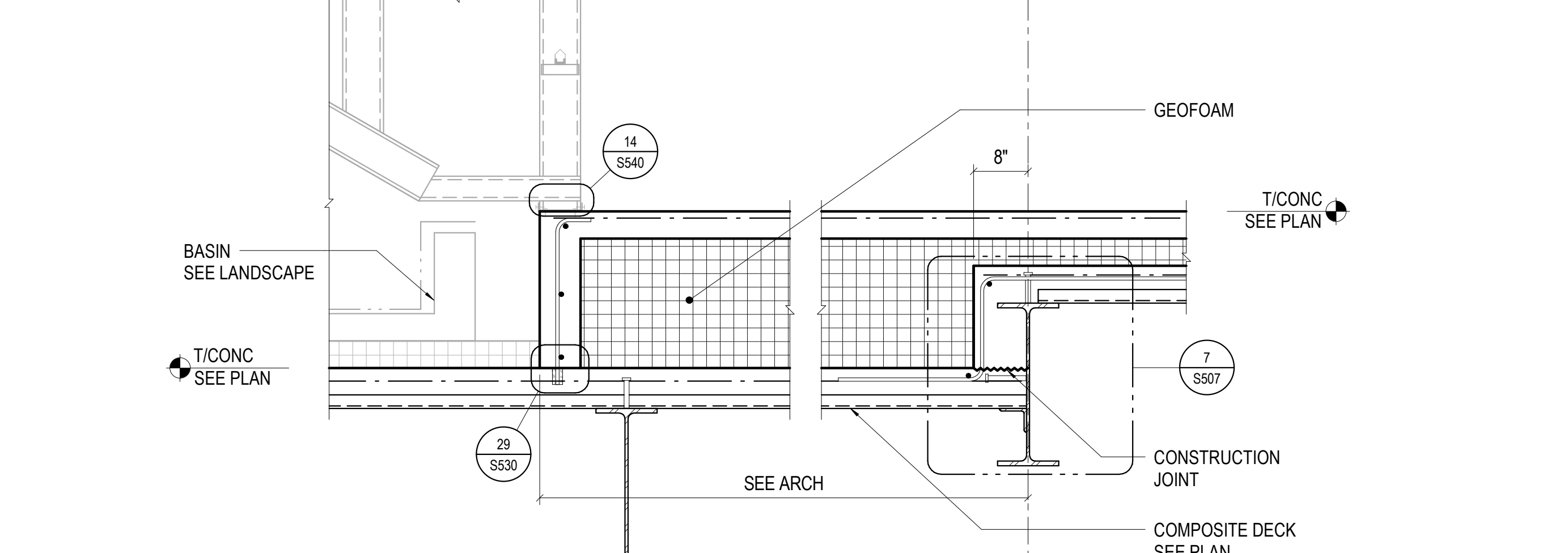
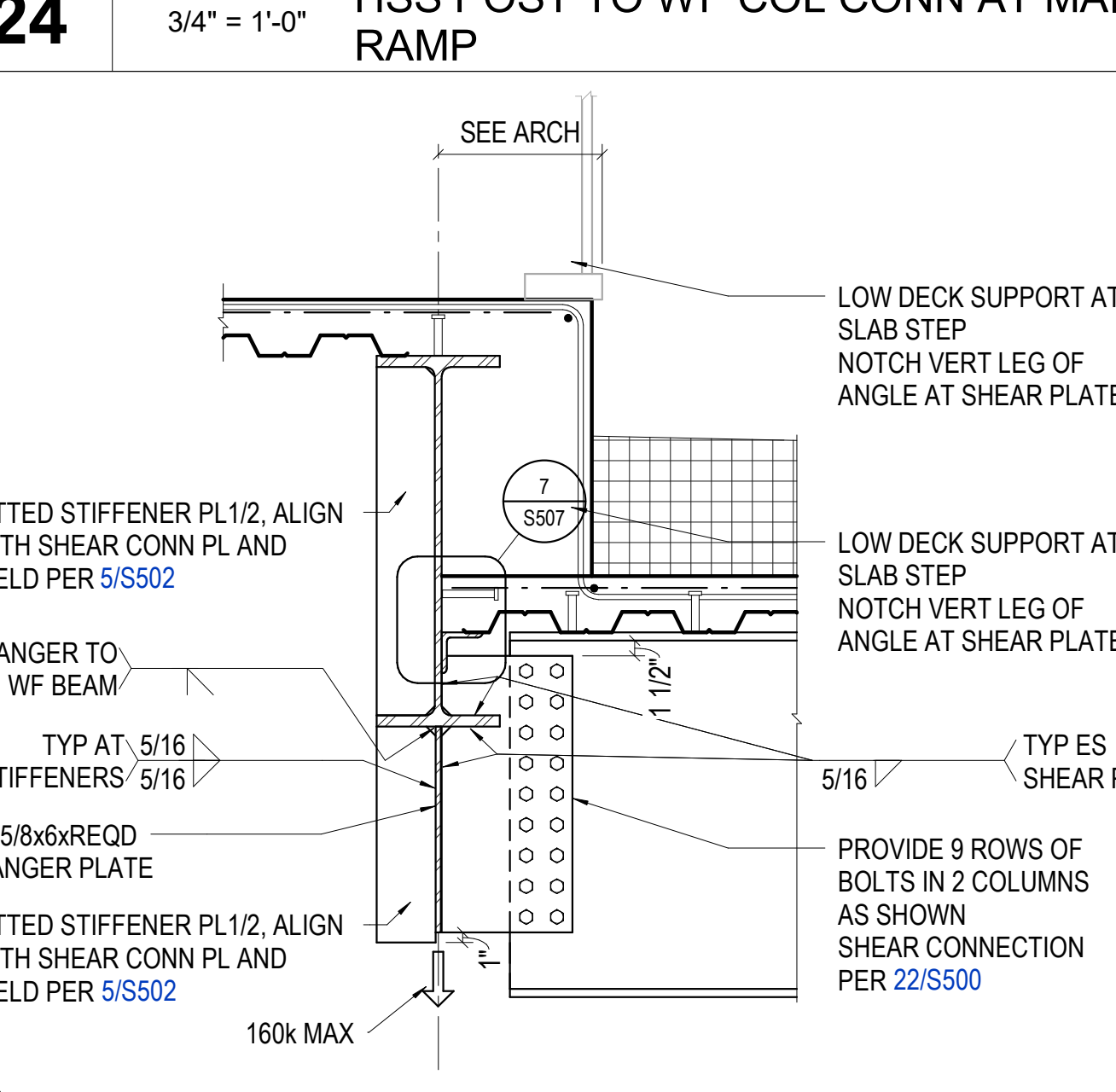
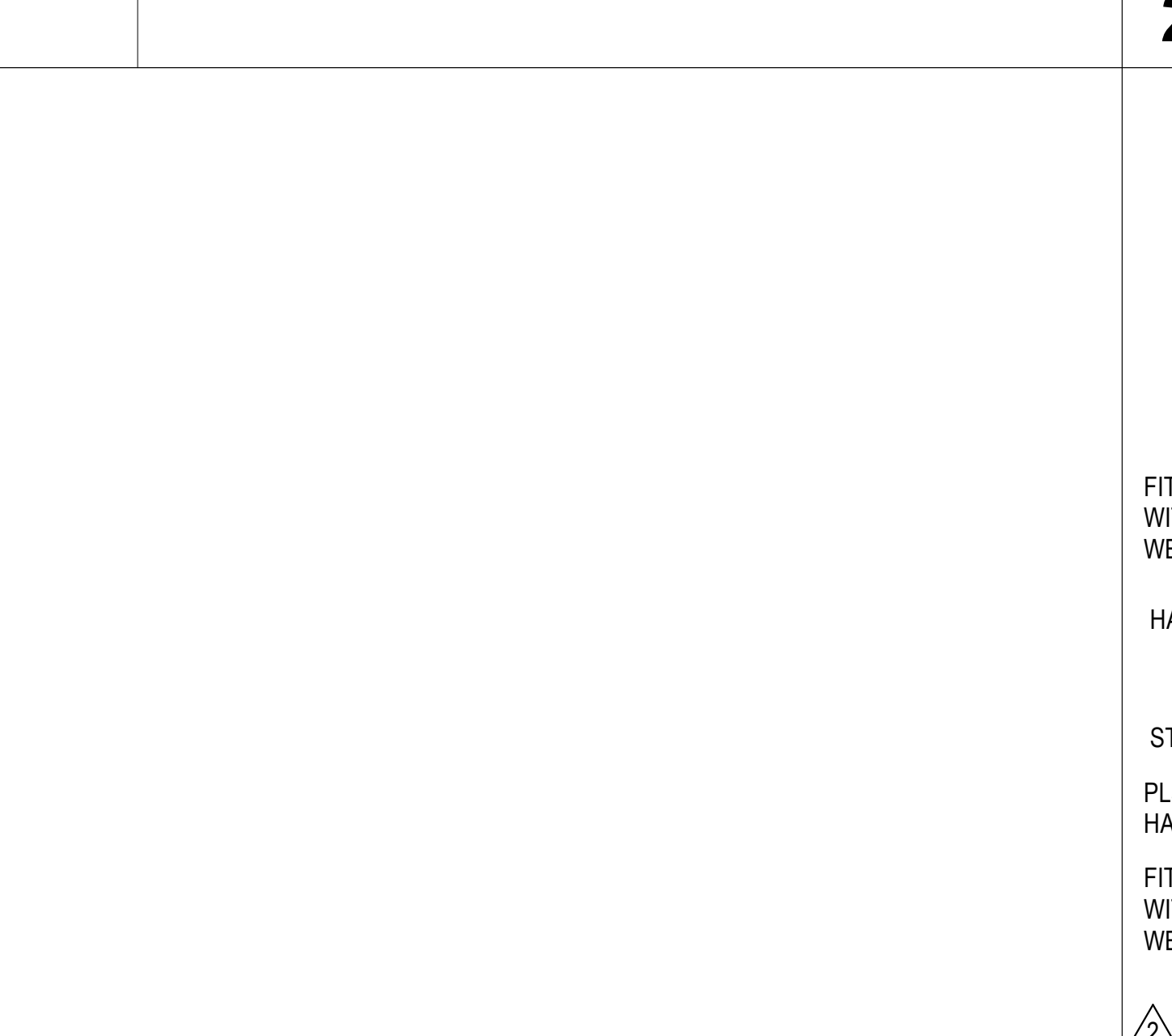
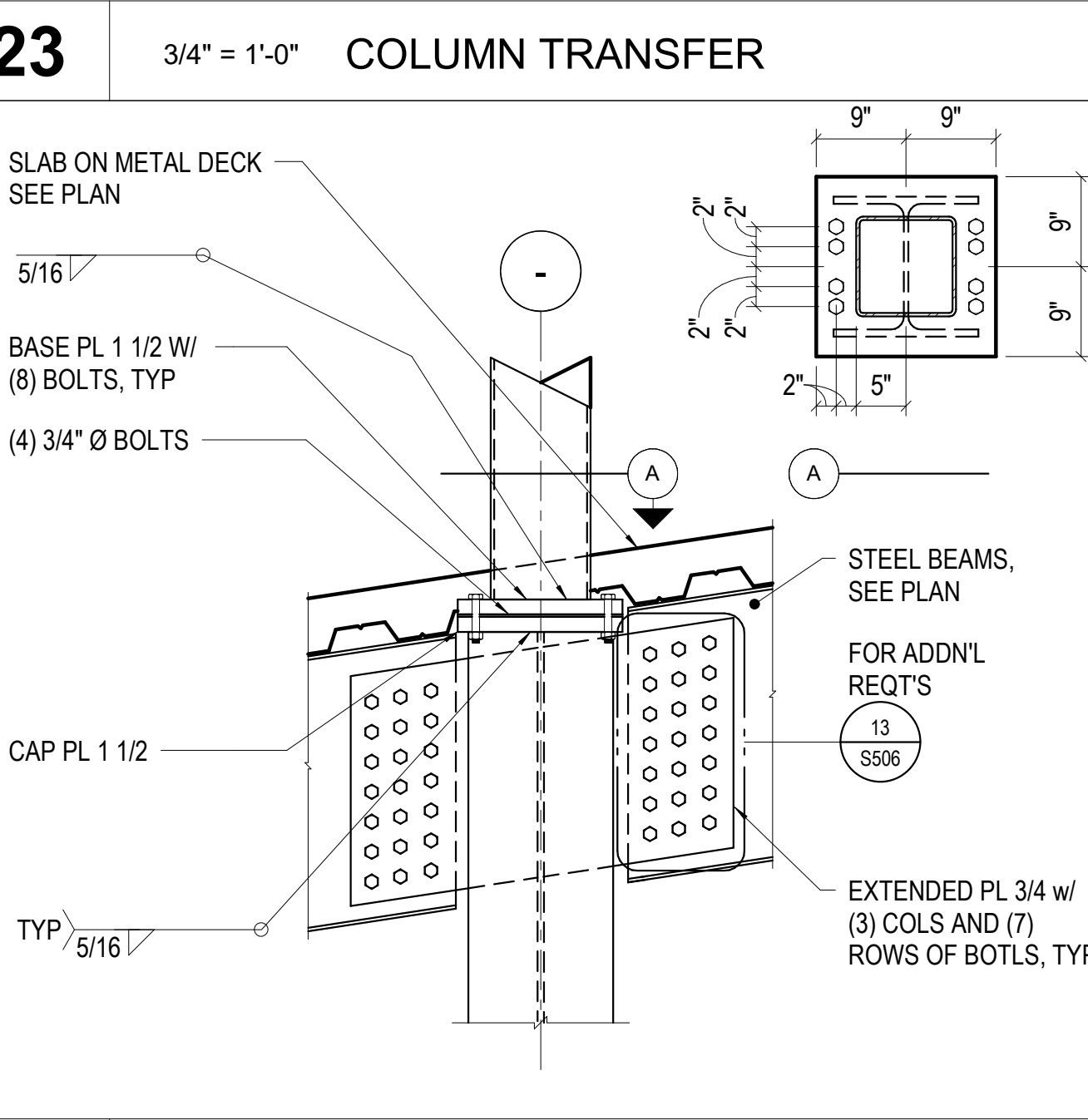
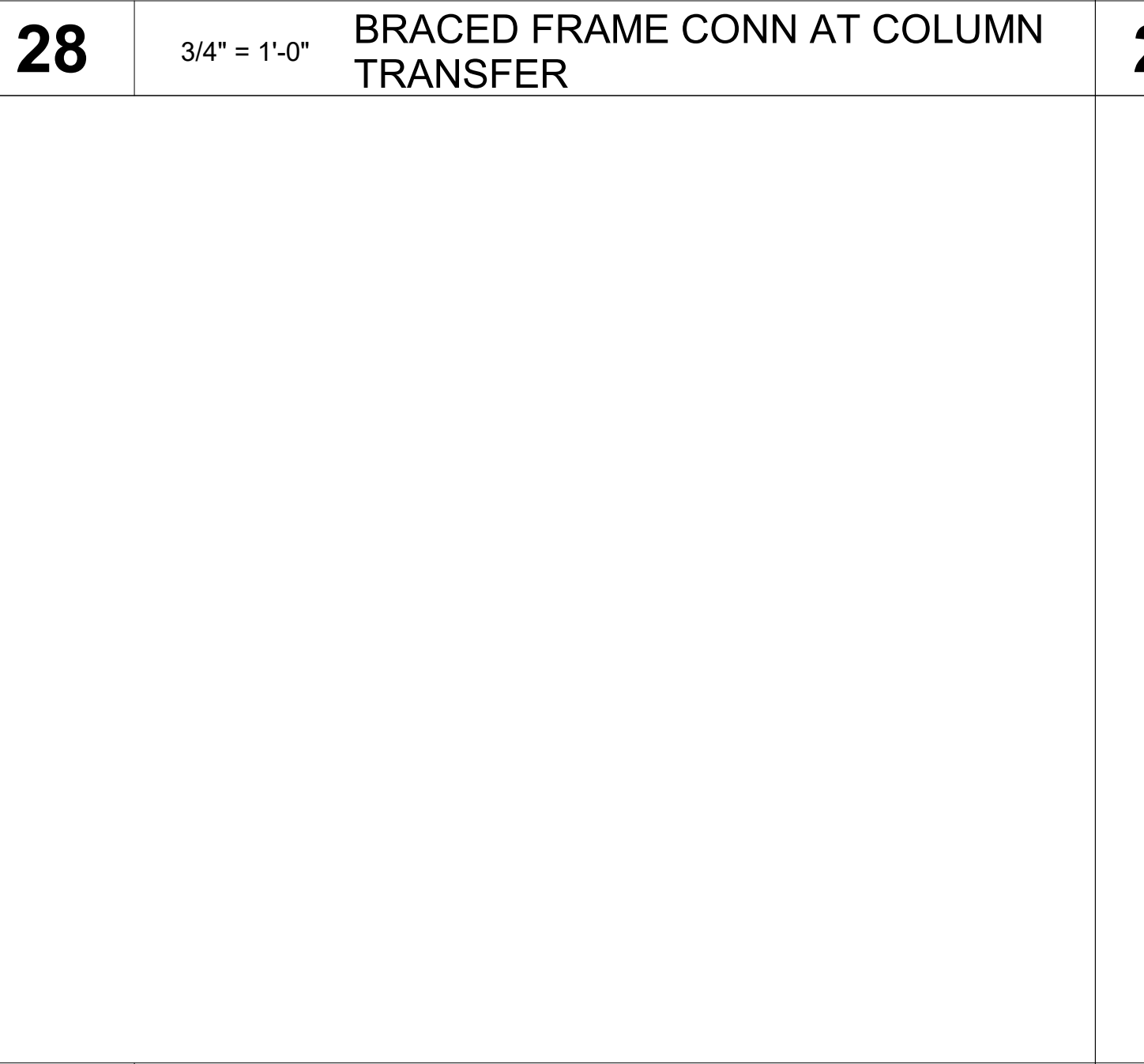
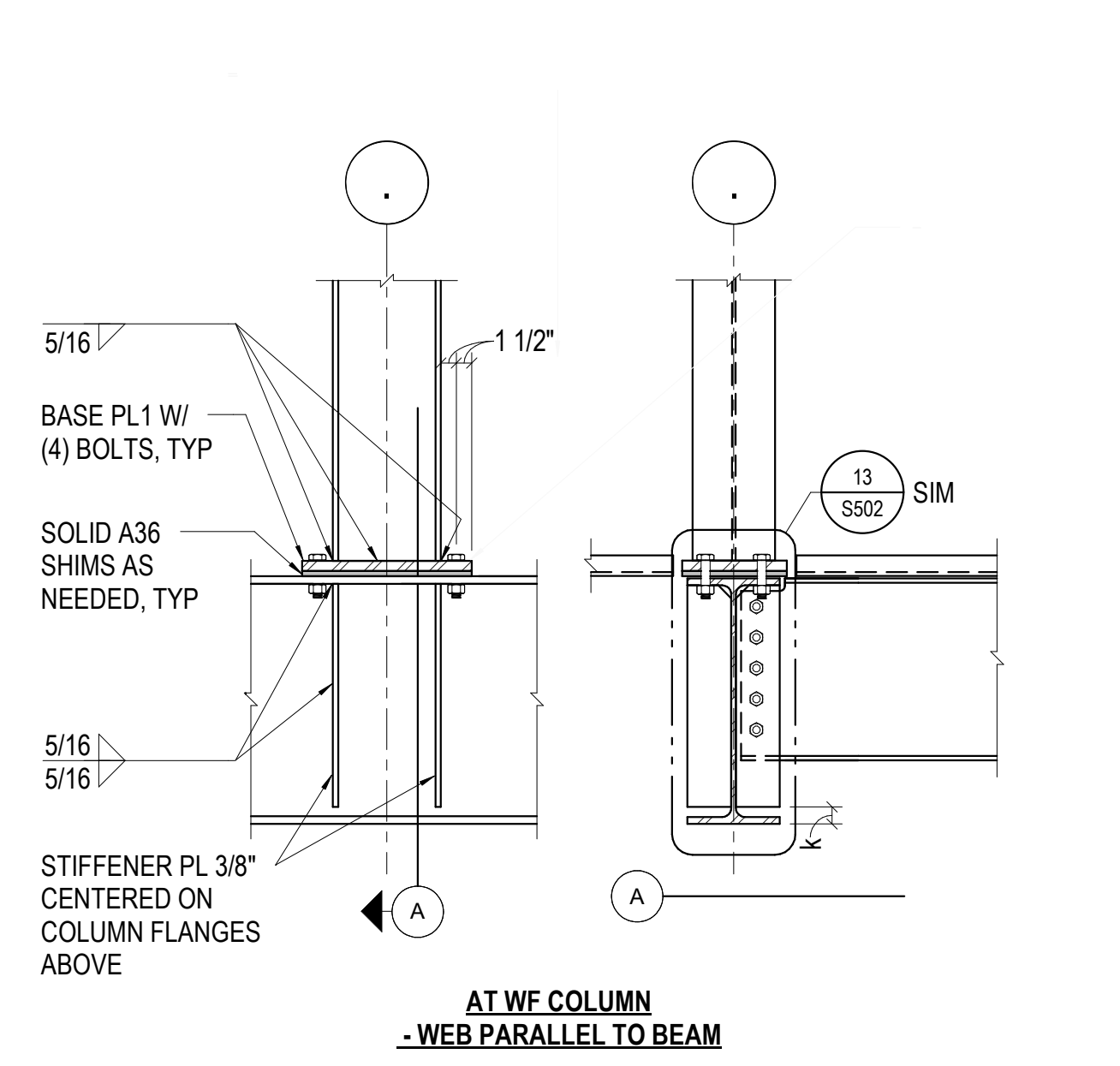
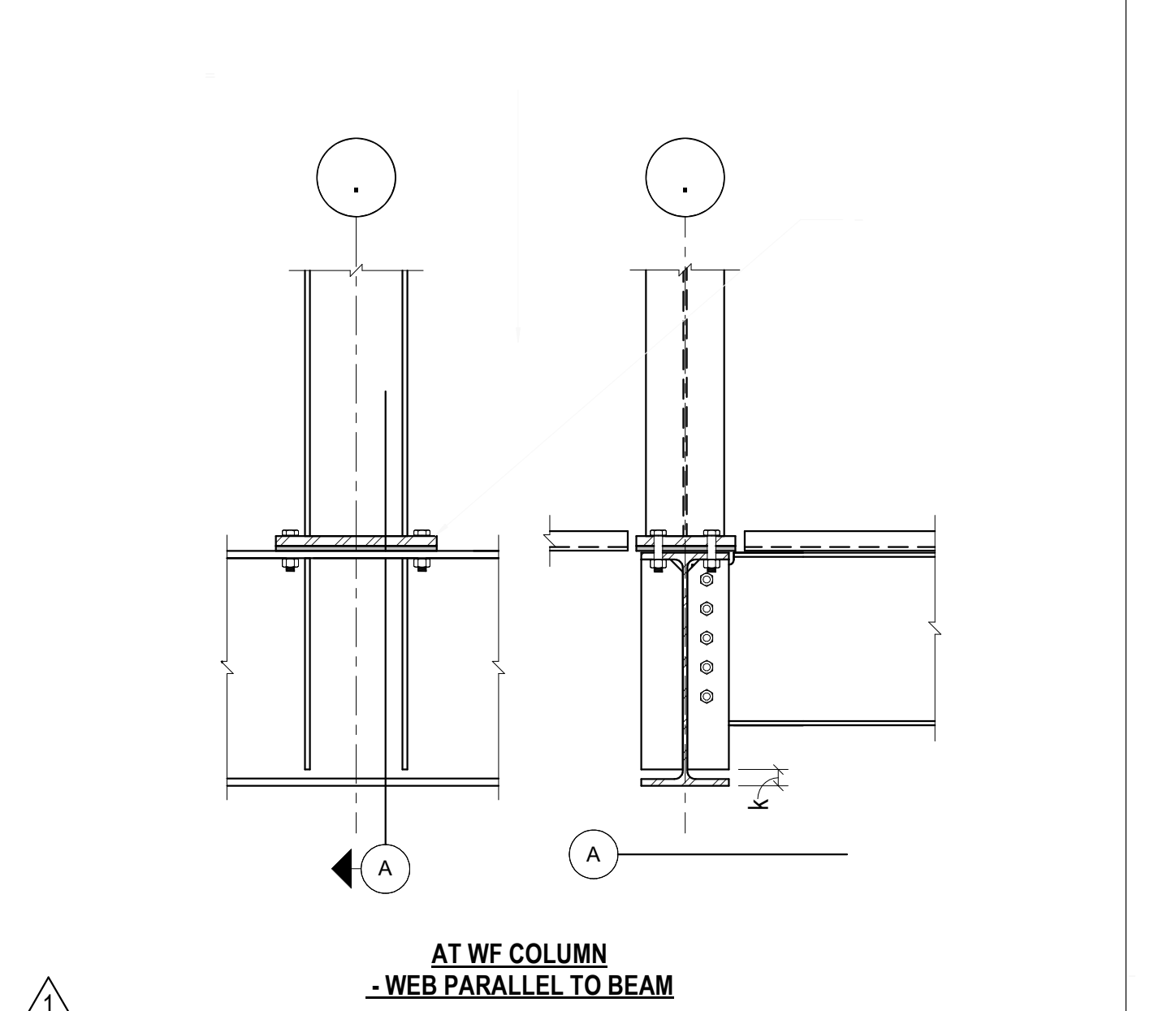
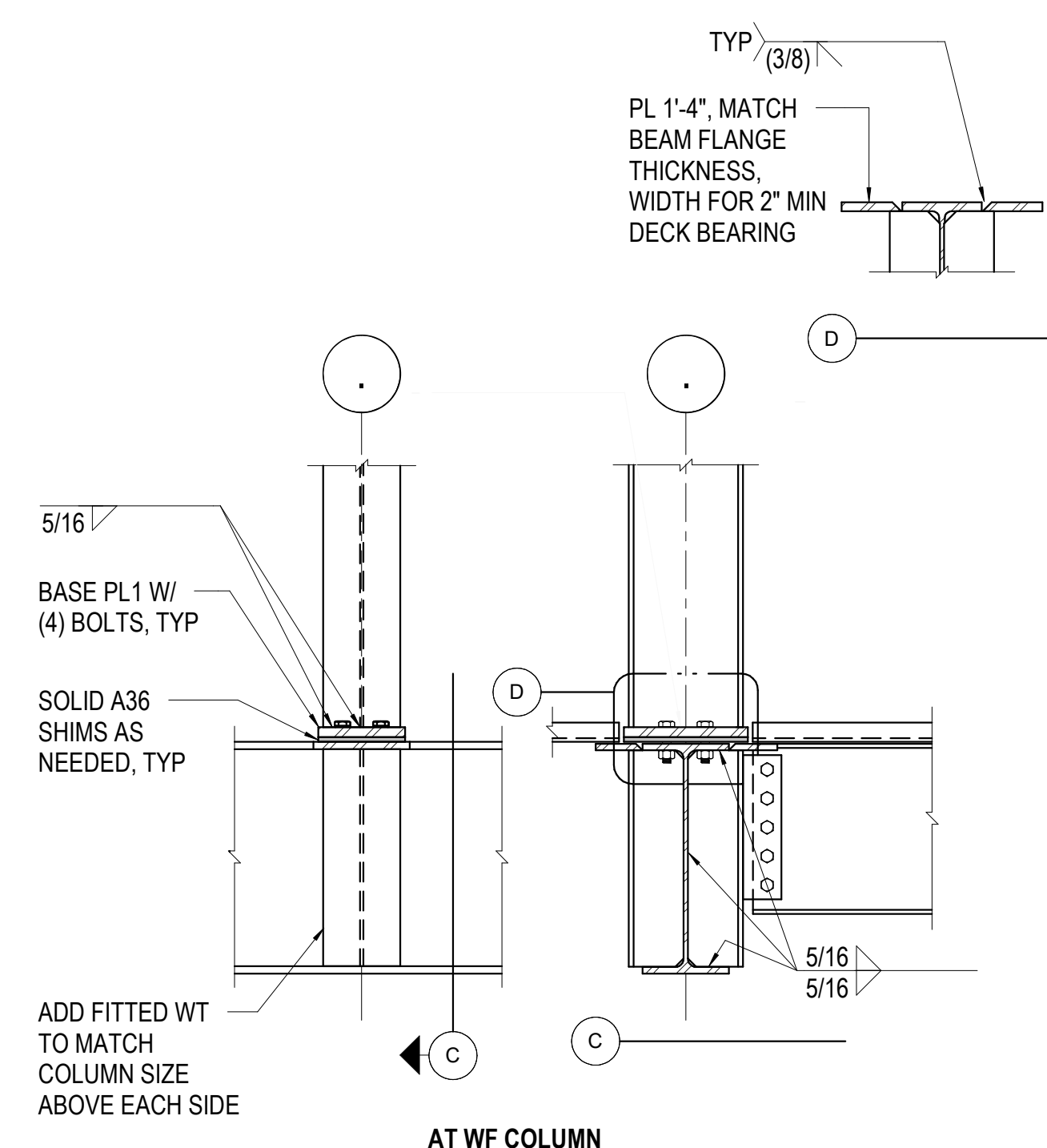
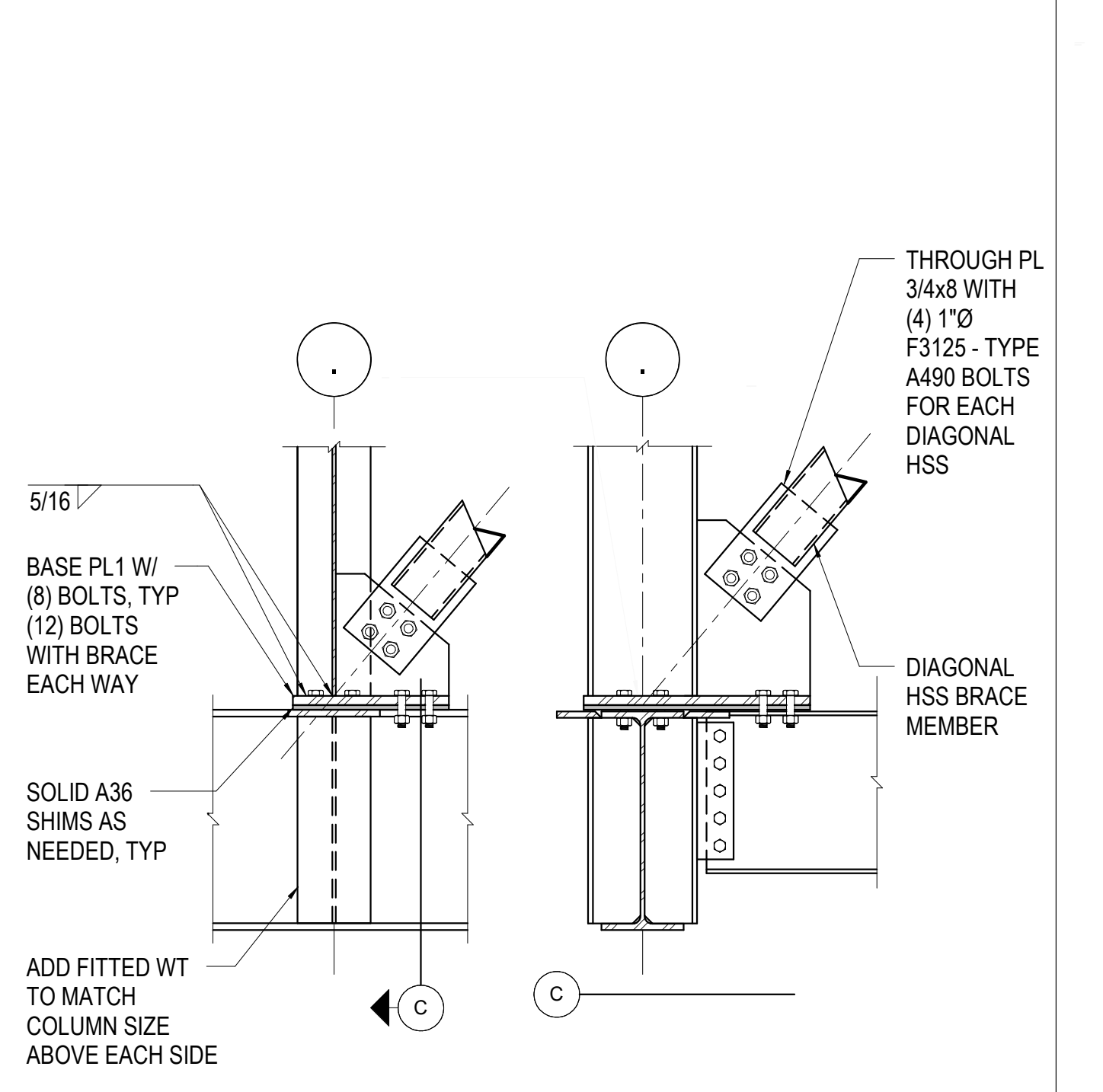
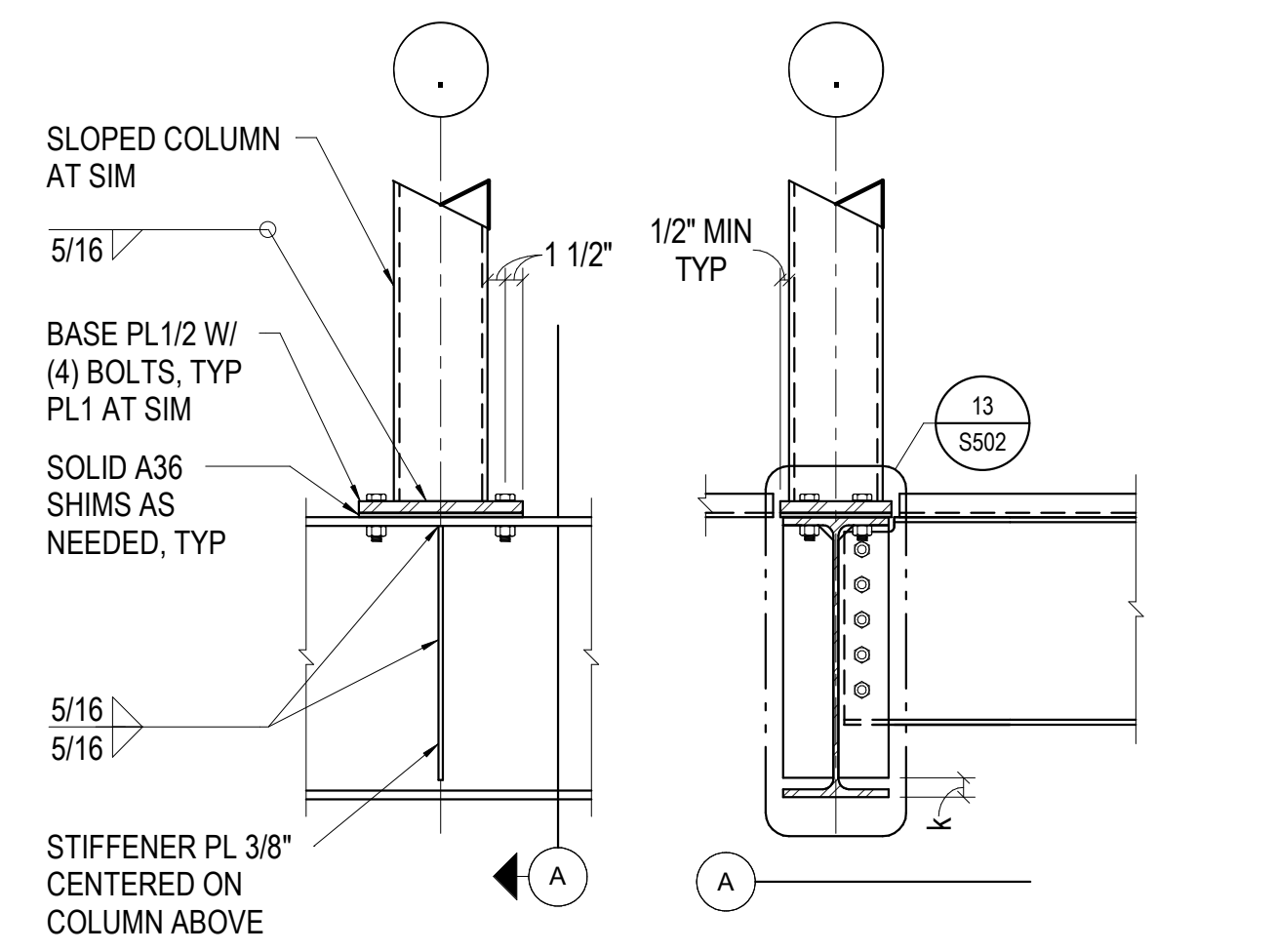
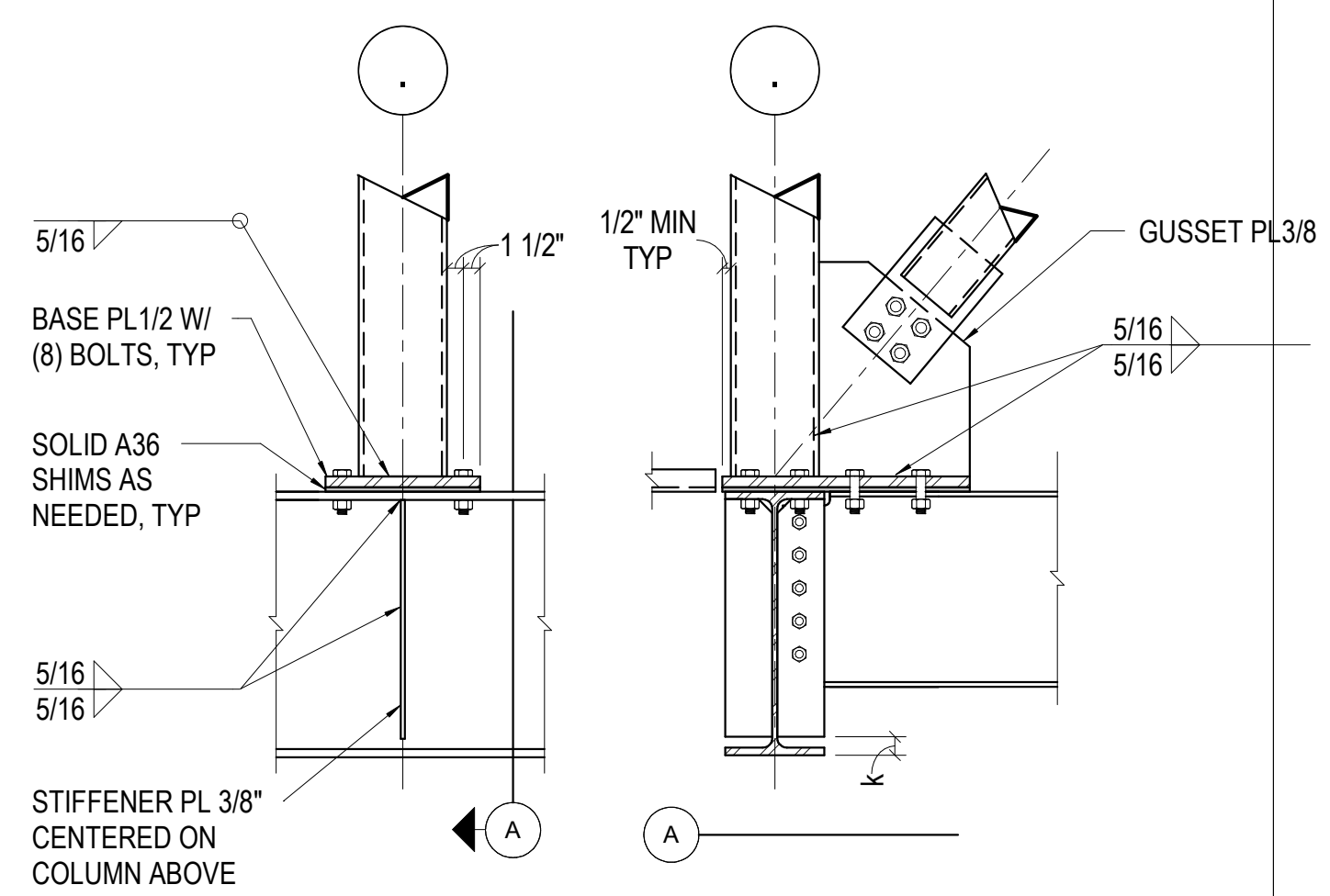
1 3/4" = 1'-0" TYP SLAB TO WEB CONNECTION

BEAM SIZE	NRB	MAX BEAM REACTION, Vu (KIPS)	NUMBER OF COLUMNS OF BOLTS, NCB				
			2	3	4	5	
W8x13 & UP	2	16	36	72	91	91	
W10x15 & UP	2	16	36	72	91	91	
W12x19 & UP	3	31	54	104	133	143	
W14	3	31	54	104	133	143	
W16	4	50	72	128	156	180	
W18	5	70	90	152	180	208	
W21 - W24	6	89	107	175	204	232	
W27	7	109	125	199	227	256	
W30	8	127	143	223	251	280	
W33	9	146	161	246	275	303	
W36 - W44	10	165	179	298	327	327	



5 3/4" = 1'-0" MONUMENTAL STAIR STRINGER

MJC:081; 19-01-2021
LEAD REVIEW: TEAH BRANNWELLS, TAM
DESIGNERS: ETIANNICKOLS, CHRISTINA CHILDRESS, CALEB CHENULT
PROJECT MANAGER: RALEIGH FISHER
AS21; 19-01-2021
LEAD REVIEW: TEAH BRANNWELLS, TAM
DESIGNERS: ETIANNICKOLS, CHRISTINA CHILDRESS, CALEB CHENULT
PROJECT MANAGER: RALEIGH FISHER
AS21; 19-01-2021
LEAD REVIEW: TEAH BRANNWELLS, TAM
DESIGNERS: ETIANNICKOLS, CHRISTINA CHILDRESS, CALEB CHENULT
PROJECT MANAGER: RALEIGH FISHER



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 Henderson Engineers
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 LENOXA, MS 39214
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 NEW YORK, NY 10001
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 JMC HOSPITALITY
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 THE WOODLANDS, TX 77380
 P: 684.41.2222

WATER FEATURES
 OTL
 2150 S. TOWNE CENTER, SUITE 100
 ANAHEIM, CA 92806
 P: 714.637.4747

IRRIGATION
 WC3 DESIGN
 11A ROBINSON MANOR BLVD.
 ROCKERS ROCK, PA 16156
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PSW Job Number:
 993A

Issue Date:
 02.24.2023

REVISIONS
 NUMBER DATE DESCRIPTION
 1 03.13.2023 ADDENDUM 1
 2 06.09.2023 ADDENDUM 2

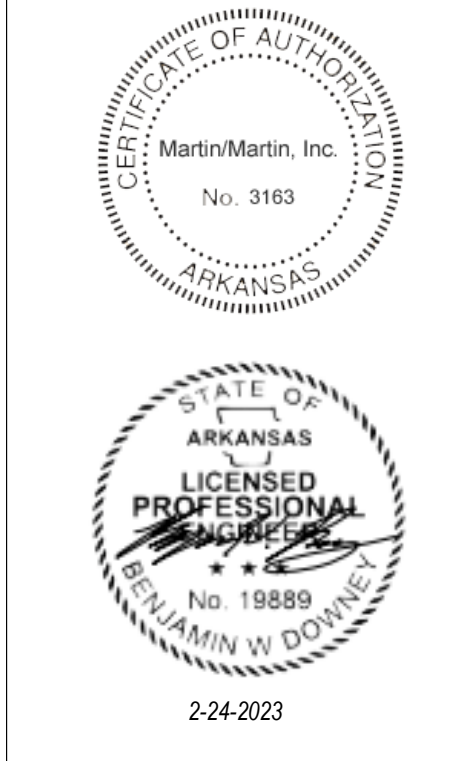
Consents:
WF BEAM STEP DETAILS

THIS PAGE IS BEST VIEWED IN COLOR

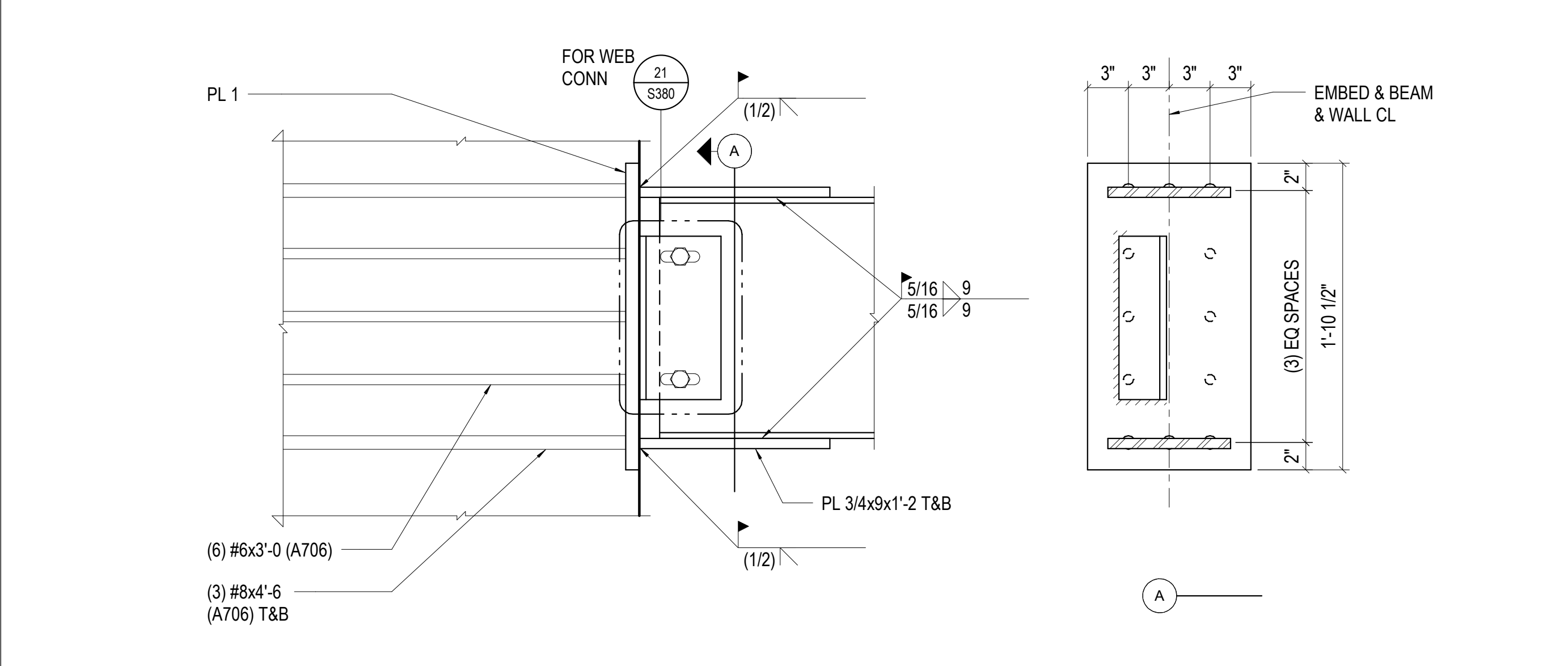
S507

DESIGNERS: ETHAN WICKS, CHRISTINA CHILDS, CALEB CHESNUT
 LEAD REVIT TECH: BRAD WELLS, JAM
 ARCHITECT: POLK STANLEY WILCOX
 ADDRESS: 801 SOUTH SPRING STREET, LITTLE ROCK, AR 72201
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 PROJECT MANAGER: BAILEIGH FISHER

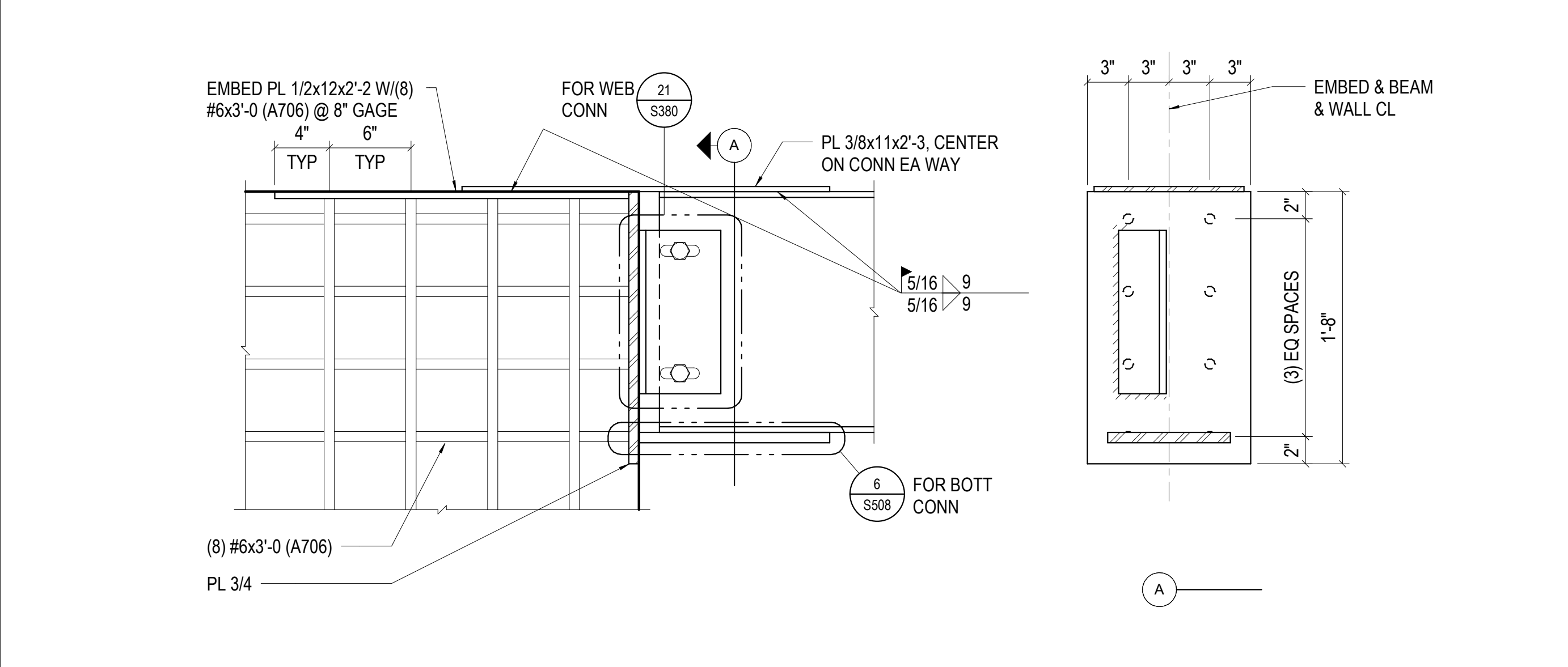
DATE: 02.24.2023
 DRAWING NO: S507



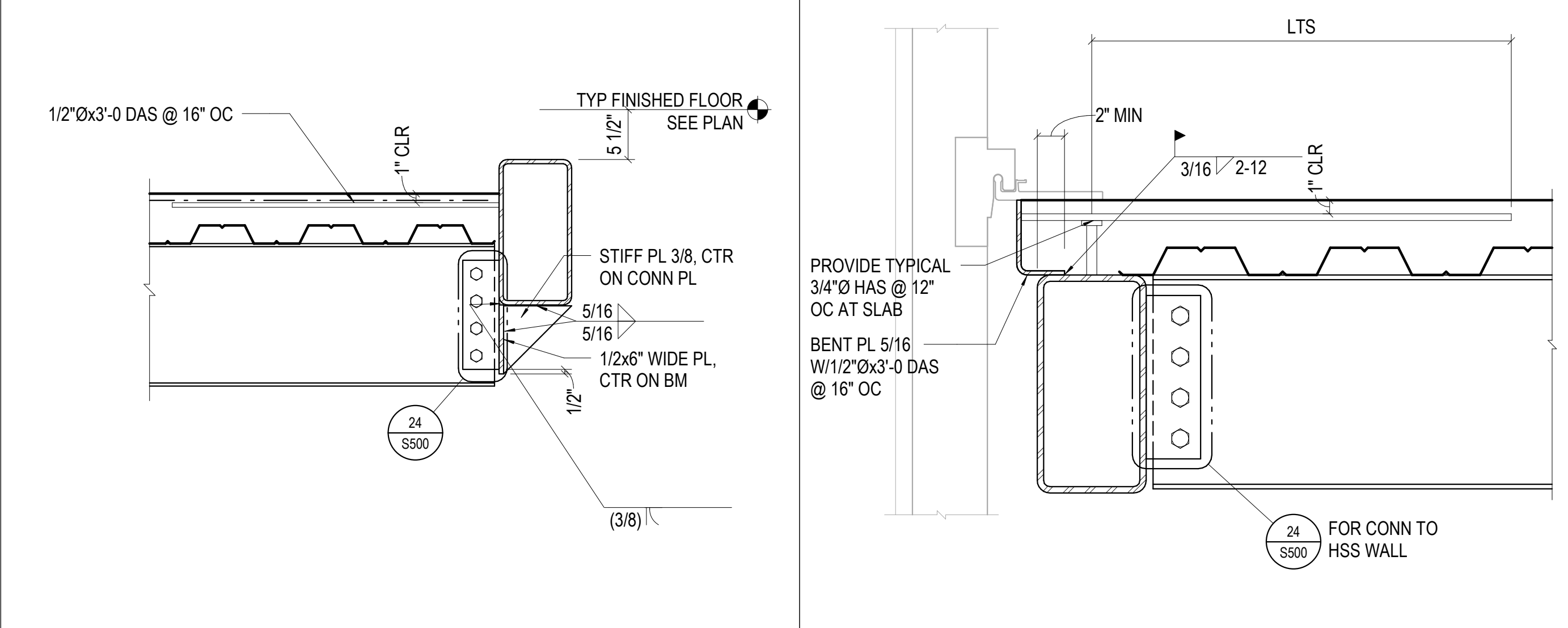
NUMBER	DATE	DESCRIPTION
1	03.13.2023	ADDENDUM 1
2	06.29.2023	ADDENDUM 2



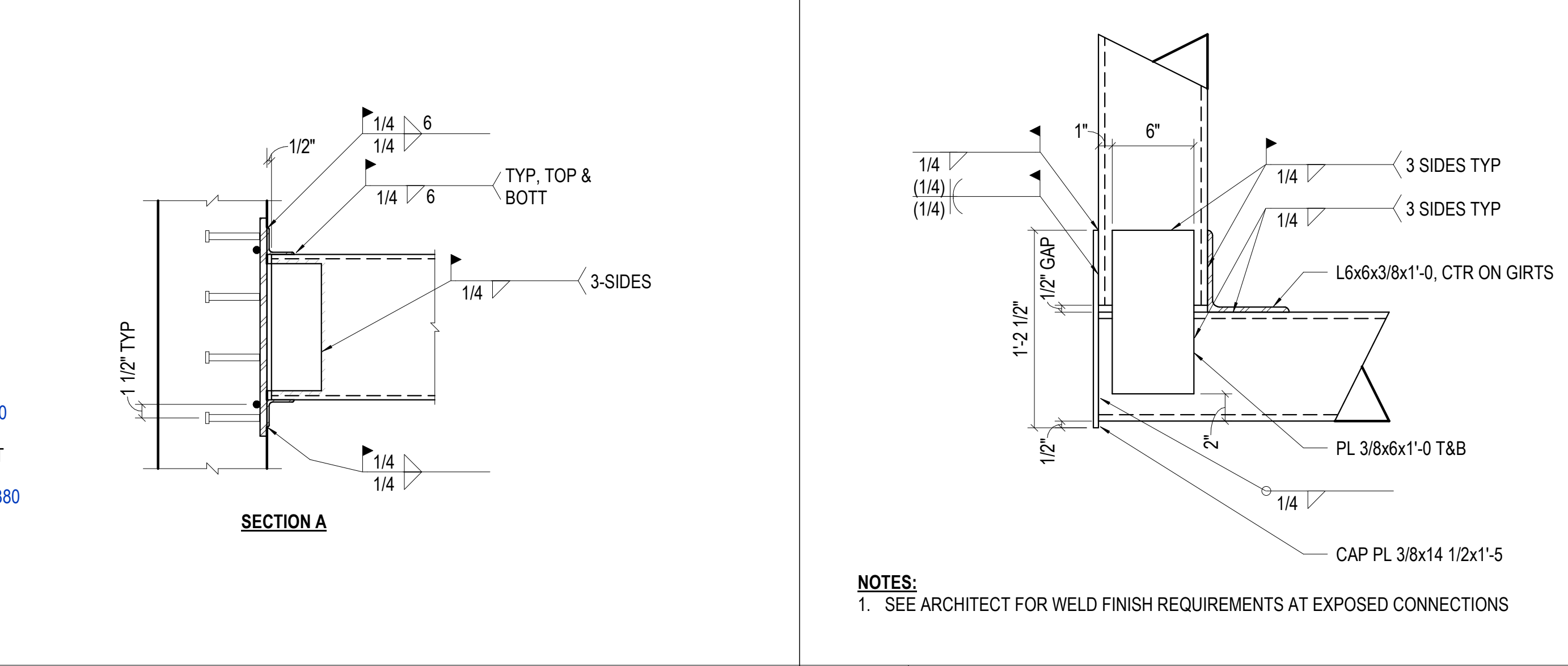
6 1 1/2" = 1'-0" BEAM TO END OF CONC WALL MOMENT CONN



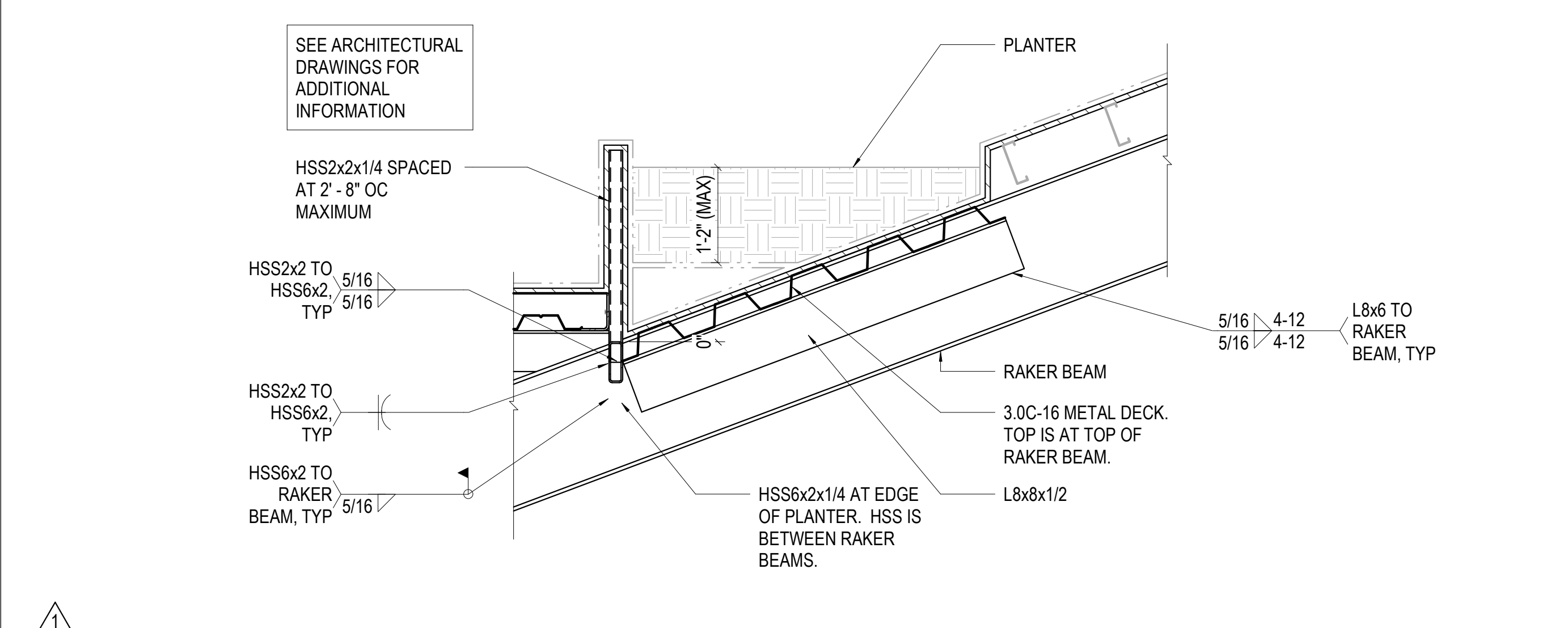
7 1 1/2" = 1'-0" BEAM TO END OF CONC WALL MOMENT CONN - TOP OF WALL



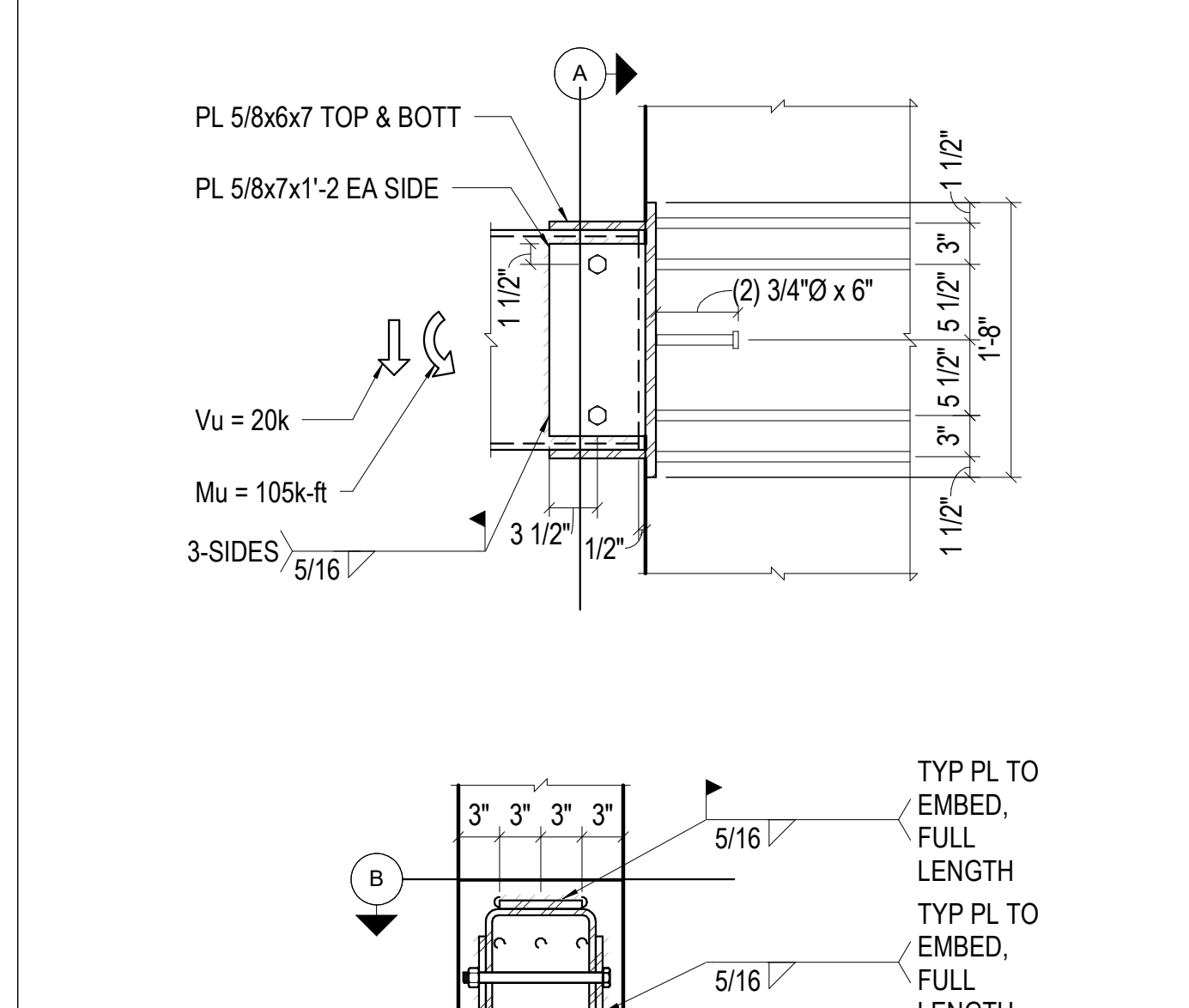
8 1" = 1'-0" SW STAIR BEAM TO GIRT



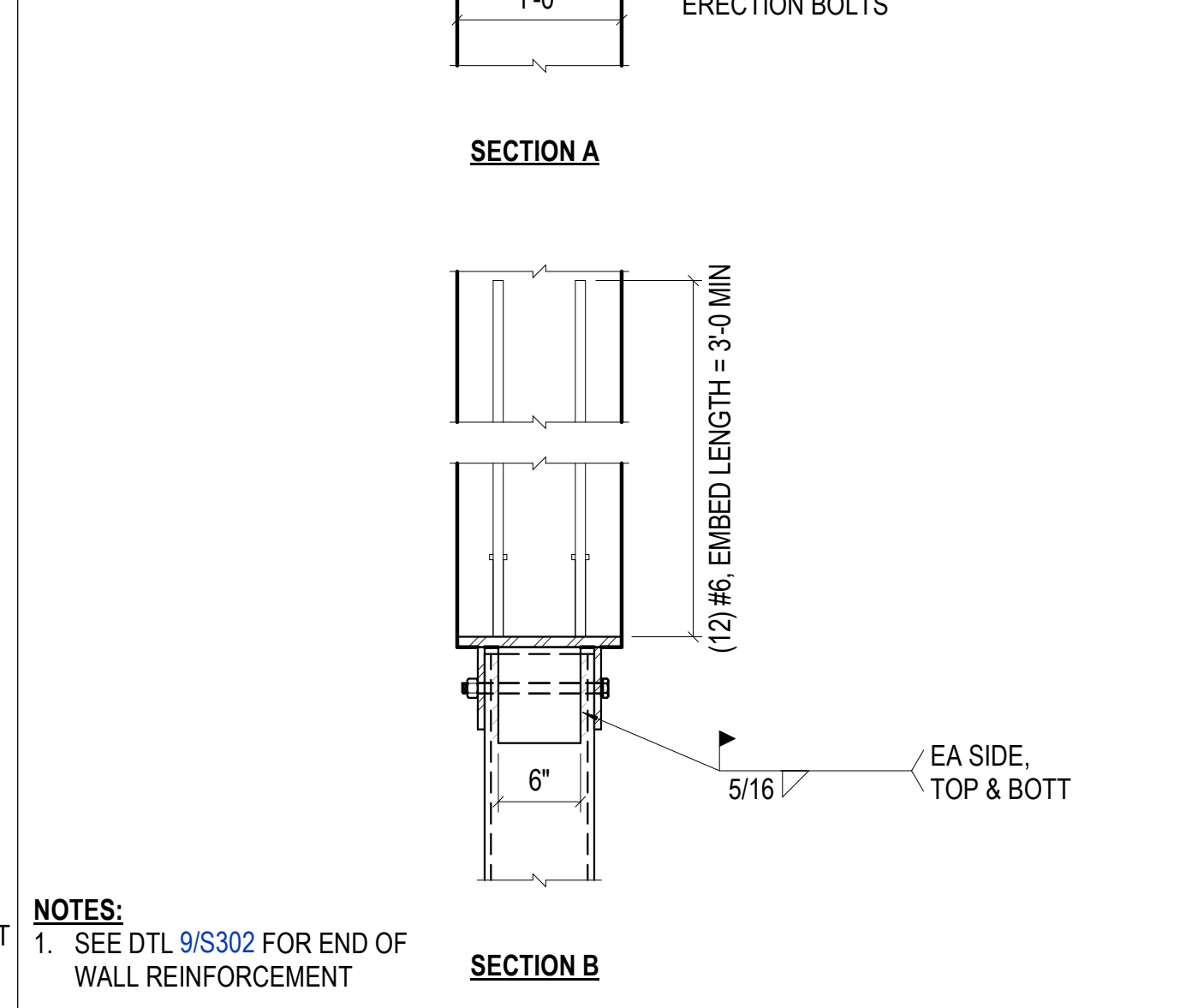
3 1 1/2" = 1'-0" STAIR SUPPORT TO HSS EDGE BM



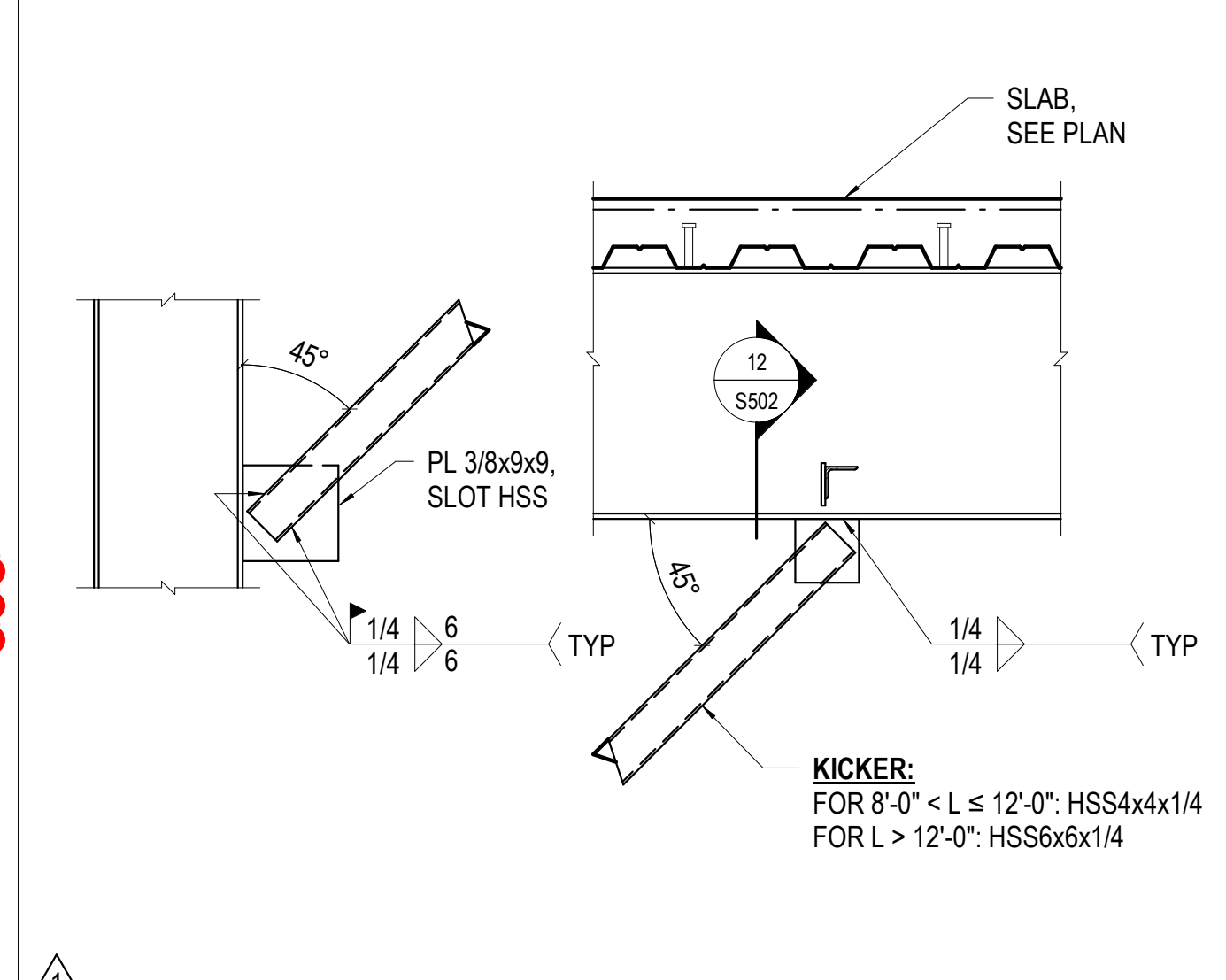
4 1 1/2" = 1'-0" HSS GIRT CORNER CONN AT STAIR



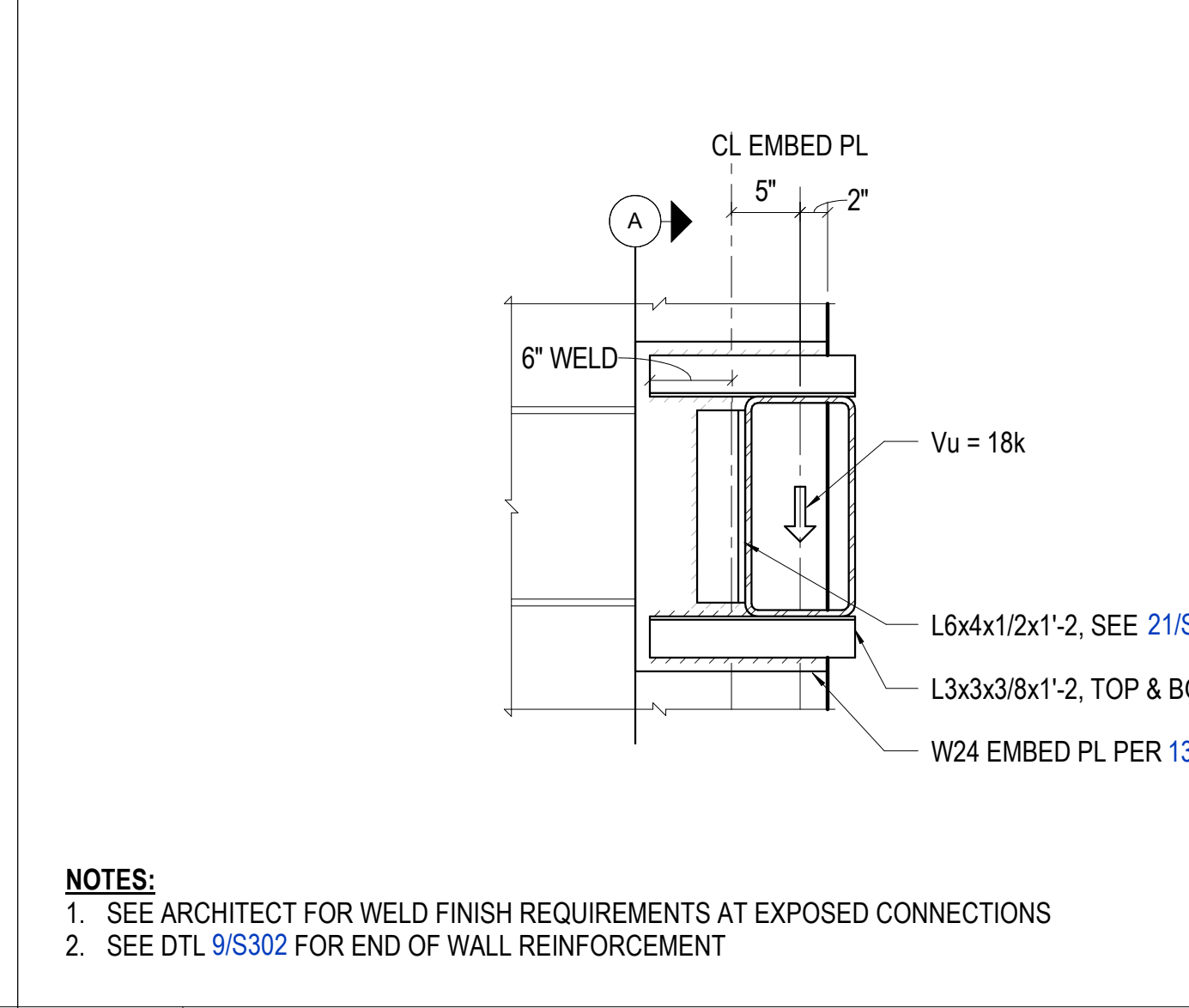
12 1" = 1'-0" GIRT TO END OF CONC WALL MOMENT CONN



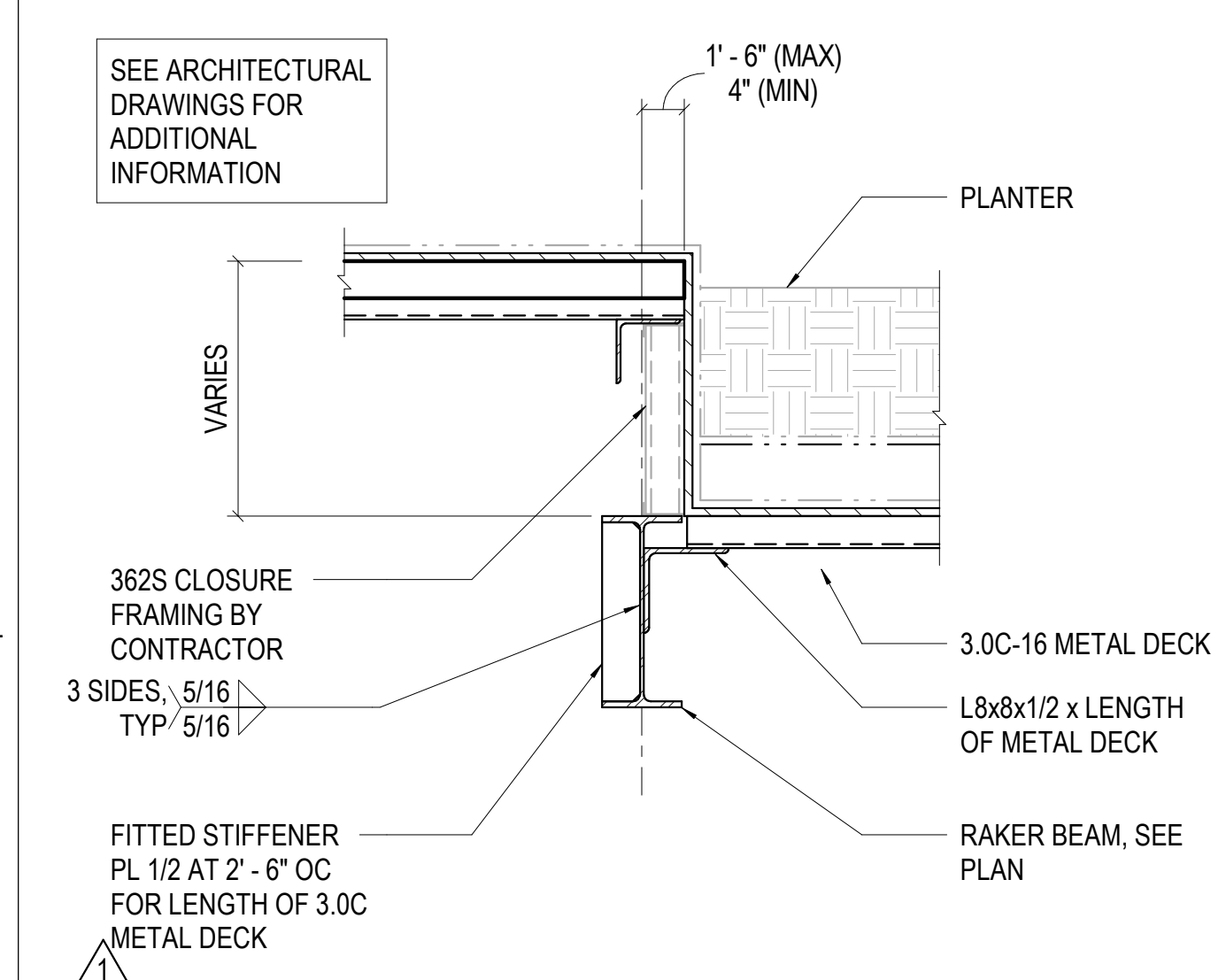
13 3/4" = 1'-0" TYP HSS KICKER



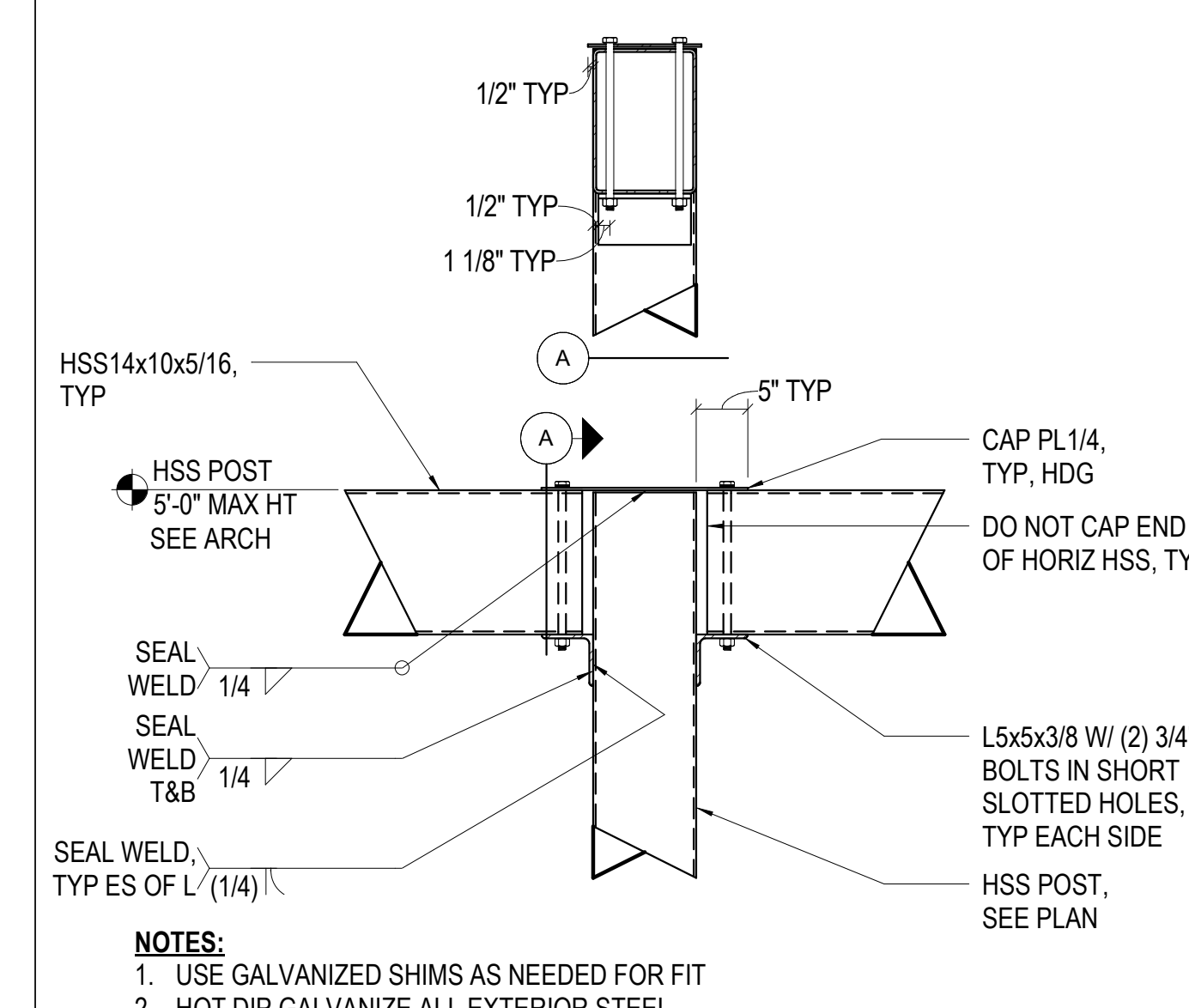
14 1" = 1'-0" GIRT TO CONC WALL CONN



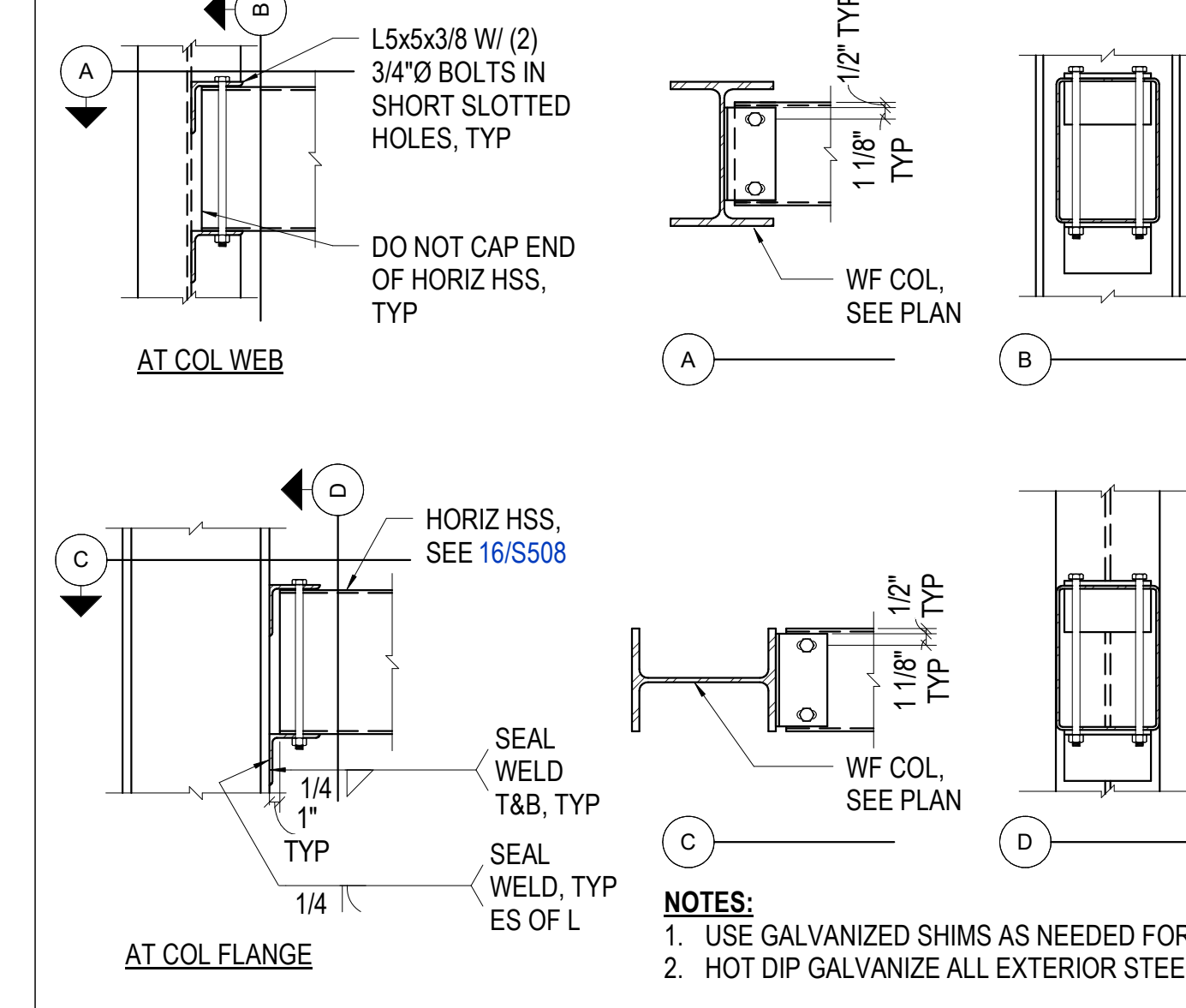
15 3/4" = 1'-0" TRANSITION FROM SEATING SURFACE TO PLANTER FRAMING



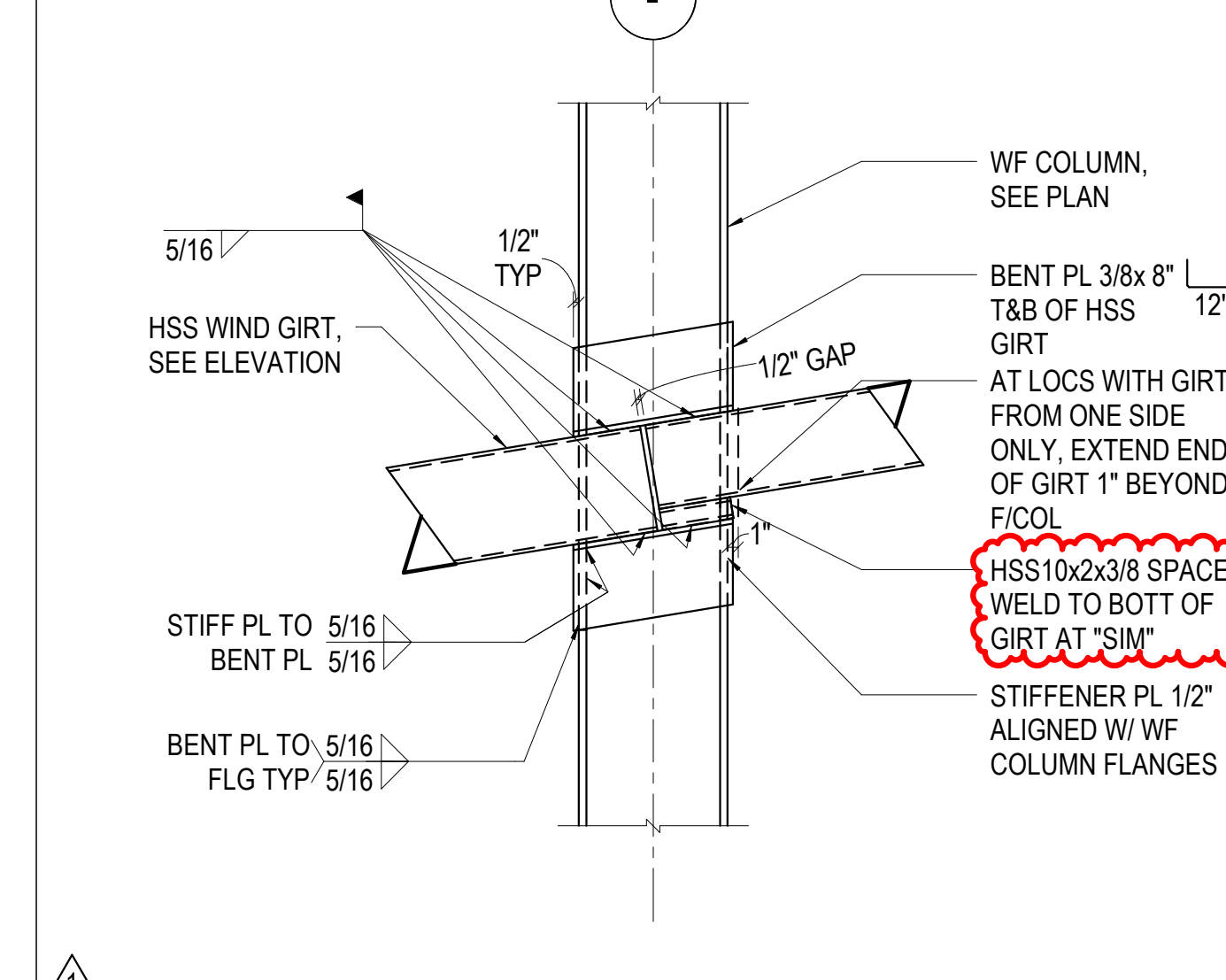
10 3/4" = 1'-0" TYPICAL GALLERY SEATING PLANTER FRAMING



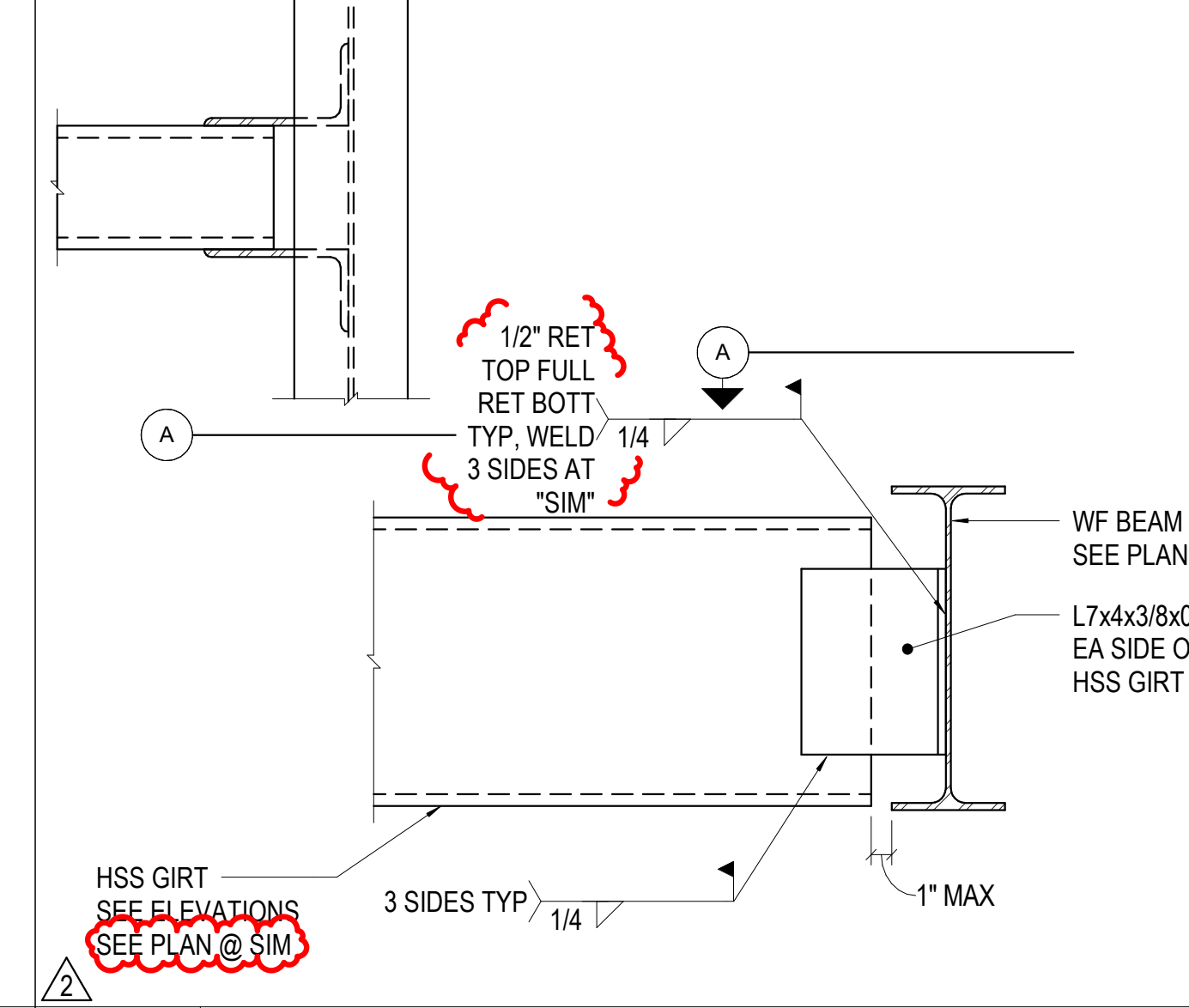
16 3/4" = 1'-0" SCREENWALL HORIZONTAL HSS BROKEN AT VERTICAL HSS



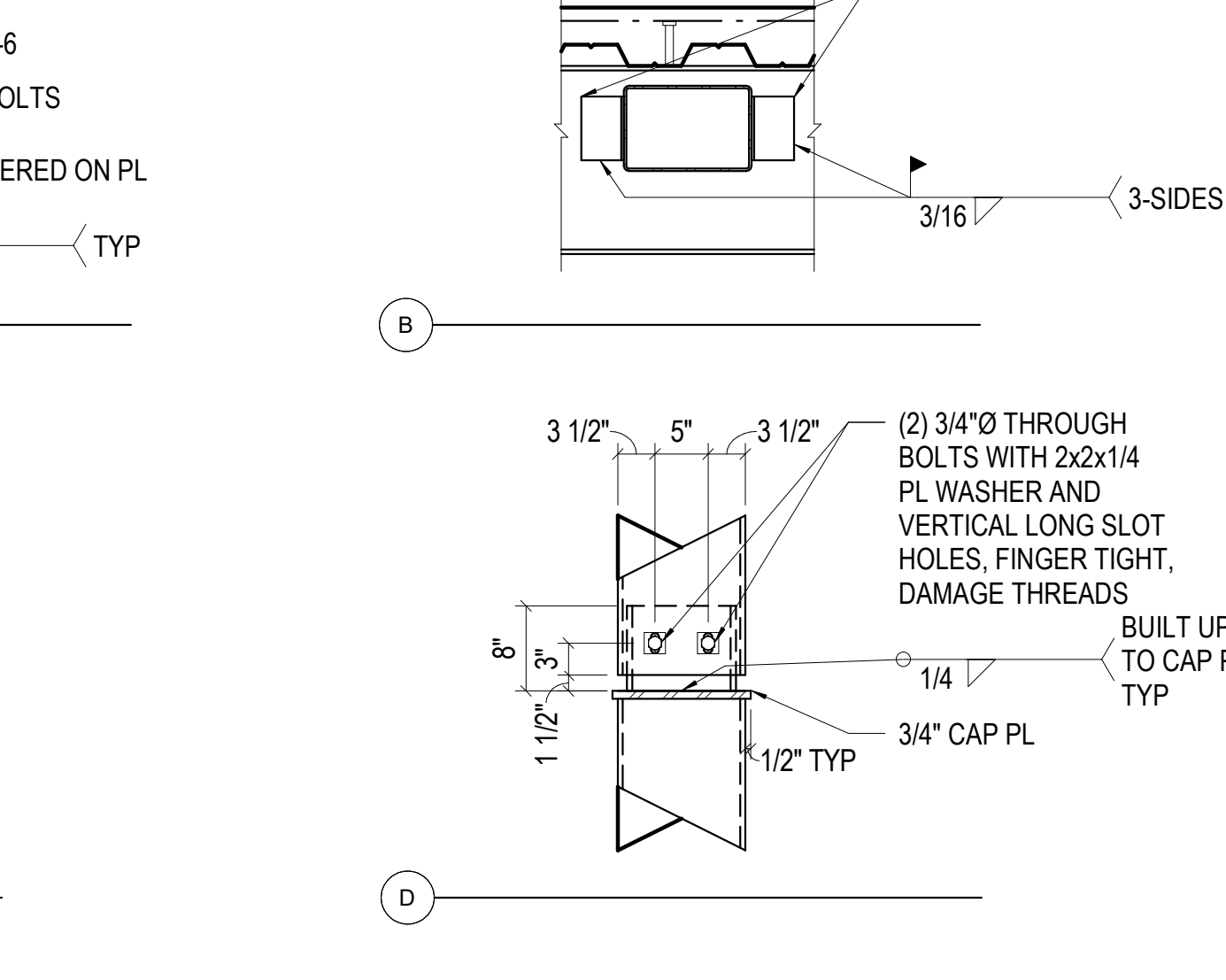
17 3/4" = 1'-0" HORIZONTAL HSS TO COL CONN



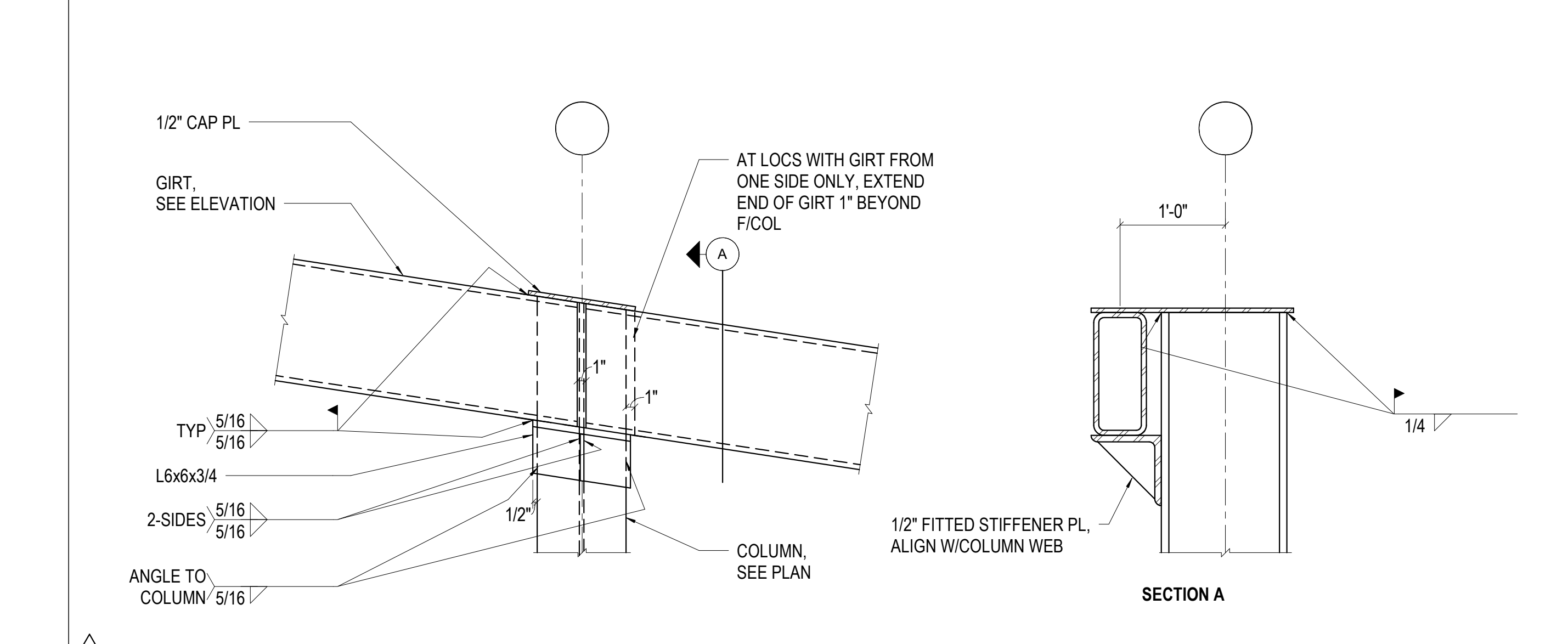
18 3/4" = 1'-0" SLANTED GIRT COL CONN AT MONUMENTAL STAIR



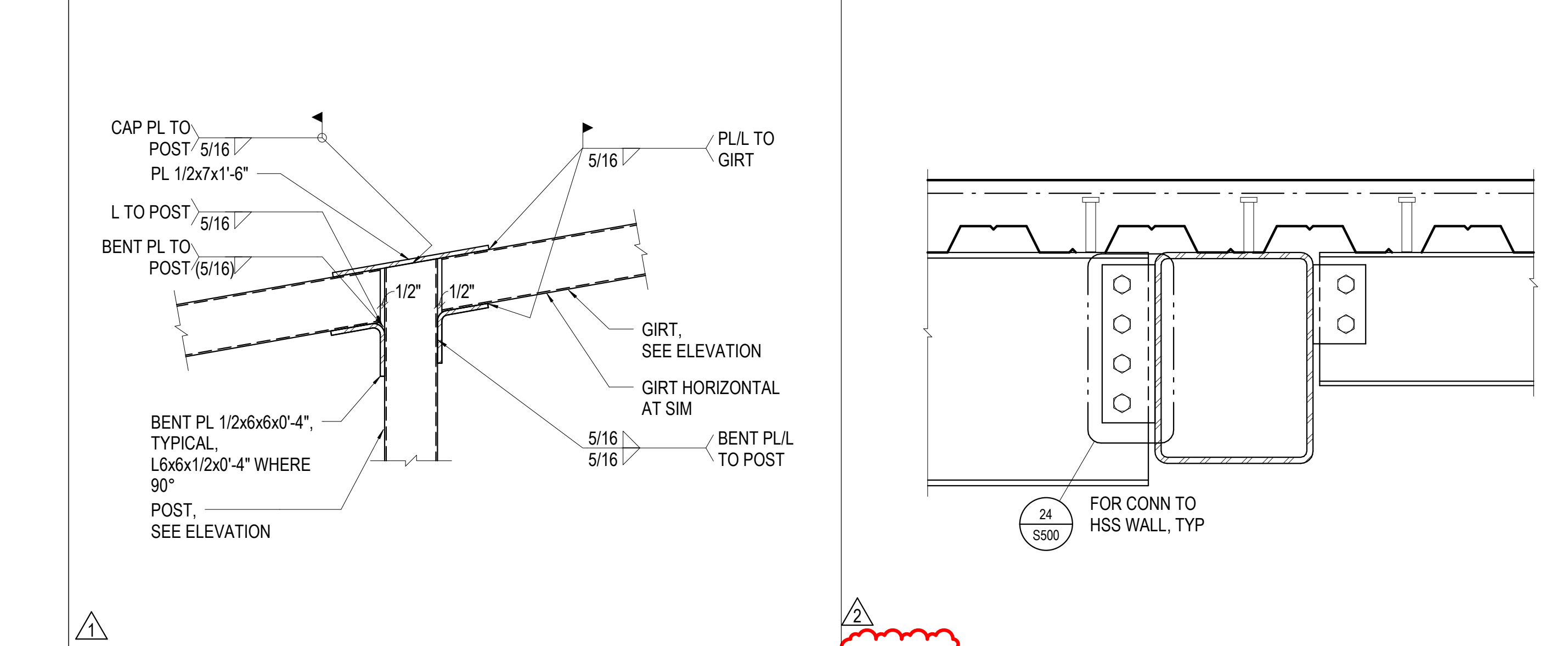
19 1 1/2" = 1'-0" HSS GIRT TO WF BEAM CONNECTION



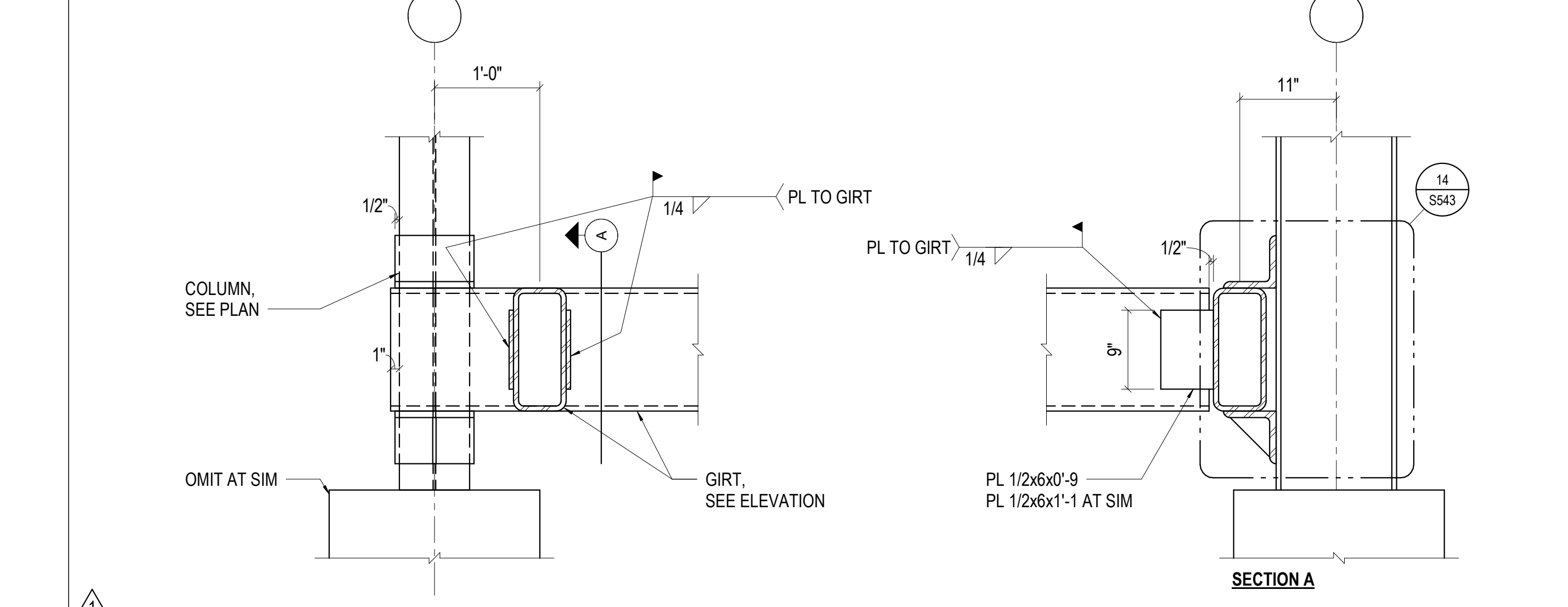
19 1 1/2" = 1'-0" HSS GIRT TO WF BEAM CONNECTION



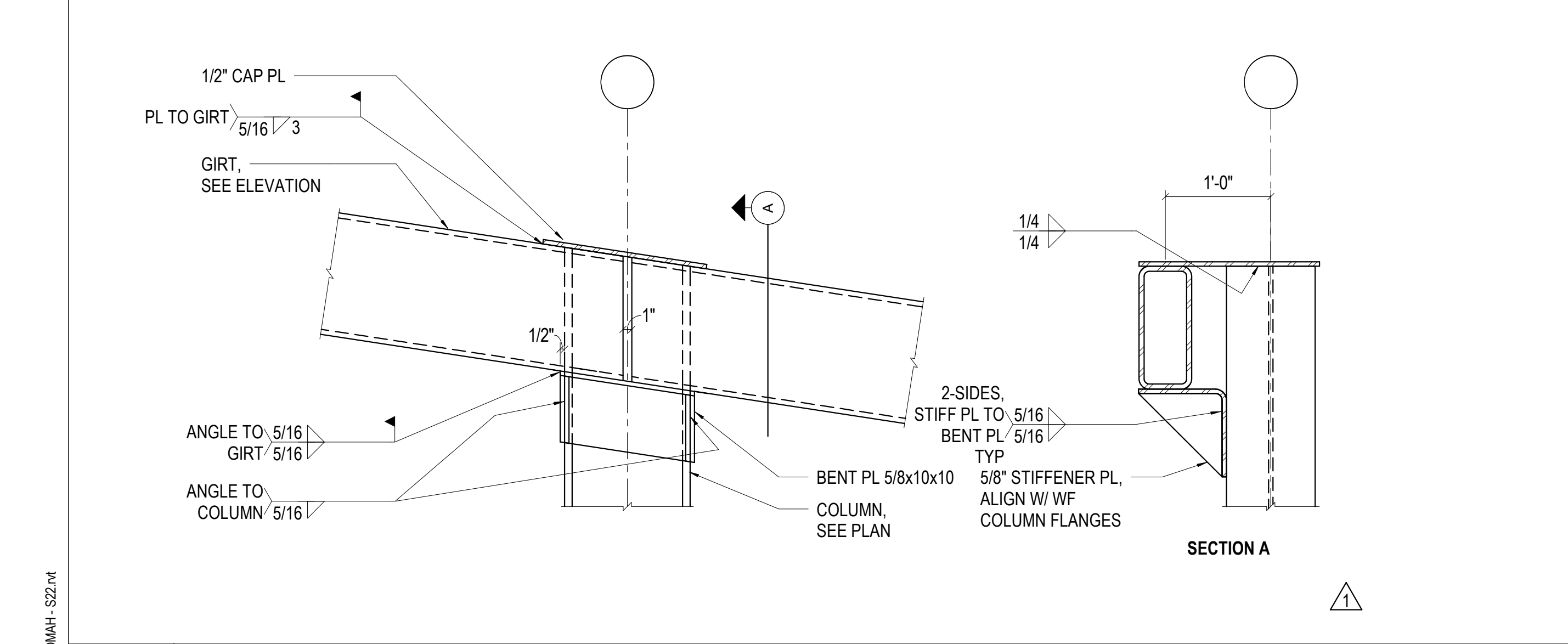
26 1" = 1'-0" GIRT TOP BYPASS CONNECTION



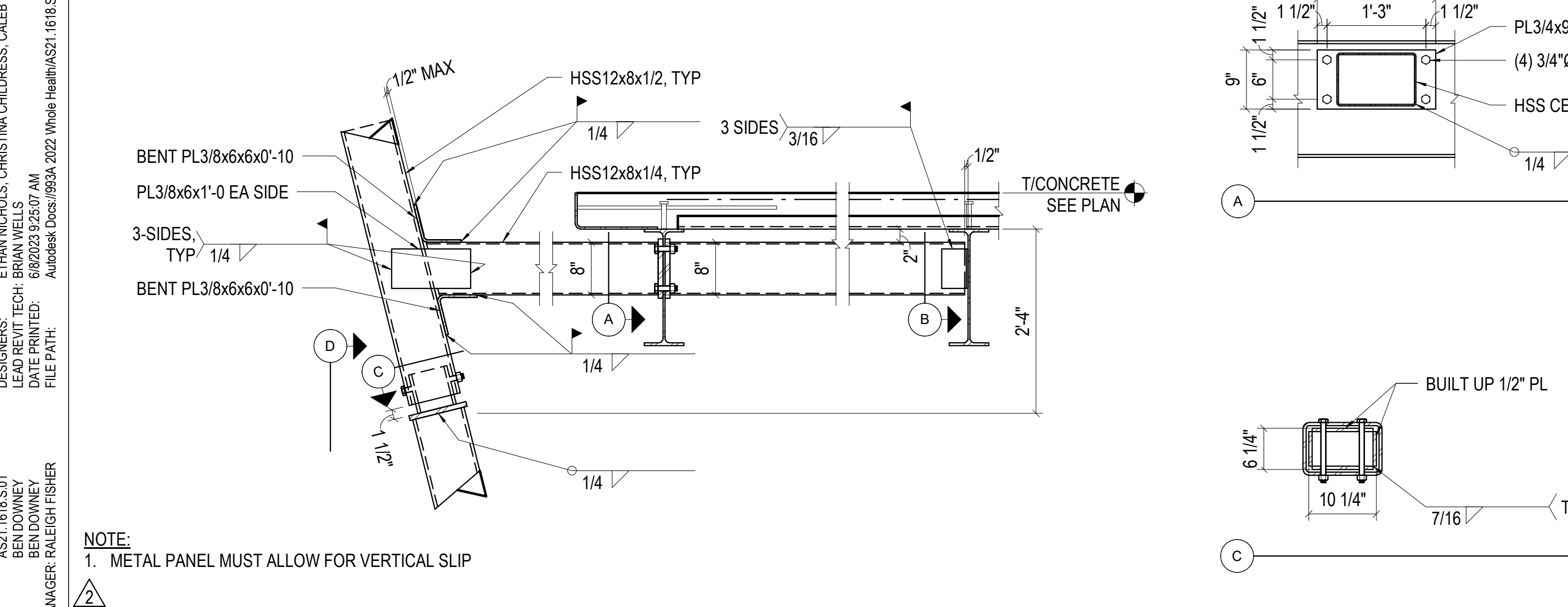
27 1" = 1'-0" GIRT CONNECTION DETAIL



28 1" = 1'-0" GIRT CONNECTION DETAIL

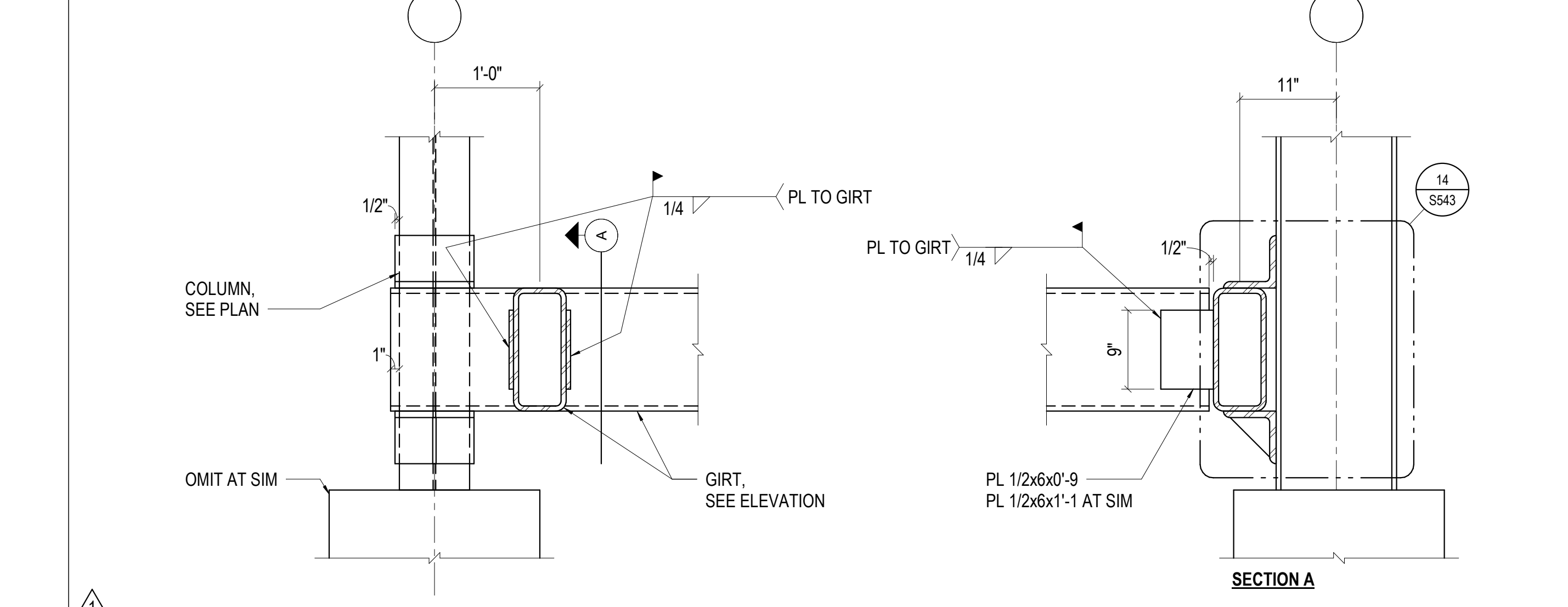


29 1" = 1'-0" GIRT TOP CONNECTION



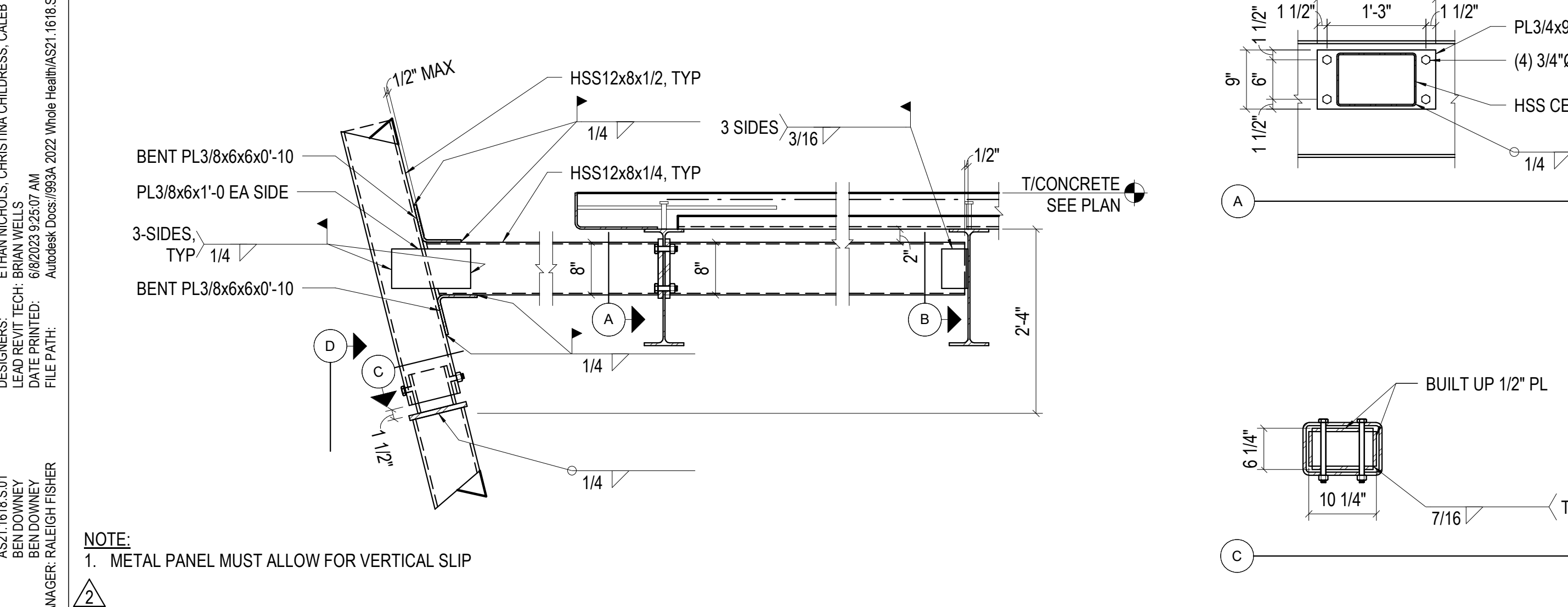
30 3/4" = 1'-0" FIN CONNECTION

22 1 1/2" = 1'-0" WF TO HSS BEAM CONNECTION



22 1 1/2" = 1'-0" WF TO HSS BEAM CONNECTION

22 1 1/2" = 1'-0" WF TO HSS BEAM CONNECTION



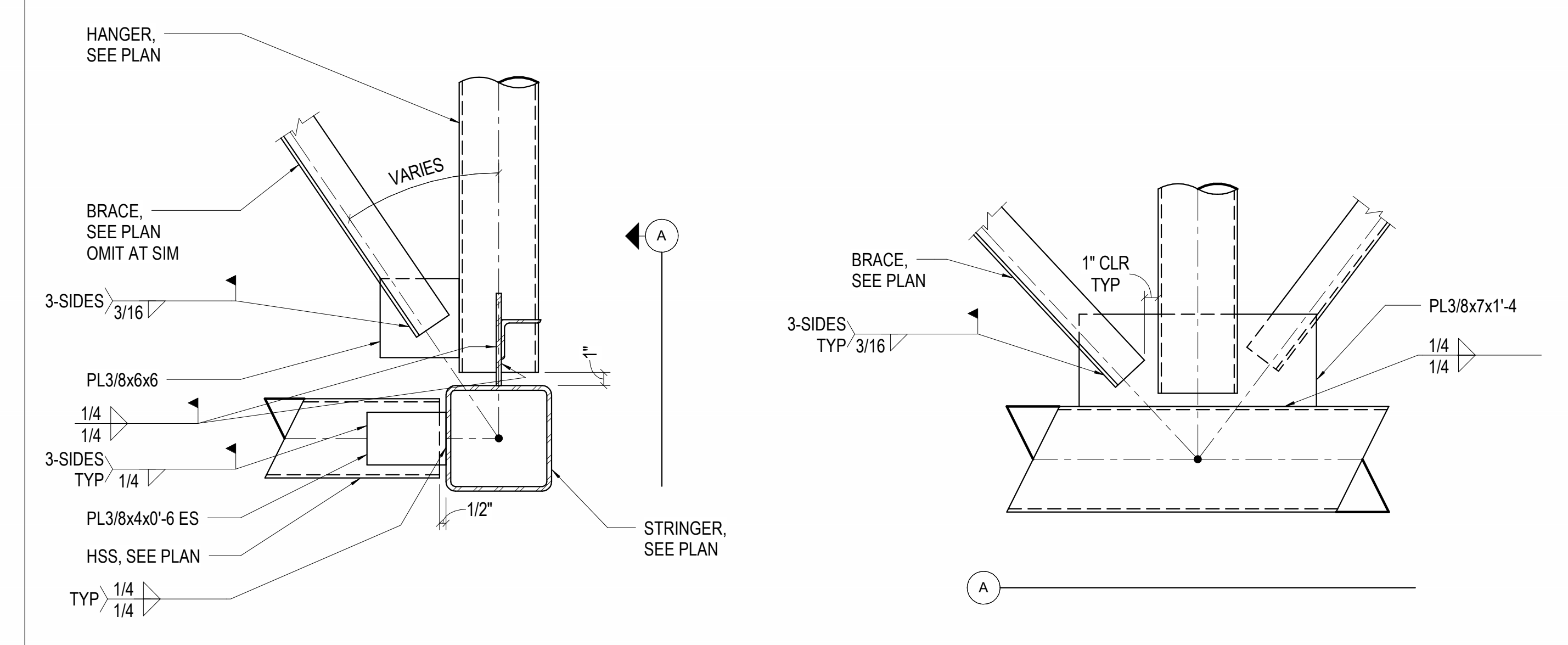
22 1 1/2" = 1'-0" WF TO HSS BEAM CONNECTION

DESIGNERS: ETHAN WICKS, CHRISTINA CHURCH, CALEB CHESNUT
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 FAX: 501.378.0879
 PROJECT MANAGER: BAILEIGH FISHER
 DATE: 02.24.2023
 DRAWING NO: S508-01

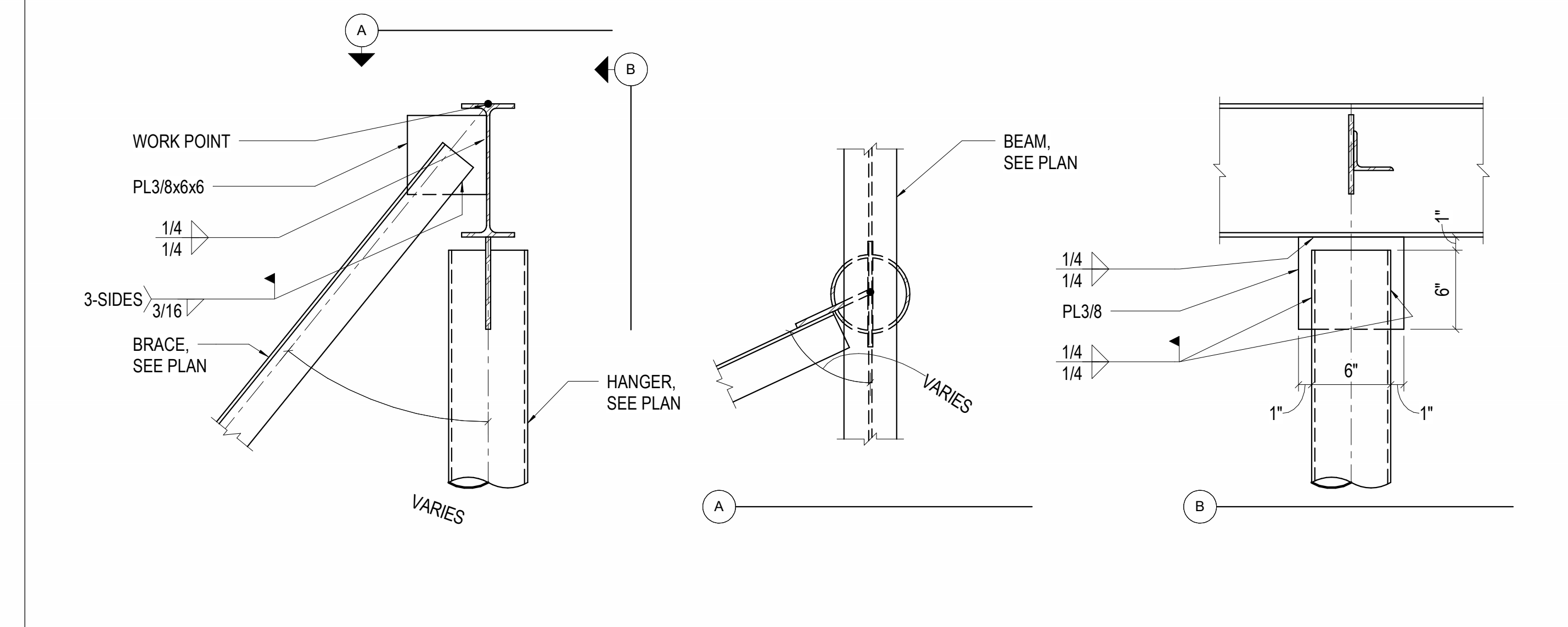
NOTE:
1. METAL PANEL MUST ALLOW FOR VERTICAL SLIP

PSW Job Number:
983A

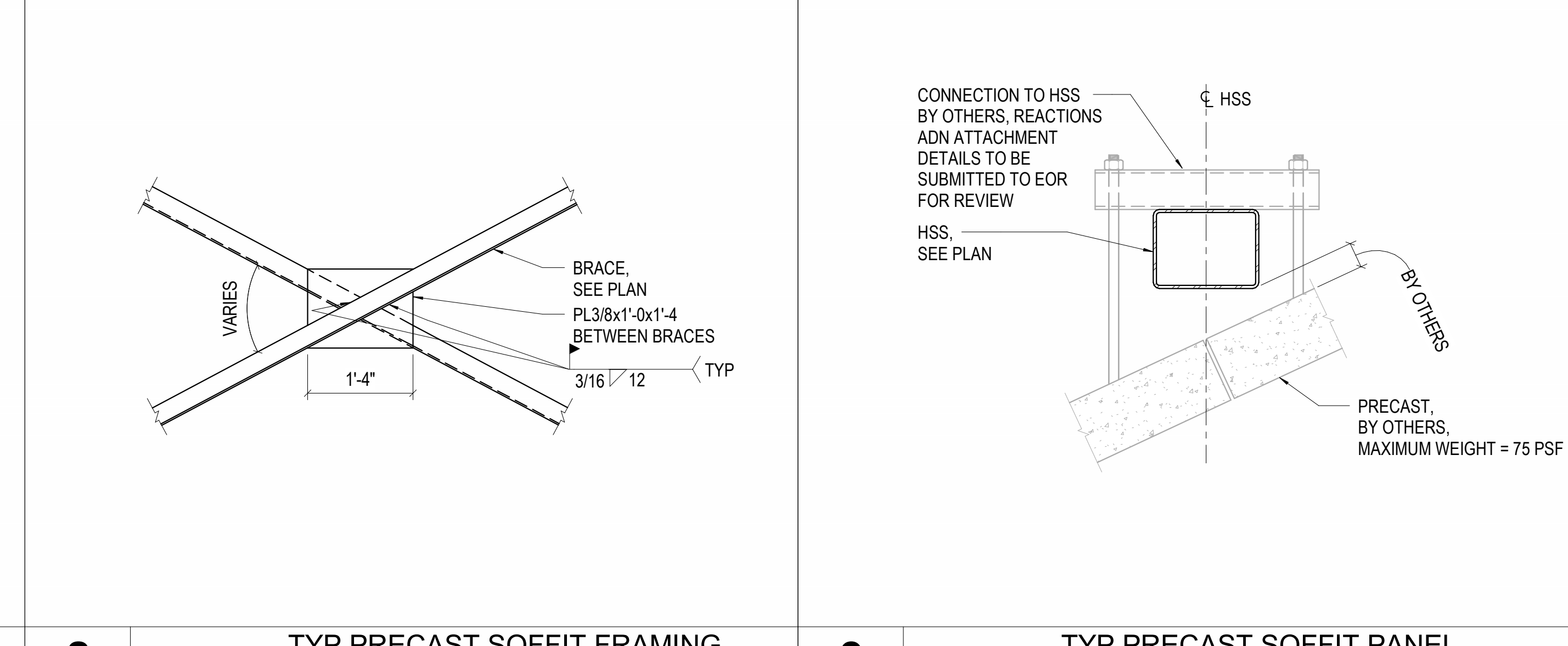
2-24-2023



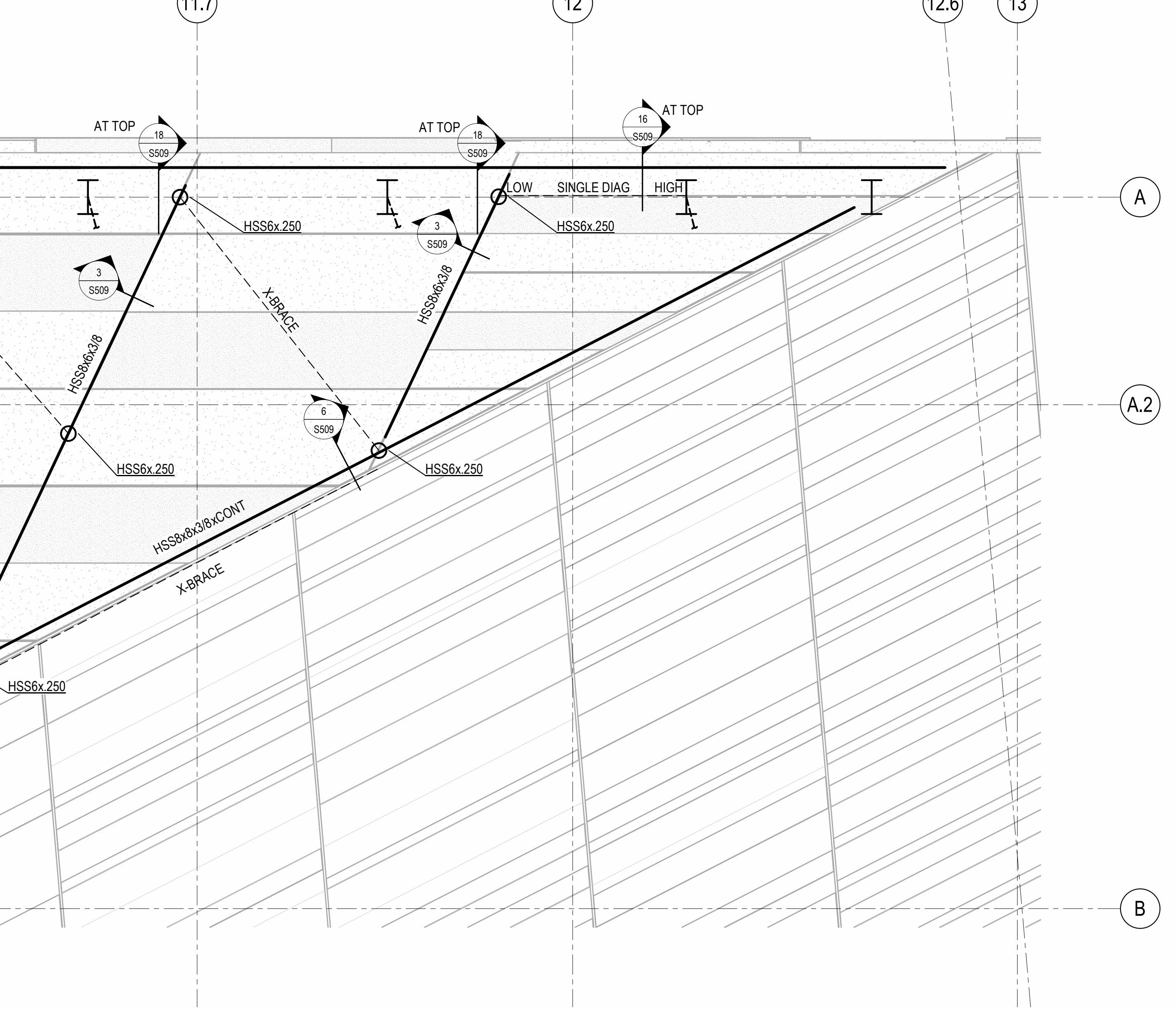
6 1 1/2" = 1'-0" TYP PRECAST SOFFIT BOTTOM CONNECTION AT STRINGER



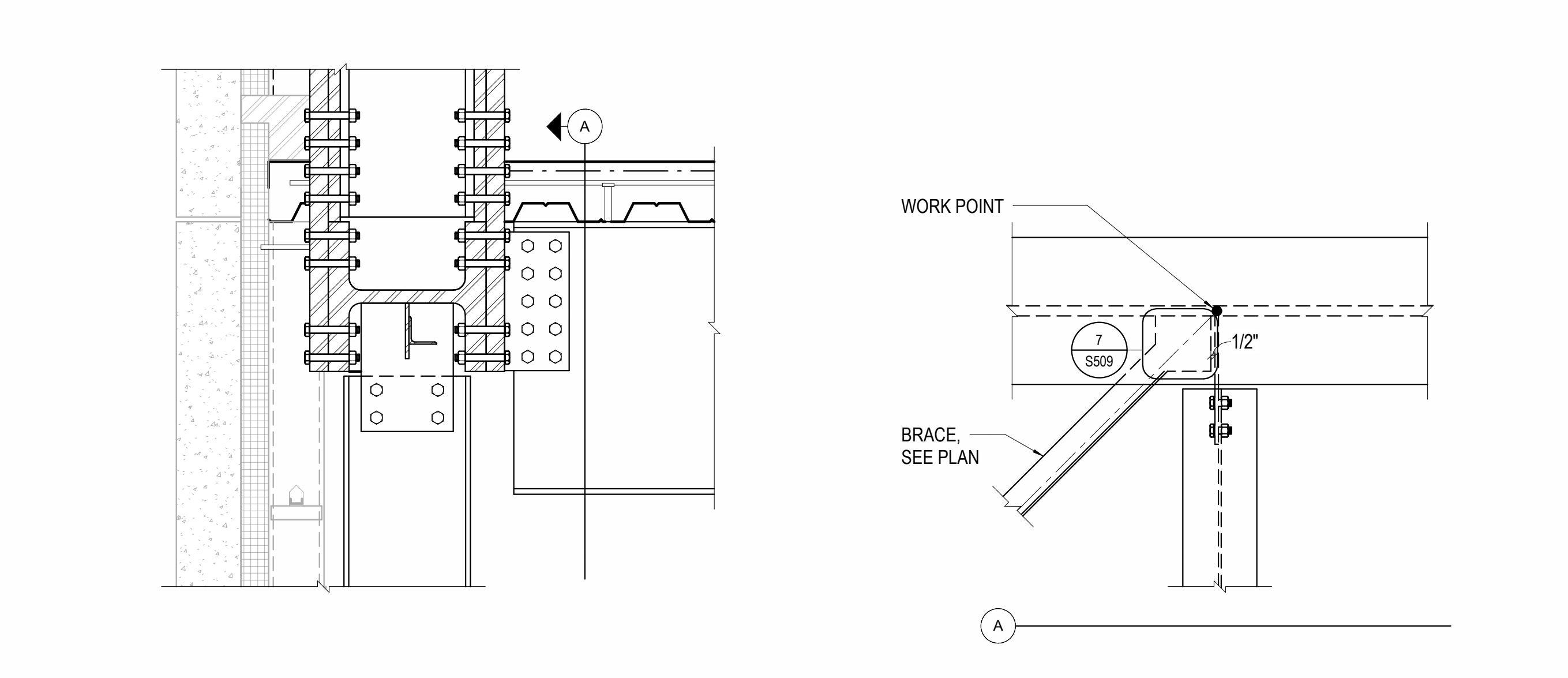
7 1 1/2" = 1'-0" TYP PRECAST SOFFIT TOP CONNECTION



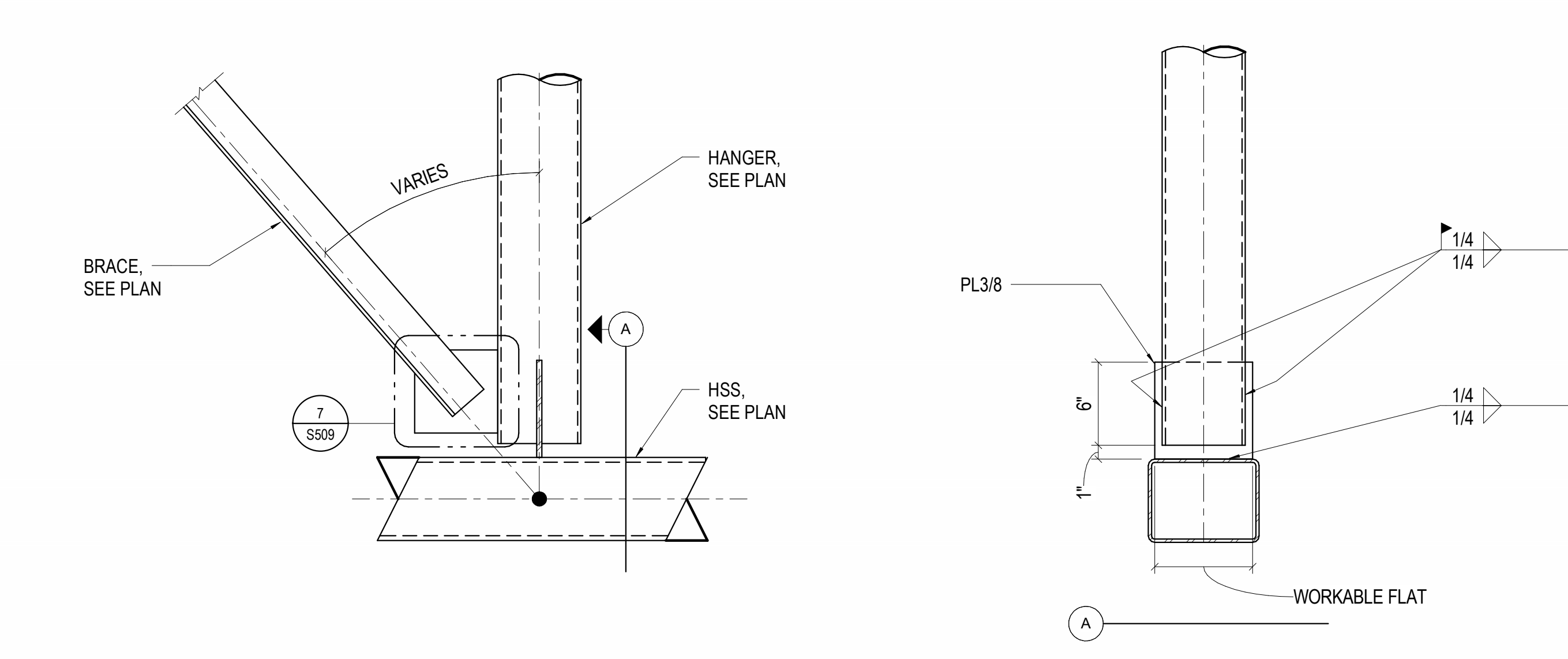
8 3/4" = 1'-0" TYP PRECAST SOFFIT FRAMING BRACE



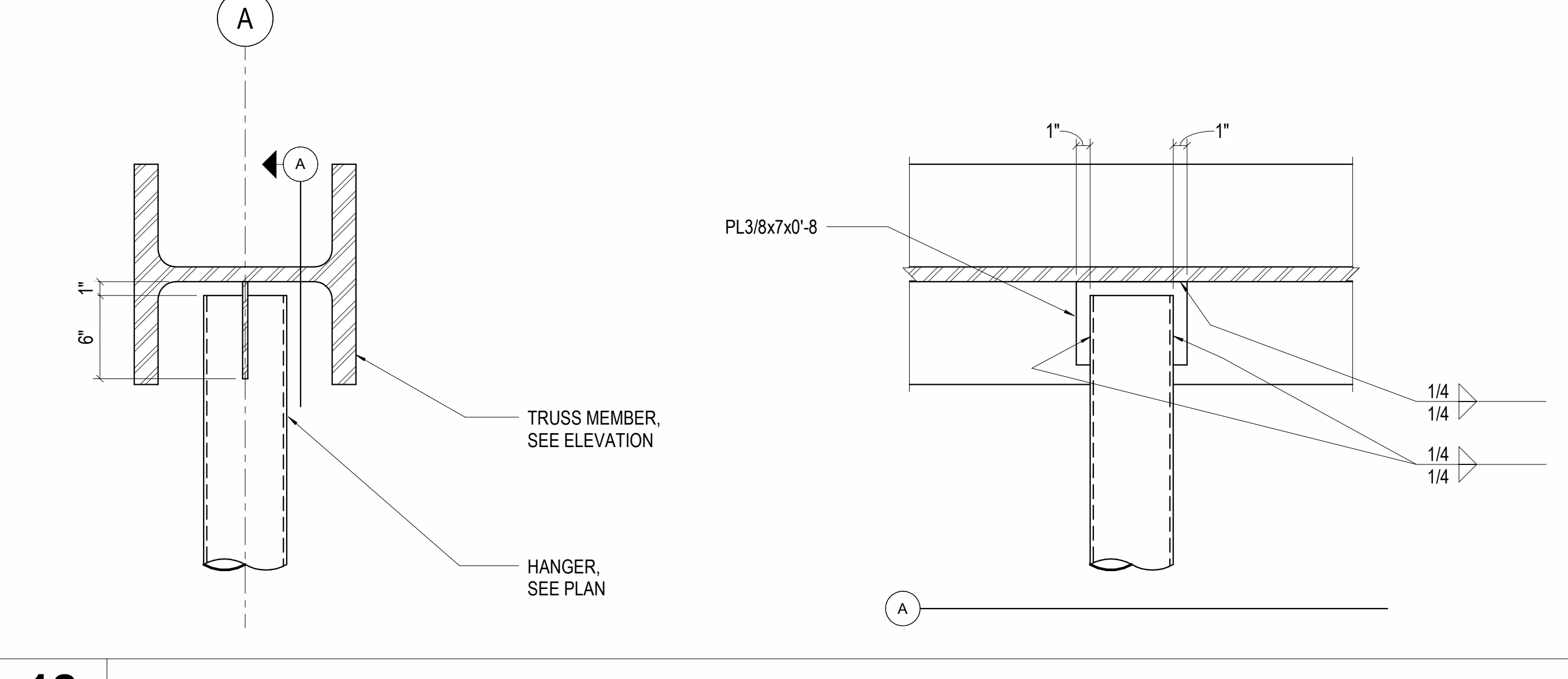
15 3/8" = 1'-0" PARTIAL PLAN - PRECAST SOFFIT FRAMING



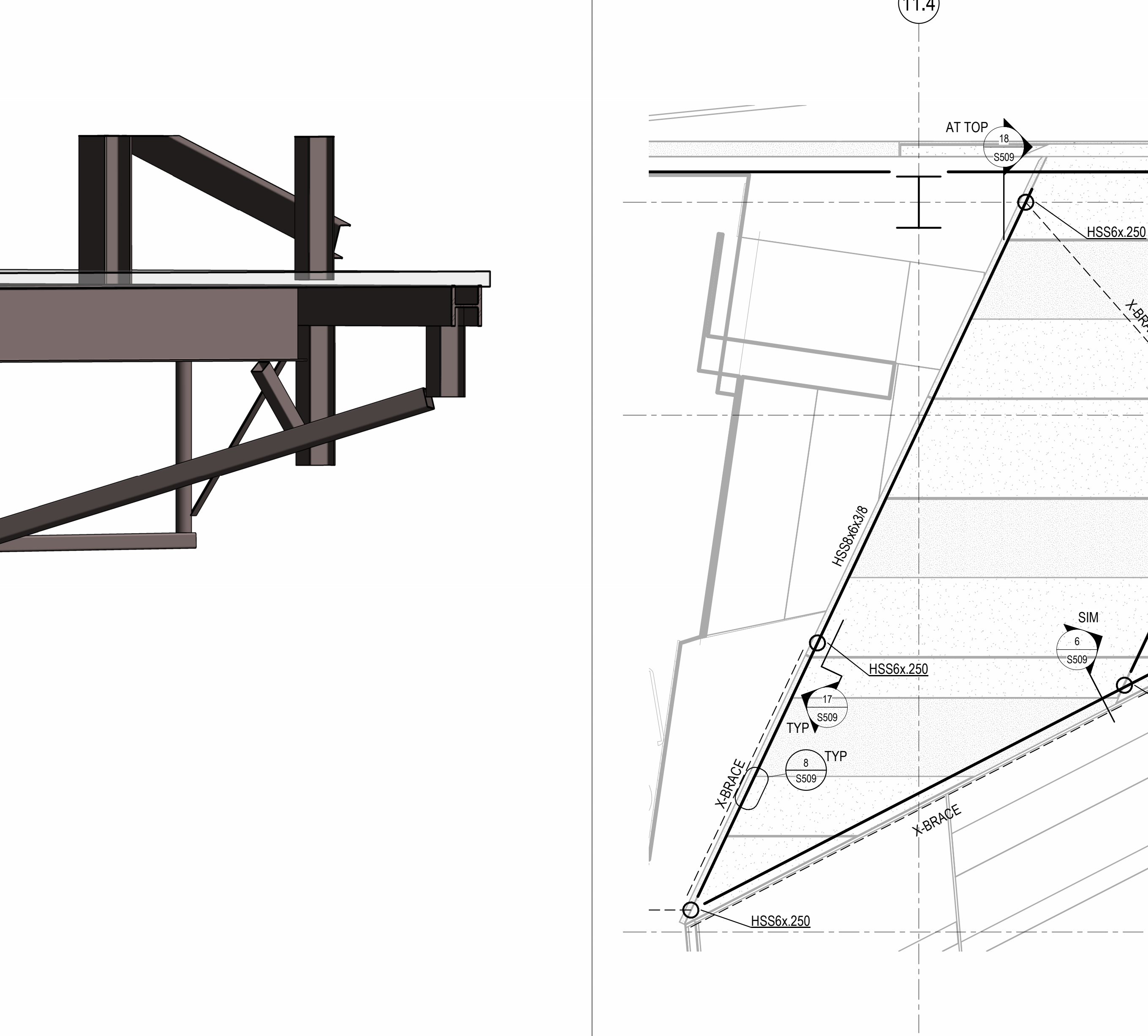
16 1" = 1'-0" PRECAST SOFFIT FRAMING BRACE



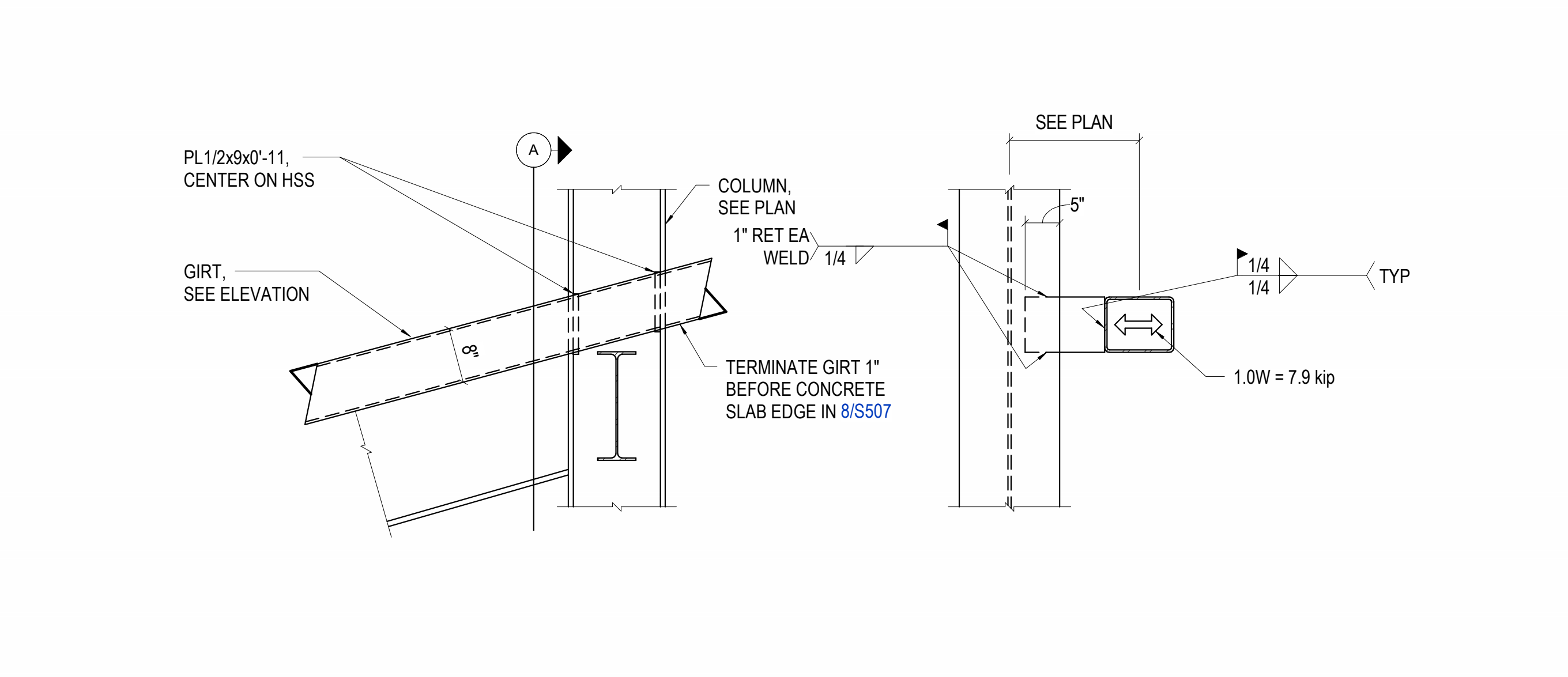
17 1 1/2" = 1'-0" TYP PRECAST SOFFIT FRAMING CONNECTION



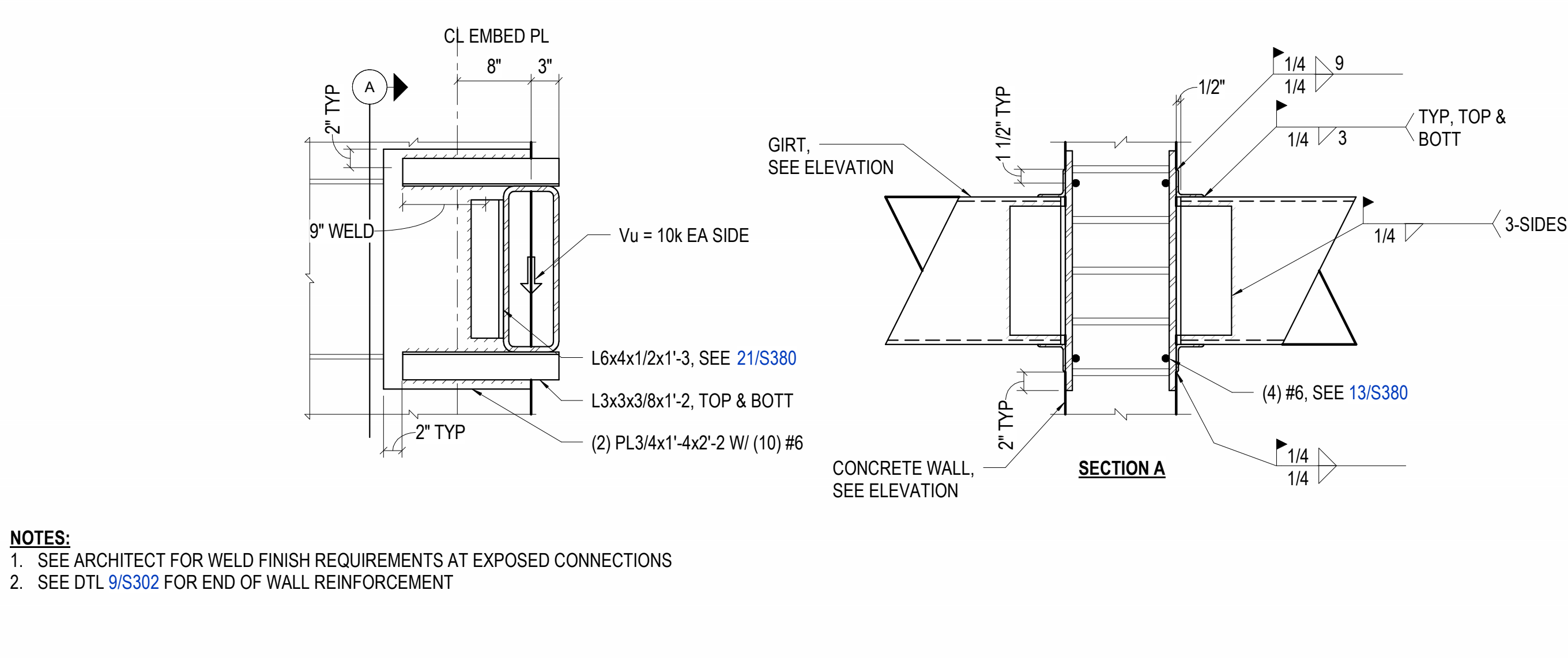
18 1 1/2" = 1'-0" PRECAST SOFFIT FRAMING TRUSS CONNECTION



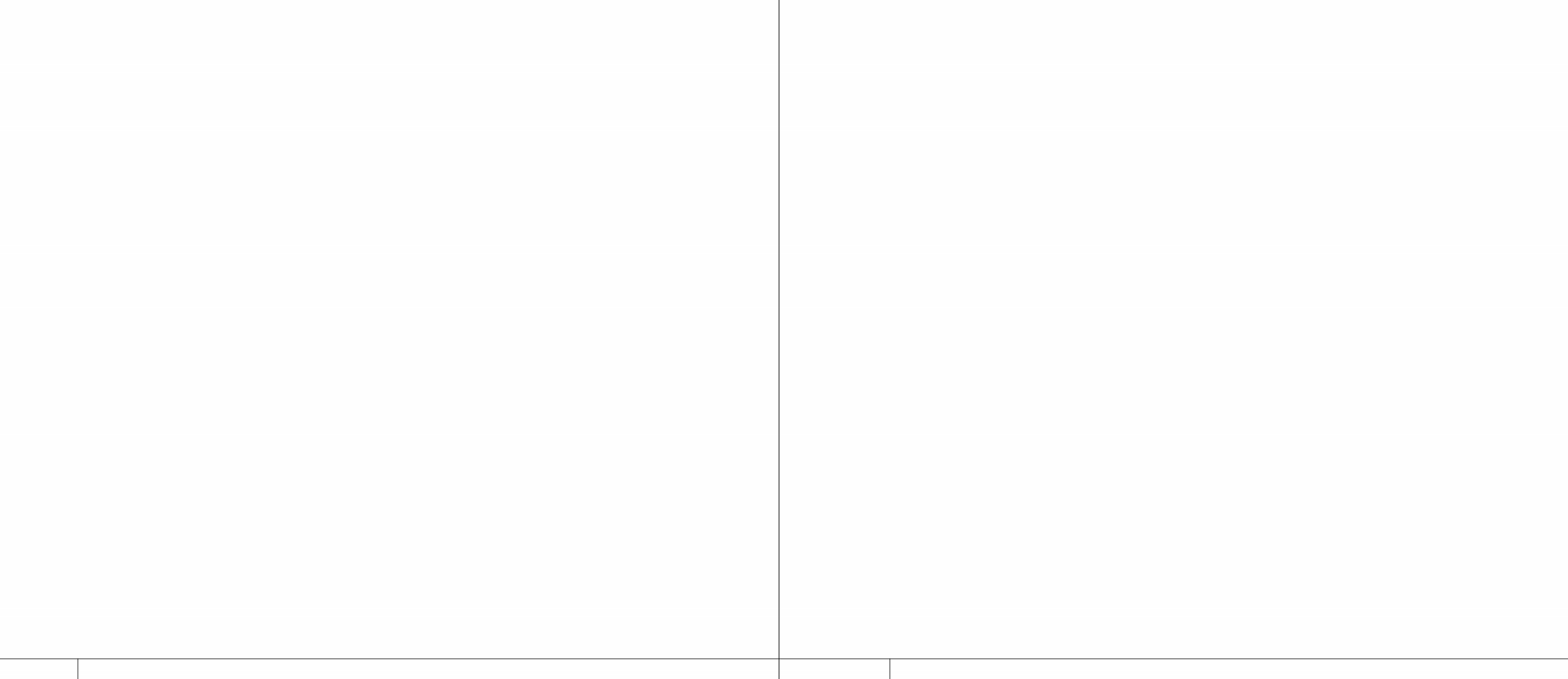
3 1 1/2" = 1'-0" TYP PRECAST SOFFIT PANEL CONNECTION



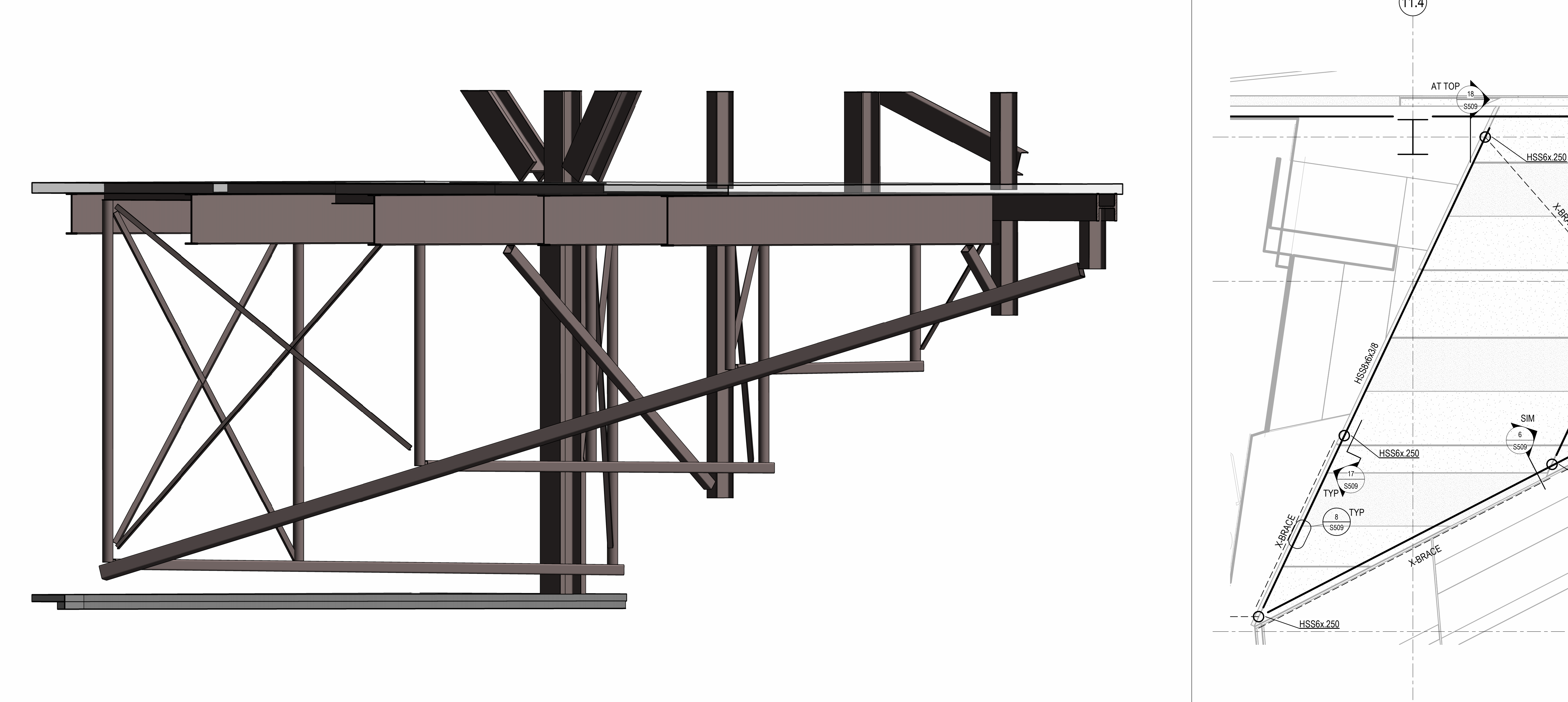
26 3/4" = 1'-0" MONUMENTAL STAIR GIRT CONNECTION AT GRID E



27 1" = 1'-0" GIRT TO CONC WALL CONN GRID A



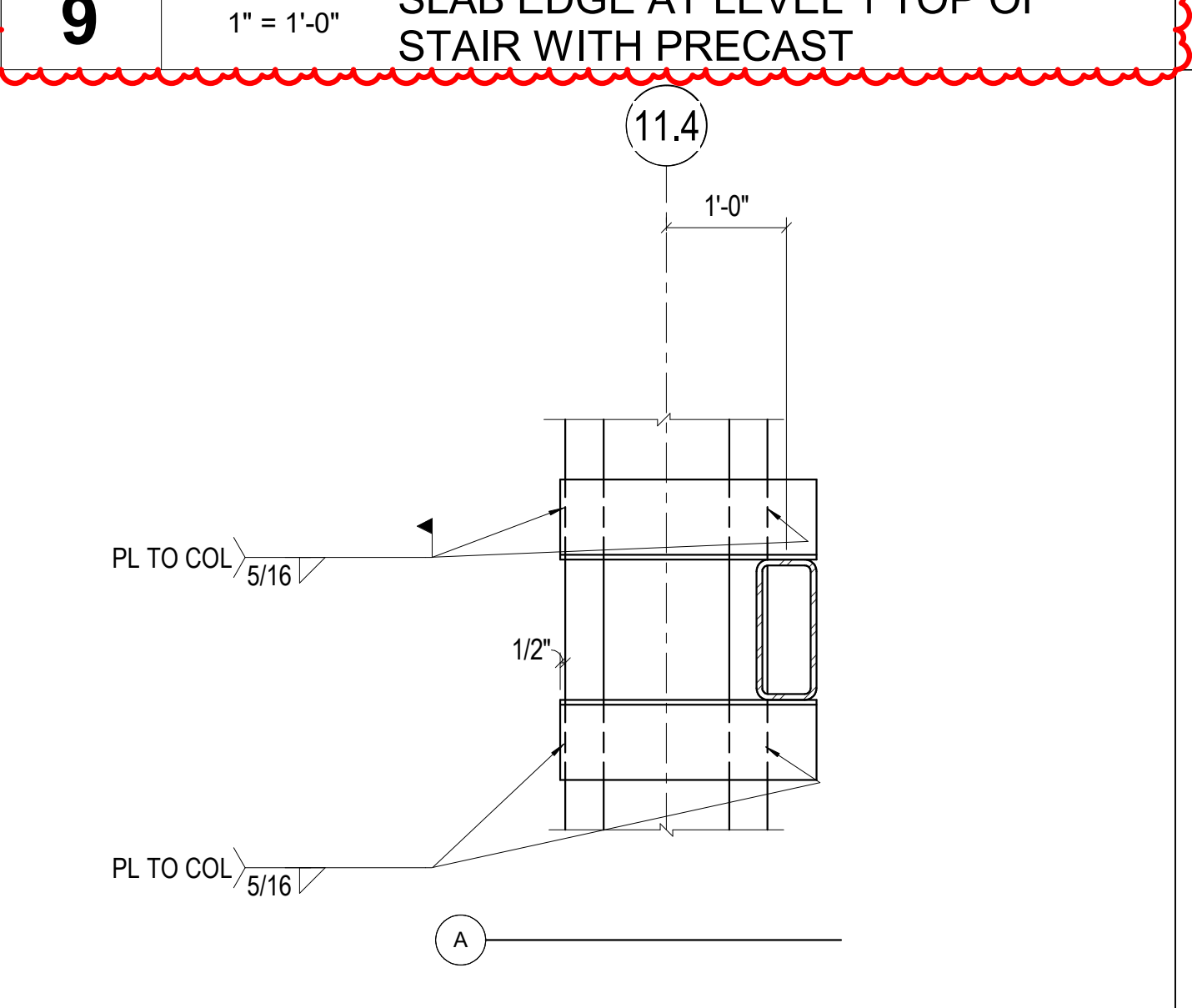
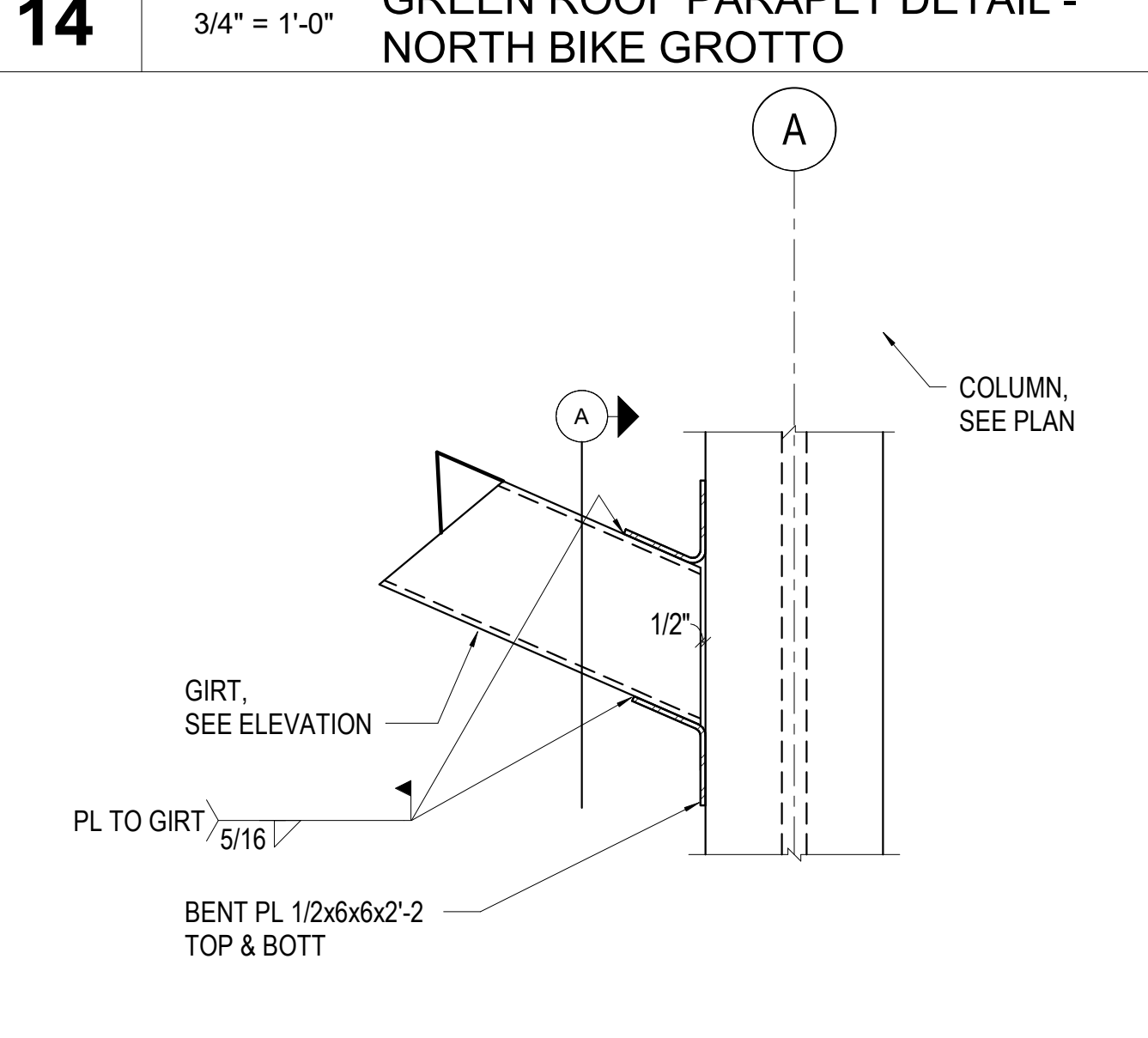
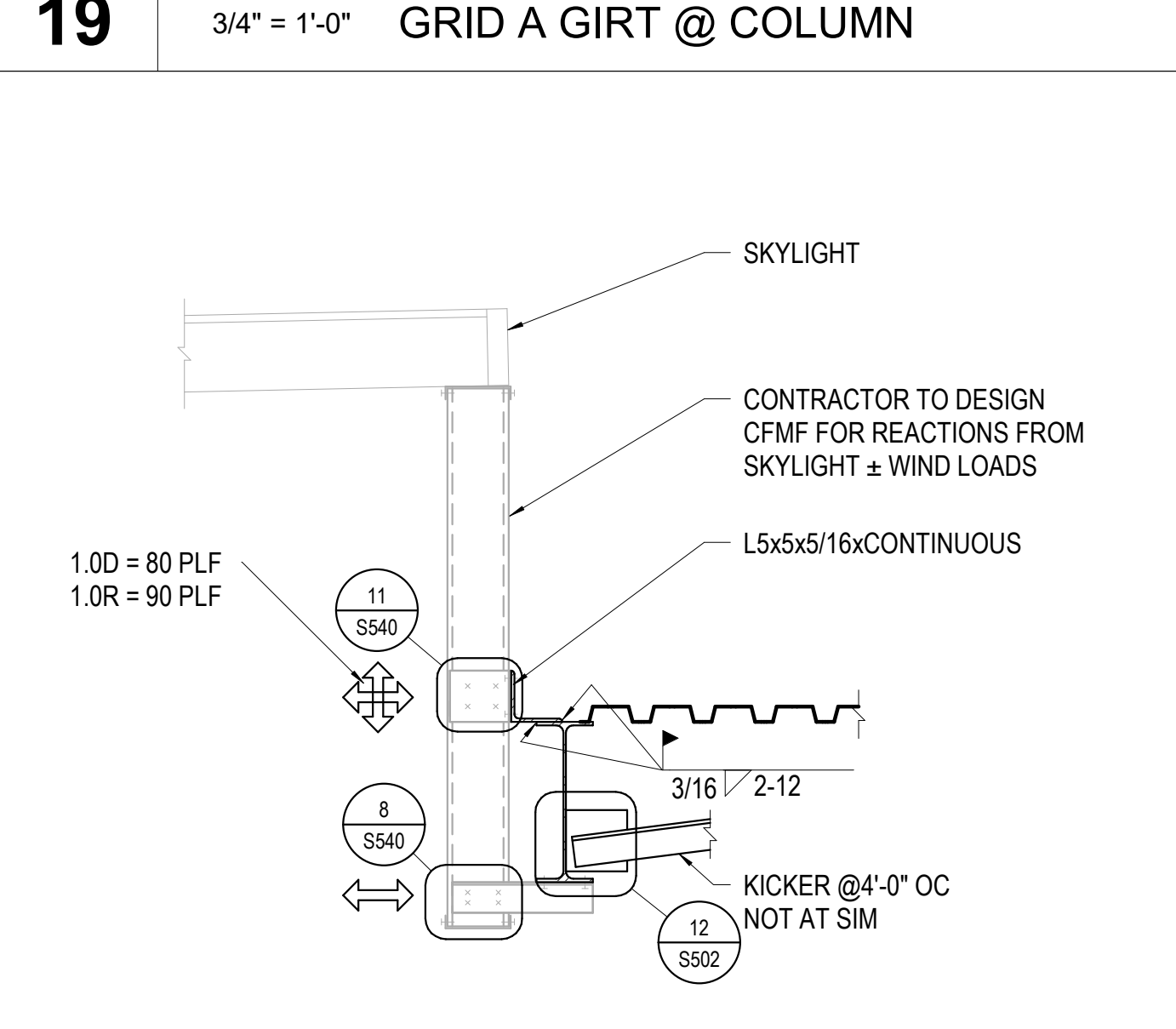
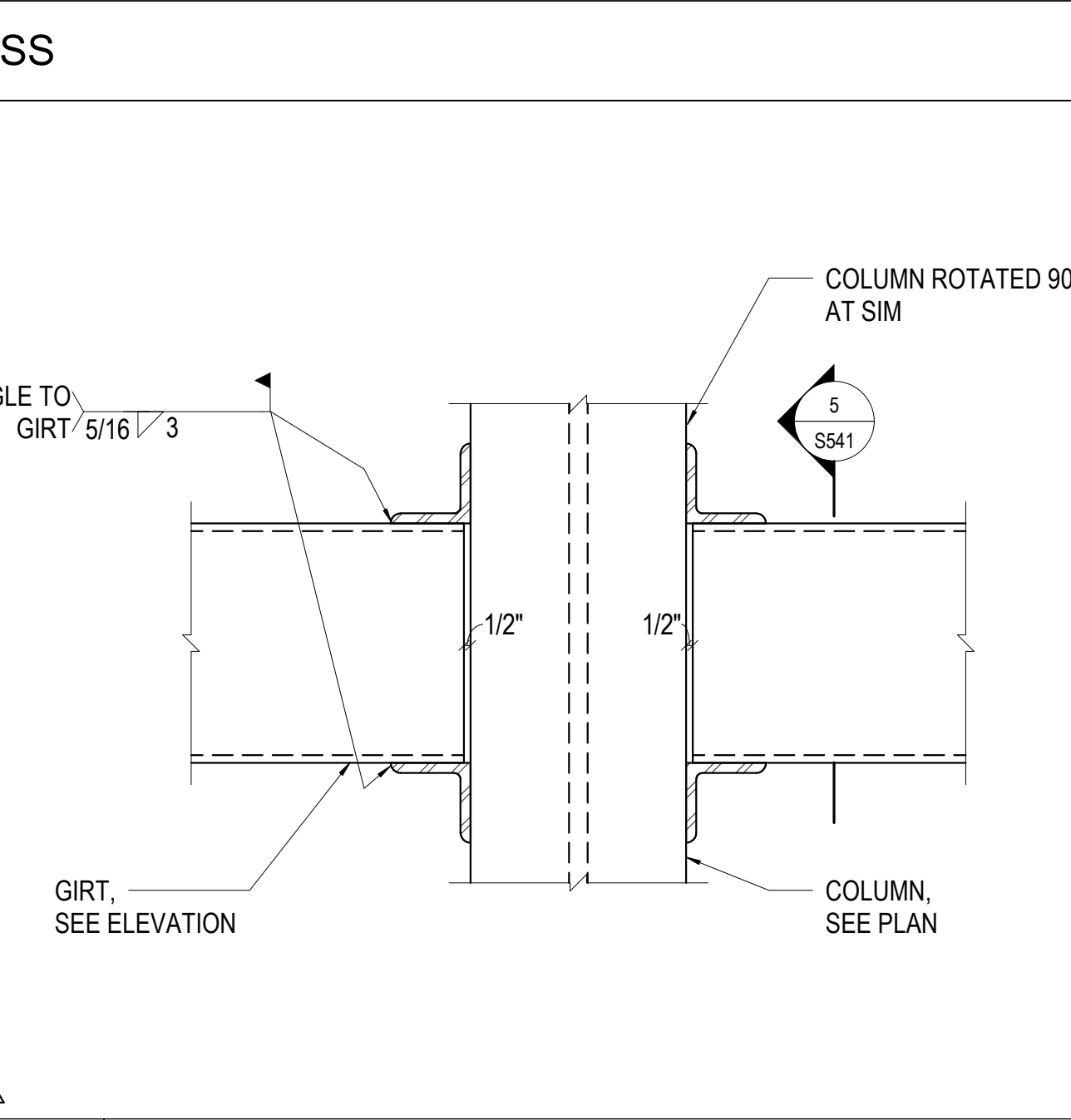
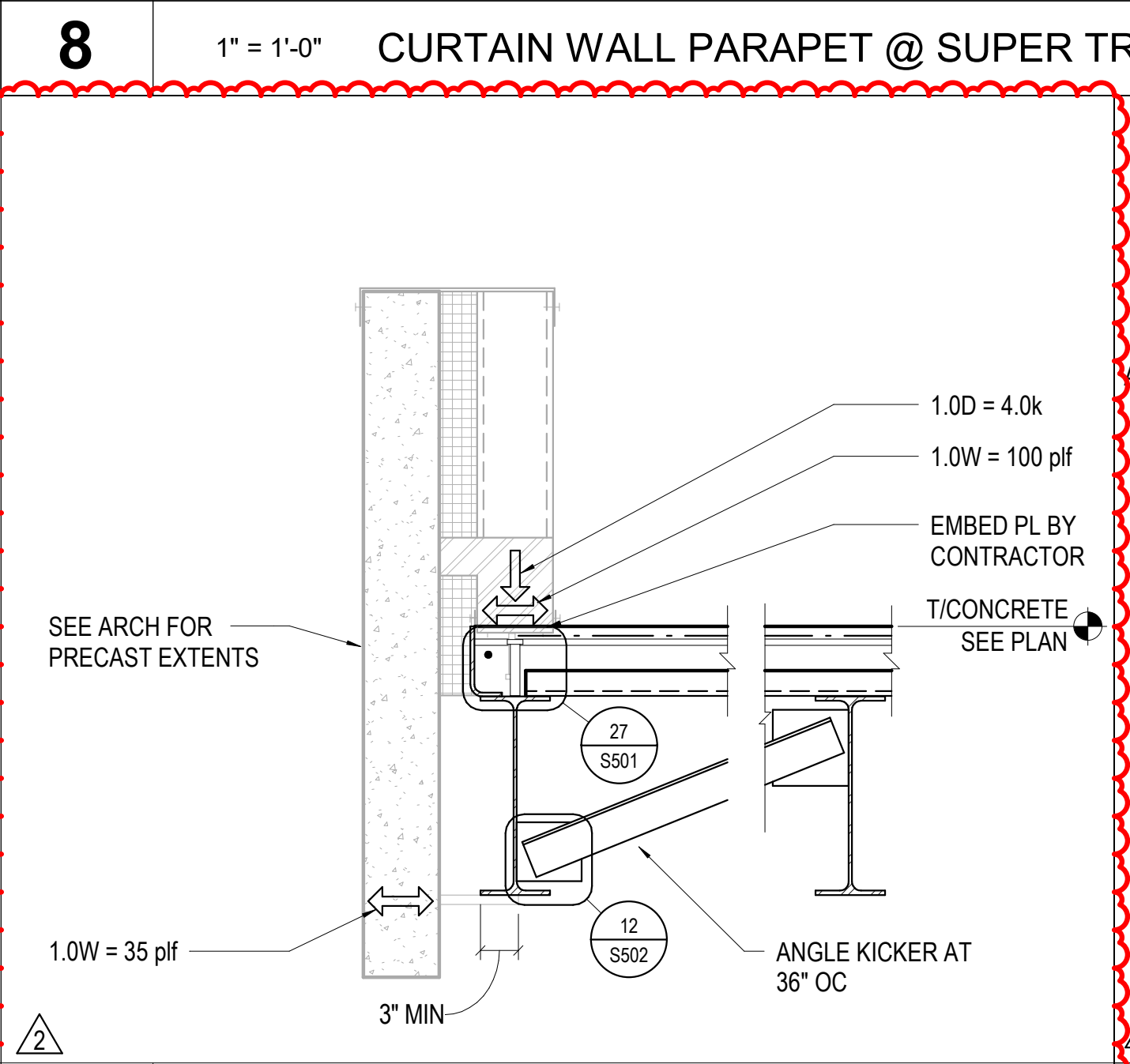
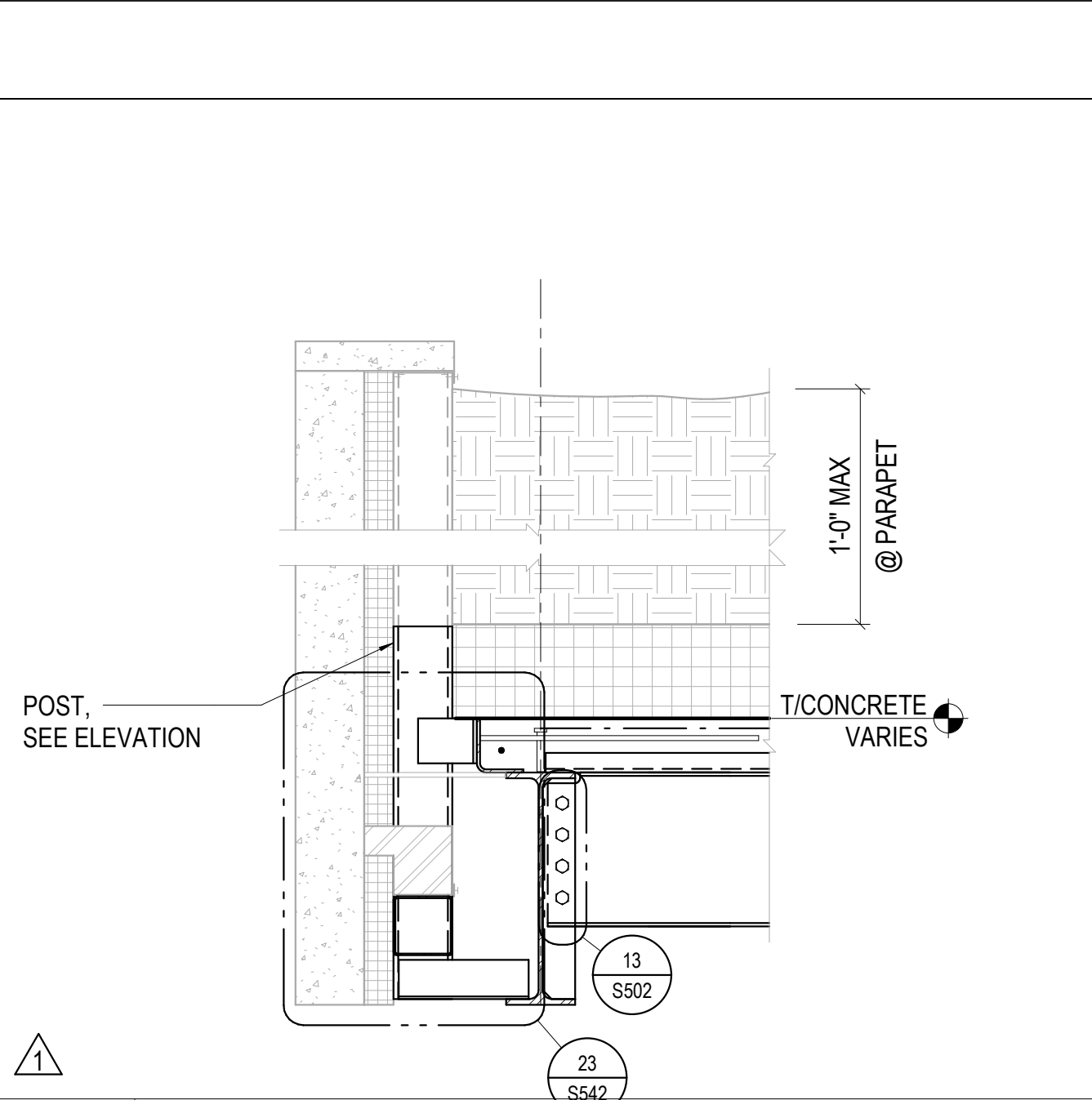
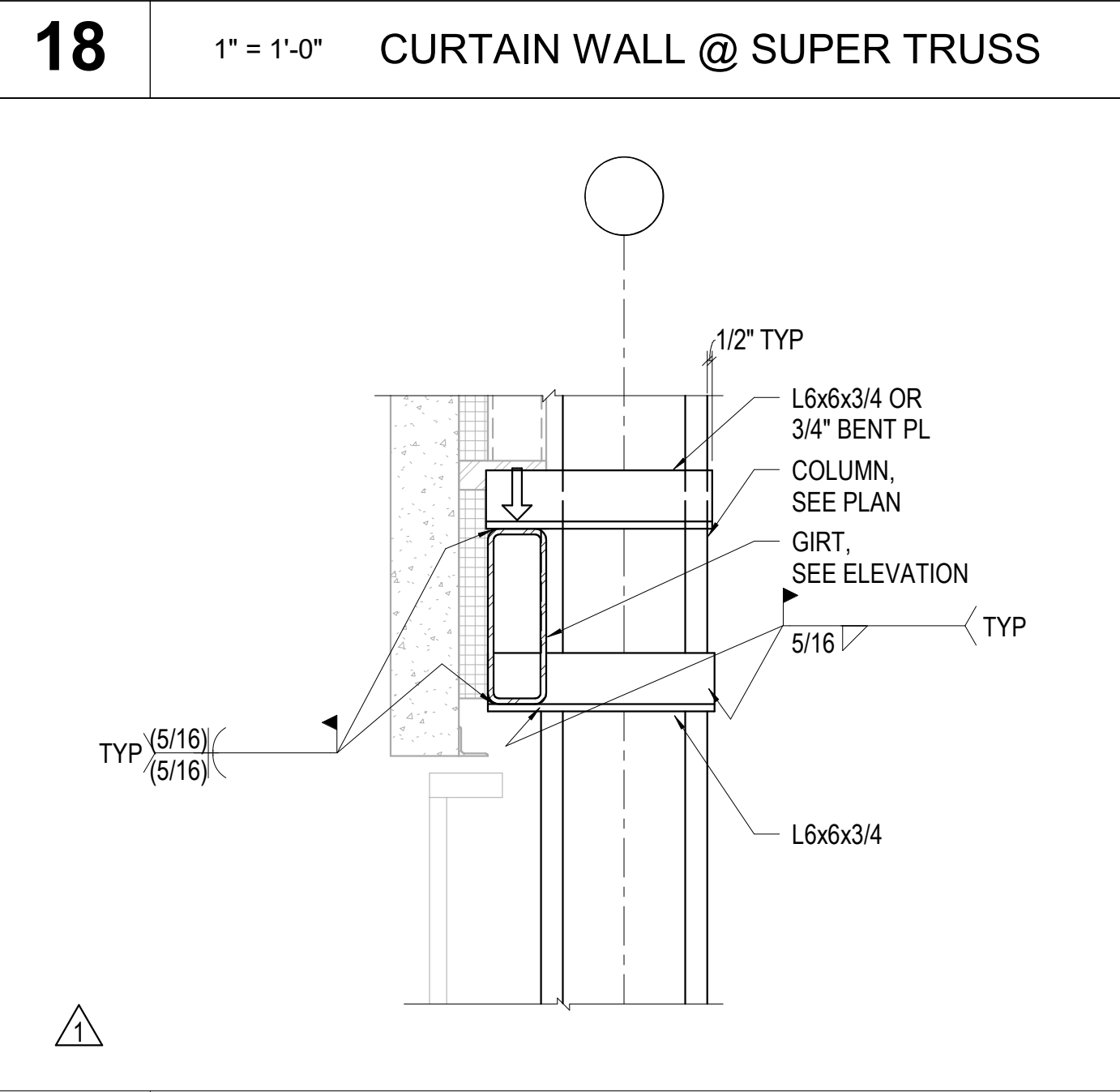
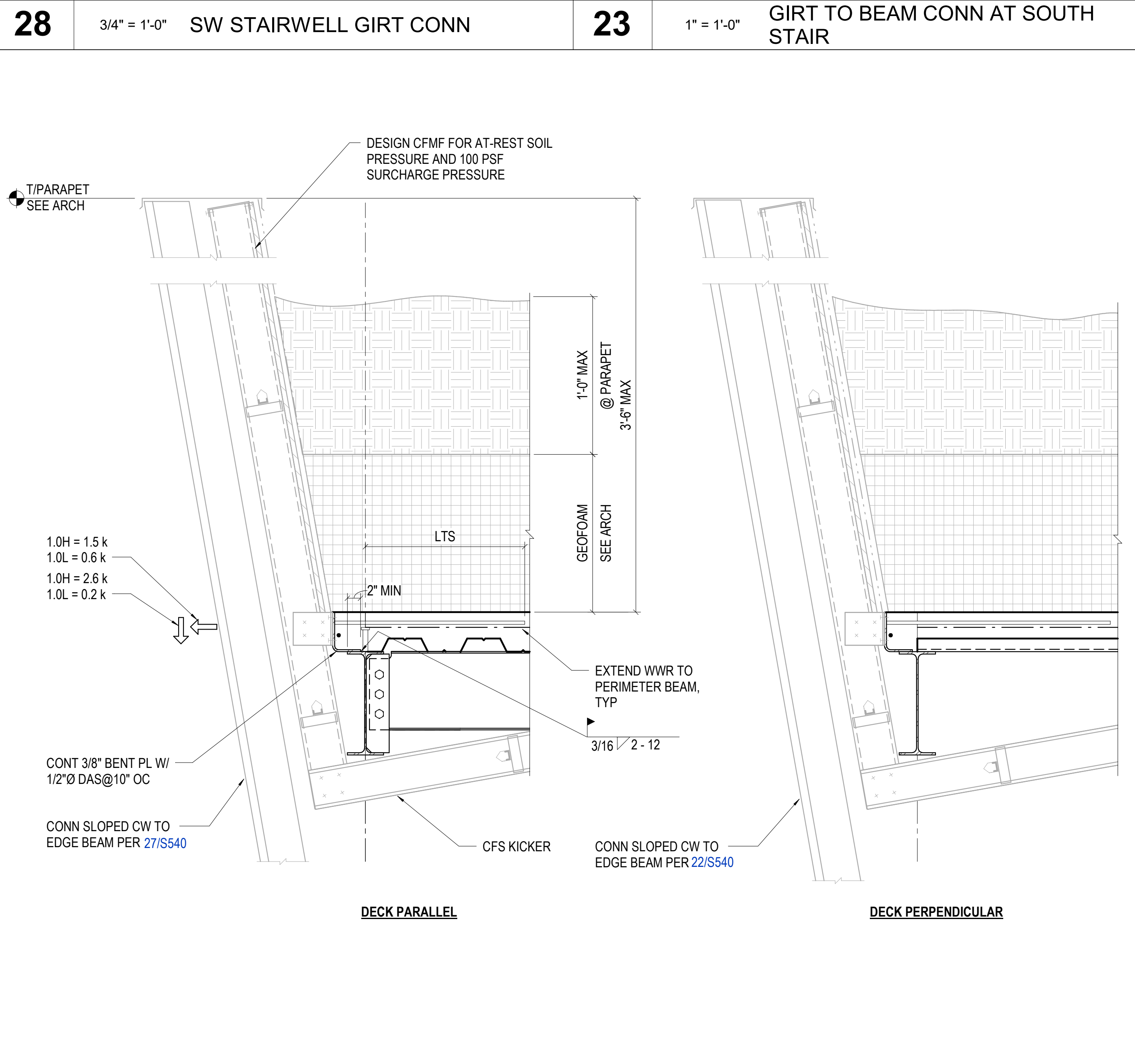
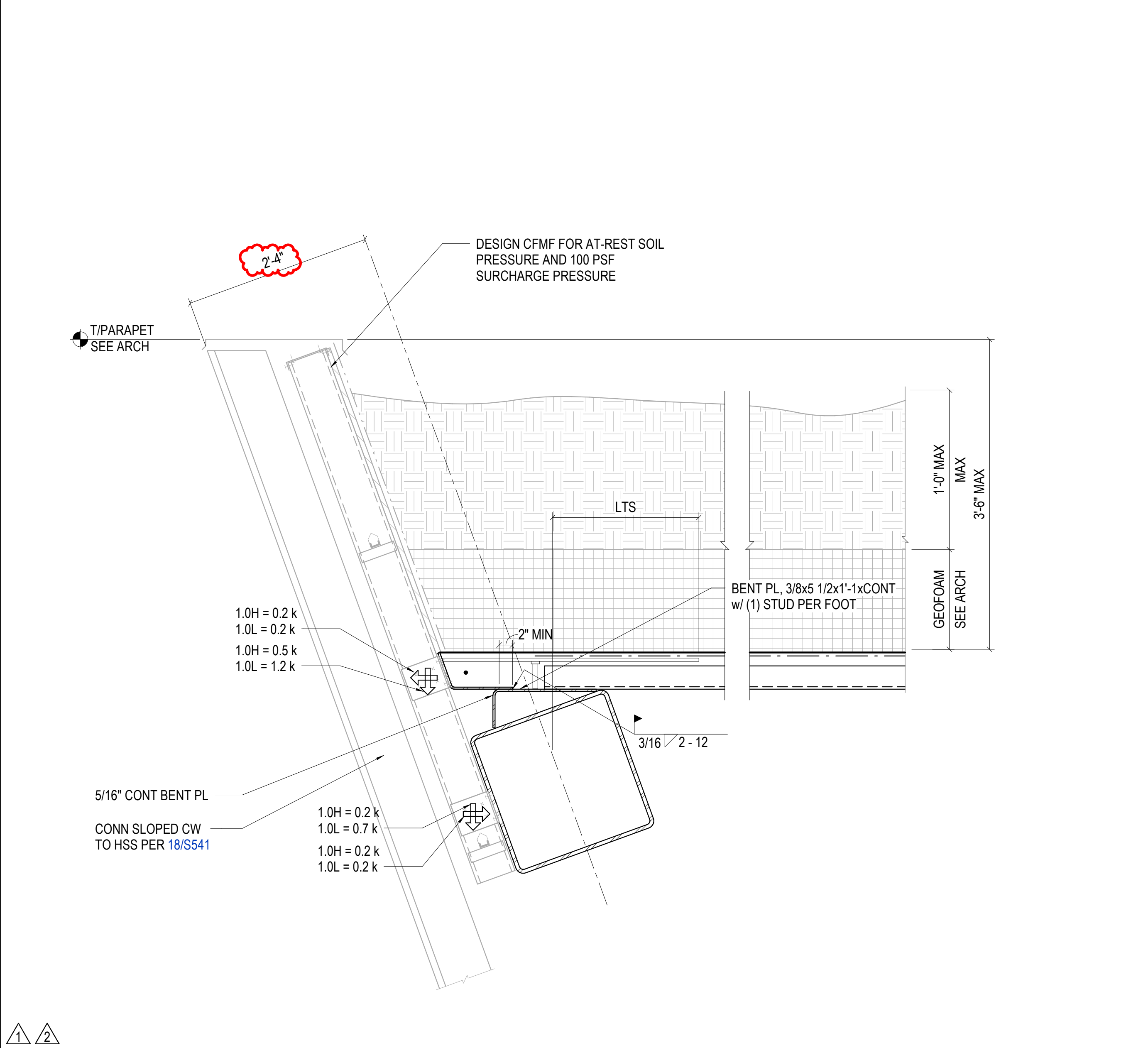
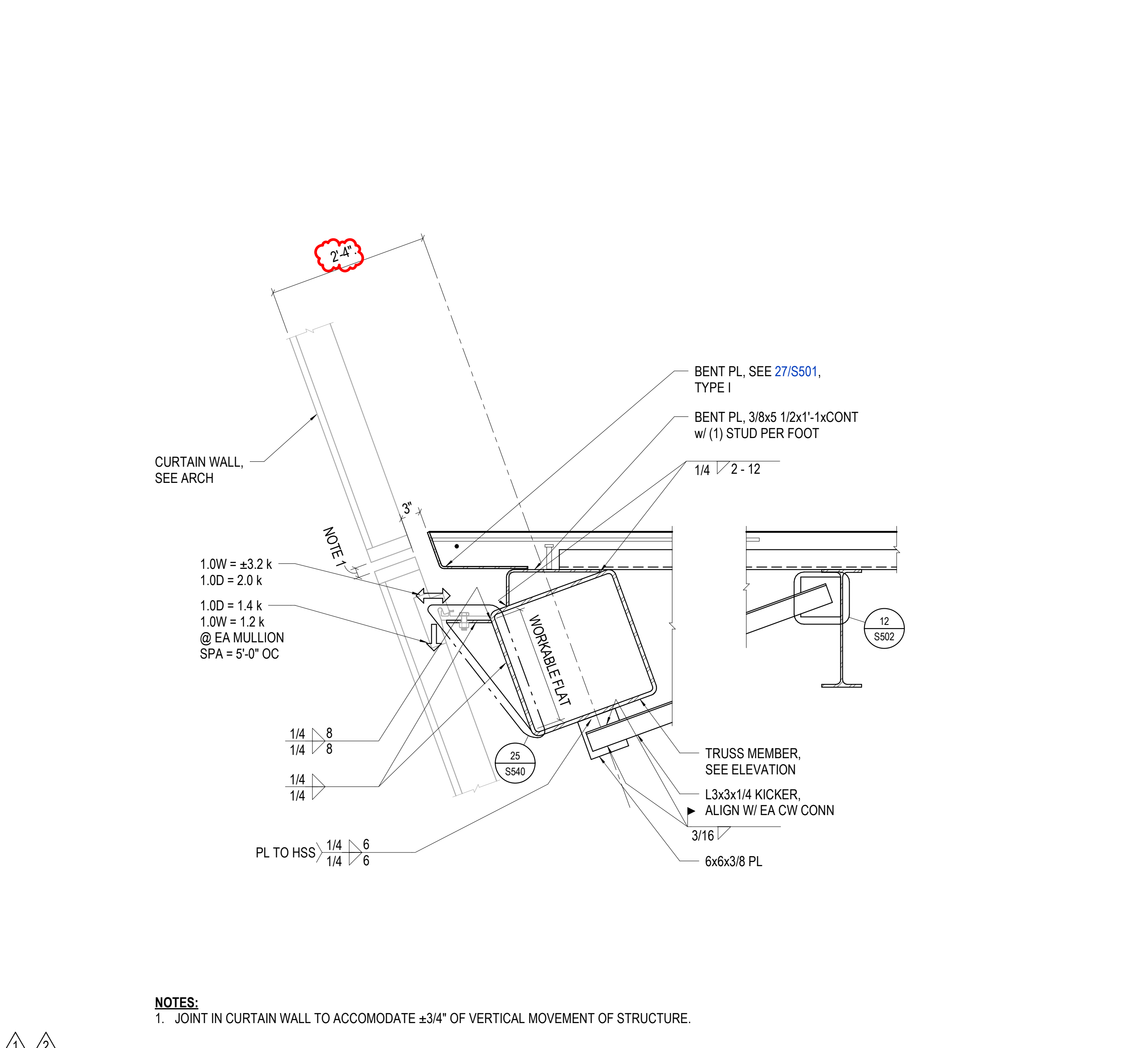
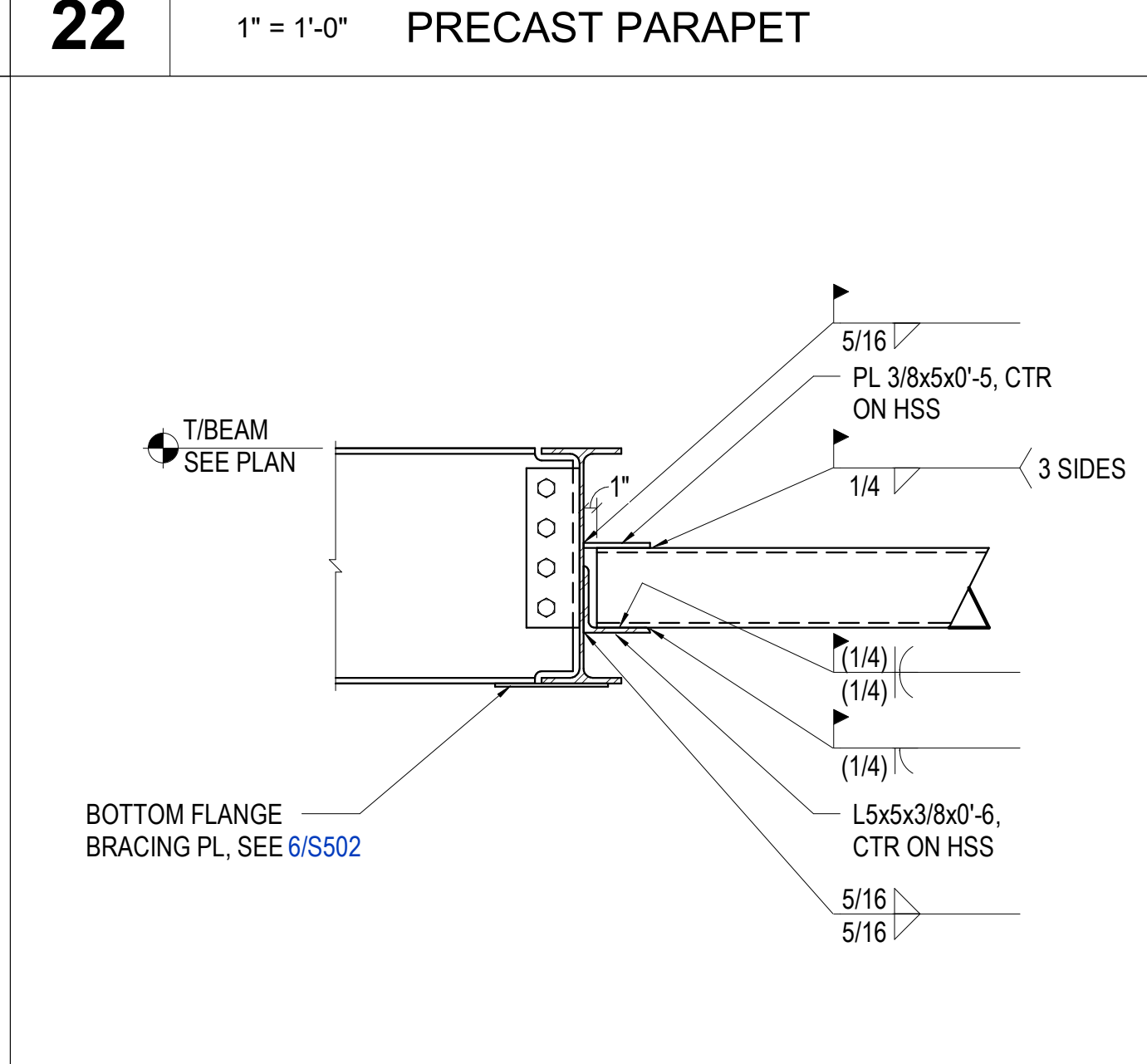
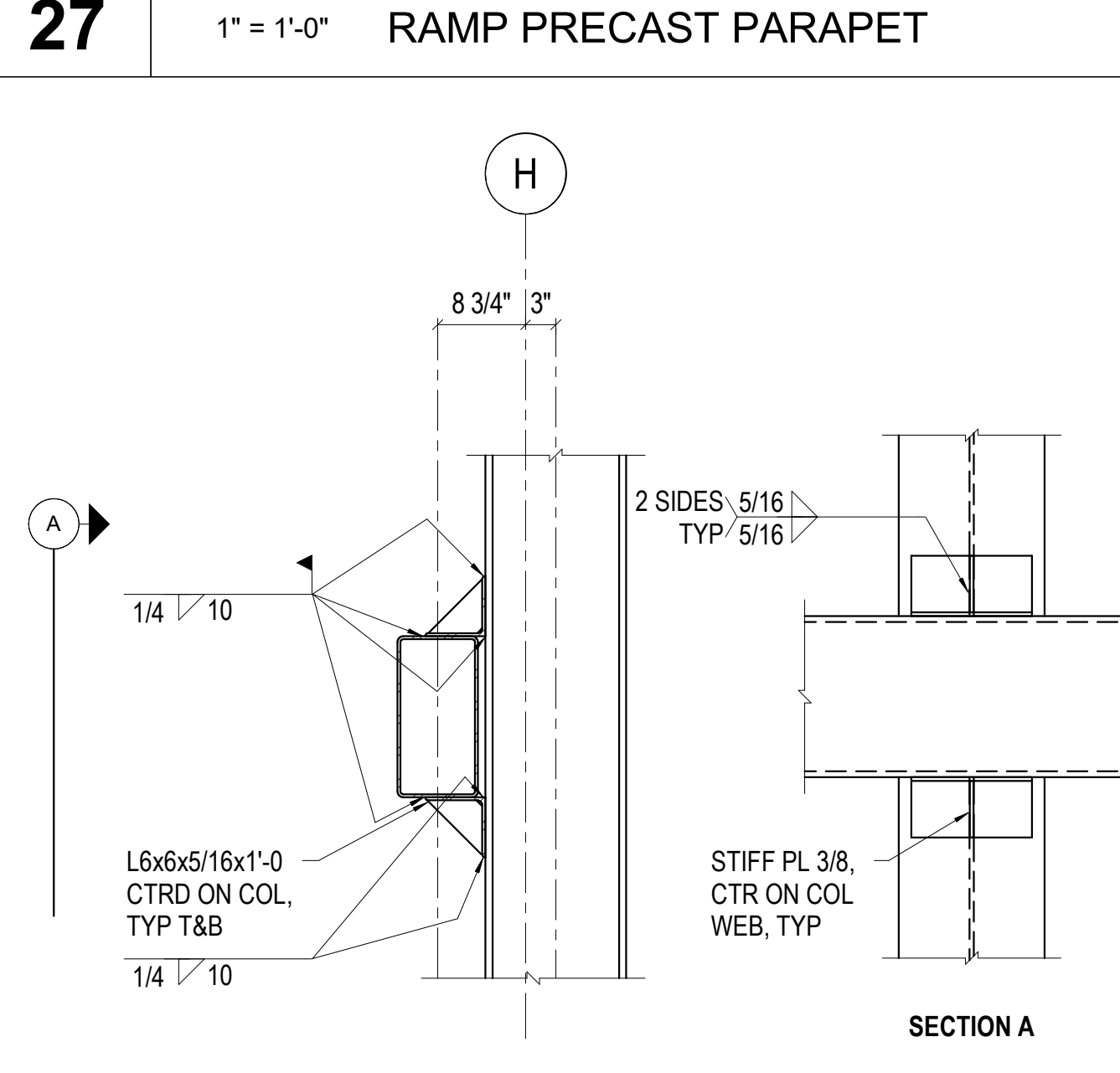
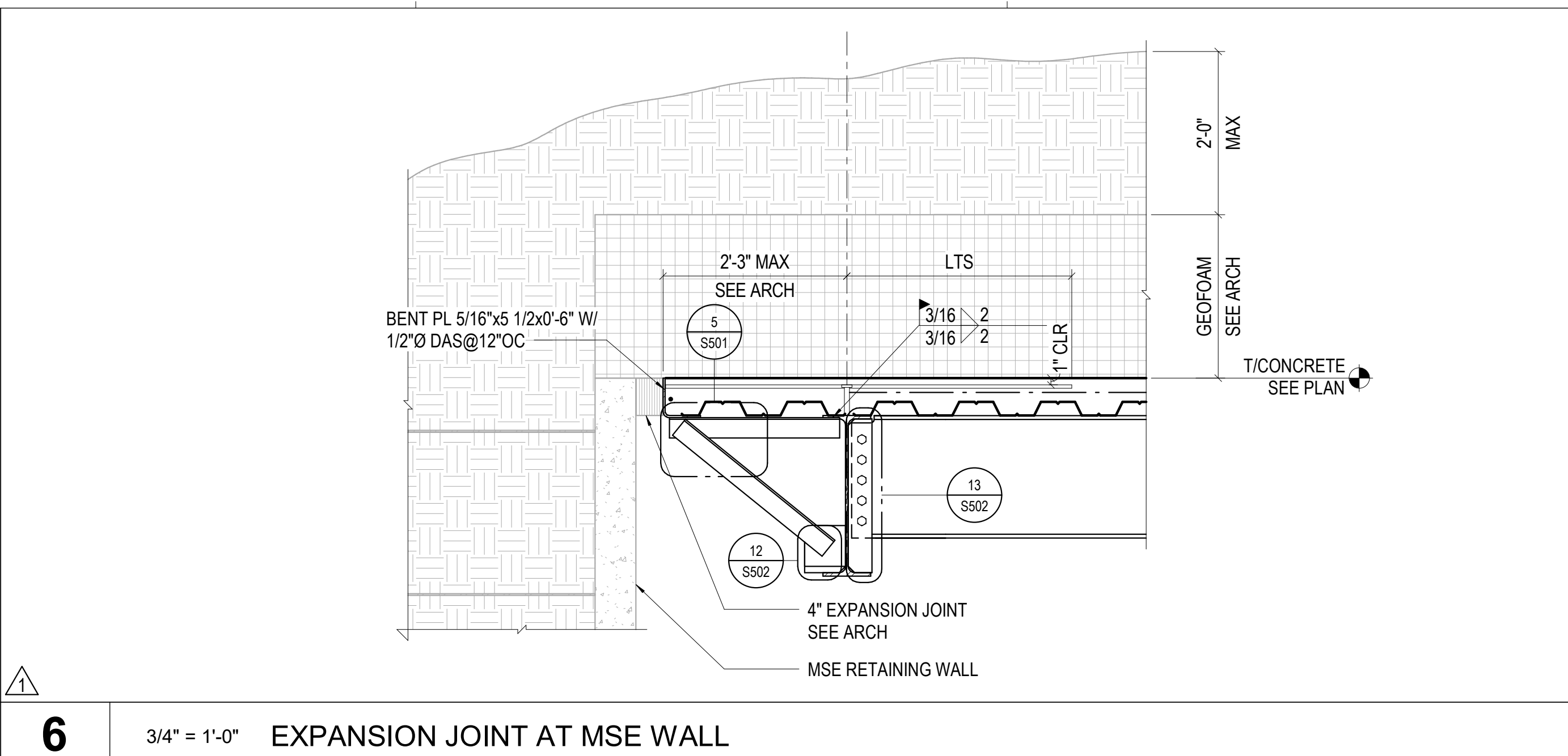
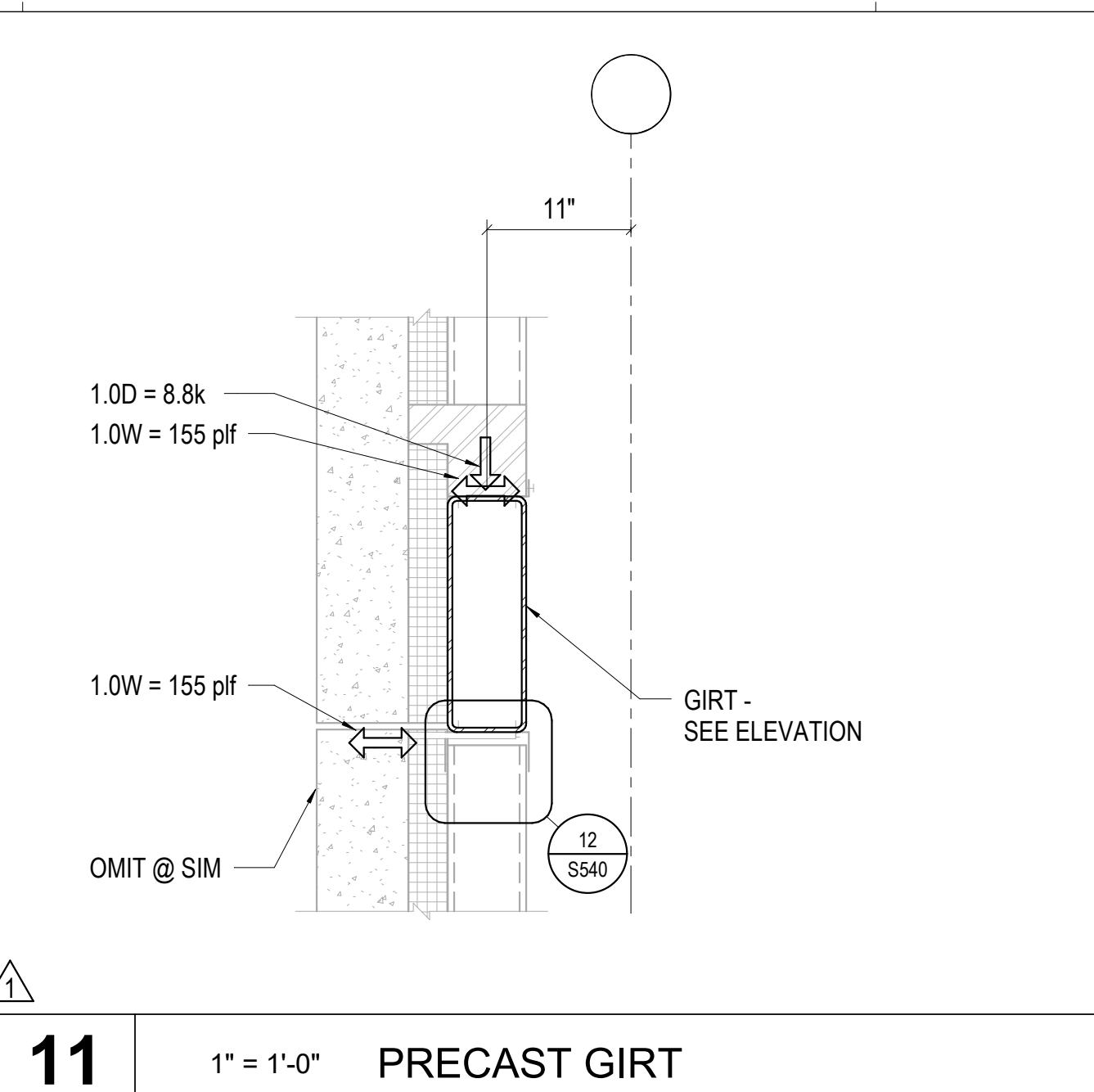
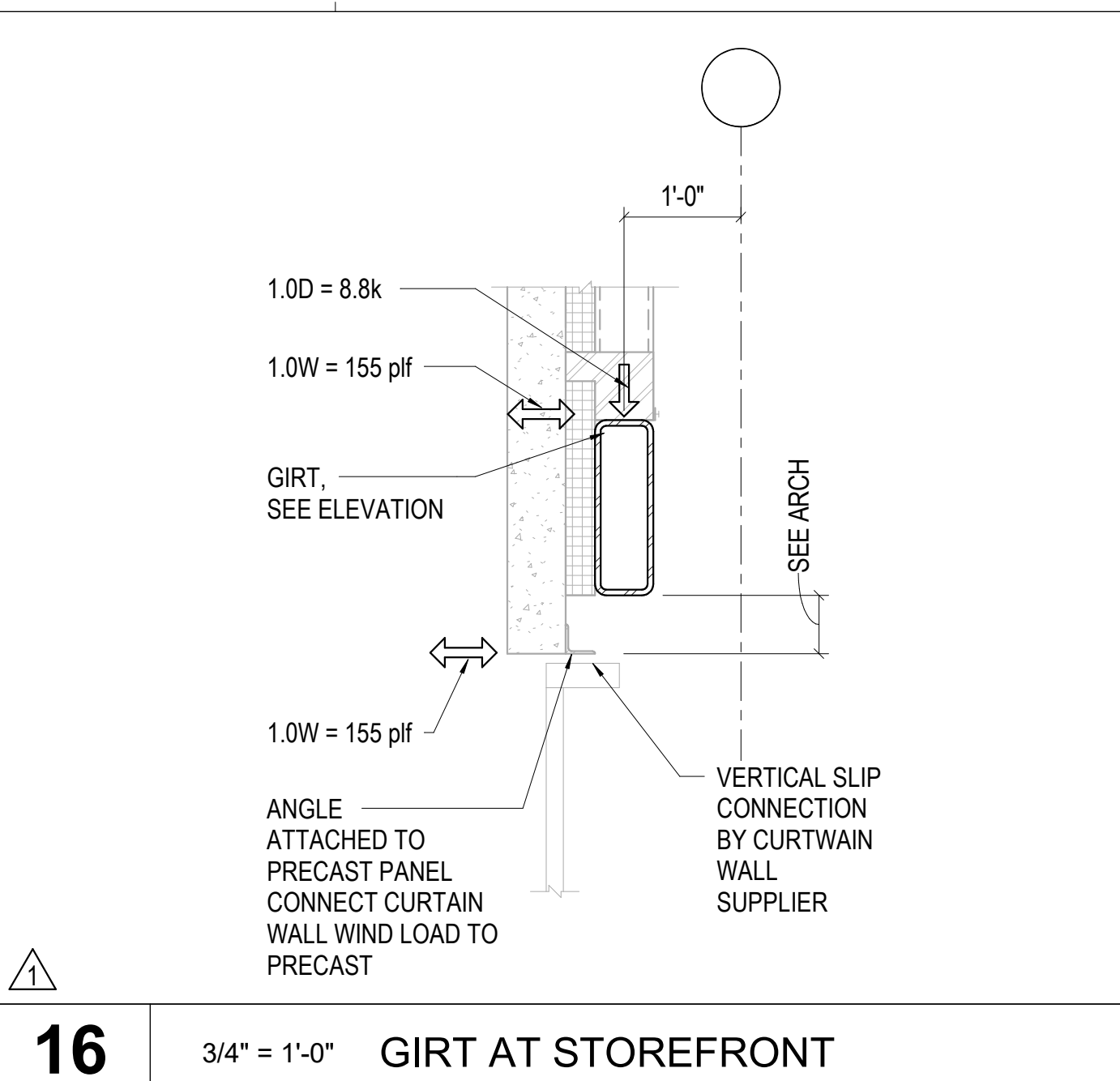
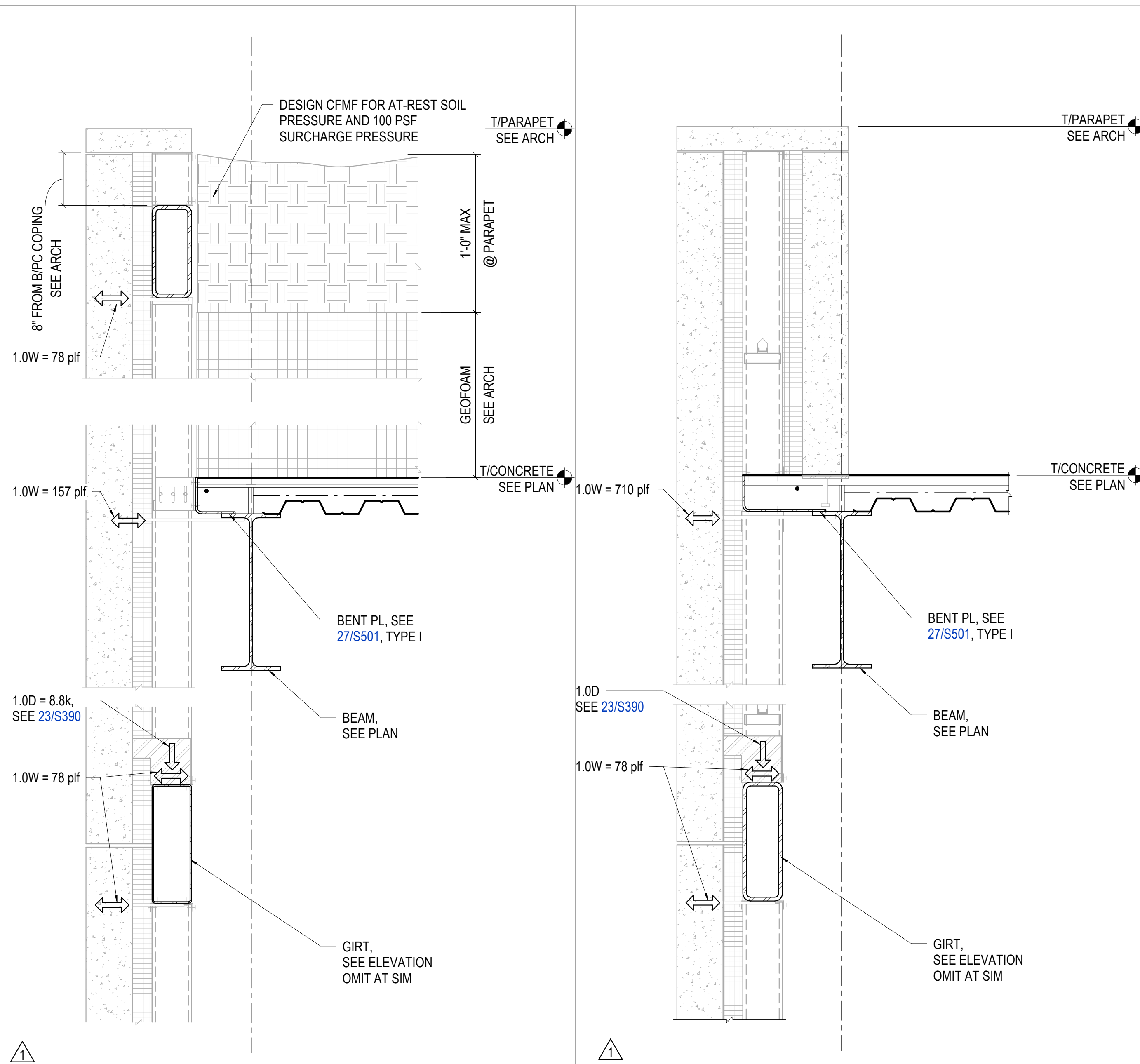
30 ISOMETRIC - PRECAST SOFFIT FRAMING

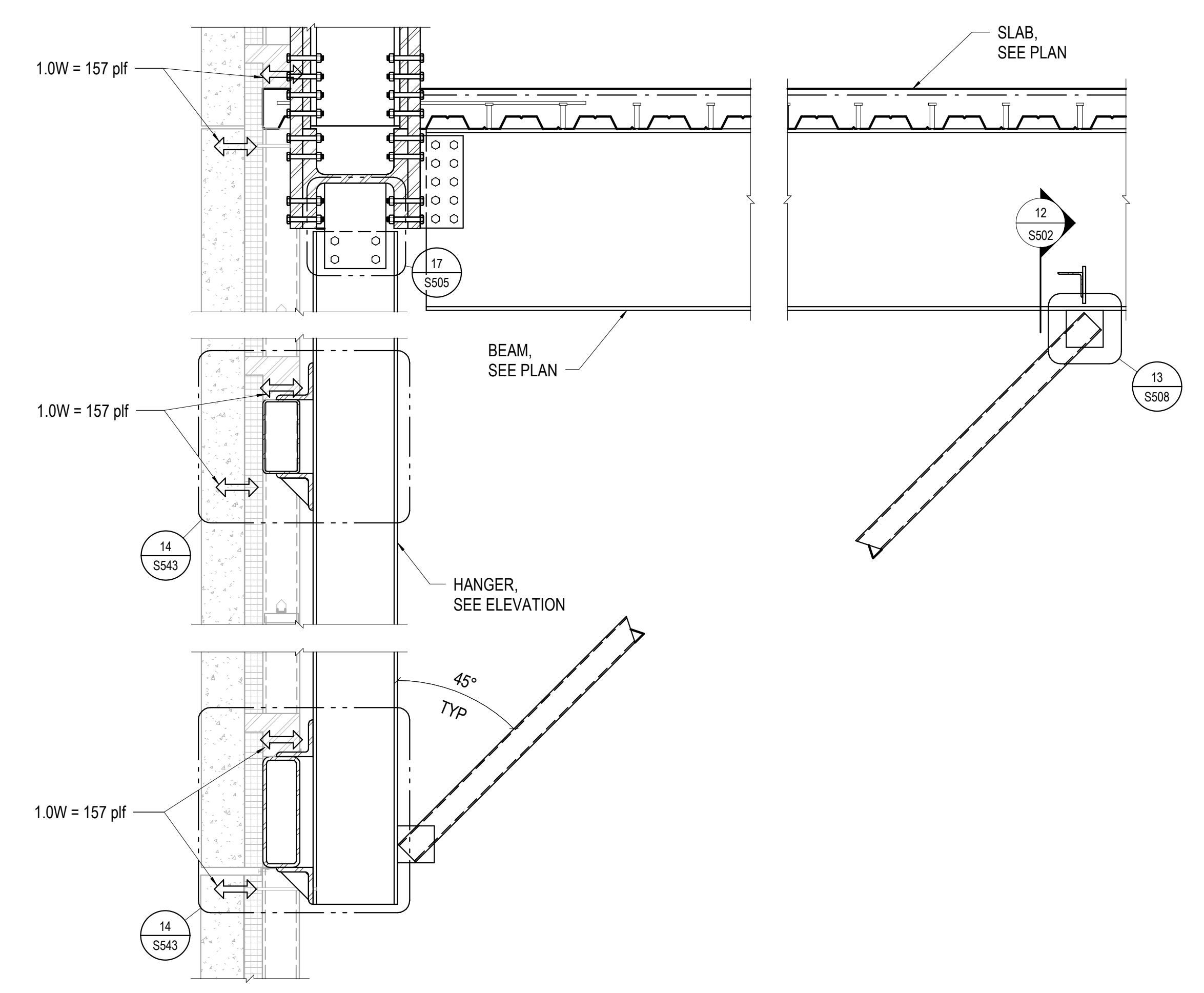


15 3/8" = 1'-0" PARTIAL PLAN - PRECAST SOFFIT FRAMING

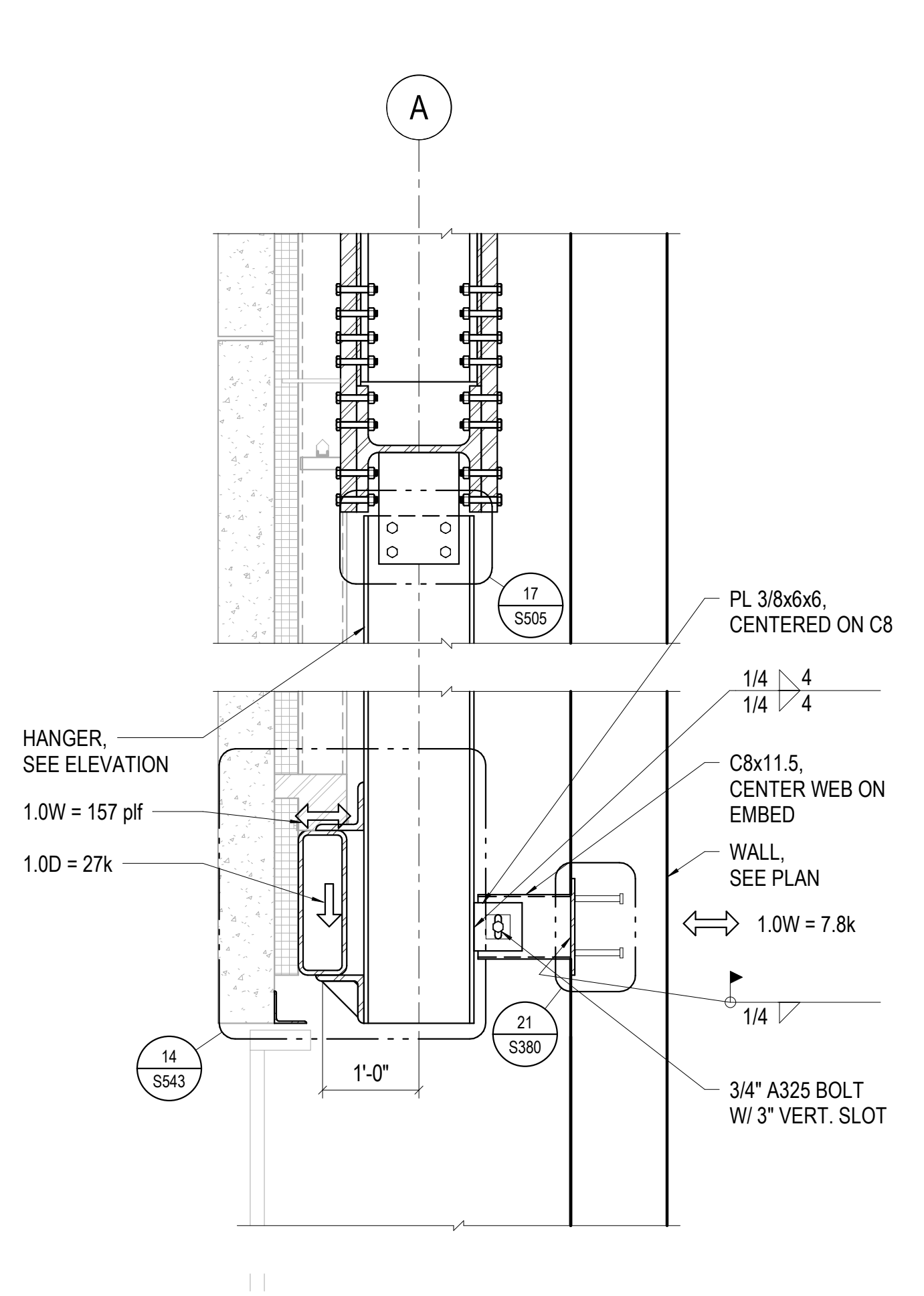


NUMBER	DATE	DESCRIPTION
1	03.13.2023	ADDITIONAL 1
2	06.09.2023	ADDITIONAL 2

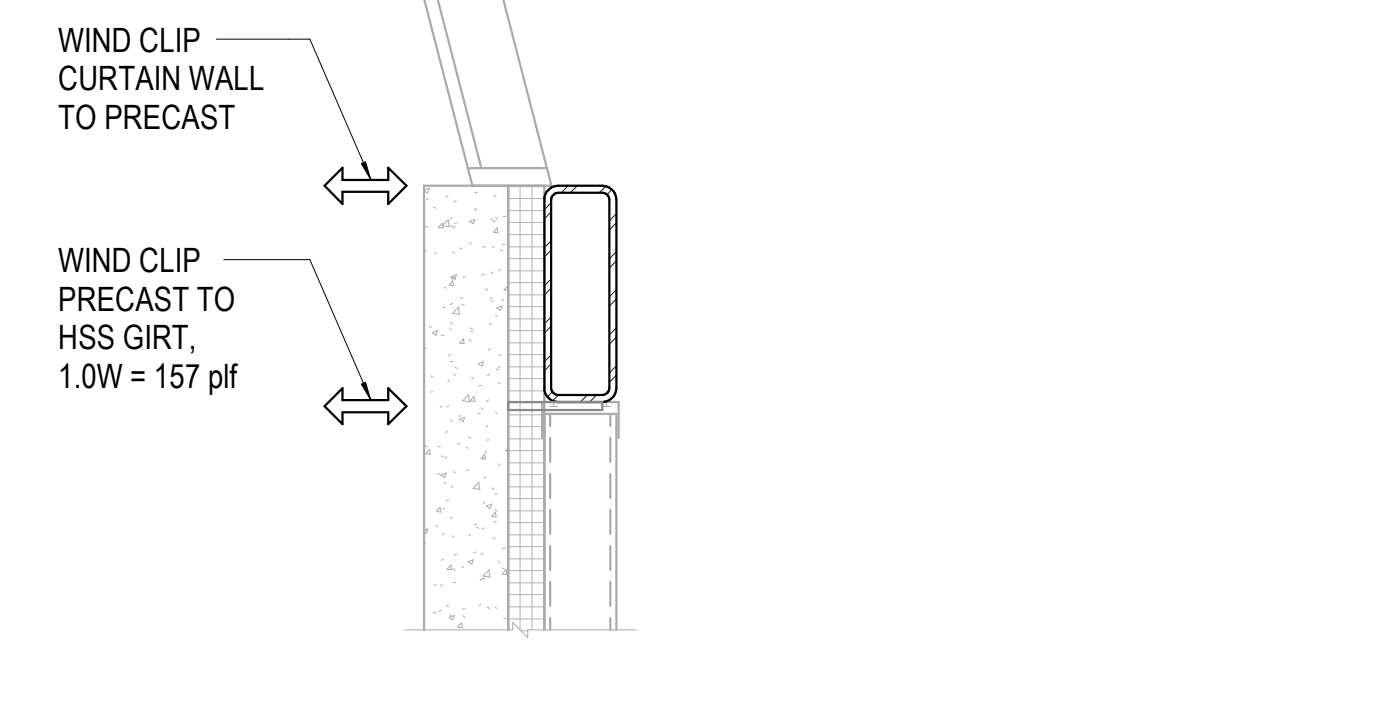




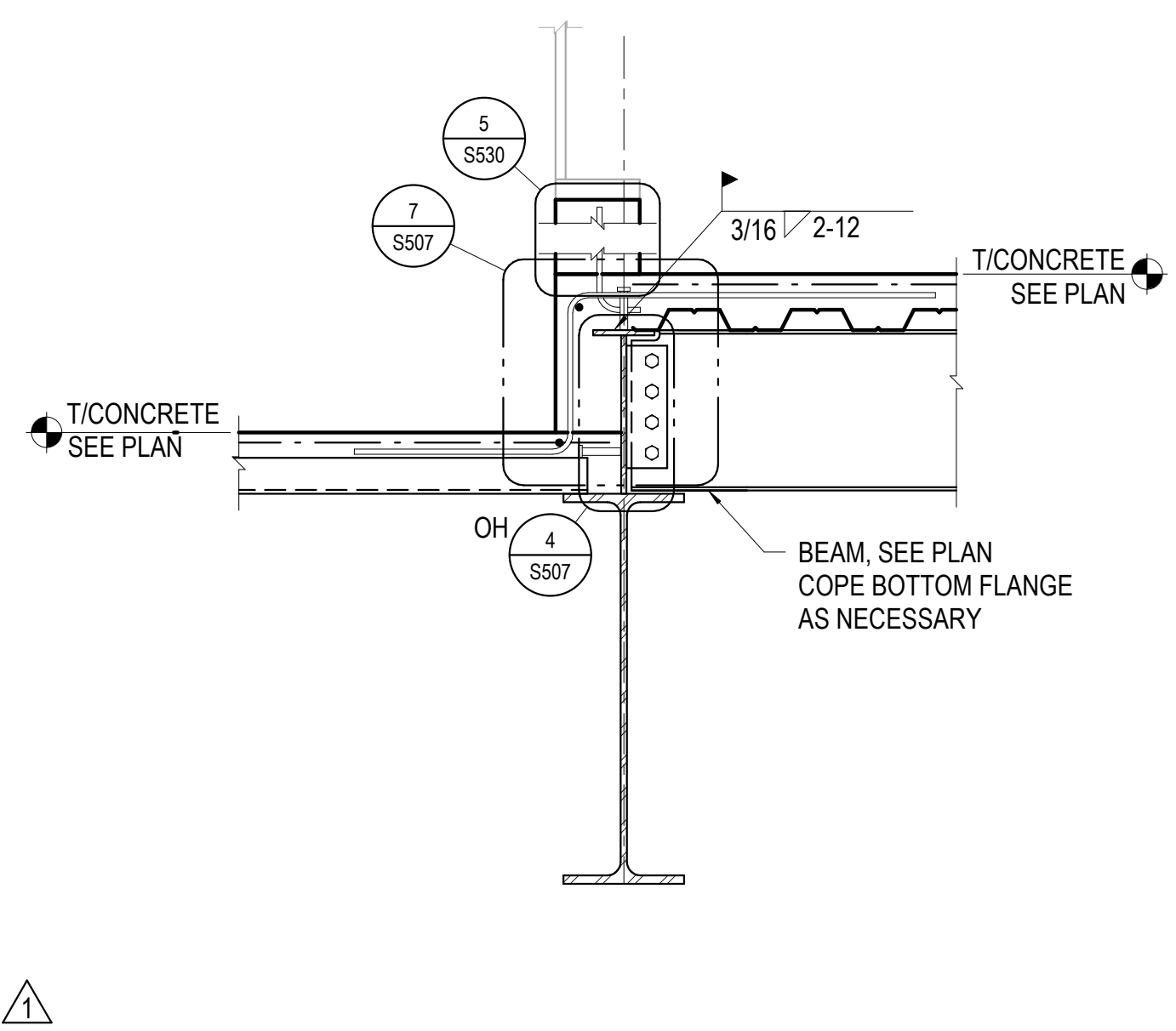
7 3/4" = 1'-0" HANGING GIRT @ CANTILEVER



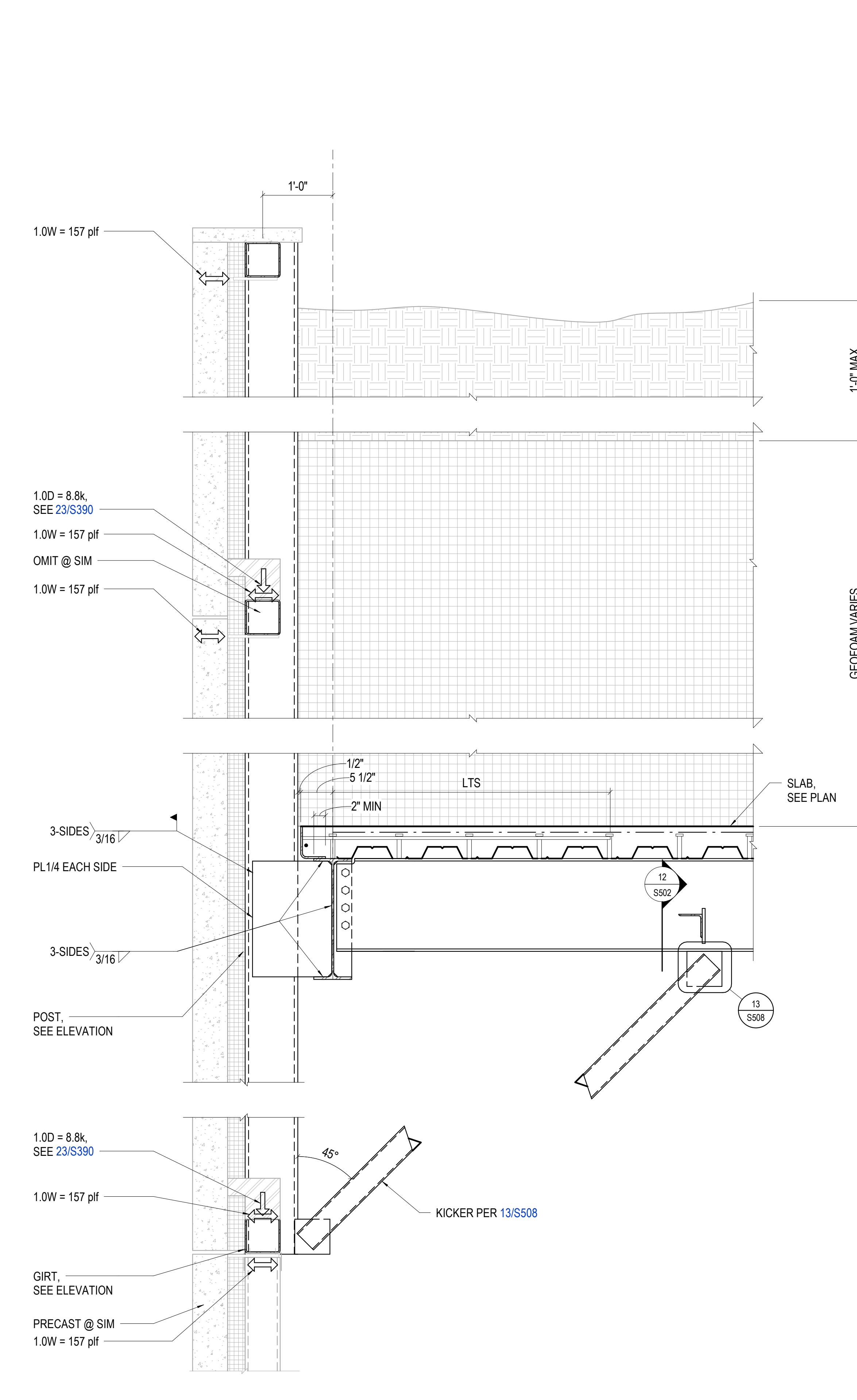
12 3/4" = 1'-0" LONG GIRTS SUPPORT



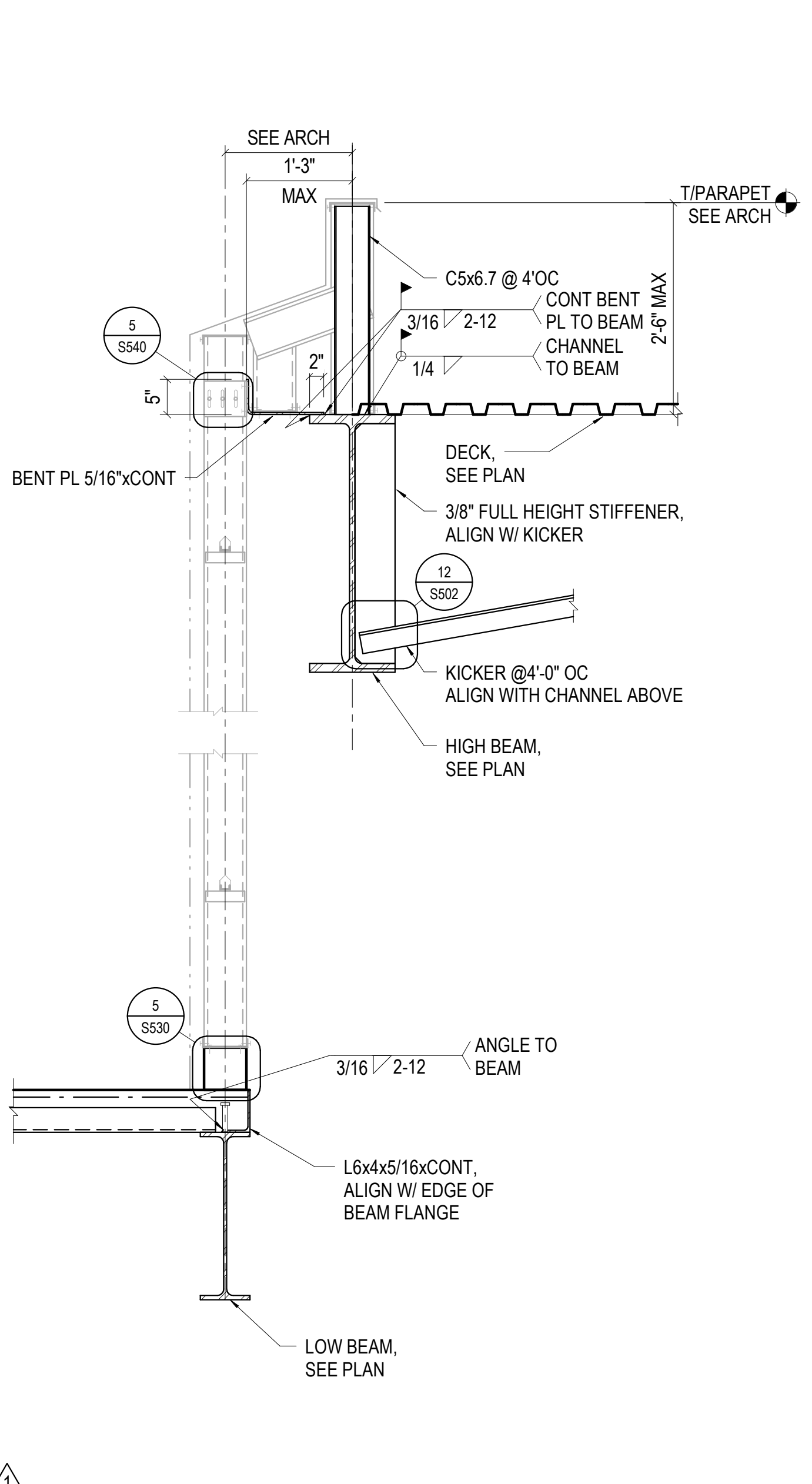
16 3/4" = 1'-0" SLANTED GIRTS COL CONN SECTION



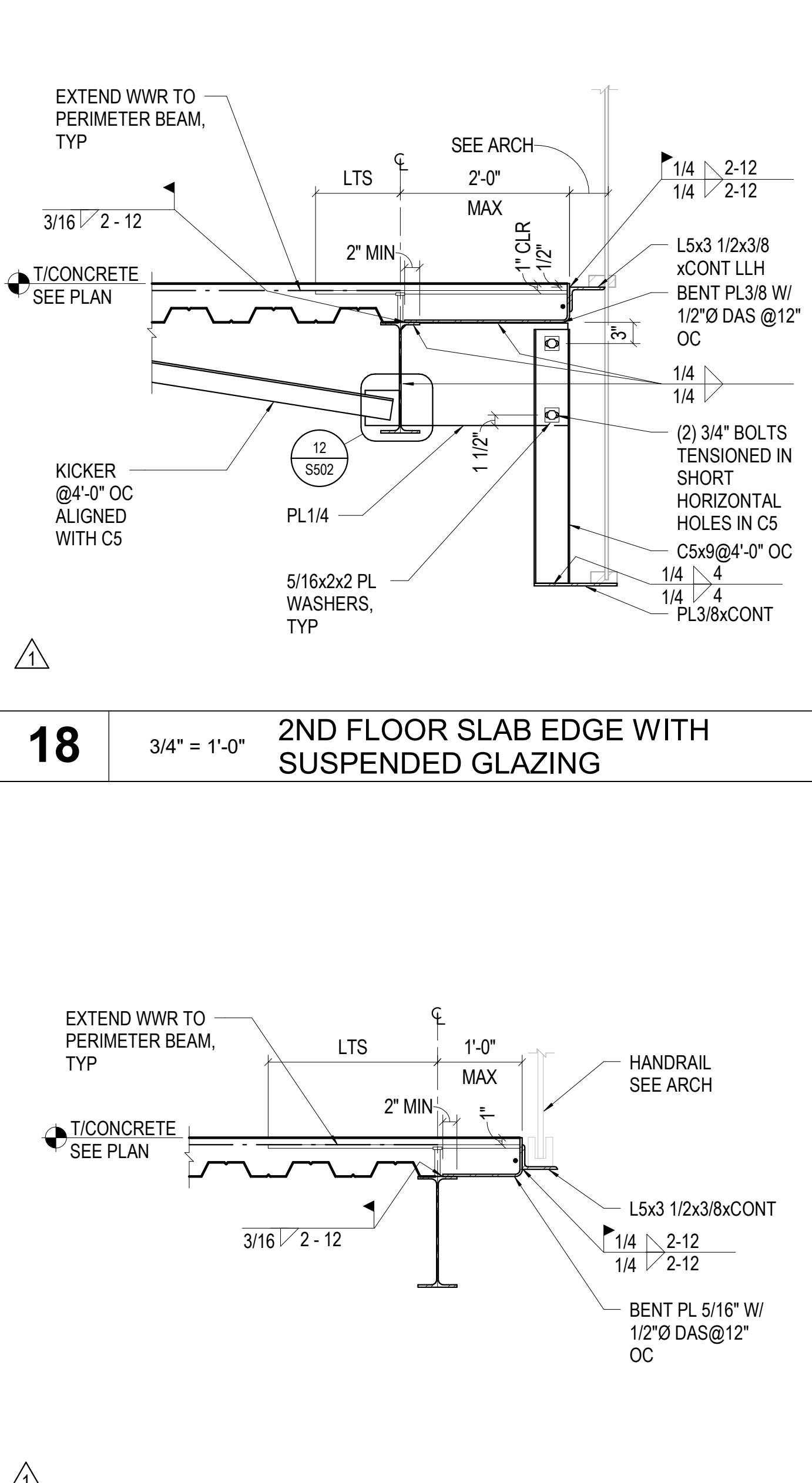
17 3/4" = 1'-0" W SHAPE WITH BUILT UP PLATE



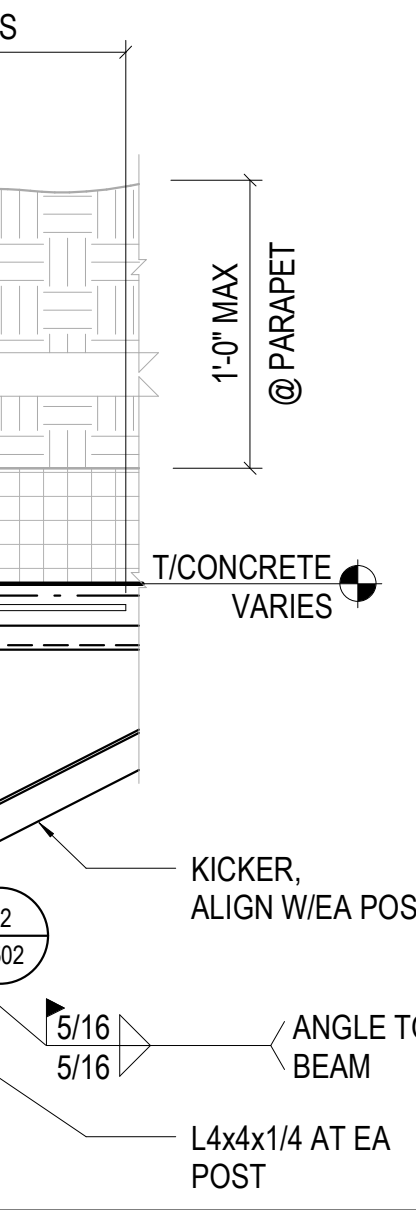
10 1'-0" HANGING GIRTS SLAB EDGE



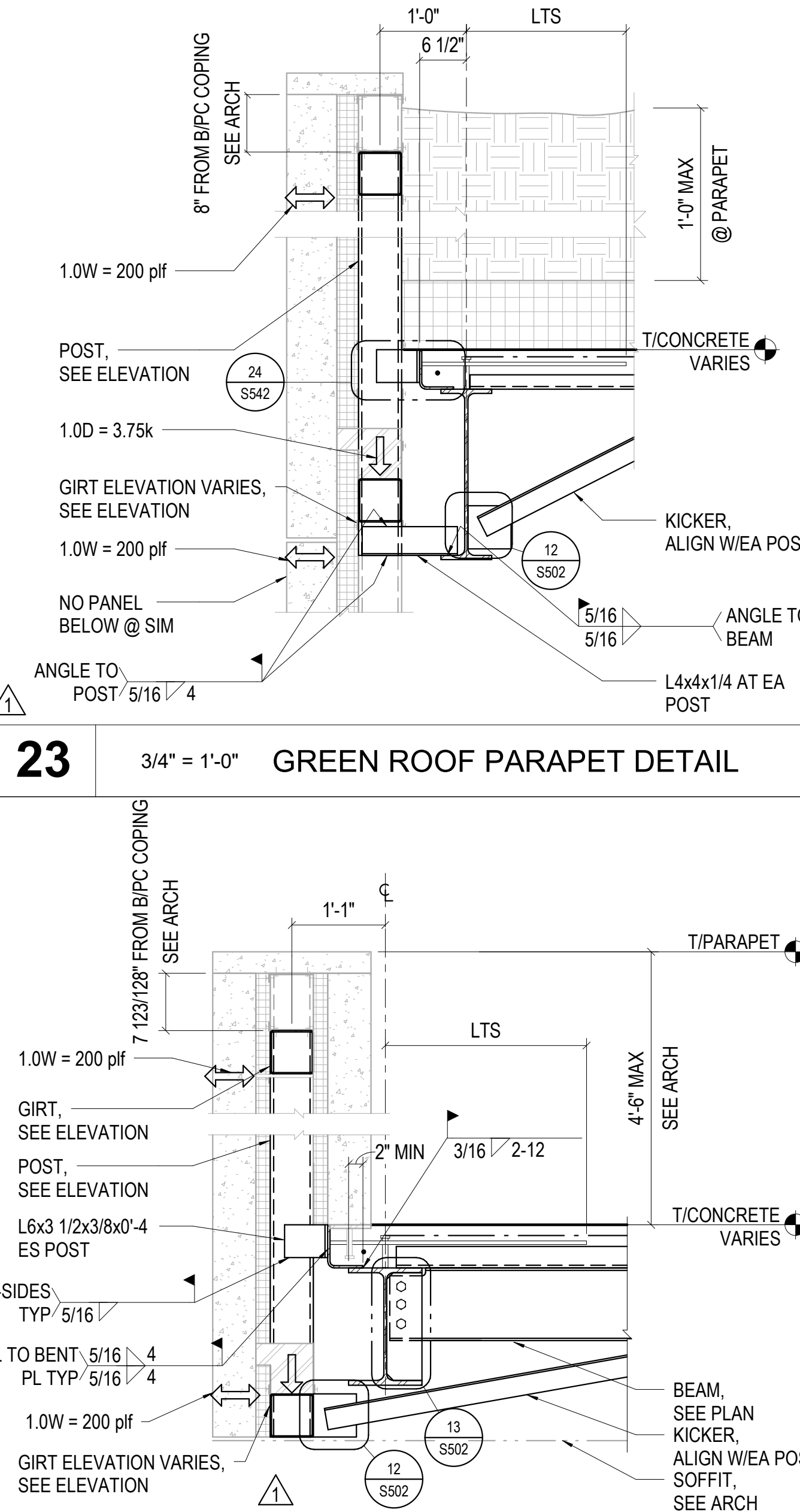
14 3/4" = 1'-0" UPPER ROOF PARAPET AT SOFFIT



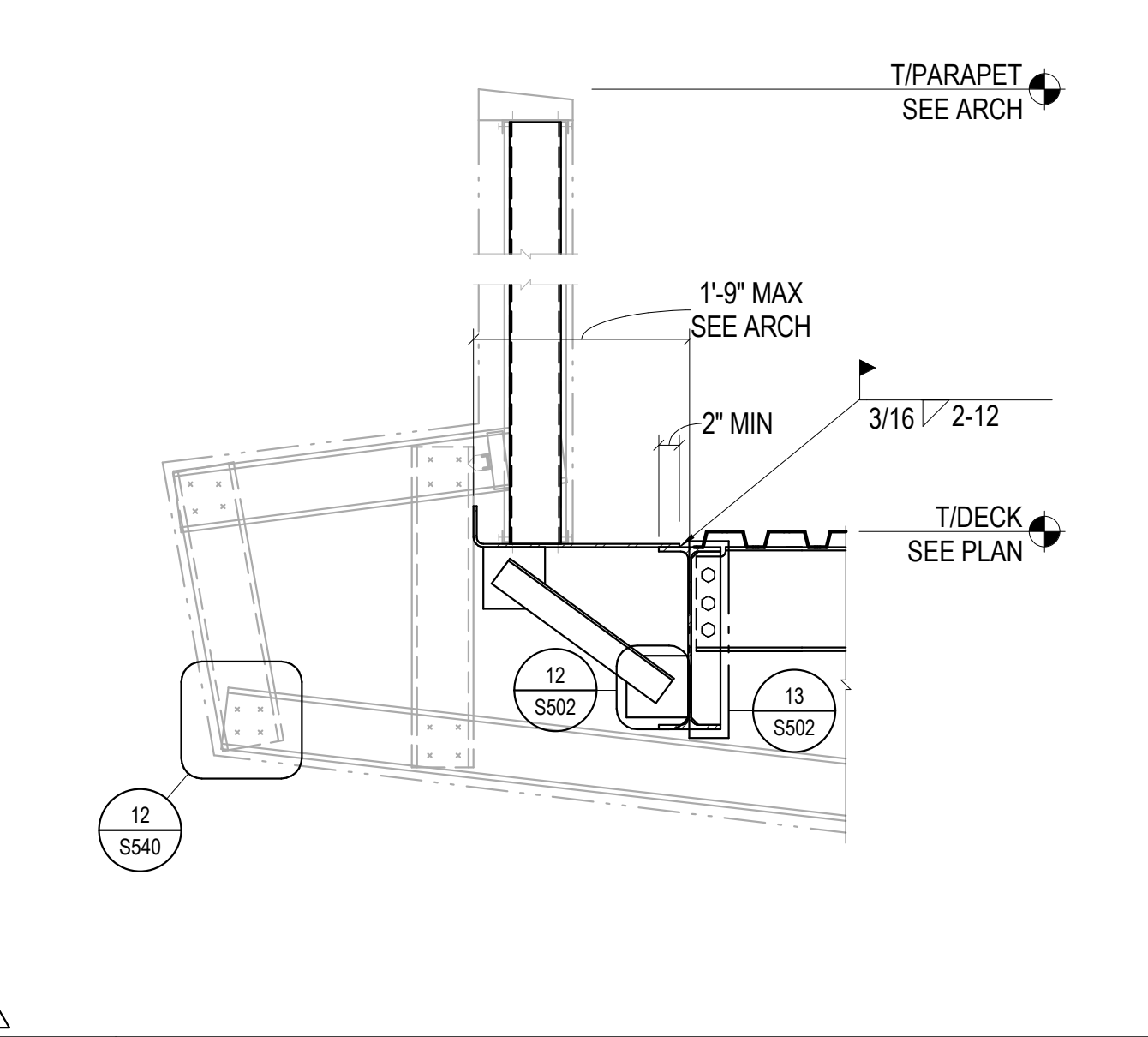
18 3/4" = 1'-0" 2ND FLOOR SLAB EDGE WITH SUSPENDED GLAZING



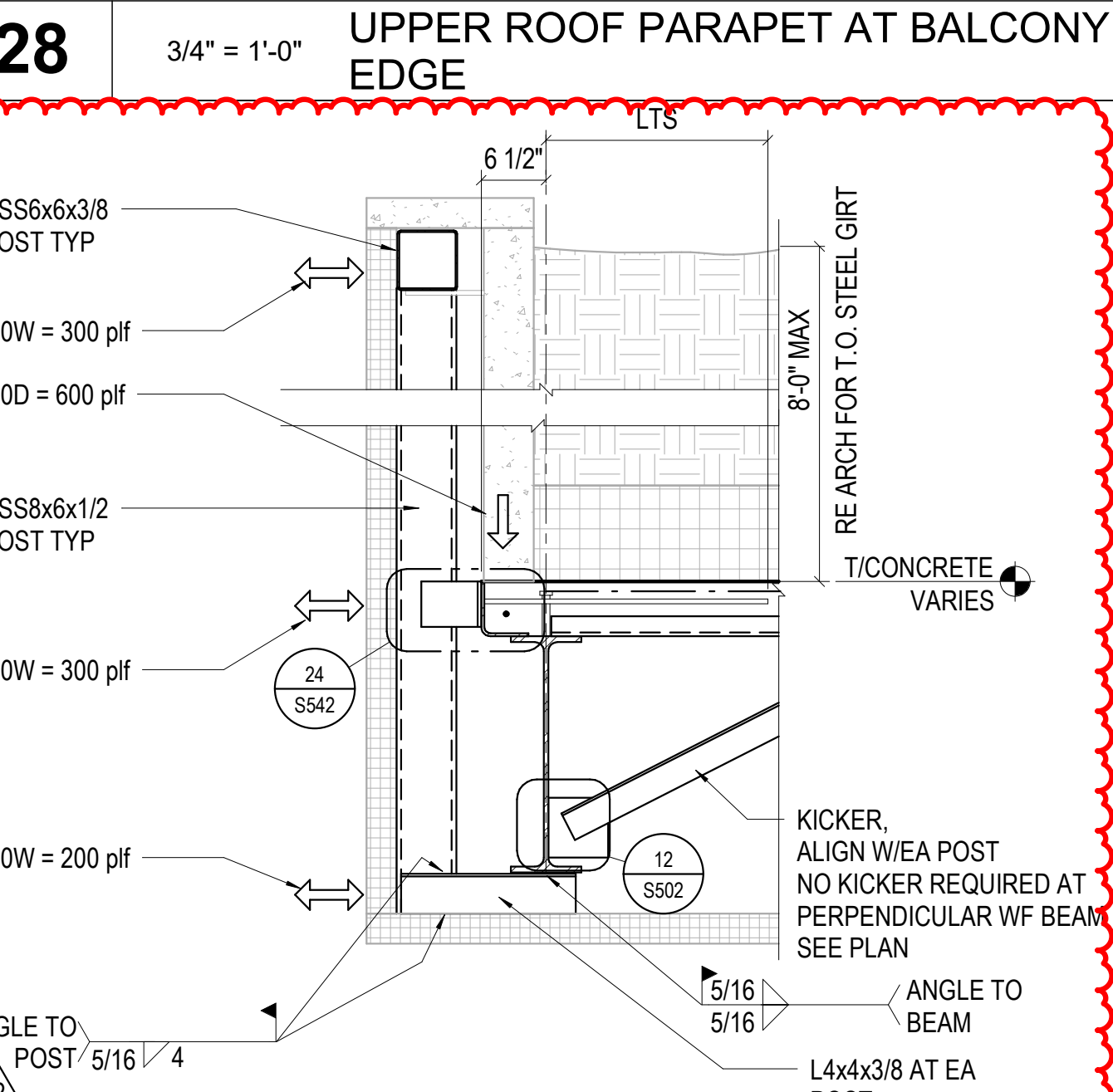
19 3/4" = 1'-0" HANDRAIL AT LEVEL 4



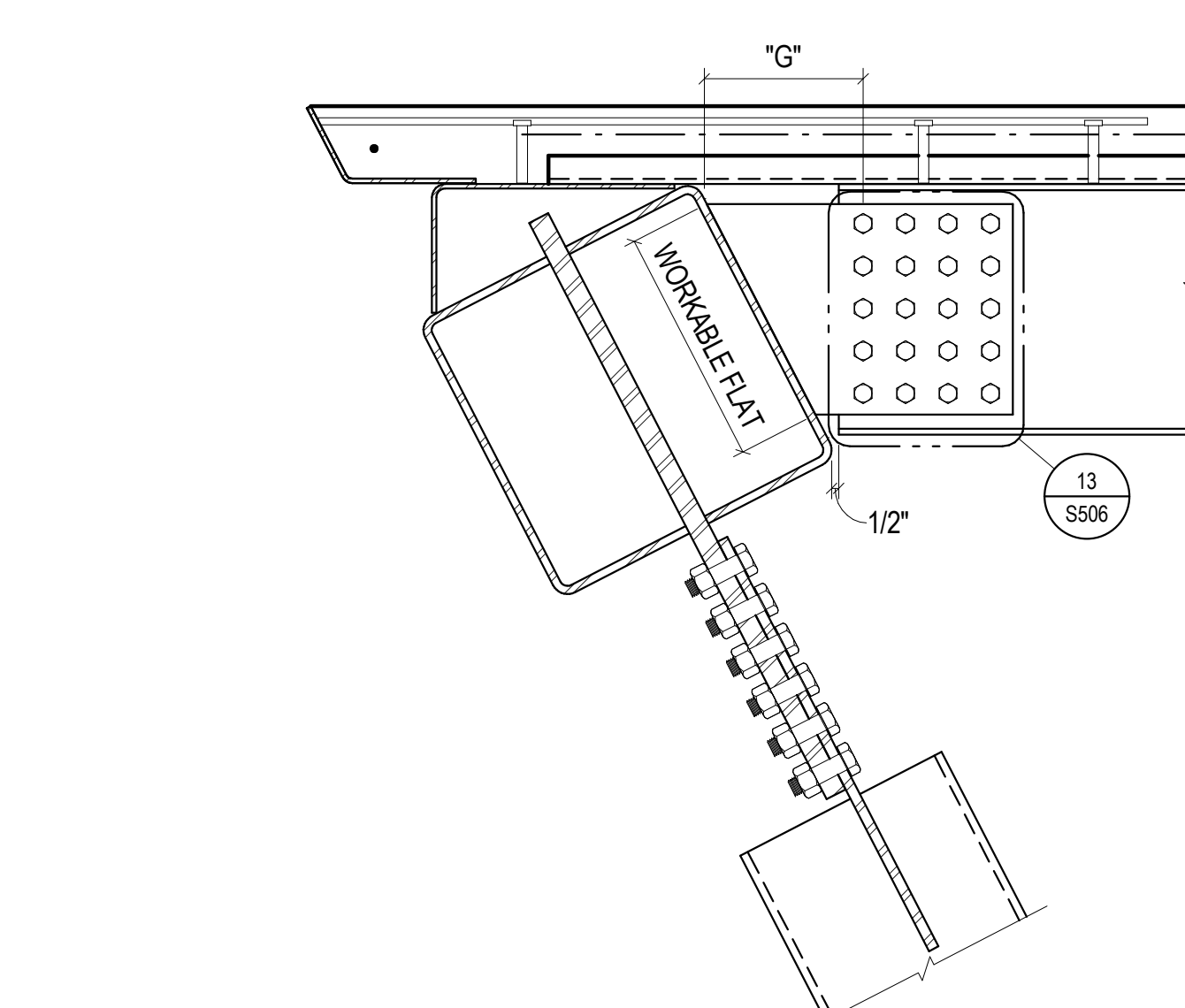
23 3/4" = 1'-0" GREEN ROOF PARAPET DETAIL



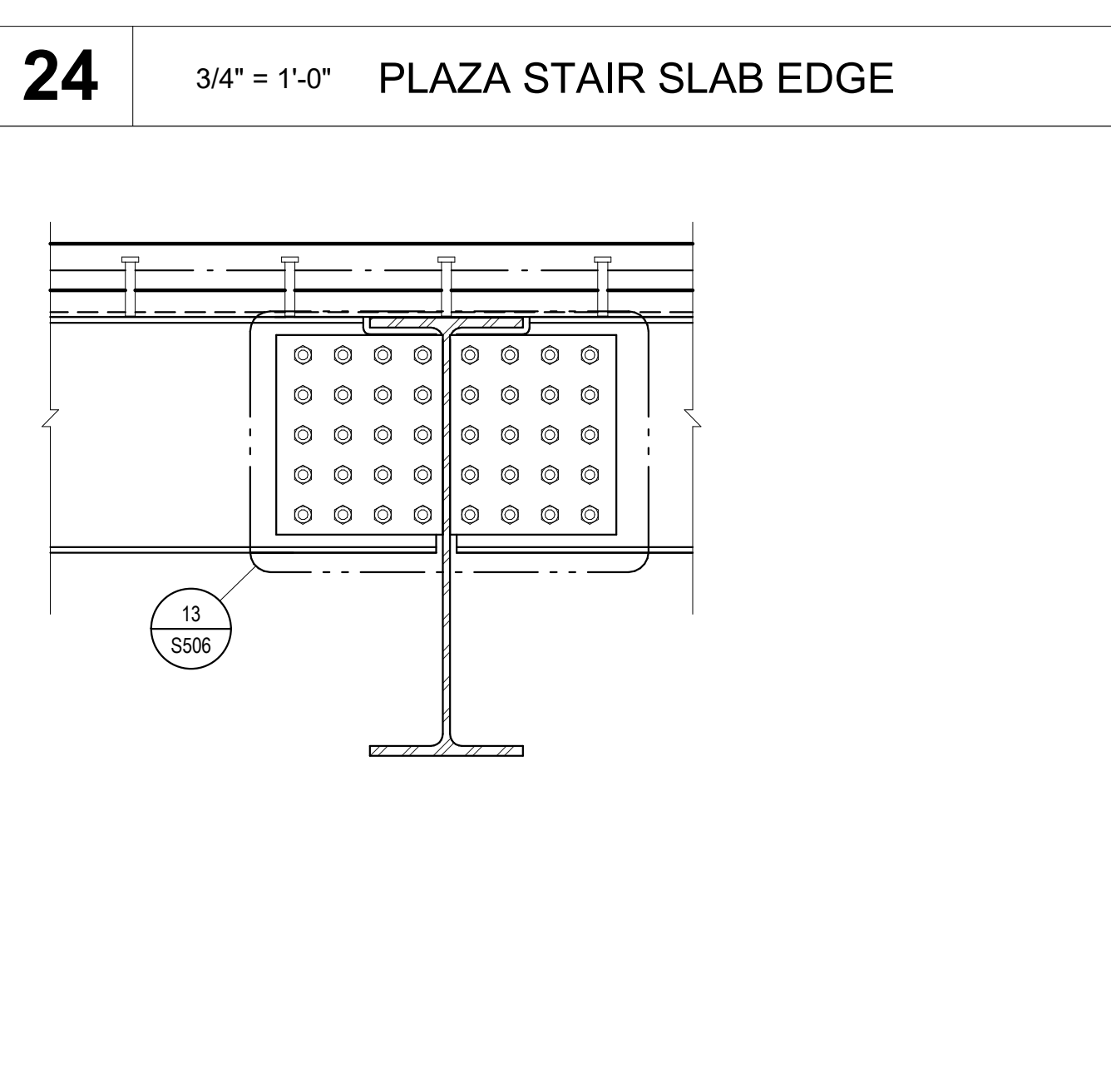
27 1'-0" UPPER ROOF PARAPET AT BALCONY SIDE



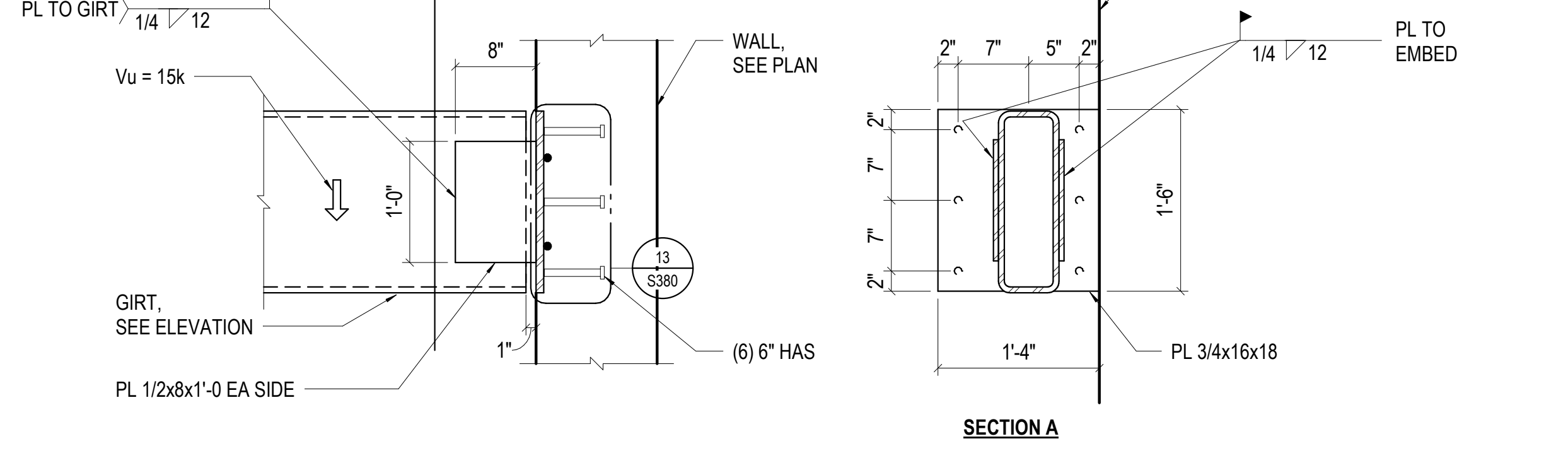
28 3/4" = 1'-0" UPPER ROOF PARAPET AT BALCONY EDGE



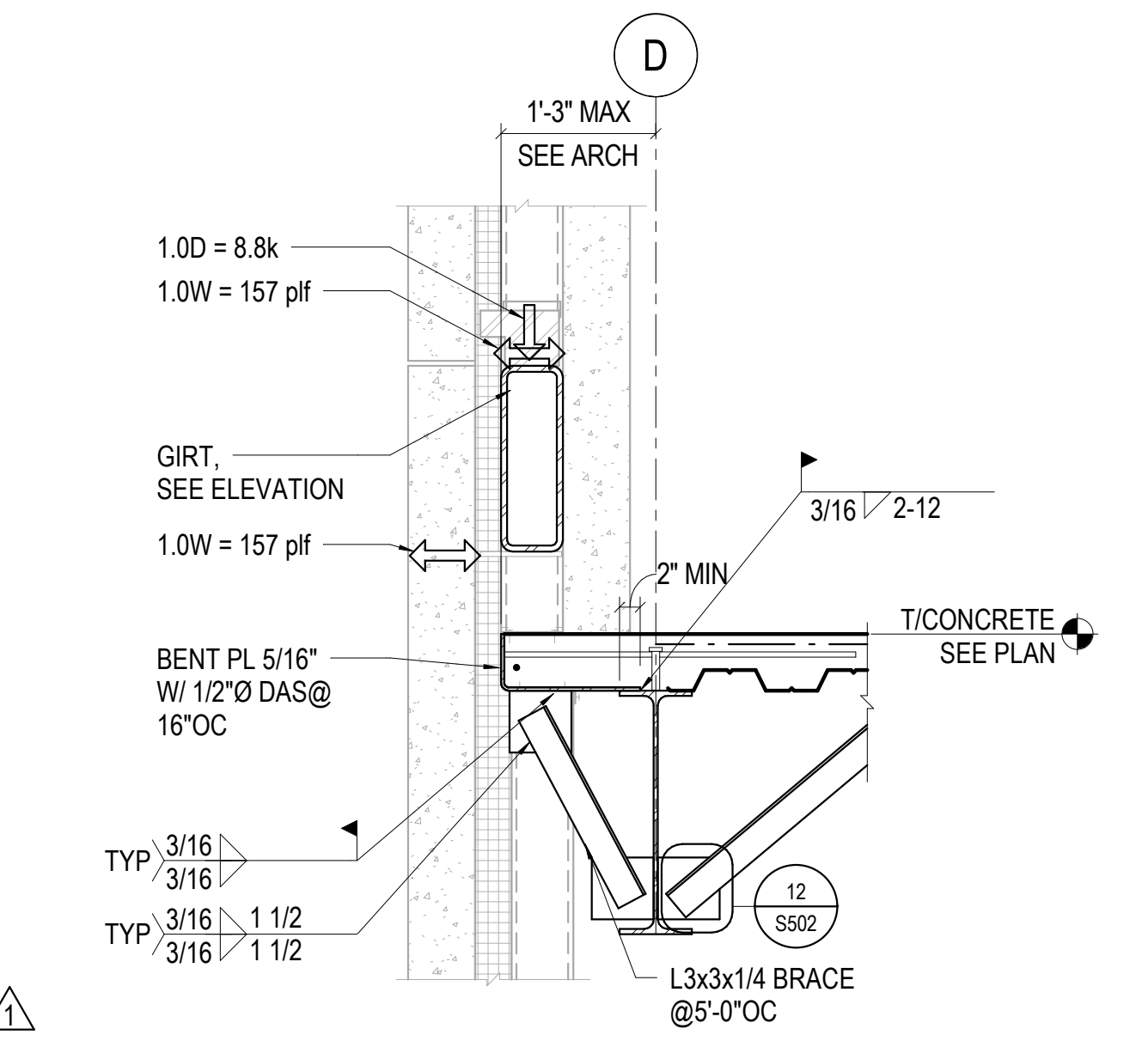
29 3/4" = 1'-0" OCULUS PARAPET DETAIL



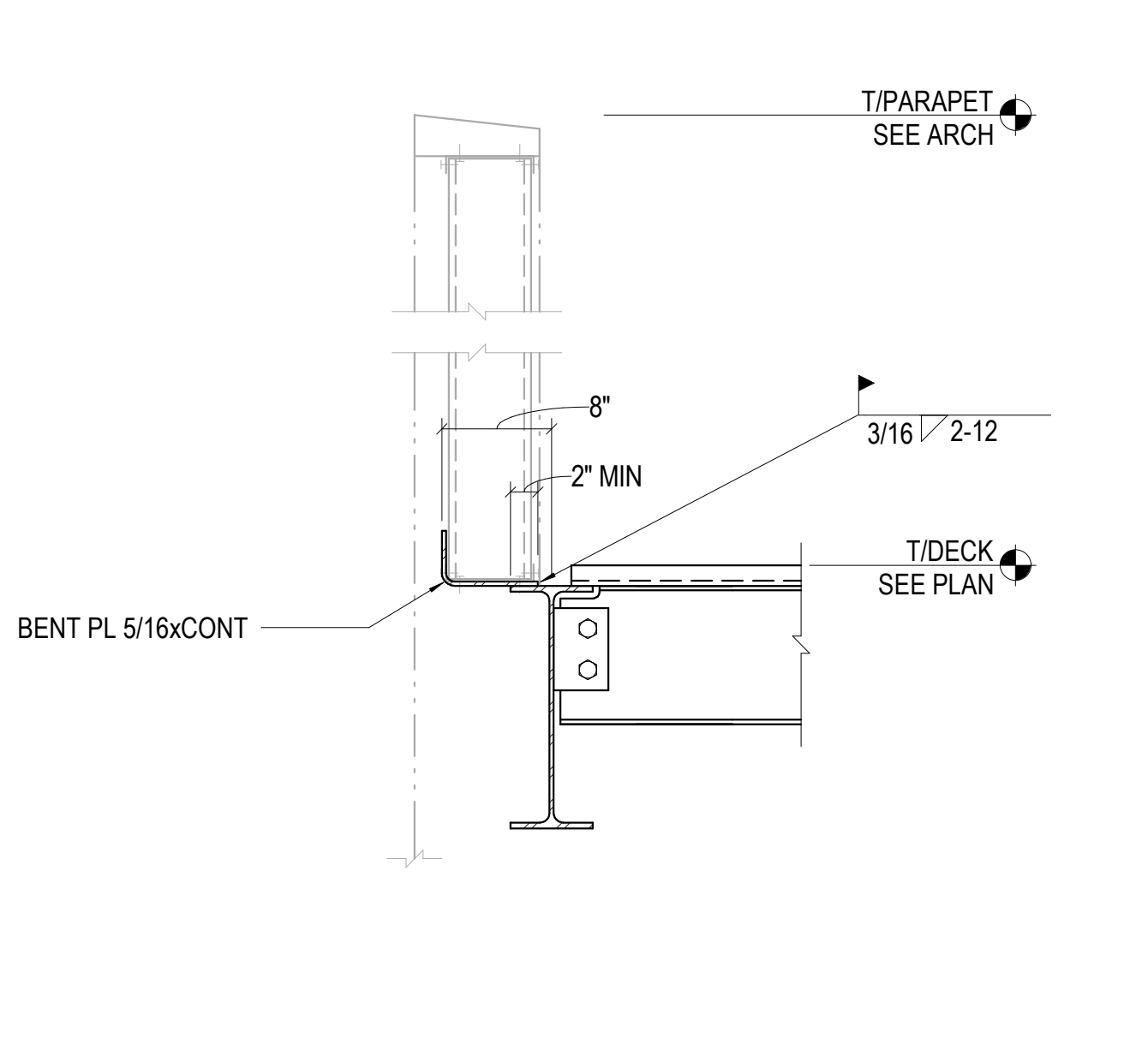
24 3/4" = 1'-0" PLAZA STAIR SLAB EDGE



26 1'-0" GIRTS CONNECTION DETAIL



22 3/4" = 1'-0" GIRTS DETAIL

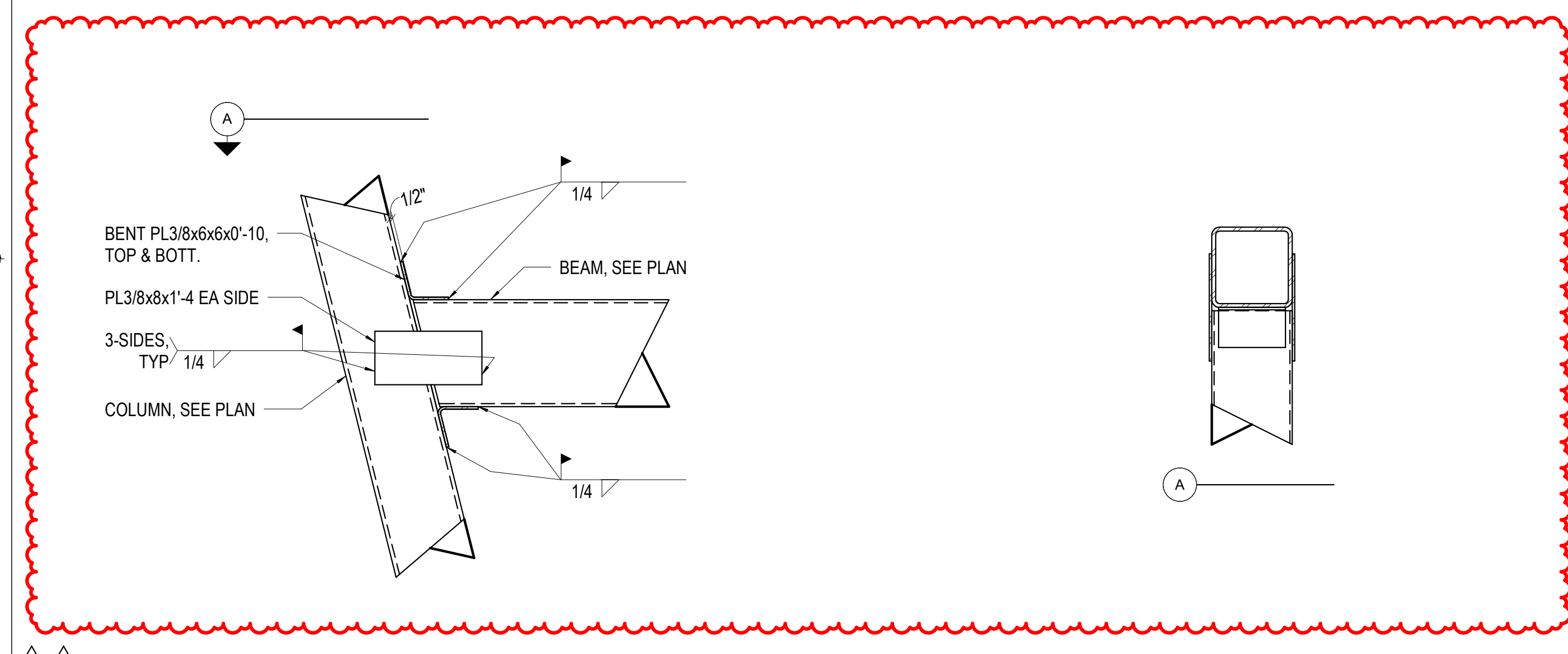


25 1'-0" UPPER ROOF PARAPET AT BALCONY CORNER

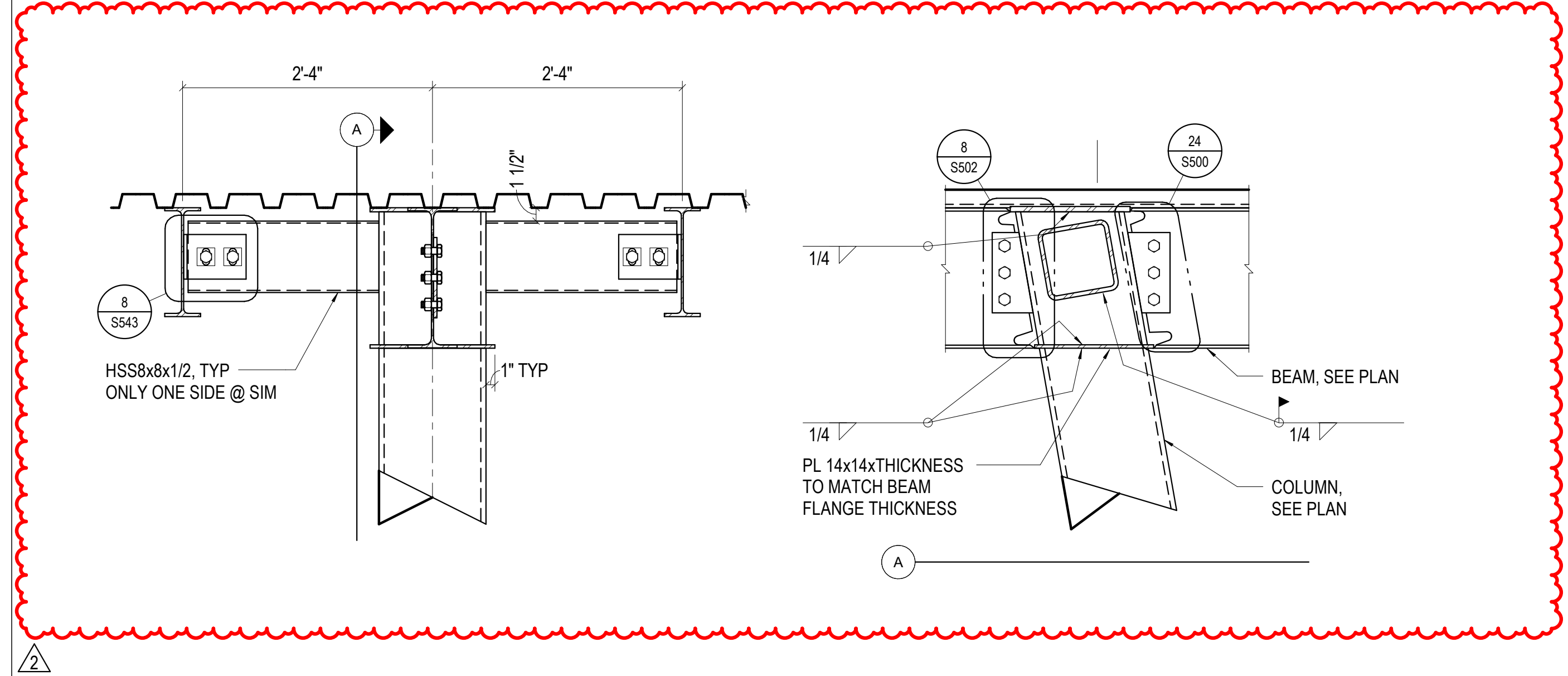
30 1'-0" SUPER TRUSS DRAG BEAM CONNECTION

15 3/4" = 1'-0" SLANTED GIRTS COL CONN

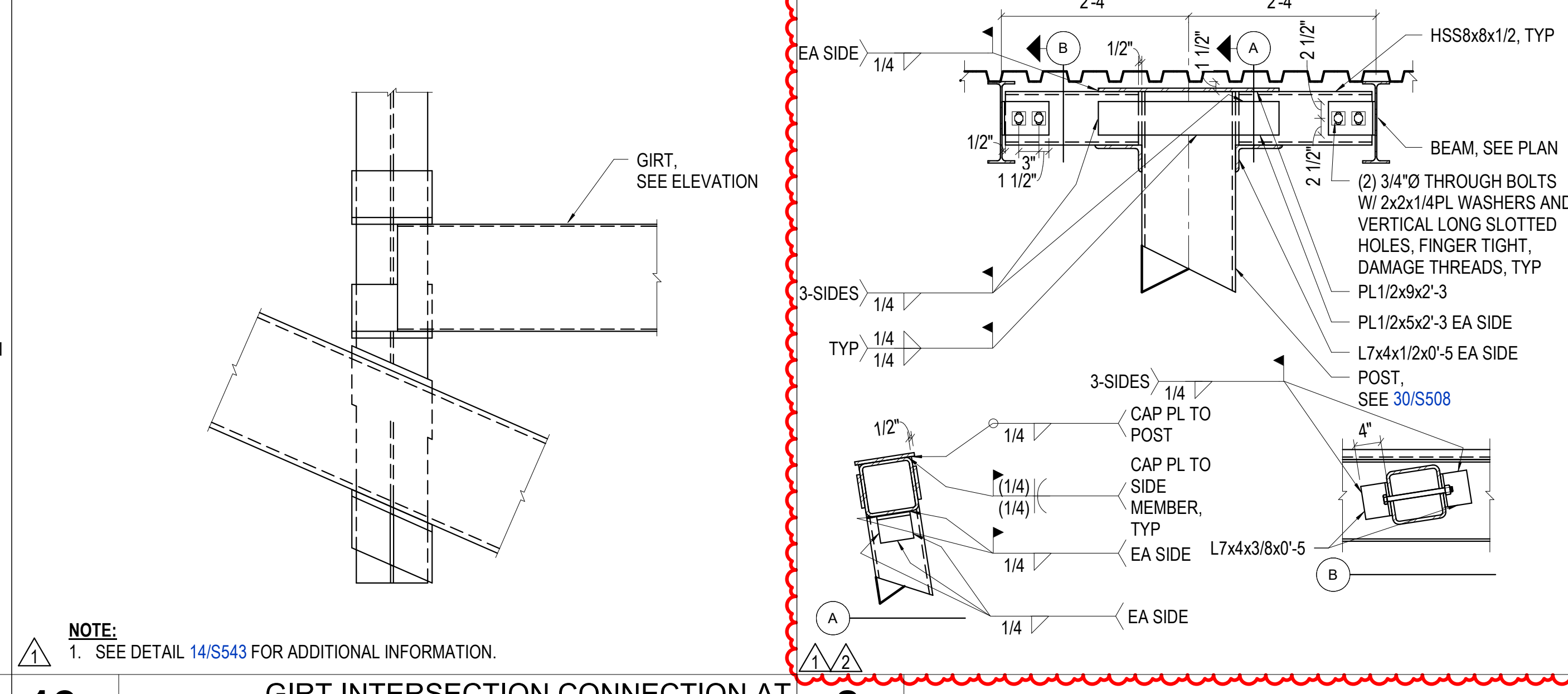
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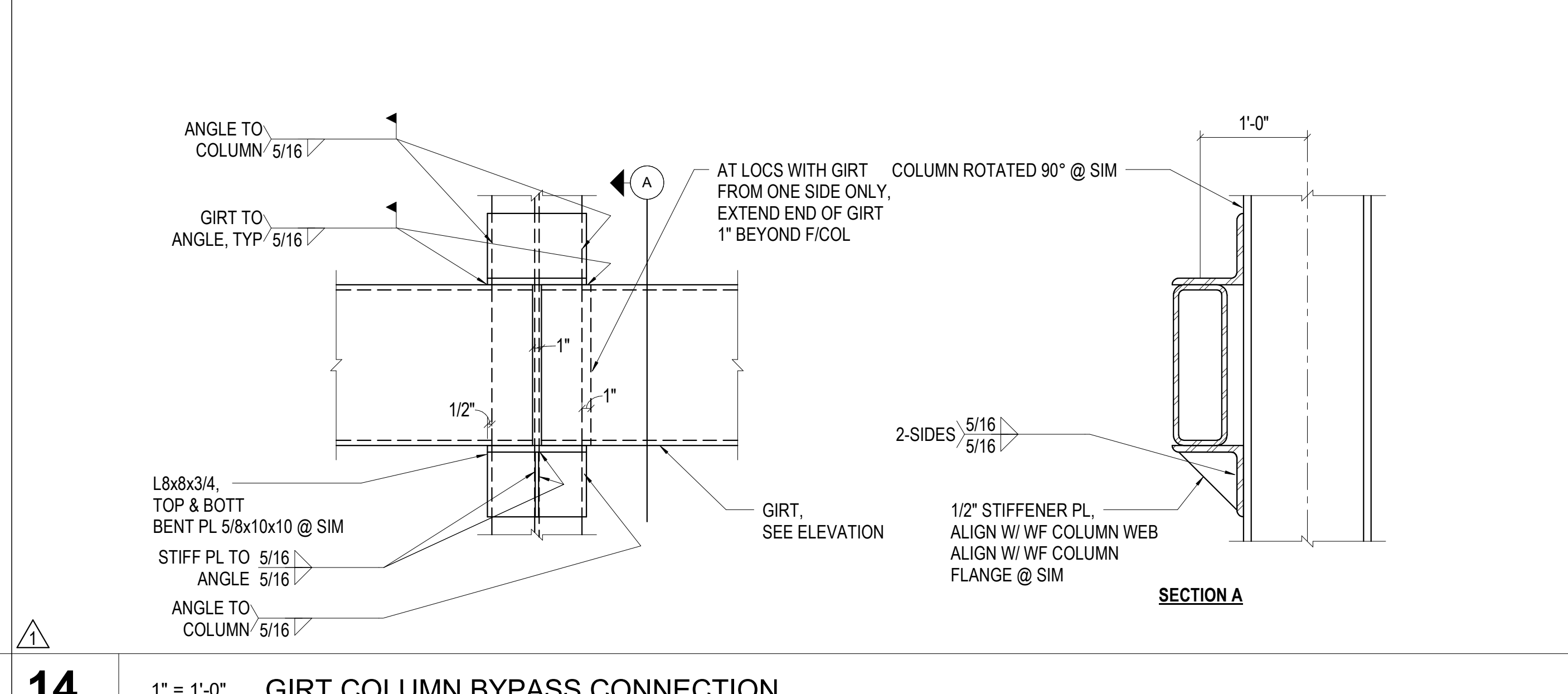
11 3/4" = 1'-0" GRAVITY FIN COLUMN CONNECTION



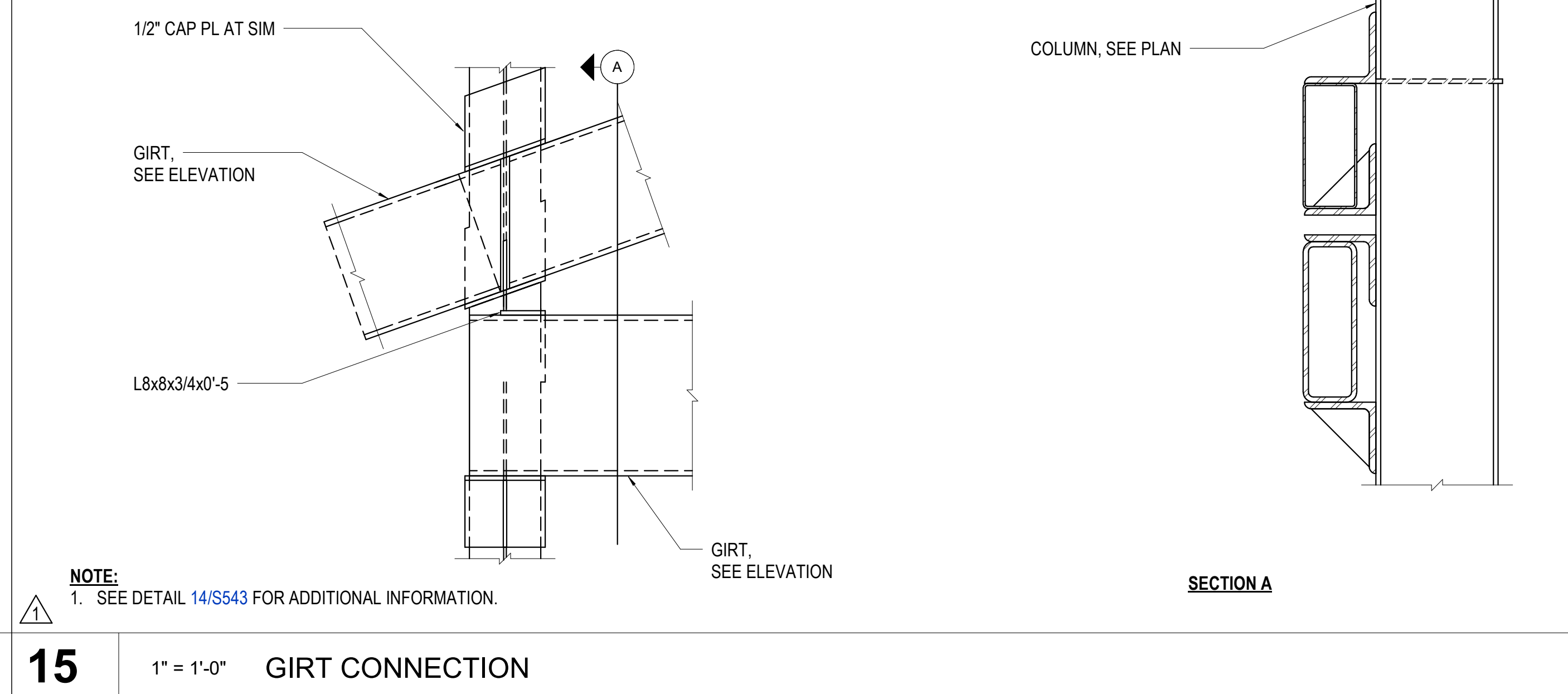
12 1" = 1'-0" HSS COLUMN TO WF BEAM CONN



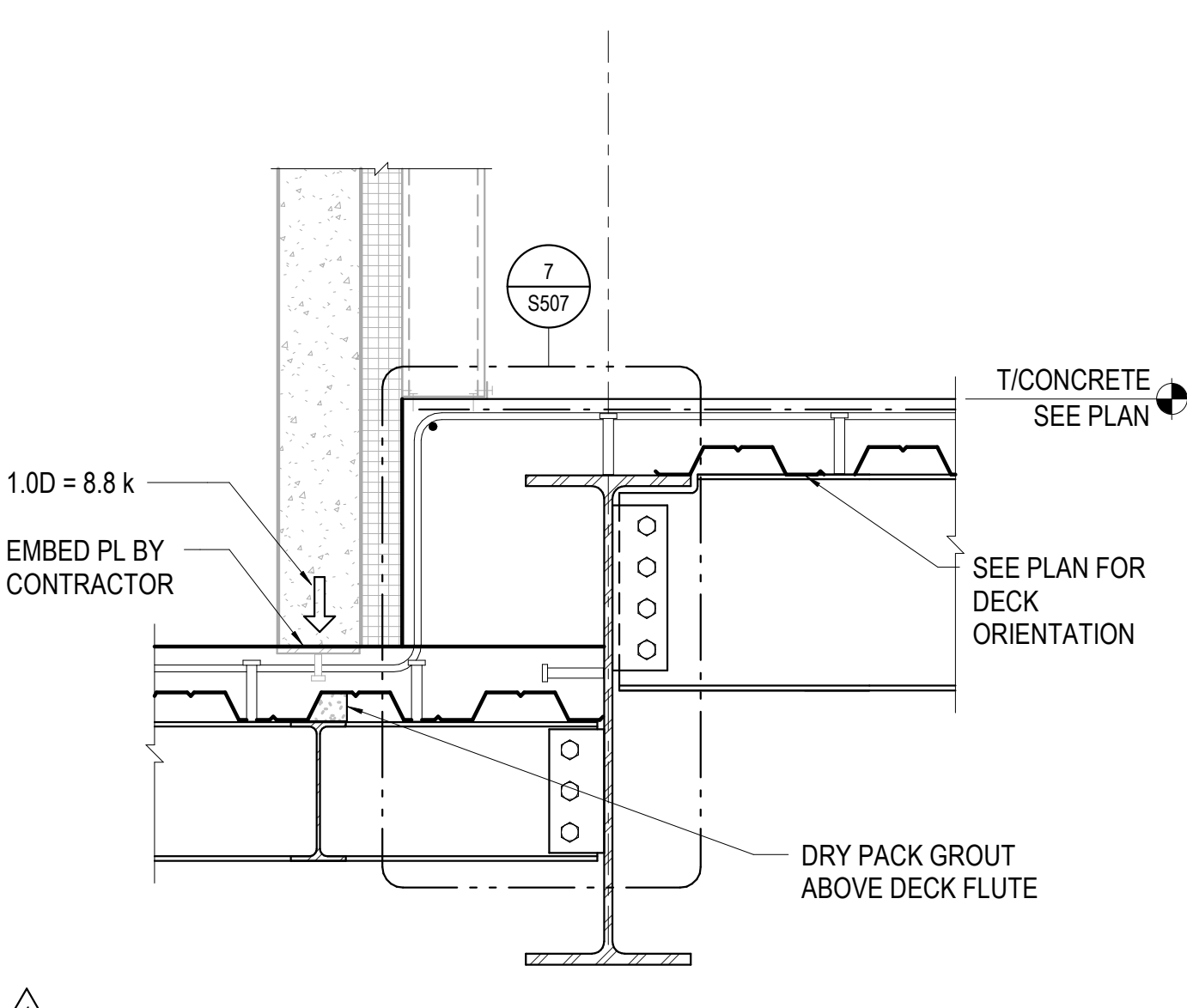
8 3/4" = 1'-0" FIN POST TOP CONNECTION



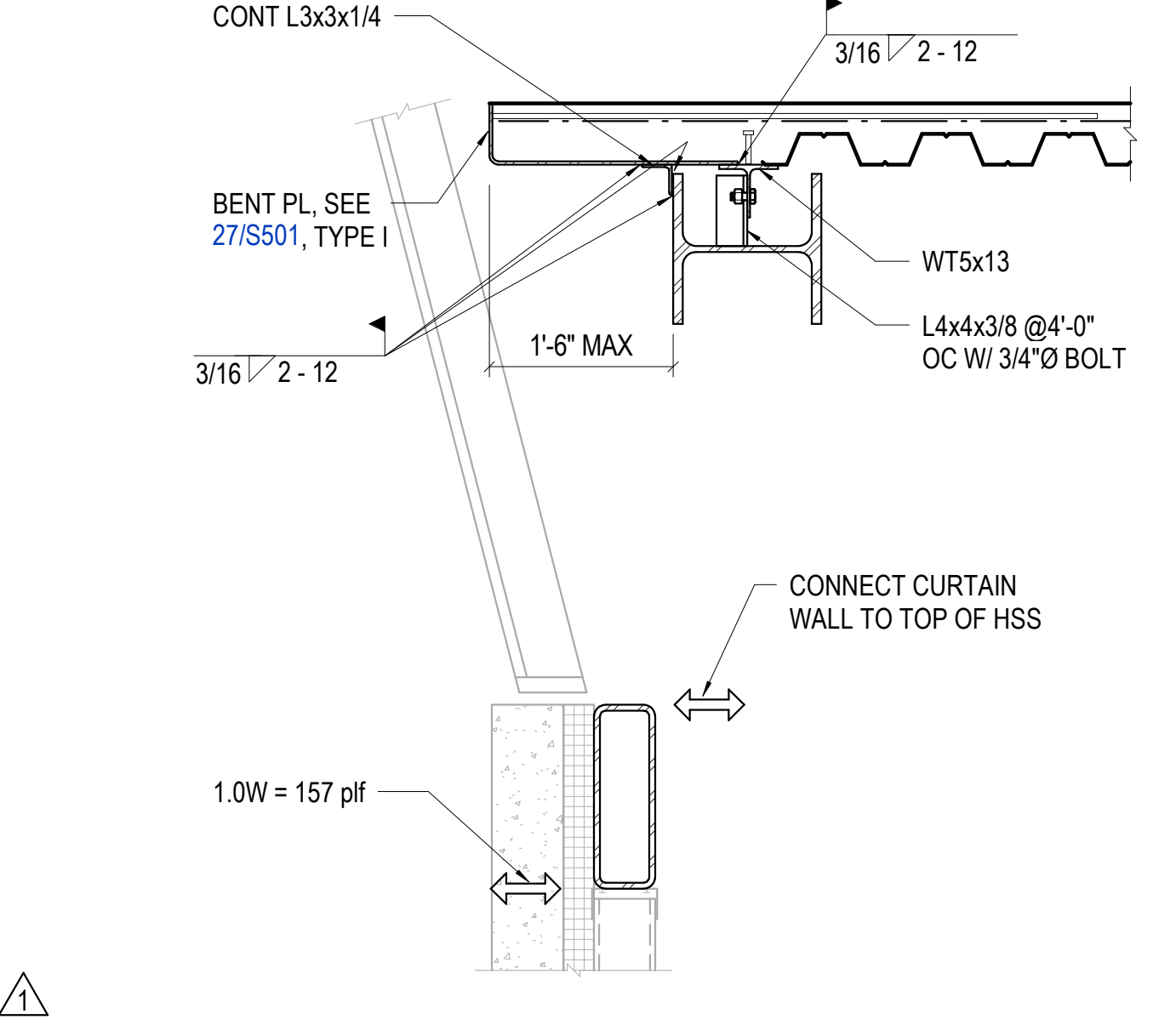
14 1" = 1'-0" GIRT COLUMN BYPASS CONNECTION



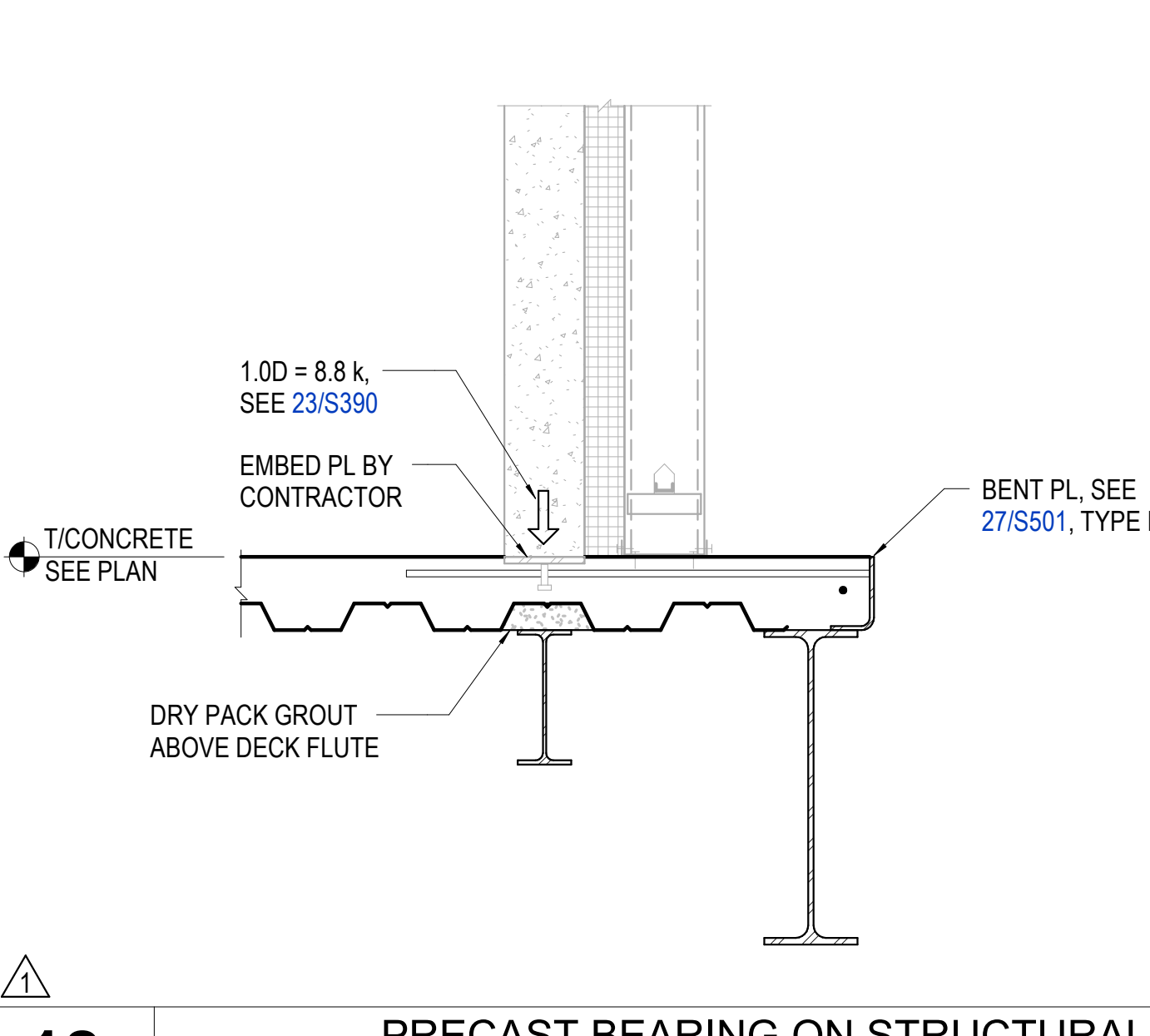
15 1" = 1'-0" GIRT CONNECTION



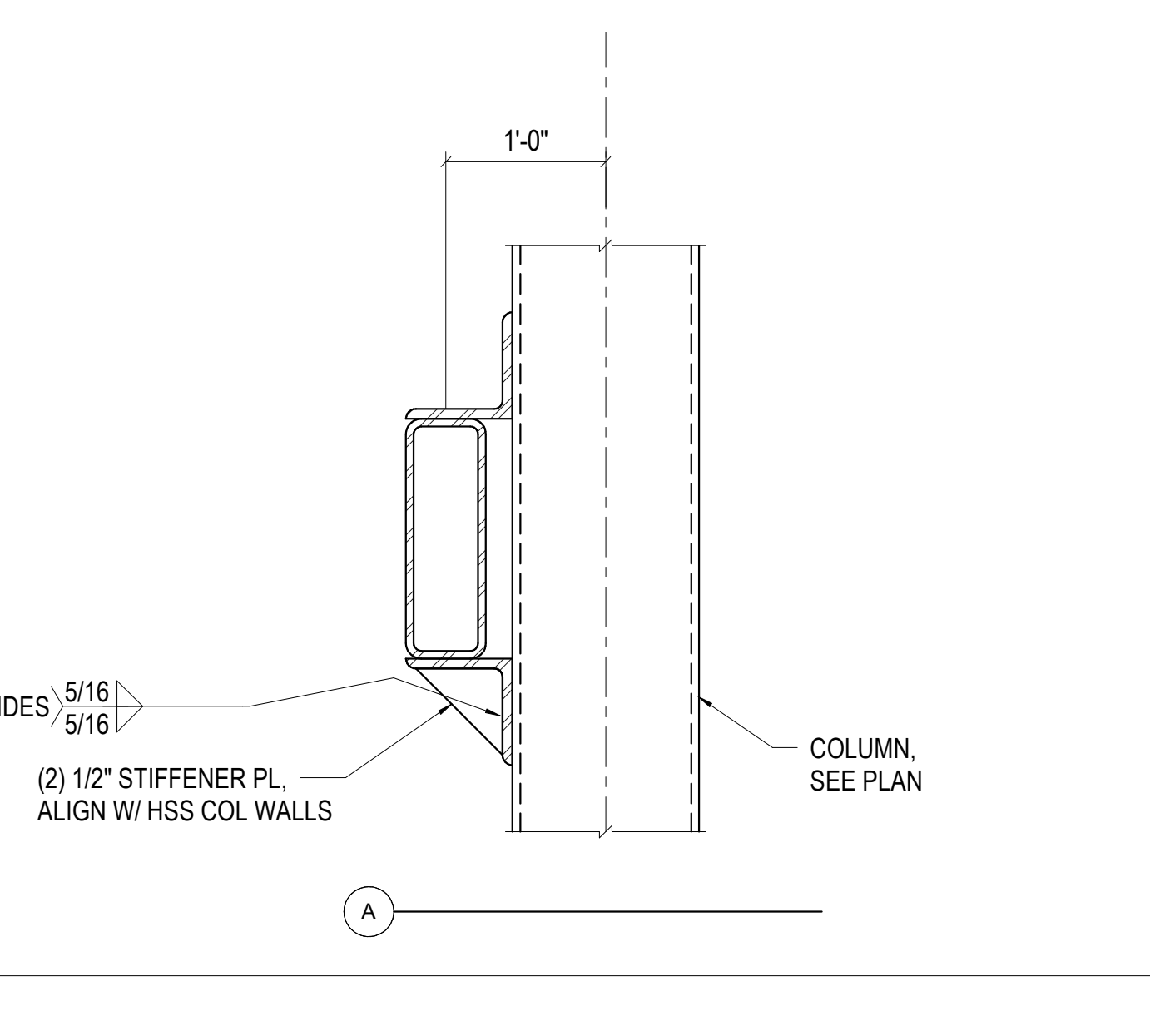
16 1" = 1'-0" SLAB STEP AT PRECAST



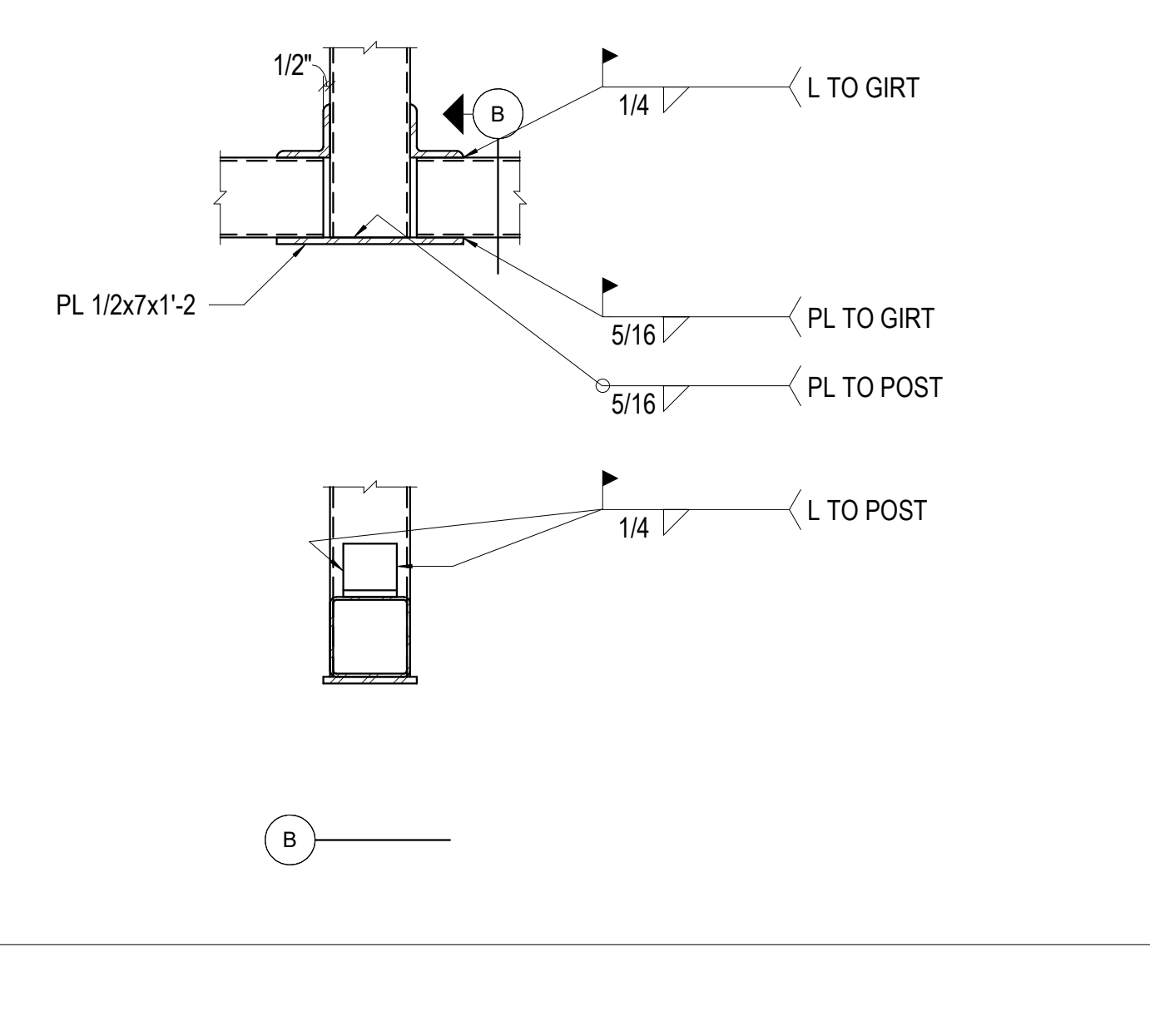
17 3/4" = 1'-0" SLANTED GIRT COL CONN SECTION AT SLAB EDGE



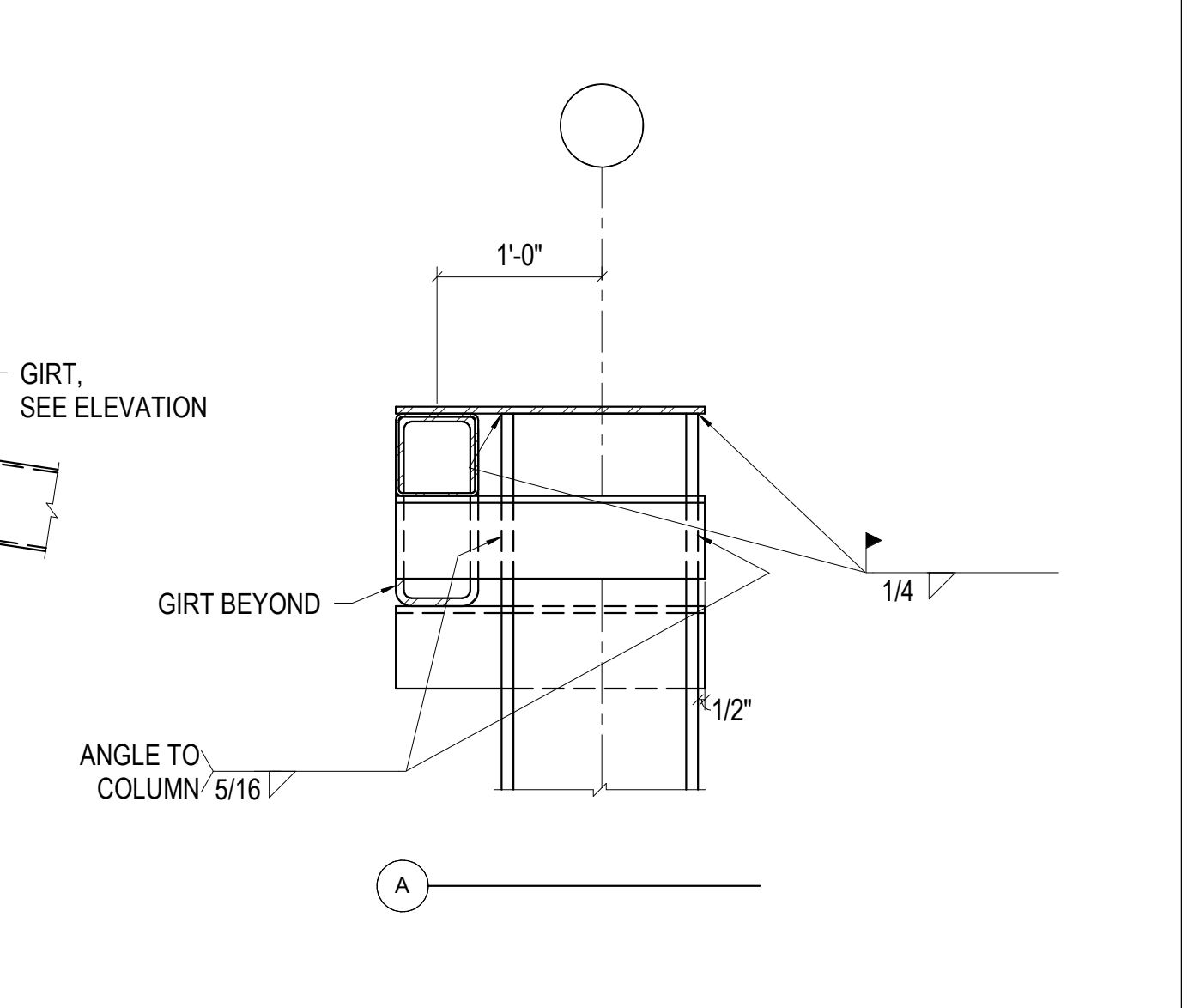
18 1" = 1'-0" PRECAST BEARING ON STRUCTURAL SLAB



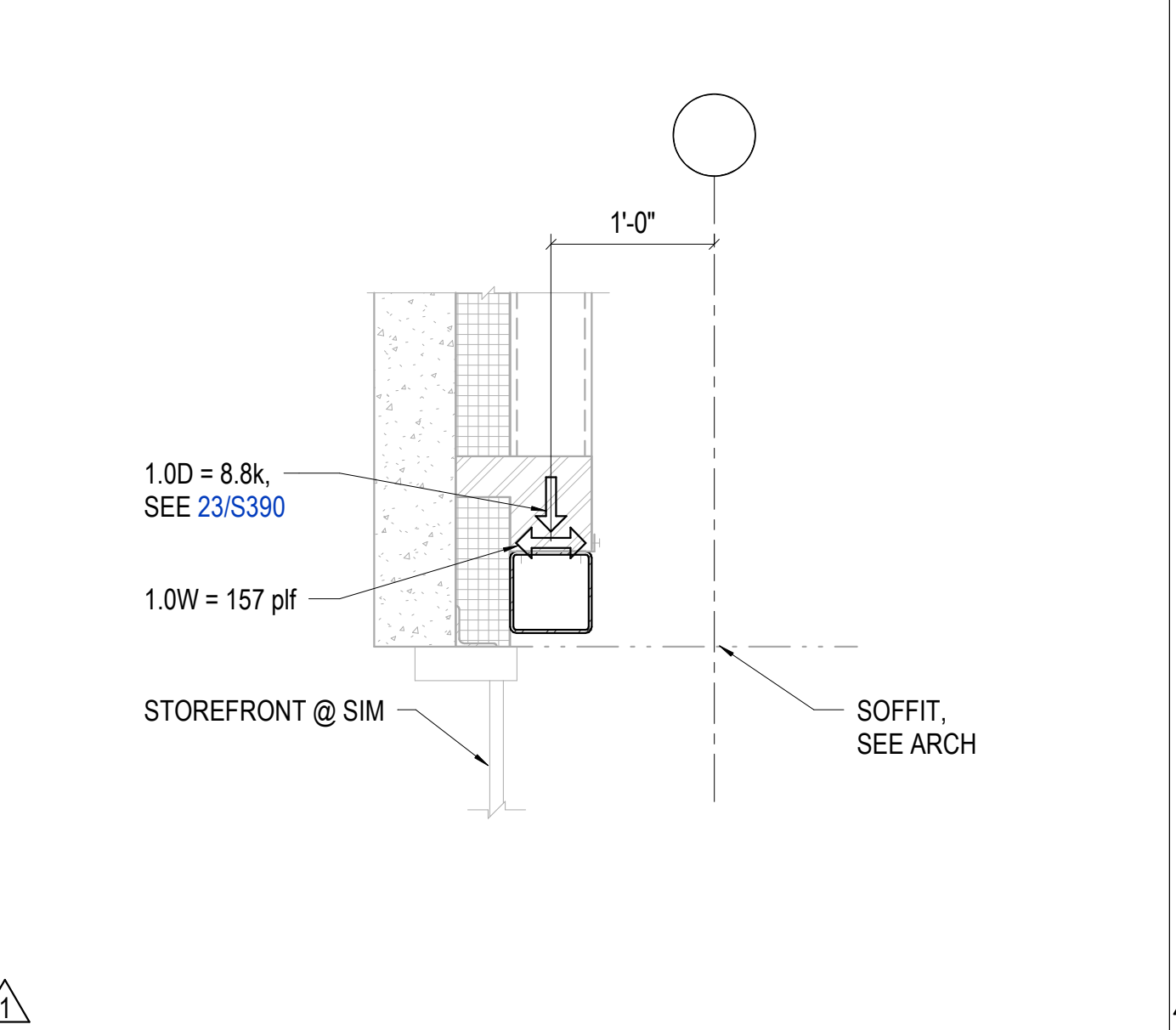
24 1" = 1'-0" GIRT TO HSS COLUMN CONNECTION



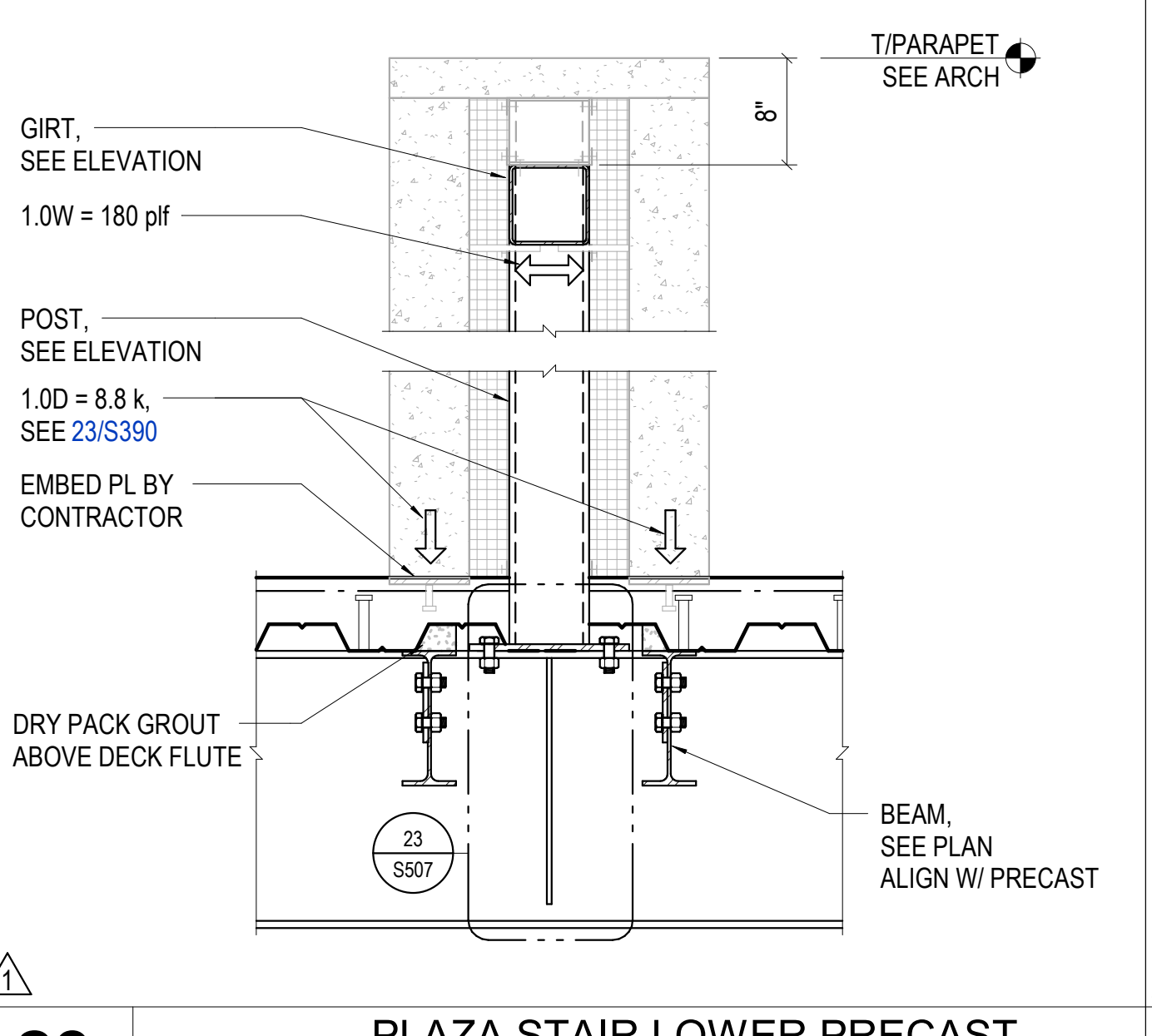
25 1" = 1'-0" TYP HSS GIRT TO POST



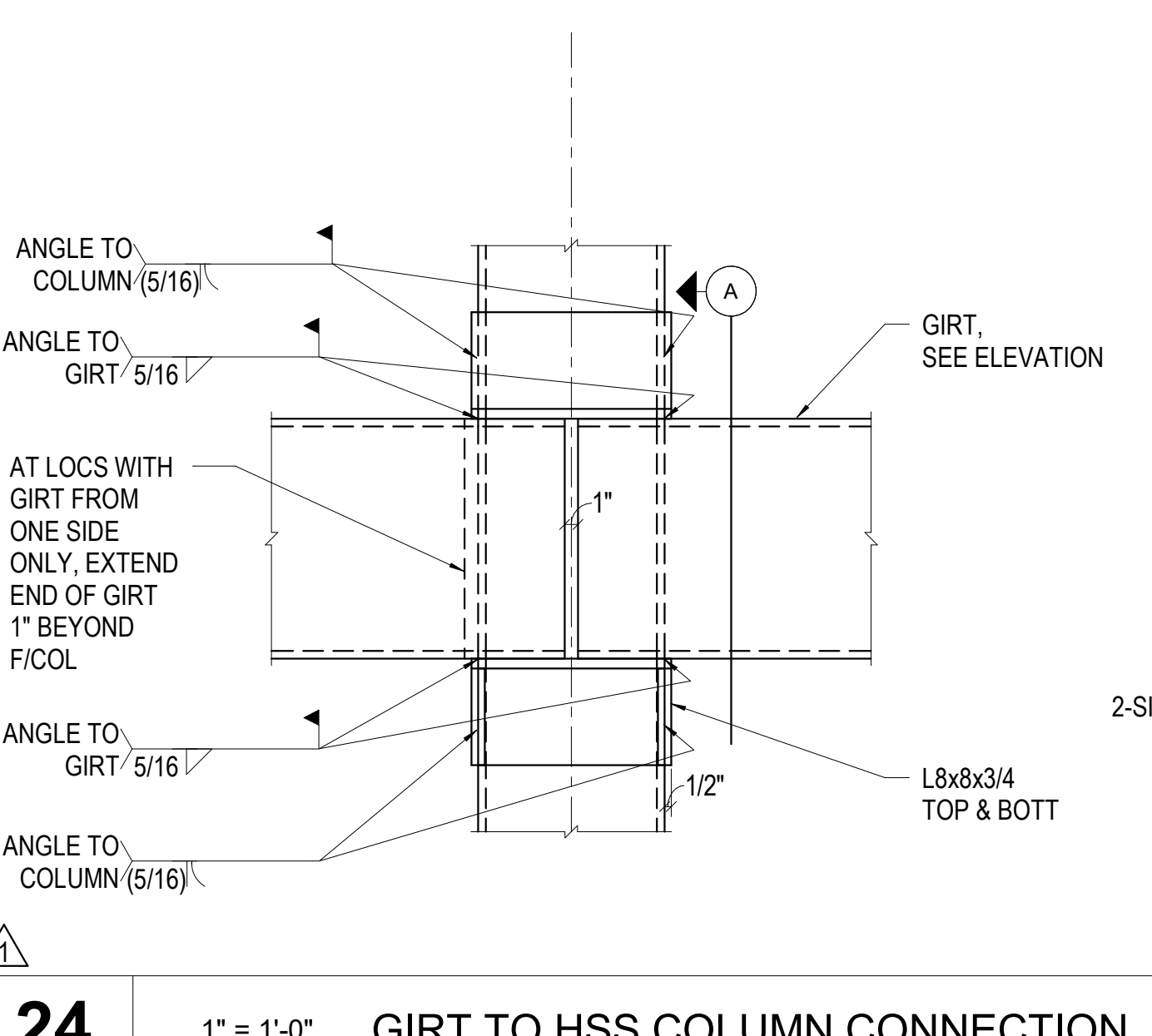
26 1" = 1'-0" ALT GIRT TOP CONNECTION



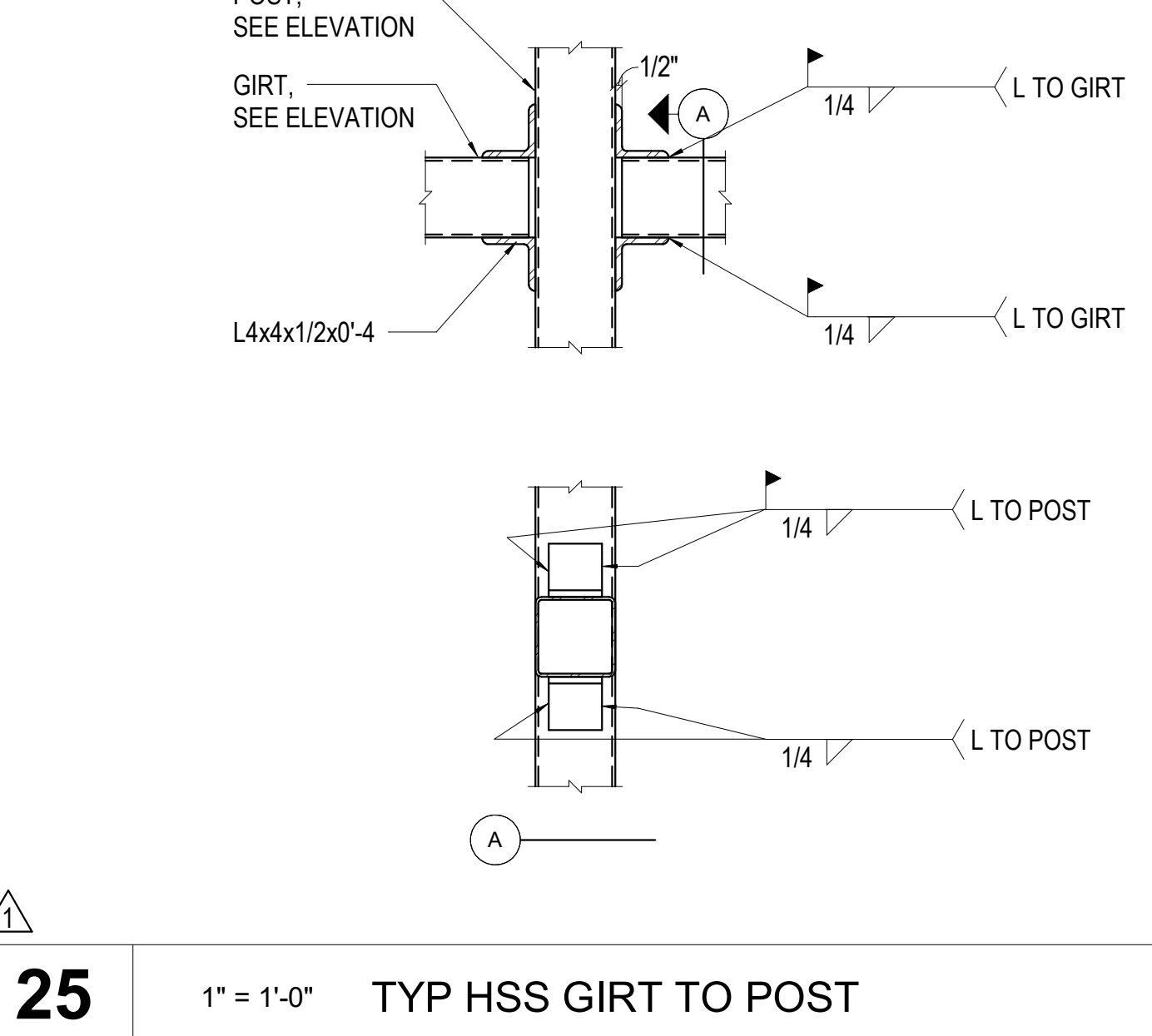
22 1" = 1'-0" PRECAST SOFFIT



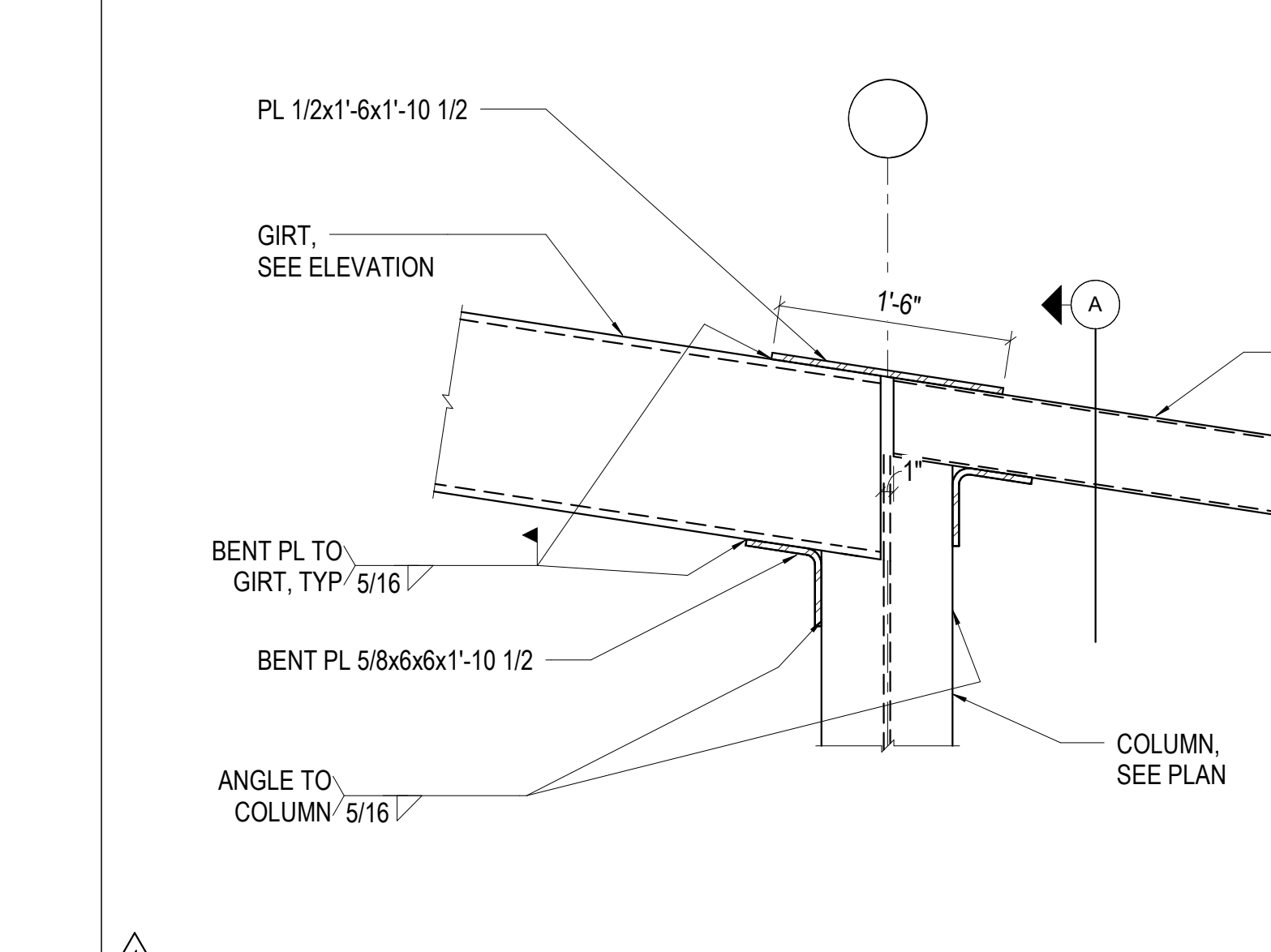
23 1" = 1'-0" PLAZA STAIR LOWER PRECAST SUPPORT



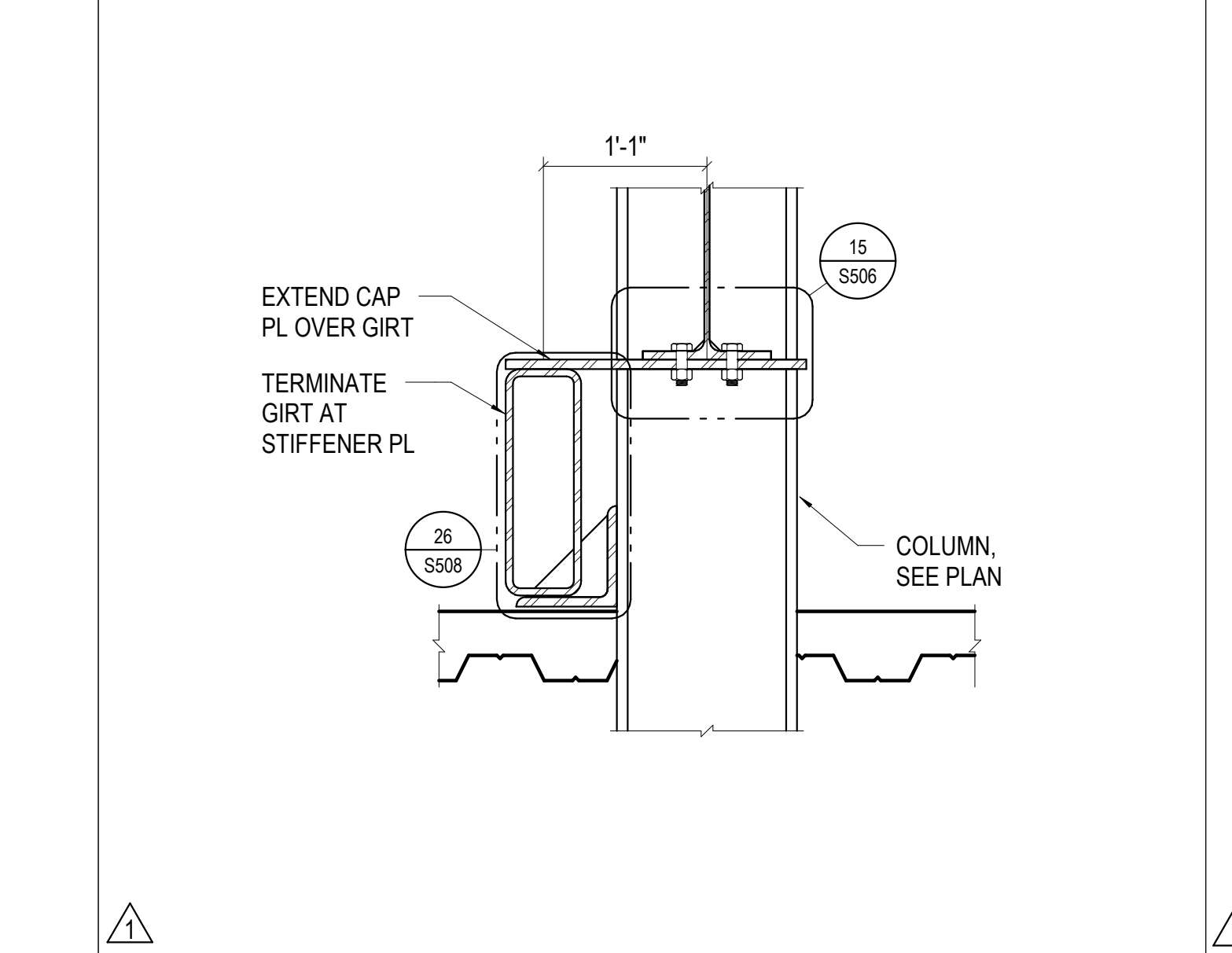
24 1" = 1'-0" GIRT TO HSS COLUMN CONNECTION



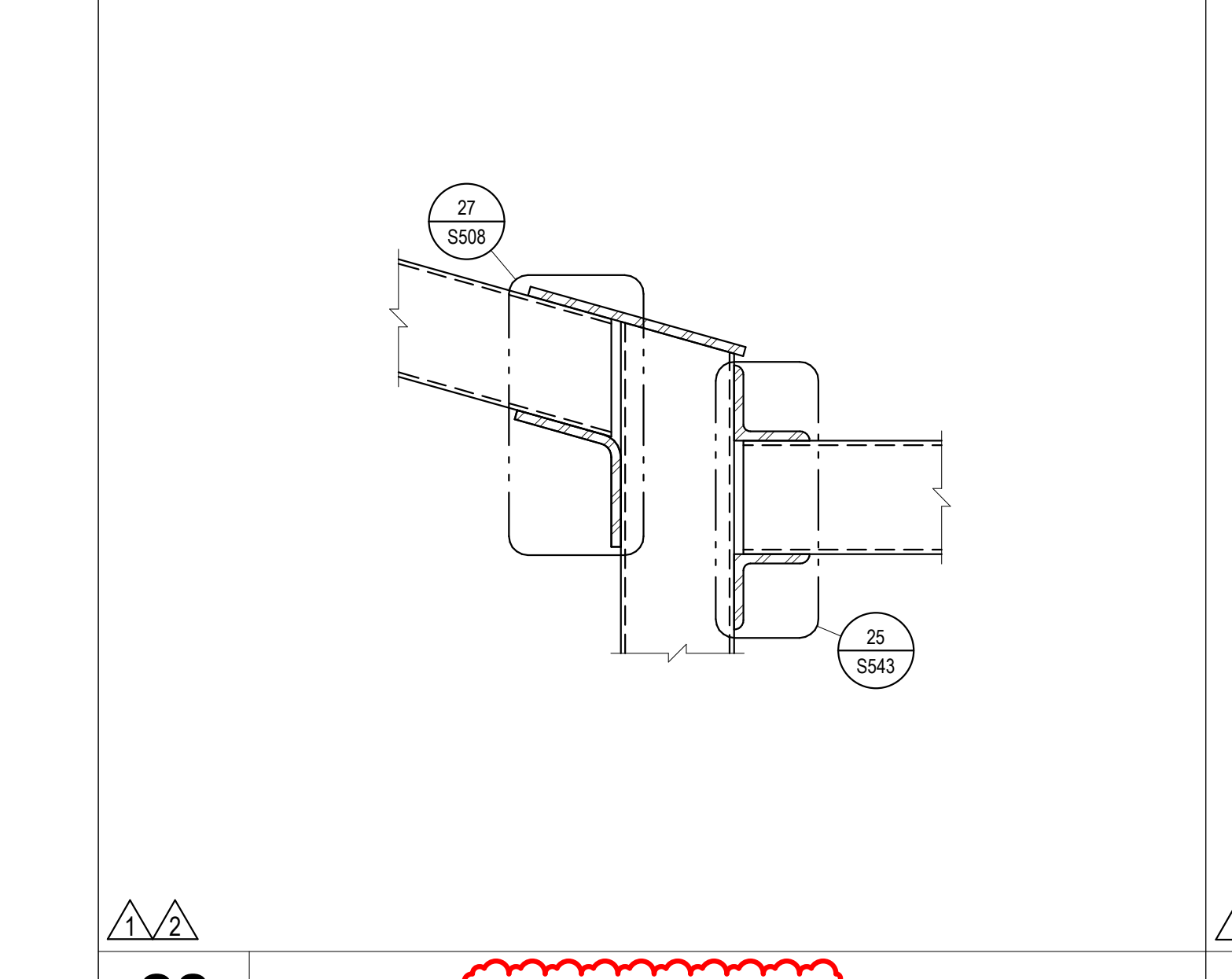
25 1" = 1'-0" TYP HSS GIRT TO POST



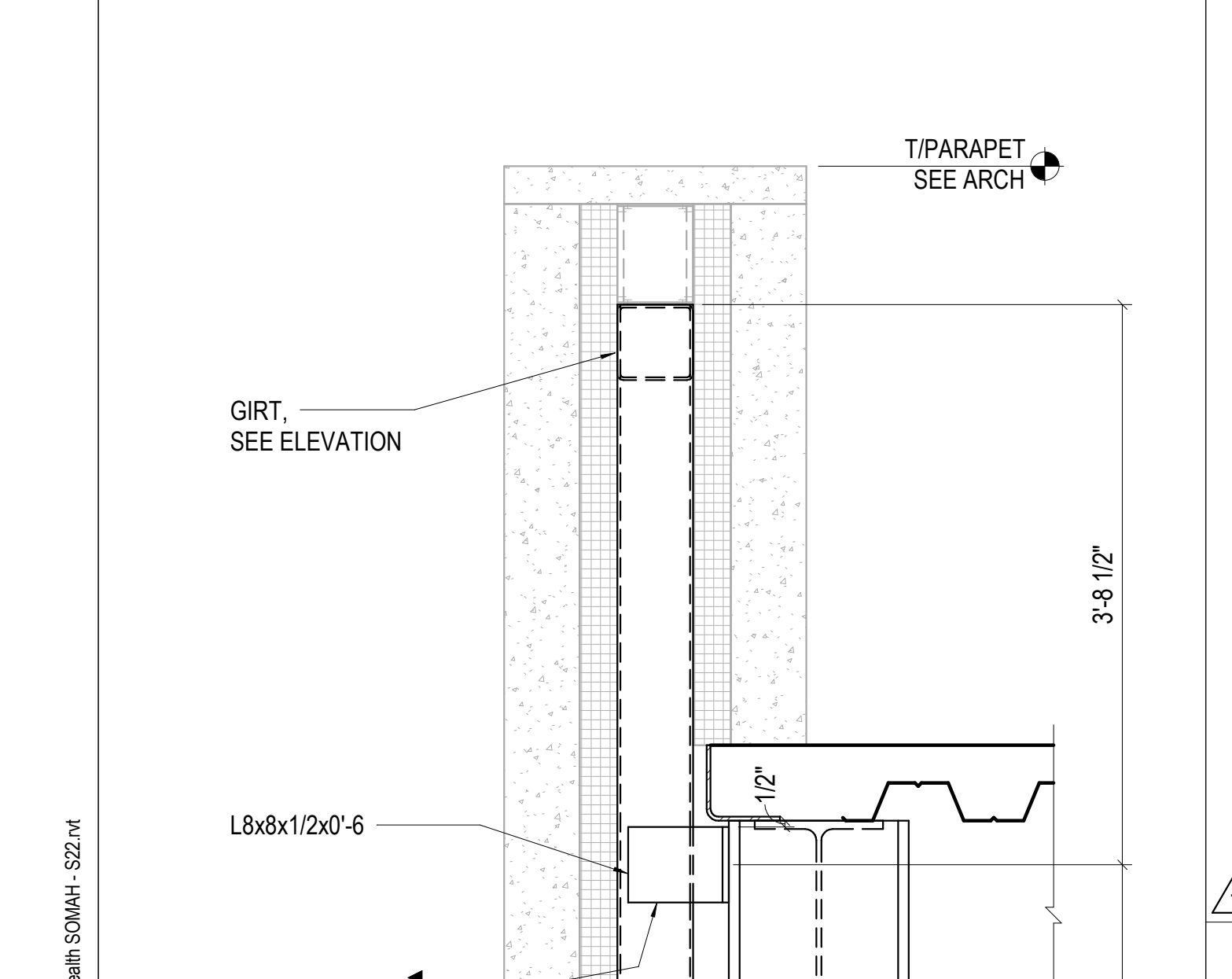
27 1" = 1'-0" END GIRT CONNECTION AT PLAZA STAIR



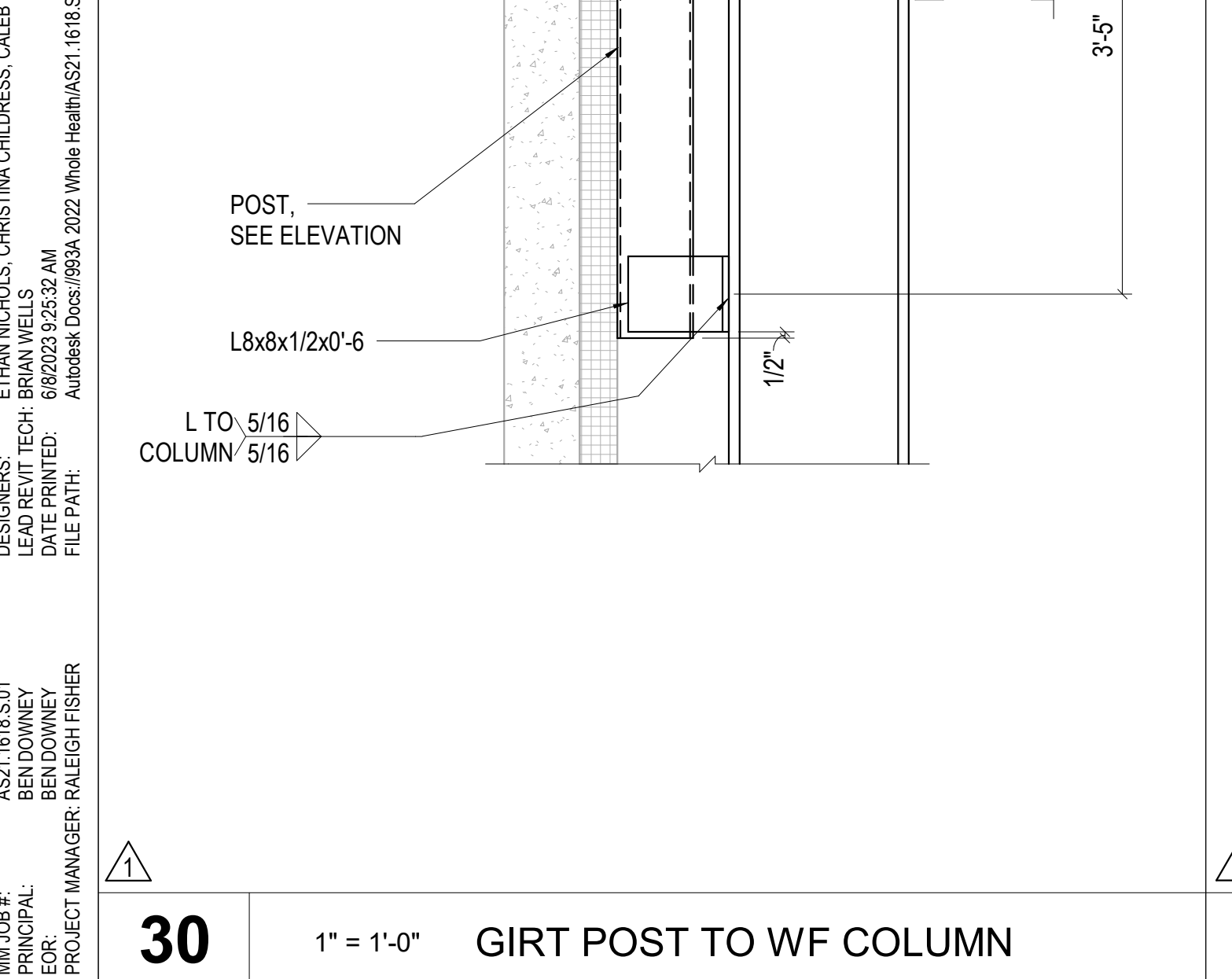
28 1 1/2" = 1'-0" HSS CONNECTION



29 1" = 1'-0" GIRT POST TO WF COLUMN



30 1" = 1'-0" GIRT CONNECTION



31 1" = 1'-0" GIRT CONNECTION

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