



MECHANICAL AND CHEMICAL ANCHORS:

- A. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY MANUFACTURER OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- B. INSTALL ANCHORS PER THE MANUFACTURER'S INSTRUCTIONS.
- C. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- D. AT ALL LOCATIONS WHERE ADHESIVES ARE CALLED OUT, THE PRODUCTS LISTED BELOW SHALL BE ACCEPTABLE. ALTERNATE PRODUCTS MUST BE APPROVED BY EOR PRIOR TO USE.
- E. THE MANUFACTURER RECOMMEND PRODUCT SCREEN TUBE SHALL BE USED FOR ALL ADHESIVES INSTALLED IN HOLLOW MASONRY.
- F. EXPANSION ANCHORS AND MECHANICAL SCREW ANCHORS MAY BE USED AS NOTED ON THE DRAWINGS. THE PRODUCTS LISTED BELOW SHALL BE ACCEPTABLE. ALTERNATE PRODUCTS MUST BE APPROVED BY EOR PRIOR TO USE.
- G. CONCRETE AND MASONRY ANCHORS ARE ASTM F1554 GRADE 36 ALL THREAD RODS, UNO.
- H. CONCRETE DOWELS ARE ASTM A615 GRADE 60, UNO.
- I. THE CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION TRAINING FOR ALL PRODUCTS TO BE USED. PRIOR TO COMMENCEMENT OF WORK, UNO TRAINED INSTALLERS SHALL PERFORM POST INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE PROVIDED TO THE EOR AS REQUESTED.
- J. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AA) AS CERTIFIED THROUGH AGICRCSI (ACI 318-19 D.9.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
- K. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS PRIOR TO INSTALLATION.
- L. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE BUILDING CODE AND PER THE CURRENT ICES-REPORT (ICR-1003) AND PER THE MANUFACTURER'S INSTRUCTIONS.
- M. THE FOLLOWING TABLE LISTS DEFAULT PRODUCTS USED IN DESIGN. WHERE SPECIFIC MODEL PRODUCTS ARE NOT OTHERWISE CALLED OUT IN STRUCTURAL DRAWINGS, THIS TABLE SHALL CONTROL.

PRODUCTS BY MANUFACTURER

| BASE MATERIAL | ADHESIVE EPOXY | EXPANSION ANCHOR | SCREW ANCHOR | POWDER ACTUATED FASTENER |
|---|----------------|-------------------|-----------------------|--------------------------|
| HILTI | | | | |
| CONCRETE | HY 200 | KWIK BOLT T22 | KWIK HUS-EZ | X-C* |
| CMU (GROUTED) | HY 270 | KWIK BOLT T22 | KWIK HUS-EZ | X-C |
| CMU (HOLLOW) | HY 270 | HLC SLEEVE | KWIK CON II* | X-C |
| URM (CLAY BRICK) | HY 270 | HLC SLEEVE | KWIK CON II* | --- |
| STEEL | --- | --- | SELF DRILL HWH2 10-10 | X-U |
| * USE X-CP FOR WOOD SILL PLATE TO CONCRETE | | | | |
| SIMPSON STRONG-TIE | | | | |
| CONCRETE | SET-3G | STRONG BOLT 2 | TITEN HD | PDPA** |
| CMU (GROUTED) | SET-3G | STRONG BOLT 2 | TITEN HD | PDP |
| CMU (HOLLOW) | SET-3G | --- | TITEN HD (STAINLESS) | PDP |
| URM (CLAY BRICK) | ET-3G | --- | --- | --- |
| STEEL | --- | --- | X METAL SCREW #10 | PDPA |
| ** USE PHN FOR WOOD SILL PLATE TO CONCRETE | | | | |
| DEWALT FASTENERS | | | | |
| CONCRETE | PURE 110 | STUD* SD-1 | WEDGE BOLT* | 0.300" HEAD DRIVE PIN*** |
| CMU (GROUTED) | AC100+ | STUD* SD-1 | WEDGE BOLT* | 0.300" HEAD DRIVE PIN*** |
| CMU (HOLLOW) | AC100+ | HOLLOW-SET DROPIN | --- | 0.300" HEAD DRIVE PIN*** |
| URM (CLAY BRICK) | AC100+ | LOK-OUT* AS | WEDGE BOLT* | 0.300" HEAD DRIVE PIN*** |
| STEEL | --- | --- | --- | --- |
| *** 0.145" SHANK | | | | |
| MISCELLANEOUS: | | | | |
| A. COORDINATE CONCRETE ERECTION PAD AND HOUSE KEEPING PAD LOCATIONS AND DIMENSIONS WITH ARCH, MECHANICAL, ELECTRICAL, PLUMBING, AND OWNER REQUIREMENTS. | | | | |
| B. RE ARCHITECT FOR MISCELLANEOUS METAL STAIR DETAILS. | | | | |

SUBMITTALS:

- A. PRIOR TO FABRICATION OF STRUCTURAL ELEMENTS, SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD. SHOP DRAWINGS SHALL INCLUDE INFORMATION AS REQUIRED BY THE BUILDING CODE AND AS LISTED ELSEWHERE IN THESE SPECIFICATIONS. SUBMITTALS FOR THE FOLLOWING STRUCTURAL ELEMENTS ARE TO BE PROVIDED FOR REVIEW (THIS LIST DOES NOT PRECLUDE SUBMITTAL OF ADDITIONAL STRUCTURAL ITEMS NOT LISTED, AS APPLICABLE).
- B. CONTRACTOR NOTE: COPIES OF STRUCTURAL DRAWINGS (PLANS AND/OR DETAILS) WILL NOT BE ACCEPTED BY CSA AS SHOP DRAWINGS. ALL SHOP DRAWINGS MUST BE REPRODUCED BY THE RESPECTIVE SUPPLIERS AND DETAILED AS NECESSARY.
- C. CAST-IN-PLACE CONCRETE
- D. CONCRETE MASONRY UNITS (DOES NOT INCLUDE MASONRY VENEERS)
- E. REINFORCEMENT
- F. HOT ROLLED STEEL
- G. OPEN WEB METAL JOISTS
- H. METAL DECKING
- I. ELEVATED CONCRETE
- J. PRECAST PANELS, PLANKS, AND/OR OTHER
- K. COLD-FORMED FRAMING

PRECAST CONCRETE:

- A. EXTERIOR PRECAST PANELS, UNO SHALL BE 8" OR 10" THICK, UNO BEARING PANELS. INTERIOR PRECAST PANELS SHALL BE 8" THICK, UNO PANELS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE PROJECT STATE.
- B. WIND AND SEISMIC LATERAL LOAD ANALYSIS SHALL BE INCLUDED IN THE DESIGN CALCULATIONS. DETAILS, ELEVATIONS, AND SECTIONS SHOWN ON PLANS ARE DIAGRAMMATIC ONLY. PANELS SHALL BE DESIGNED, CONSTRUCTED, FINISHED, LEVELNESS, ACCORDING TO THE REQUIREMENTS OF THE PARTICULAR CONNECTION AND THE SPECIFICATIONS. (COORDINATE SPEC DESIGN)
- C. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL PLATES, ANCHORS, INSERTS, BOLTS, LUGS, ETC. TO BE PROVIDED AND ANY DISCREPANCIES SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT. PRECASTER SHALL ALSO FIELD VERIFY LOCATIONS PRIOR TO ERECTION.
- D. THE DESIGN AND ANCHORAGE WHERE WELDING IS REQUIRED SHALL CONFORM TO CHAPTER 6 OF THE PCI HANDBOOK, 5TH EDITION.
- E. ANY DEVIATION FROM PRECAST CONCRETE DESIGN OR DETAILS SHOWN HEREIN SHALL BE IMMEDIATELY REPORTED, IN WRITING, BY THE ARCHITECT. CONNECTION DETAILS ARE NOT EXCLUSIVE AND MAY BE ALTERED BY THE PRECASTER TO HIS STANDARD OF SUGGESTED DETAILS, PROVIDED THAT THESE STANDARDS SATISFY THE STRENGTH REQUIREMENTS OF THE PARTICULAR CONNECTION AND ARE SUBJECTED TO THE STRUCTURAL ENGINEERS APPROVAL PRIOR TO CASTING.
- F. PRECASTER SHALL BE RESPONSIBLE FOR THE COMPLETE ERECTION OF PRECAST CONCRETE ELEMENTS, INCLUDING BRACING, SHORING, LEVELING, WELDING, BOLTING, ETC. ALL FABRICATION AND ERECTION SHALL COMPLY WITH APPROPRIATE PCI TOLERANCES.
- G. PRECASTER SHALL PROVIDE ONLY THOSE OPENINGS AND SAW CUTS SHOWN ON THEIR APPROVED DRAWINGS. HOLES EQUAL TO OR SMALLER THAN 8" x 8" WILL NOT BE SHOWN ON PRECAST DRAWINGS. HOLES EQUAL TO SMALLER THAN 8" x 8" AND ALL OTHER HOLES WILL BE CUT BY THE RESPECTIVE TRADES IN THE FIELD. HOLES WHICH WILL APPARENTLY CUT PRIMARY REINFORCING IN THE FIELD, CUTTING THE PRESTRESSED REINFORCING IS NOT PERMITTED UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- H. PRECAST PRODUCTS WILL BE FABRICATED TO TOLERANCES SPECIFIED IN SECTION 5 OF THE PCI MANUAL, MIN-116 "MANUAL FOR QUALITY CONTROL, PLANTS AND PRODUCTION OF PRECAST/PRESTRESSED CONCRETE PRODUCTS."
- I. ALL CONNECTION MATERIALS, LEVELING DEVICES, AND ANCHOR BOLTS, WHEN PRECAST MEMBERS AND OTHER PRECAST MEMBERS OR TO SLAB-ON-GRADE, OR TO CAST-IN-PLACE CONCRETE SHALL BE PROVIDED BY THE PRECASTER.
- J. ALL MEMBERS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF F'c = 5,000 PSI AT 28 DAYS.
- K. PRECAST DESIGN SHALL COMPLY WITH ACI 318 AND THE DESIGN RECOMMENDATIONS OF TEH PCI DESIGN HANDBOOK, 6TH EDITION.

PRECAST ERECTION NOTES:

- A. STABILITY OF THE STRUCTURE SHALL BE MAINTAINED AT ALL TIMES UNTIL ALL CONNECTIONS ARE COMPLETED.
- B. PRECAST DESIGN SHALL BE BRACED AT THE TIME OF ERECTION UNTIL FINAL CONNECTIONS ARE MADE AND THE BUILDING IS STABLE.
- C. DRY PACK GROUT BETWEEN COLUMN AND FOOTING. BETWEEN WALL PANELS AND SLAB-ON-GRADE, AND FOR POCKETS AT THE BOTTOM OF THE COLUMNS SHALL HAVE A MINIMUM STRENGTH OF 5,000 PSI, UNLESS NOTED OTHERWISE.
- D. DRY PACKING UNDER COLUMN BASE PLATES AND AT POCKETS FOR ANCHOR BOLTS SHALL BE DONE IMMEDIATELY AFTER ERECTION OF COLUMNS AND PRIOR TO ERECTION OF SUPPORTED BEAMS.
- E. EXTERIOR WALL PANELS TO BE ERECTED ON MIN OF 4" x 4" SHIMS LOCATED UNDER THE ENDS OF THE PANEL.
- F. ALL RECESSED CONNECTIONS SHALL BE PATCHED WITH NON-SHRINK GROUT OR AS DETAIL.
- G. PRECAST DESIGN SHALL BE ERECTED TO OBTAIN APPROXIMATELY THE SAME BEARING LENGTH AT BOTH ENDS. A MINIMUM OF 3" BEARING IS REQUIRED.
- H. BEARING PADS SHALL BE PLACED SQUARE AND FLUSH WITH BEAM LEDGES AND COLUMN CORBELS.

LIGHT GAGE METAL FRAMING:

- A. ALL STUD AND/OR JOIST FRAMING MEMBERS SHALL BE THE TYPE, SIZE, AND GAGE AS SHOWN ON THE PLANS AND SHALL CONFORM TO THE "North American Specification for the Design of Cold-Formed Steel Structural Members (AISI-NASPEC)".
- B. PRIOR TO FABRICATION OF FRAMING, SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD.
- C. DESIGN OF COLD-FORMED CARBON OR LOW-ALLOY STEEL STRUCTURAL MEMBERS SHALL BE IN ACCORDANCE WITH THE "North American Specification for the Design of Cold-Formed Steel Structural Members (AISI-NASPEC)".
- D. INSTALLATION AND CONSTRUCTION OF COLD-FORMED CARBON OR LOW-ALLOY STEEL STRUCTURAL AND NON-STRUCTURAL STEEL FRAMING SHALL BE IN ACCORDANCE WITH THE "Standard for Cold-Formed Steel Framing - General Provisions, American Iron and Steel Institute - (AISI-General)", AND "AISI-NASPEC".
- E. INSTALLATION OF COLD-FORMED STEEL BOX AND BACK-TO-BACK HEADERS, AND DOUBLE-L HEADERS USED IN SINGLE SPAN CONDITION FOR LOAD CARRYING PURPOSES SHALL BE IN ACCORDANCE WITH THE "Standard for Cold-Formed Steel Framing - Header Design, American Iron and Steel Institute - (AISI-Header)", SUBJECT TO THE LIMITATIONS THEREIN.
- F. COLD-FORMED FRAMING CONNECTIONS SHALL BE PROVIDED BY THE MANUFACTURER AND SHALL MEET MINIMUM FASTENER AND SCREW PATTERNS IN ACCORDANCE WITH CHAPTER 22 OF THE APPLICABLE BUILDING CODE, UNO ON THE PLANS.
- G. FRAMING CONNECTIONS, FASTENERS, HOLDDOVS, OR OTHER SHOWN ON THE PLANS SHALL BE SIMPSON STRONG-TIE OR EQUAL.
- H. THIS SECTION SHALL APPLY TO ALL LIGHT GAGE STUD WALLS; BEAMS; AND/OR SHEAR (TYP UNO). NON-STRUCTURAL LIGHT GAGE STUD WALLS SHALL BE CONSTRUCTED AS DETAILED BY THE ARCHITECT AND PROJECT SPECIFICATIONS (TYP).
- I. ALL CONNECTIONS SHALL BE SCREWED OR WELDED. USE A MINIMUM OF (2) #10 SCREWS AT EACH CONNECTION. POWER DRIVEN FASTENERS (PDFS) SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
- J. STUDS WALLS SHALL HAVE LATERAL BRACING AT 48" OC, MAX PER AISI.
- K. LIGHT GAGE TRACK SHALL MATCH GAGE AND SIZE OF VERTICAL STUDS. RE: FASTENER SCHEDULE FOR CONNECTION TO CONCRETE OR MASONRY.
- L. ALL FRAMING MEMBERS SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR 33X AND 43 MIL AND 50 KSI FOR 54, 68, AND 97 MIL MATERIAL (TYP UNO).
- M. LIGHT GAGE FRAMING MEMBERS AND CONNECTIONS SHALL BE DESIGNED AND SIGNED AND SEALED BY SUPPLIER'S SPECIALTY ENGINEER REGISTERED IN THE PROJECT STATE. RE: ARCH FOR ADDITIONAL DETAILS AS REQUIRED.

BRACING CONNECTORS:

- A. SHEAR STUDS AND THEIR INSTALLATION SHALL MEET ALL REQUIREMENTS SPECIFIED IN SECTION 7 OF AWS D1.1 STRUCTURAL WELDING CODE-STEEL.
- B. SIZES FOR COMPOSITE METAL DECK CONSTRUCTION STUD LENGTH SHALL EXTEND A MINIMUM LENGTH OF 1 1/2" PLUS COMPOSITE DECK DEPTH AND SHALL HAVE A MAXIMUM LENGTH OF TOTAL SLAB THICKNESS MINUS 3/4" CLR FROM FINISHED CONCRETE SURFACE TO BE ONE SHEAR CONNECTOR. RE: THE STRUCTURAL DRAWINGS AND NOTES BELOW FOR NUMBER AND LOCATION OF SHEAR CONNECTORS.
- C. RE: FLOOR PLANS & GENERAL DETAILS FOR TYPICAL COMPOSITE BEAM CONSTRUCTION DETAILS AND REQUIREMENTS.
- D. INSTALLATION FOR COMPOSITE BEAMS
 1. STUDS SHALL BE WELDED IN THE FIELD USING AUTOMATICALLY TIMED STUD WELDING EQUIPMENT.
 2. THE TOP FLANGE OF THE BEAMS MUST BE UNPAINTED AND FREE OF HEAVY RUST, MILL SCALE, DIRT, SAND, OR OTHER FOREIGN MATERIAL WHICH WILL INTERFERE WITH THE WELDING OPERATION.
 3. THE METAL DECK MUST BE FREE OF DIRT, SAND, OIL, OR OTHER FOREIGN MATERIAL AND MUST BE DRY AND FREE OF MOISTURE. METAL DECK MUST REST LIGHTLY ON BEAM FLANGE. WELDING MUST TAKE PLACE THROUGH ONLY ONE THICKNESS OF THE METAL DECK.
 4. STUDS SHALL BE SPACED ON UNIFORMLY LOADED BEAMS HAVING DECK WITH DECK FLUTES PARALLEL TO THE SPAN SHALL HAVE STUDS PLACED AT FLUTES ACROSS THE SPAN. ADDITIONAL STUDS SHALL BE SPACED BEGINNING FROM THE ENDS OF THE BEAM AND REPEATING UNTIL ALL STUDS ARE PLACED.
 5. BEAMS WITH CONCENTRATED LOADS SHALL HAVE STUDS PLACED IN THE ZONES INDICATED, IN THE SAME MANNER AS DESCRIBED ABOVE. GIRDERS WITH DECK FLUTES PARALLEL TO THE SPAN SHALL HAVE STUDS PLACED UNIFORMLY WITHIN THE ZONES INDICATED ON THE DRAWINGS.
 6. MINIMUM TRANSVERSE STUD SPACING SHEAR STUDS IN GROUPS SHALL NOT BE SPACED CLOSER THAN 9" OC.
 7. STUDS WILL BE PLACED DIRECTLY OVER THE BEAM WEB FOR BEAMS HAVING A FLANGE THICKNESS OF 300 OR LESS.

SHEAR CONNECTORS:

- A. SHEAR STUDS AND THEIR INSTALLATION SHALL MEET ALL REQUIREMENTS SPECIFIED IN SECTION 7 OF AWS D1.1 STRUCTURAL WELDING CODE-STEEL.
- B. SIZES FOR COMPOSITE METAL DECK CONSTRUCTION STUD LENGTH SHALL EXTEND A MINIMUM LENGTH OF 1 1/2" PLUS COMPOSITE DECK DEPTH AND SHALL HAVE A MAXIMUM LENGTH OF TOTAL SLAB THICKNESS MINUS 3/4" CLR FROM FINISHED CONCRETE SURFACE TO BE ONE SHEAR CONNECTOR. RE: THE STRUCTURAL DRAWINGS AND NOTES BELOW FOR NUMBER AND LOCATION OF SHEAR CONNECTORS.
- C. RE: FLOOR PLANS & GENERAL DETAILS FOR TYPICAL COMPOSITE BEAM CONSTRUCTION DETAILS AND REQUIREMENTS.
- D. INSTALLATION FOR COMPOSITE BEAMS
 1. STUDS SHALL BE WELDED IN THE FIELD USING AUTOMATICALLY TIMED STUD WELDING EQUIPMENT.
 2. THE TOP FLANGE OF THE BEAMS MUST BE UNPAINTED AND FREE OF HEAVY RUST, MILL SCALE, DIRT, SAND, OR OTHER FOREIGN MATERIAL WHICH WILL INTERFERE WITH THE WELDING OPERATION.
 3. THE METAL DECK MUST BE FREE OF DIRT, SAND, OIL, OR OTHER FOREIGN MATERIAL AND MUST BE DRY AND FREE OF MOISTURE. METAL DECK MUST REST LIGHTLY ON BEAM FLANGE. WELDING MUST TAKE PLACE THROUGH ONLY ONE THICKNESS OF THE METAL DECK.
 4. STUDS SHALL BE SPACED ON UNIFORMLY LOADED BEAMS HAVING DECK WITH DECK FLUTES PARALLEL TO THE SPAN SHALL HAVE STUDS PLACED AT FLUTES ACROSS THE SPAN. ADDITIONAL STUDS SHALL BE SPACED BEGINNING FROM THE ENDS OF THE BEAM AND REPEATING UNTIL ALL STUDS ARE PLACED.
 5. BEAMS WITH CONCENTRATED LOADS SHALL HAVE STUDS PLACED IN THE ZONES INDICATED, IN THE SAME MANNER AS DESCRIBED ABOVE. GIRDERS WITH DECK FLUTES PARALLEL TO THE SPAN SHALL HAVE STUDS PLACED UNIFORMLY WITHIN THE ZONES INDICATED ON THE DRAWINGS.
 6. MINIMUM TRANSVERSE STUD SPACING SHEAR STUDS IN GROUPS SHALL NOT BE SPACED CLOSER THAN 9" OC.
 7. STUDS WILL BE PLACED DIRECTLY OVER THE BEAM WEB FOR BEAMS HAVING A FLANGE THICKNESS OF 300 OR LESS.

REINFORCING STEEL:

- A. WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH ASTM A185. WIRE FABRIC LOCATED IN CONCRETE SLABS SHALL BE LOCATED IN THE CENTER OF THE SLAB, UNO BY STRUCTURAL DOCUMENTS, SUPPORTS USED SHALL BE UNLESS EXPLICITLY APPROVED BY ENGINEER OF RECORD.
- B. REINFORCING STEEL SHALL COMPLY WITH ASTM A615 GRADE 60 WITH THE FOLLOWING REQUIREMENTS: (a) ACTUAL YIELD STRENGTH BASED ON MILL TEST SHALL NOT EXCEED 78 KSI. RETESTS SHALL NOT EXCEED THIS VALUE BY MORE THAN ADDITIONAL 3000 PSI. (b) F_y/F_t SHALL NOT BE LESS THAN 1.25. (c) F_t - ACTUAL YIELD TENSILE STRENGTH, F_t = ACTUAL ULTIMATE TENSILE STRENGTH.
- C. REINFORCING STEEL SHALL COMPLY WITH ASTM A706 AT ALL LOCATIONS WHERE REBAR MUST BE WELDED.
- D. ALL SLAB-ON-GRADE CONSTRUCTION SHALL FOLLOW THE RECOMMENDATIONS OF "GUIDE FOR FLOOR AND SLAB CONSTRUCTION, ACI 302.1R-04".
- E. CONTROL JOINT LOCATIONS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 3X THE THICKNESS OF SLAB SECTION IN FEET. IN EACH DIRECTION, JOINTS ARE NOT SHOWN ON PLAN FOR CLARITY. CONTRACTORS SHALL PROVIDE AN INTENDED JOINT PLAN PRIOR TO POURING. RE: TYPICAL CONTROL JOINT DETAILS FOR ADDITIONAL INFORMATION. INDUSTRY STANDARD PRACTICE SHALL BE FOLLOWED. JOINTS MUST BE OUT WITHIN 24 HOURS OF POUR, BUT IDEALLY SHOULD BE IN THE 6 TO 18 HOUR POST POUR WINDOW.
- F. A VAPOR BARRIER SHALL BE PROVIDED BELOW SLAB-ON-GRADE AT ALL LOCATIONS. VAPOR BARRIER SHALL BE LAPPED AND TAPED AS REQUIRED BY MANUFACTURER. RE: ARCH FOR VAPOR BARRIER REQUIREMENTS INCLUDING THICKNESS. (PROVIDE A MINIMUM 10 MIL THICKNESS IF NOT OTHERWISE SPECIFIED).
- G. FLY ASH MAY BE USED TO REPLACE A PORTION OF THE PORTLAND CEMENT IN A MIX AND SHALL CONFORM TO ASTM C618. ADDITIVE IS SUBJECT TO REVIEW/APPROVAL BY ENGINEER. MIXES USING FLY ASH SHALL BE PROPORTIONED TO ACCOUNT FOR THE PROPERTIES OF THE SPECIFIC FLY ASH TO BE USED. THE SCHEDULE FOR AMOUNT OF FLY ASH AND CEMENT IN THE MIX SHALL NOT EXCEED 20 PERCENT.
- H. UNLESS NOTED OTHERWISE BY STRUCTURAL DOCUMENTS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:
 - (a) EXIST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
 - (b) CAST TO EARTH OR WEATHER #5 OR SMALLER 1 1/2"
 - OR LARGER 2"
 - (c) NOT EXPOSED TO EARTH OR WEATHER OR IN CONTACT WITH GROUND SLABS, WALLS, JOISTS #1 OR SMALLER 3/4"
 - ALL OTHER 1 1/2"
 - BEAMS 3"
 - PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1 1/2"

BRACING CONCRETE AND MASONRY WALLS

- A. CONTRACTOR SHALL PROVIDE ANY NECESSARY TEMPORARY BRACING FOR ALL WALLS BACK FILLING SHALL NOT OCCUR UNTIL PERMANENT LATERAL RESTRAINTS ARE IN PLACE.

MASONRY VENEER:

- A. THE WEIGHT OF EXTERIOR MASONRY VENEER SHALL NOT EXCEED 40 PSF
- B. FOR ADJUSTABLE TWO-PIECE ANCHORS, ANCHORS OF WIRE SIZE W1 AND Z2 GAGE CORRUGATED SHEET-METAL ANCHORS, PROVIDE AT LEAST ONE ANCHOR FOR EACH 2.63 FT²
- C. ANCHOR SHALL BE SPACED AT A MAXIMUM OF 16" O.C. EACH DIRECTION.
- D. PROVIDE ADDITIONAL ANCHORS AROUND OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS AROUND PERIMETER OF OPENING AT A MAXIMUM OF 3 FT ON CENTER. PLACE ANCHORS WITHIN 12" OF OPENINGS.
- E. COLD-FORMED STEEL BACKING, ATTACHED EACH ANCHOR TO BACKING WITH CORROSION-RESISTANT SCREWS HAVING A MINIMUM NOMINAL SHANK DIAMETER OF 0.190 IN.

STRUCTURAL STEEL:

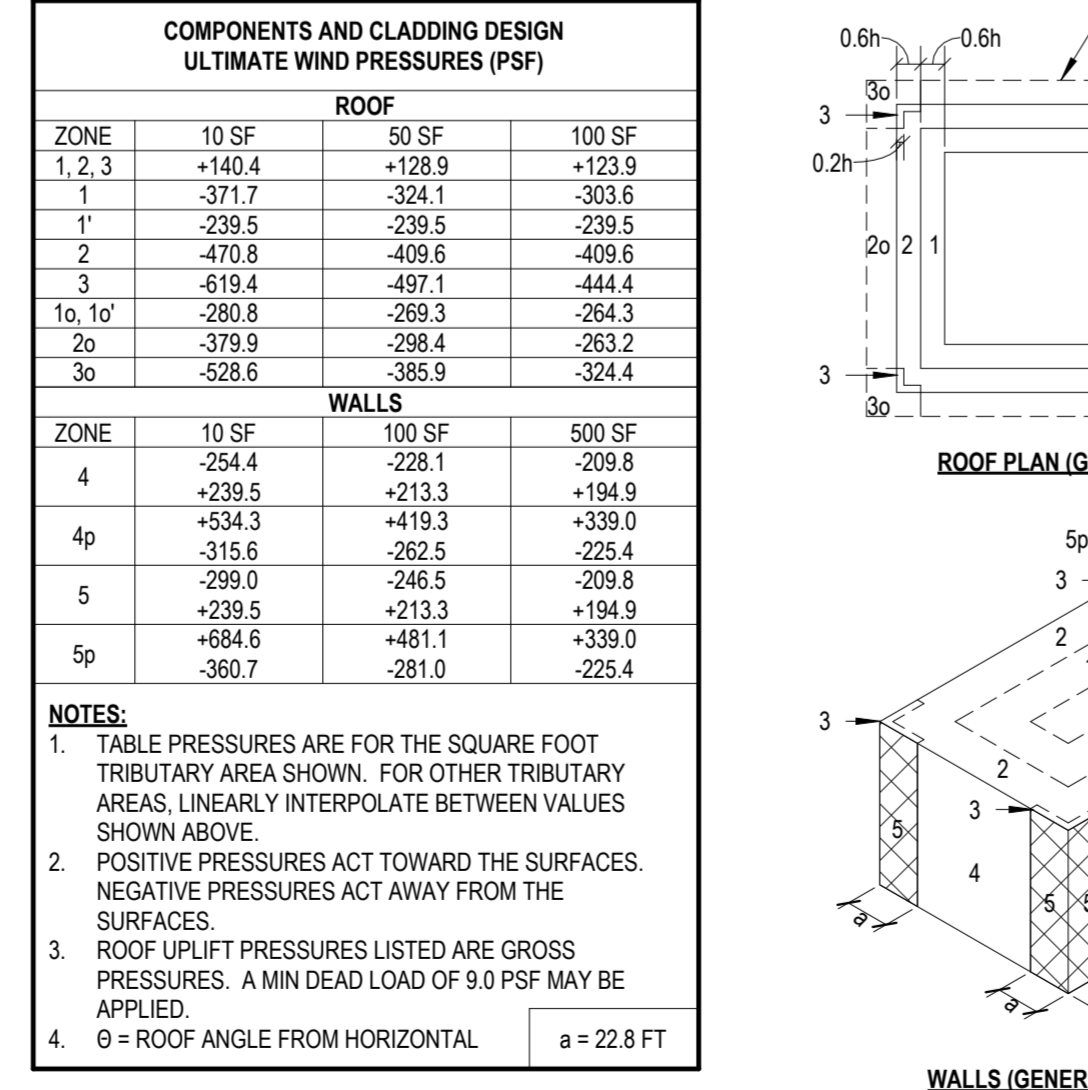
- A. ALL ANCHOR BOLTS SHALL BE ASTM F1554-GR105, UNLESS NOTED OTHERWISE.
- B. PROVIDE MIN 1" NON-SHRINK GROUT UNDER COLUMN BASE PLATES. UNO FABRICATOR SHALL SUPPLY ADEQUATE GROUT BED FOR INSTALLATION AND ADJUSTMENT OF LEVELING NUTS.
- C. ALL PLATES, CHANNELS, AND ANGLES SHALL CONFORM TO ASTM A36. ALL STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A992, GRADE 50. RECTANGULAR HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B WITH YIELD STRENGTH = 46 KSI. CIRCULAR HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B WITH YIELD STRENGTH = 42 KSI. UNO MEMBERS AND ANCHORS THAT ARE TO BE EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED.
- D. ALL SHEAR CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE STANDARD AISI WELDED OR BOLTED CONNECTIONS AND SHALL HAVE SUFFICIENT CAPACITY TO DEVELOP AN END CONNECTION EQUAL TO HALF THE GIVEN VALUE IN THE TABLE "ALLOWABLE UNIFORM LOADS IN KIPS FOR BEAMS LATERALLY SUPPORTED" IN PART 2 OF THE THIRTEENTH EDITION OF AISC MANUAL.
- E. ALL BOLTS FOR BEAM CONNECTIONS SHALL BE ASTM A325 WITH A MINIMUM DIAMETER OF 3/4". UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED AS BEARING TYPE CONNECTIONS. WASHERS SHALL BE INSTALLED UNDER NUTS AND FASTENERS WHEN REQUIRED BY THE SPECIFICATION FOR STRUCTURAL JOINTS.
- F. ALL WELDS SHALL BE MADE IN ACCORDANCE WITH THE LATEST PRACTICES OF AWS. USE E-70XX SERIES ELECTRODES.
- G. CAP PLATES FOR COLUMNS SHALL BE 5/8" THICK UNLESS NOTED OTHERWISE.
- H. ALL HORIZONTAL TUBE MEMBERS SHALL HAVE A MINIMUM 1/4" END PLATE, TYP.
- I. COLUMN BASE PLATES SHALL BE A MINIMUM 3/4" THICK W/ (4) 3/4" DIA. F1554-36 ANCHOR RODS. REFER TO BASE PLATE SCHEDULES FOR MORE STRINGENT REQUIREMENTS, TYP. UNO.
- J. DIAGONAL ANGLE SUPPORTS AT COLUMNS AND ANY OTHER MISCELLANEOUS SUPPORTS REQUIRED TO CARRY STEEL DECK SHALL BE FURNISHED AND INSTALLED BY THE STEEL FABRICATOR, WHERE OPENINGS OCCUR AND FRAMING IS NOT SHOWN, CONTRACTOR SHALL PROVIDE ADEQUATE SUPPORT IN ACCORDANCE WITH GOOD ENGINEERING PRACTICE.
- K. ALL HOT-ROLLED STEEL BEARING ON MASONRY BLOCK WALLS SHALL HAVE A MINIMUM 6" BEARING UNO.
- L. STEEL STAIRS SHALL BE DESIGNED AND SIGNED AND SEALED BY STEEL FABRICATOR'S SPECIALTY ENGINEER REGISTERED IN THE PROJECT STATE. RE: ARCH FOR STAIR CONFIGURATION AND STAIR DETAIL. DESIGN OF FRAMING MEMBERS AND CONNECTIONS TO BE BY SPECIALTY ENGINEER, WORKING DIRECTLY WITH THE STAIR DETAILER AND FABRICATOR.
- M. CONNECTION DETAILING IS TO BE DONE BY THE STEEL FABRICATOR. CONNECTIONS SHALL BE DESIGNED AND SIGNED AND SEALED BY STEEL FABRICATOR'S SPECIALTY ENGINEER REGISTERED IN THE PROJECT STATE.
- N. WHERE STEEL BEAMS BEAR ON MASONRY AT EACH END, ONE BEAM END SHALL BE A SLIP CONNECTION W/ SLOTTED HOLES AND THE OPPOSITE END SHALL BE A TYPICAL BEAM CONNECTION FASTENED TO EMBED PLATE AS SHOWN ON GENERAL DETAILS.

JOISTS:

- A. ALL OPEN WEB STEEL JOISTS SHALL CONFORM TO SJI SPECIFICATIONS AND SHALL HAVE DOUBLE ANGLE TOP & BOTTOM CHORDS.
- B. MASONRY WORK SHALL BE INSPECTED IN ACCORDANCE WITH ACI 308 QUALITY ASSURANCE LEVEL SPECIFIED IN SPECIAL INSPECTION TABLE.
- C. PROVIDE BRIDGING EACH SIDE OF OPENINGS THAT INTERRUPT JOIST BRIDGING. RUN ADDITIONAL BRIDGING ONE JOIST SPACING EACH SIDE OF OPENINGS.
- D. JOISTS WHOSE TOP CHORDS CANTILEVER PAST THE BEARING POINT SHALL BE DESIGNED FOR 270 LBS/FT.
- E. ALL STEEL JOISTS SHALL BE DESIGNED FOR THE WIND UPLIFT SHOWN. THE ROWS OF BRIDGING SHALL BE INCREASED AS REQUIRED FOR UPLIFT. SINCE A KD FACTOR OF 0.85 HAS BEEN USED, NO INCREASE IN THE ALLOWABLE STRESS IS PERMITTED.

METAL DECK:

- A. METAL ROOF DECK SHALL BE AS SPECIFIED ON PLAN.
- B. METAL DECKING SHALL BE CONTINUOUS OVER AT LEAST TWO SUPPORTS.
- C. ALL METAL ROOF DECK SHALL BE 1.58 20GA, FASTENED TO SUPPORTS W/ 5/8" DIA. PUDDLE WELDS IN A 3607 PATTERN AND (7) #10 SIDELAP SCREWS PER SPAN. FASTEN AT EDGES TO SUPPORTS W/ 5/8" DIA. PUDDLE WELDS AT 6" O.C. TYP. UNO. RE: S004 FOR MORE STRINGENT REQUIREMENTS.



DRAWINGS:

- A. CHANGES TO THE CONTRACT DOCUMENTS SHALL BE CLOUDED ON SHOP DRAWINGS OR REQUESTED IN WRITING. THE CONTRACTOR IS LIABLE FOR ANY DEVIATIONS UNLESS REVIEWED AND ACKNOWLEDGED BY THE ENGINEER.
- B. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
- C. ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.
- D. DESIGN ENGINEERING, INC OR ANY OF ITS EMPLOYEES SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- E. RE: ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- F. OWNER SHALL DIRECTLY ENGAGE A THIRD PARTY INSPECTION AGENCY TO ENSURE THAT ALL STRUCTURAL ELEMENTS ARE APPROPRIATELY CONSTRUCTED AS INTENDED.

STRUCTURAL OBSERVATIONS:

- A. THE OWNER OR OWNER'S AUTHORIZED AGENT SHALL EMPLOY A REGISTERED DESIGN PROFESSIONAL TO PERFORM STRUCTURAL OBSERVATIONS. THIS DOES NOT INCLUDE OR WAIVE THE SEPARATE REQUIREMENT FOR SPECIAL INSPECTIONS AS SPECIFIED ON THE CONTRACT DOCUMENTS. OBSERVER SHALL PROVIDE A REPORT (WRITTEN STATEMENT) TO THE BUILDING OFFICIAL AT THE TIME OF COMPLETION.

BUILDING SYSTEMS:

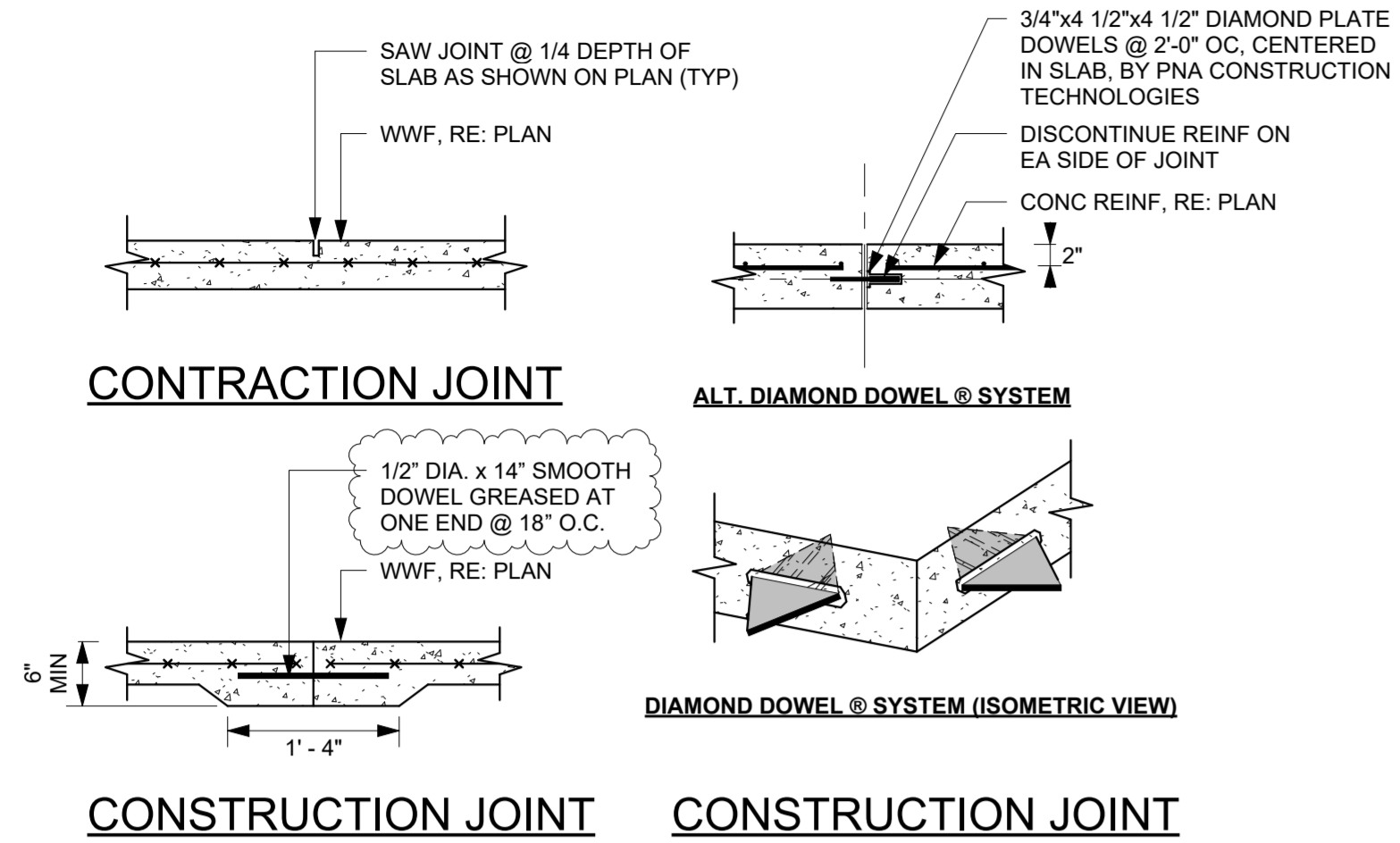
- A. CONTRACTOR SHALL PROVIDE NECESSARY BRACING & SHORING AS REQUIRED FOR ALL SYSTEMS TO BE COMPLETED. STRUCTURE SHALL NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

FOUNDATIONS:

- A. THE FOUNDATION DESIGN IS BASED UPON A SUBSURFACE EXPLORATION REPORT DATED APRIL 11, 2024 BY GEOTECHNOLOGY, LLC, DBA UES, #10000 BEARING PRESSURE IS 1500 PSF.
- B. COORDINATE ALL FOOTING STEPS AND ELEVATIONS W/ UTILITIES.
- C. IF FOOTING ELEVATIONS SHOWN OCCUR IN DISTURBED, UNSTABLE, OR UNSATURATED SOIL, THE ENGINEER SHALL BE NOTIFIED.
- D. UNDERCUTTING AND SITE PREPARATION SHALL BE COMPLETED PER GEOTECHNICAL DIRECTION AS PROVIDED IN THE ABOVE REFERENCED REPORT.

DESIGN CODES & SPECIFICATIONS

- PROJECT STATE: ARKANSAS
- BUILDING CODE: 2021 INTERNATIONAL BUILDING CODE
- DESIGN LOADS: ASCE 7-16
- MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES:
- SHELTER CODE: IC500-20
- STANDARD FOR THE DESIGN AND CONSTRUCTION OF STORM SHELTERS
- CONCRETE CODE: AC

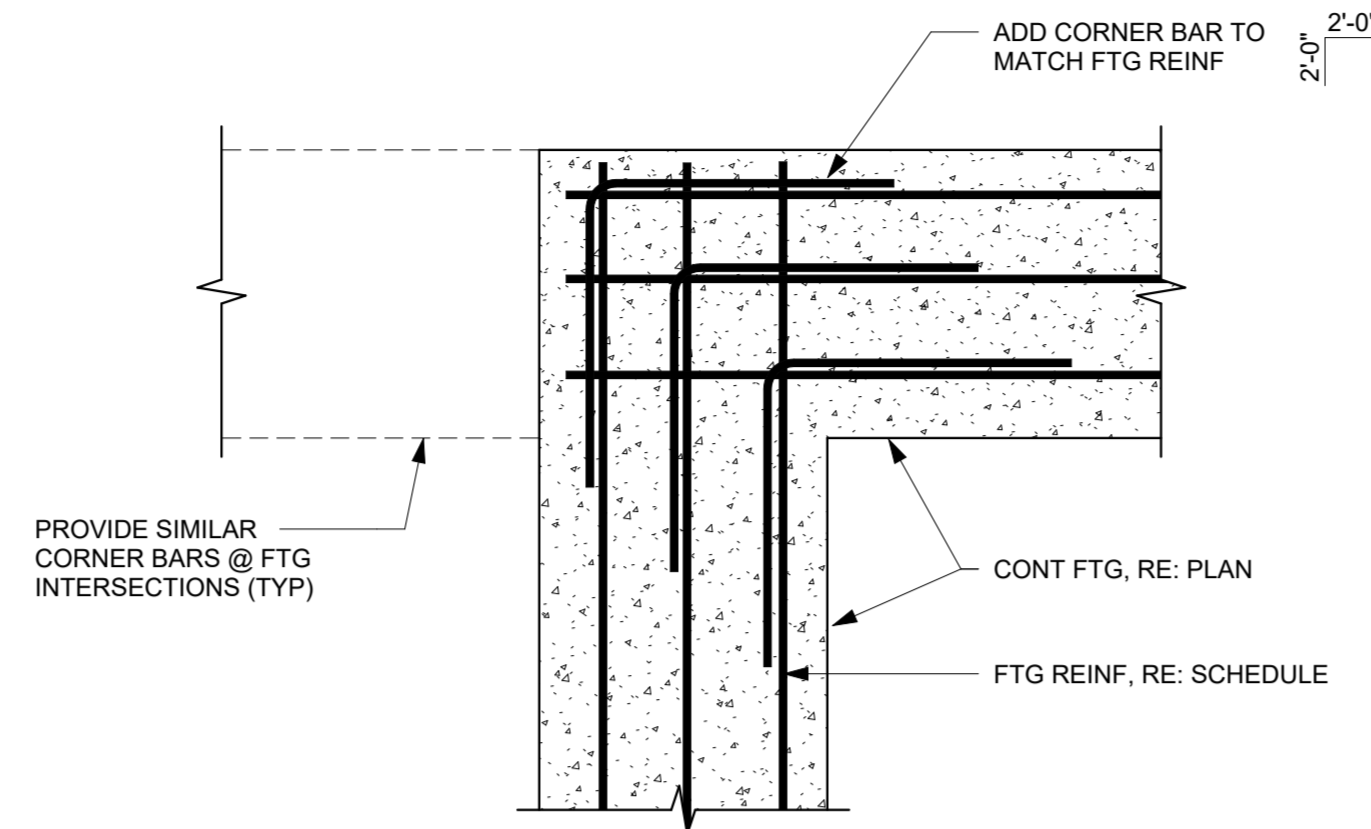


1 TYP CONC CONTROL JOINT DETAILS

3/4" = 1'-0"

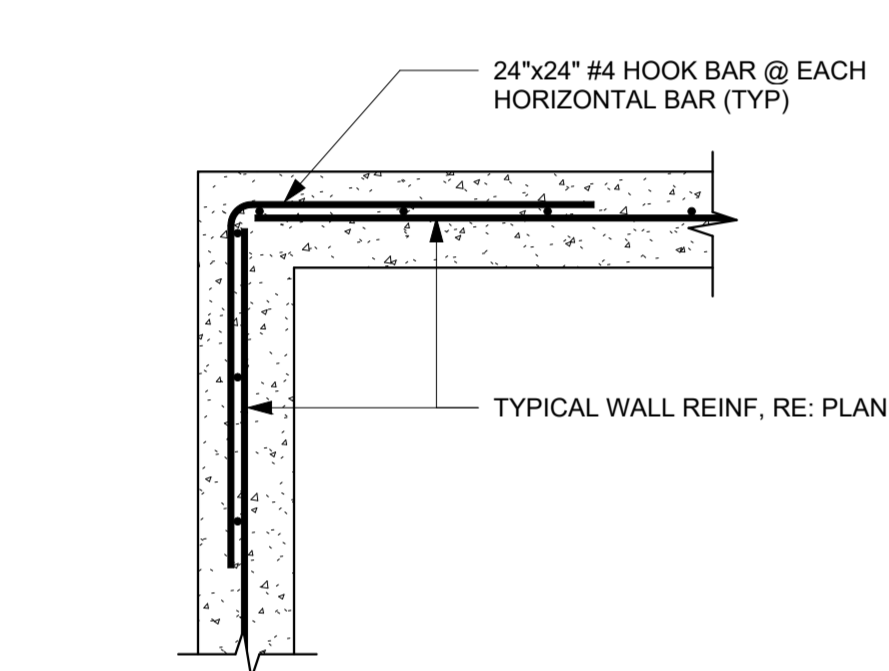
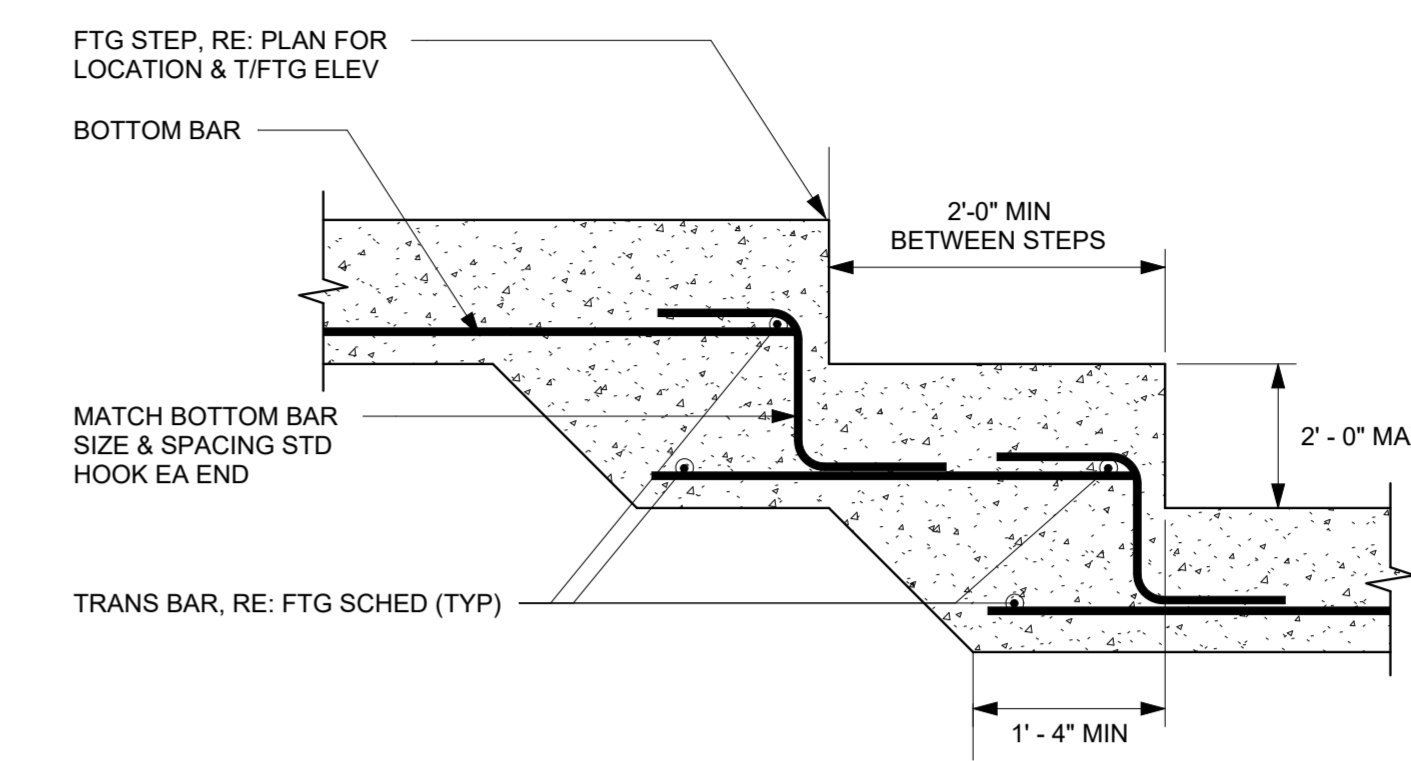
2 TYP CONC CORNER BAR DETAIL

3/4" = 1'-0"

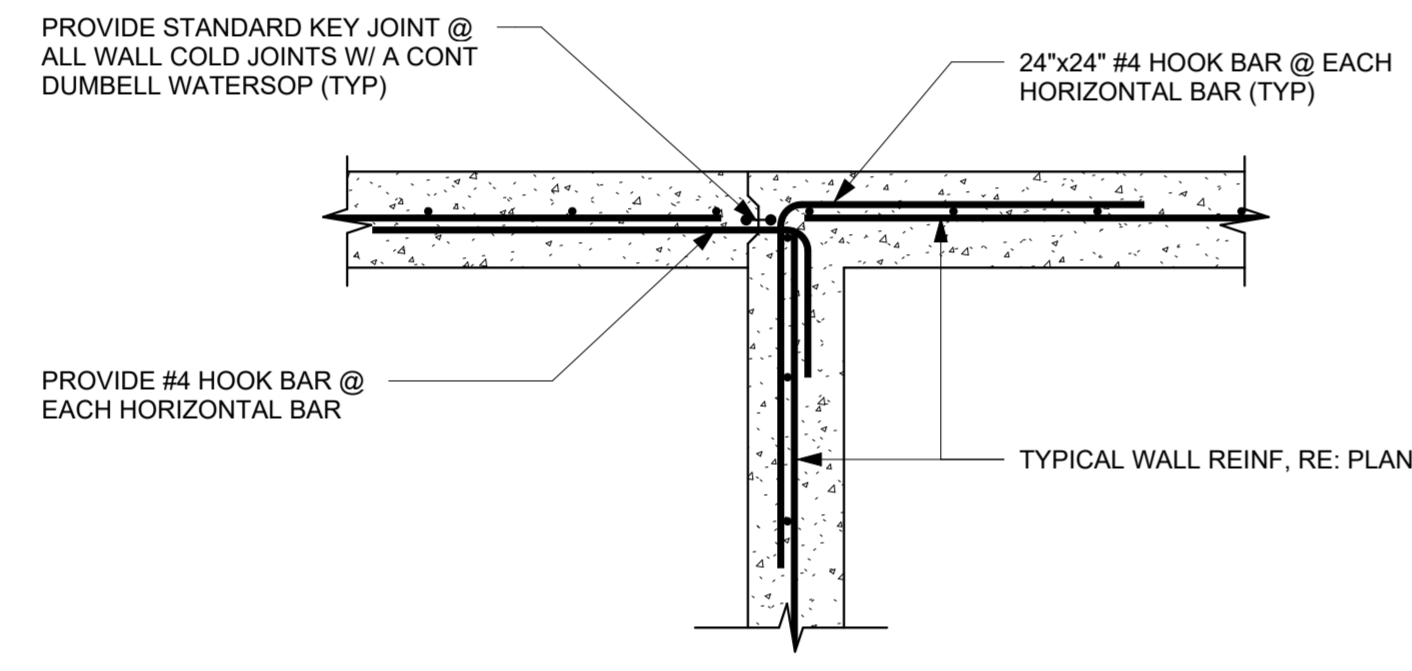


3 TYP CONC FOOTING STEP

3/4" = 1'-0"



TYPICAL WALL CORNER REINFORCING



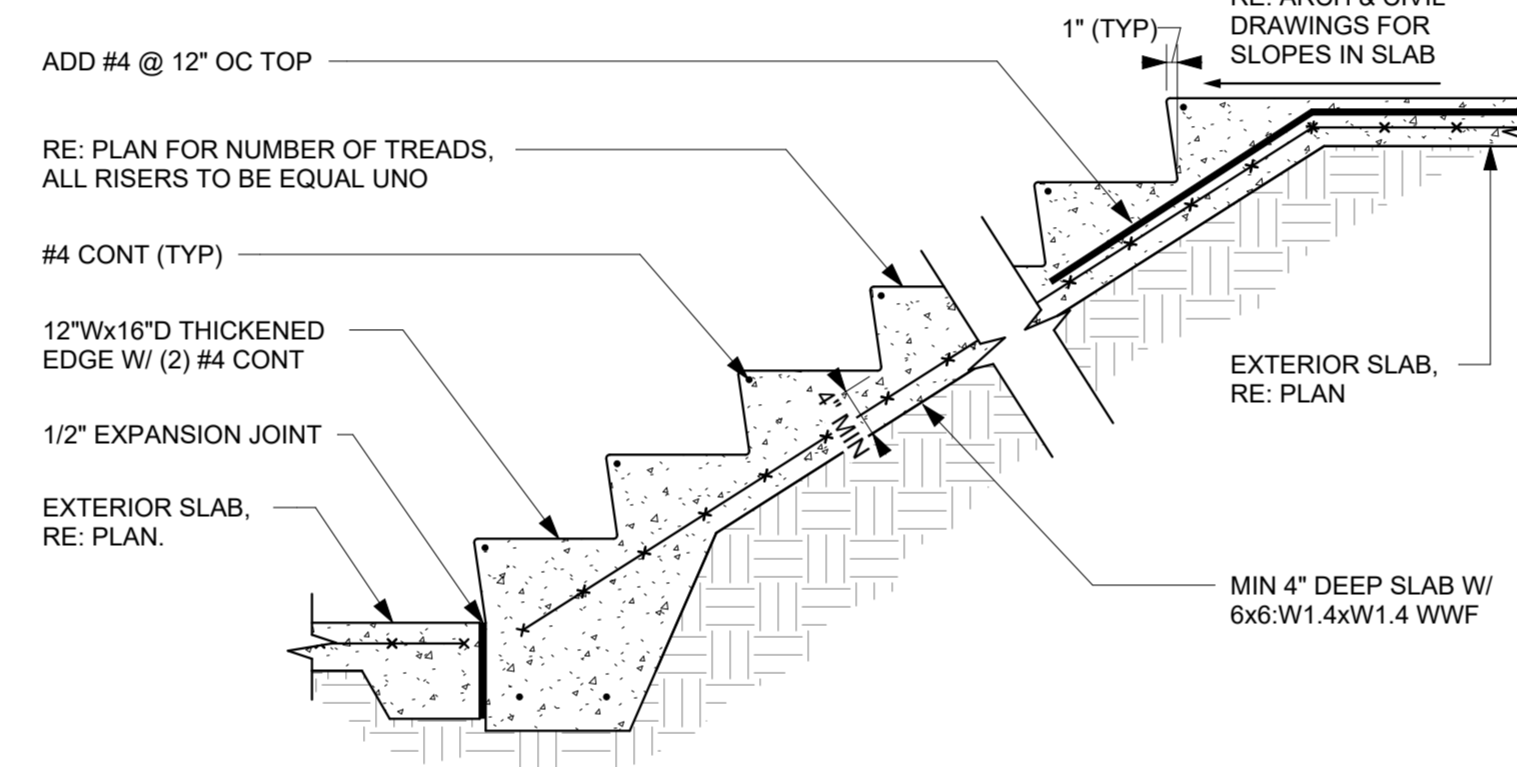
TYPICAL WALL INTERSECTION REINFORCING

4 TYP CONC STEMWALL DETAILS

3/4" = 1'-0"

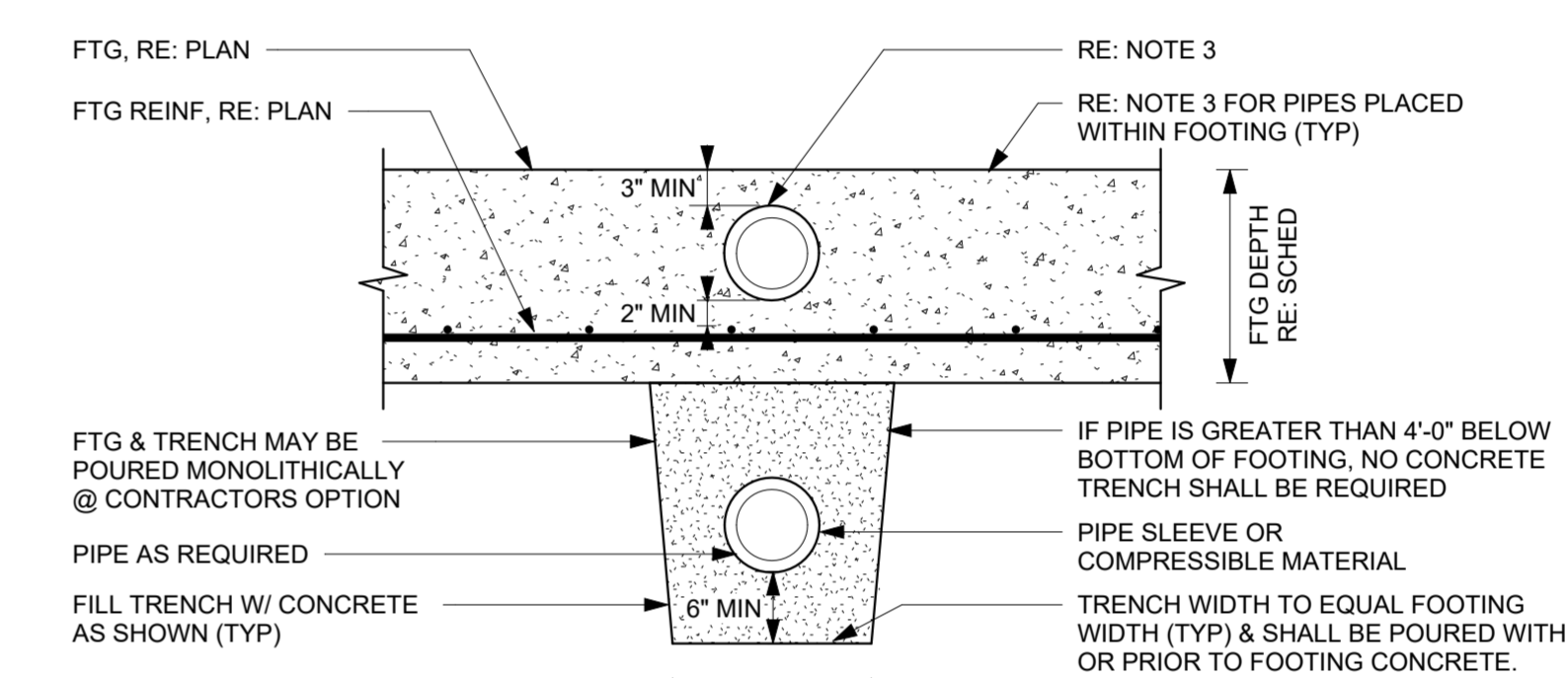
5 TYP CONC STAIR DETAIL

3/4" = 1'-0"



6 TYP CONC PIPE PENETRATION DETAIL

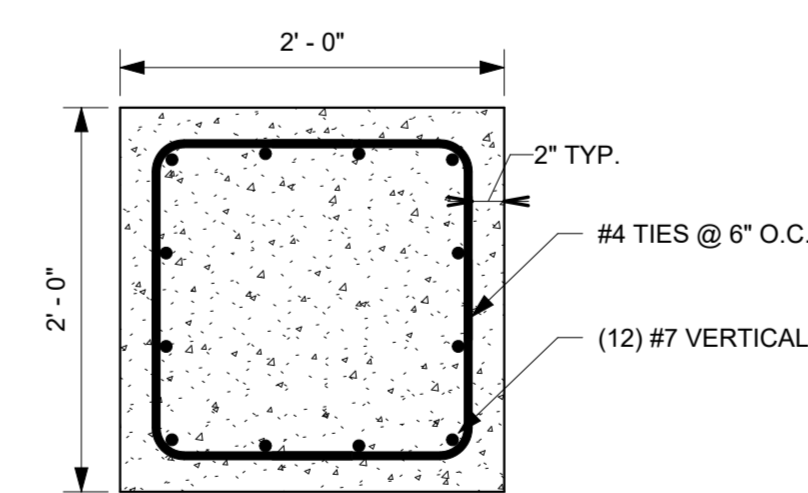
3/4" = 1'-0"



- NOTES:**
- FOR PIPES RUNNING PARALLEL TO STRIP FOOTINGS, FOOTING DEPTH AND PIPE LOCATION SHALL BE COORDINATED SO THAT THE PIPE IS NOT WITHIN THE FTG LOAD DISTRIBUTION THAT EXTENDS AT 45° ANGLE OUT FROM THE BOTTOM EDGE OF THE FOOTING.
 - PIPES MAY BE PLACED BETWEEN THE FOOTING AND THE SLAB ON GRADE THROUGH THE STEM WALL AS LONG AS A PROPER PIPE SLEEVE IS PROVIDED TO ACCOUNT FOR 1" MIN VERTICAL MOVEMENT.
 - FOR PIPES RUNNING THROUGH FOOTING, PIPE SHALL PASS ABOVE FOOTING REINF. DO NOT BREAK FOOTING REINF AND MAINTAIN 3" MIN CONCRETE COVER. MAINTAIN 2" OF CONCRETE BETWEEN PIPE SLEEVE AND REINFORCEMENT, MIN.

- NOTES:**
- NOTATIONS:
 d_b : NOMINAL BAR DIAMETER (INCHES)
 L_d : TENSION DEVELOPMENT LENGTH (INCHES) FOR REINFORCEMENT SATISFYING THE FOLLOWING REQUIREMENTS:
 SLABS AND WALLS: CLEAR SPACING > $26d_b$ AND CONCRETE CLEAR COVER > d_b
 BEAMS AND COLUMNS: CLEAR SPACING > d_b AND CONCRETE CLEAR COVER > d_b
 L_d : DEVELOPMENT LENGTH OF BARS IN THICK CONCRETE = $1.3 \times L_d$ (INCHES)
 L_c : DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = $19 \times d_b$ (INCHES)
 L_c : TIED COLUMN LAP SPLICE IN COMPRESSION = $30 \times d_b$ (INCHES)
 L_{cs} : SPIRAL COLUMN LAP SPLICE IN COMPRESSION = $22.5 \times d_b$ (INCHES)
 L_c : TYPICAL LAP SPLICE LENGTH = $1.3 \times L_d$ (INCHES)
 L_{cb} : LAP SPLICE LENGTH OF HORIZONTAL BARS IN THICK CONCRETE = $1.69 \times L_d$ (INCHES)
 2. MULTIPLY VALUES IN THE TABLE BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET THE REQUIREMENTS FOR L_d IN NOTE 1.
 3. "HORIZONTAL BARS IN THICK CONCRETE" REFERS TO BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW. THIS INCLUDES BEAMS, SLABS, FOUNDATIONS, AND WALLS.
 4. THE DEVELOPMENT AND SPLICE LENGTHS ARE BASED ON REINFORCEMENT STRENGTH $F_y = 60$ KSI.
 5. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. SEE "GENERAL NOTES".
 6. MULTIPLY VALUES IN THE TABLE BY 1.3 FOR USE WITH LIGHTWEIGHT AGGREGATE CONCRETE.

| $f_c = 3,000$ PSI | | | | | $f_c = 4,000$ PSI | | | | | $f_c = 5,000$ PSI | | | | | $f_c = 6,000$ PSI | | | | |
|-------------------|-------|-------|-------|----------|-------------------|-------|-------|-------|----------|-------------------|-------|-------|-------|----------|-------------------|-------|-------|-------|----------|
| BAR SIZE | L_d | L_c | L_s | L_{cb} | BAR SIZE | L_d | L_c | L_s | L_{cb} | BAR SIZE | L_d | L_c | L_s | L_{cb} | BAR SIZE | L_d | L_c | L_s | L_{cb} |
| #3 | 17 | 22 | 22 | 28 | #3 | 15 | 19 | 19 | 25 | #3 | 13 | 17 | 17 | 22 | #3 | 12 | 16 | 16 | 21 |
| #4 | 22 | 29 | 29 | 38 | #4 | 19 | 25 | 25 | 33 | #4 | 17 | 23 | 23 | 29 | #4 | 16 | 21 | 21 | 27 |
| #5 | 28 | 36 | 36 | 47 | #5 | 24 | 31 | 31 | 41 | #5 | 22 | 28 | 28 | 36 | #5 | 20 | 26 | 26 | 33 |
| #6 | 33 | 43 | 43 | 56 | #6 | 29 | 37 | 37 | 49 | #6 | 26 | 34 | 34 | 44 | #6 | 24 | 31 | 31 | 40 |
| #7 | 48 | 63 | 63 | 81 | #7 | 42 | 54 | 54 | 71 | #7 | 38 | 49 | 49 | 63 | #7 | 34 | 45 | 45 | 58 |
| #8 | 55 | 72 | 72 | 93 | #8 | 48 | 62 | 62 | 81 | #8 | 43 | 56 | 56 | 72 | #8 | 39 | 51 | 51 | 66 |
| #9 | 62 | 81 | 81 | 105 | #9 | 54 | 70 | 70 | 91 | #9 | 48 | 63 | 63 | 81 | #9 | 44 | 57 | 57 | 74 |
| #10 | 70 | 91 | 91 | 118 | #10 | 61 | 79 | 79 | 102 | #10 | 54 | 71 | 71 | 92 | #10 | 50 | 64 | 64 | 84 |
| #11 | 78 | 101 | 101 | 131 | #11 | 67 | 87 | 87 | 114 | #11 | 60 | 78 | 78 | 102 | #11 | 55 | 71 | 71 | 93 |
| #14 | 93 | 121 | -- | -- | #14 | 81 | 105 | -- | -- | #14 | 72 | 94 | -- | -- | #14 | 66 | 86 | -- | -- |
| #18 | 124 | 161 | -- | -- | #18 | 108 | 140 | -- | -- | #18 | 96 | 125 | -- | -- | #18 | 88 | 114 | -- | -- |



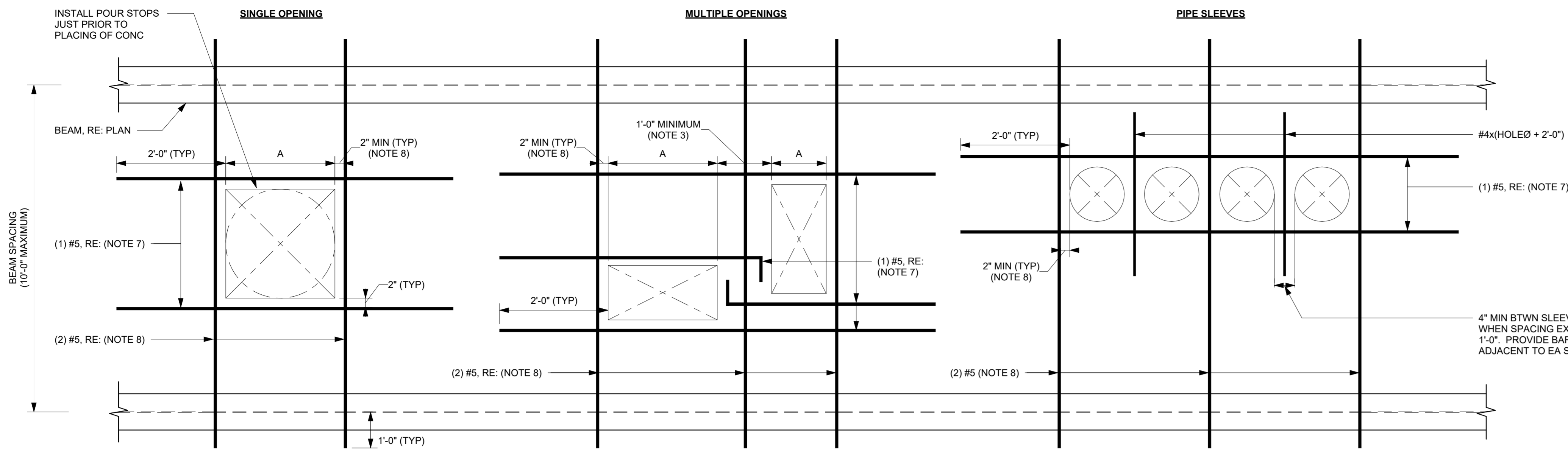
8 TYP CONC PIER DETAIL

1" = 1'-0"

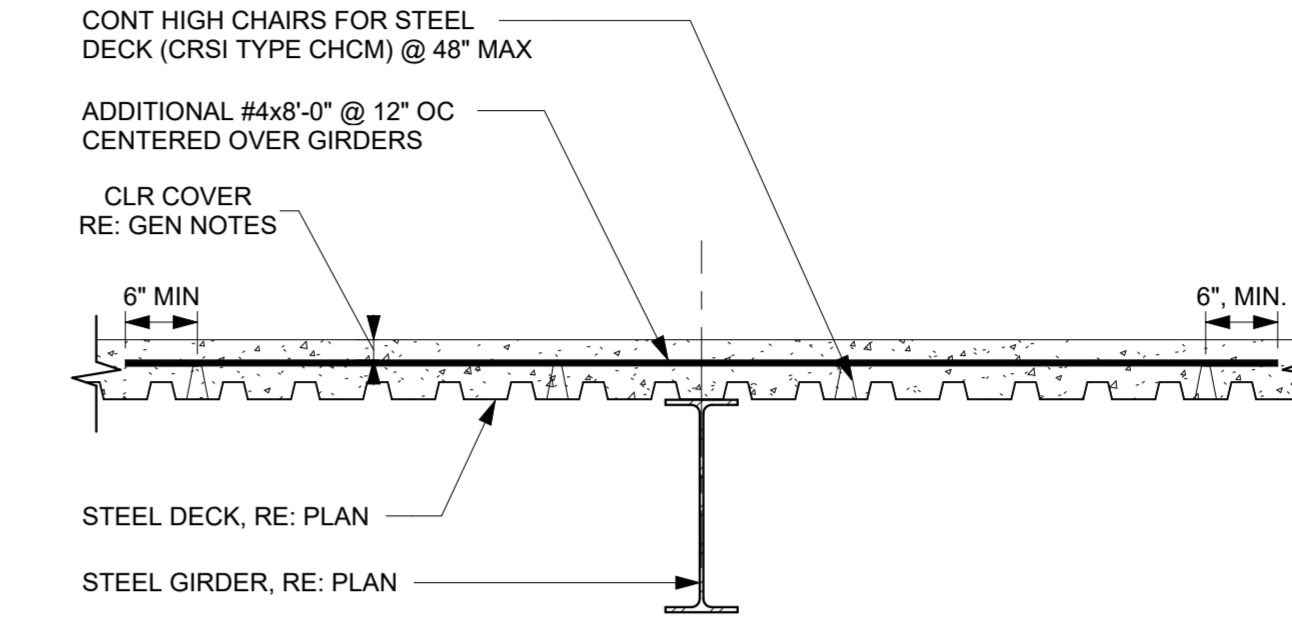
7 CONC. DEVELOP. AND SPLICE LENGTH SCHED.

3/4" = 1'-0"





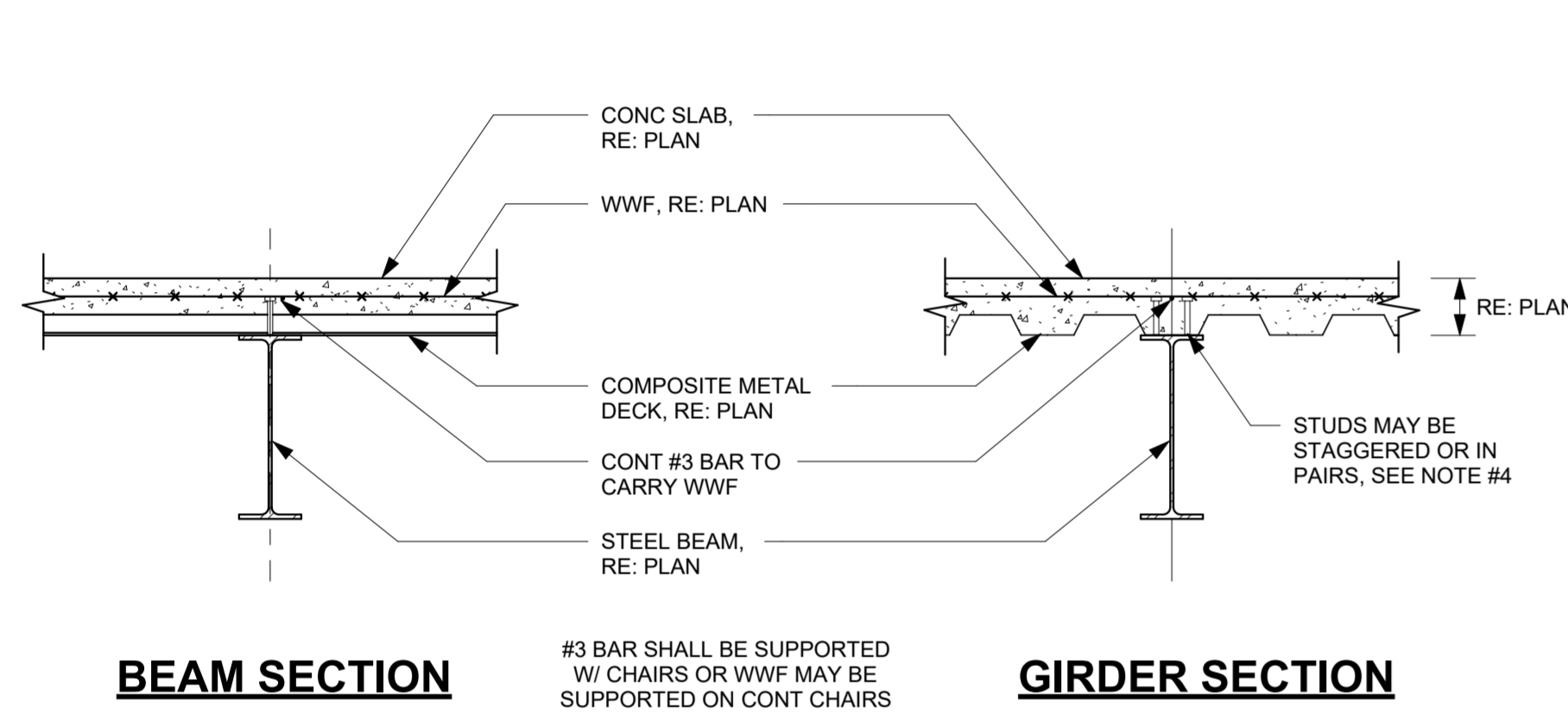
- NOTES:**
- COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
 - "A" IS THE OPENING DIMENSION IN THE DIRECTION PERPENDICULAR TO THE DECK SPAN. THIS DETAIL IS APPLICABLE FOR A MAXIMUM "A" DIMENSION OF 4'-0". RE: PLAN OR OTHER DETAILS FOR FRAMING OF OPENINGS GREATER THAN 4'-0".
 - WHEN SPACING BETWEEN MULTIPLE OPENINGS IS 2'-0" OR GREATER, REINFORCEMENT SHALL BE PLACED AS SHOWN FOR SINGLE OPENING CONDITIONS.
 - SLAB REINFORCEMENT SHALL BE CUT AROUND SLAB OPENINGS AND SHALL EXTEND TO WITHIN 2" OF OPENINGS ON ALL SIDES.
 - OPENINGS OR GROUPS OF OPENINGS 10" AND SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
 - FIELD-CUT OPENINGS IN DECK AFTER CONCRETE SLAB HAS BEEN PLACED AND HAS ATTAINED AT LEAST 75% OF ITS SPECIFIED 28-DAY COMPRESSIVE STRENGTH. DO NOT FIELD-CUT DECK UNTIL IMMEDIATELY BEFORE OPENING IS NEEDED.
 - REINFORCEMENT PERPENDICULAR TO DECK SPAN SHALL BE PLACED ON TOP OF DECK FLUTES AND SHALL BE CHAIRED TO PROVIDE ADEQUATE COVER.
 - REINFORCEMENT PARALLEL TO DECK SPAN SHALL BE PLACED IN THE NEAREST LOWER FLUTE THAT PROVIDES AT LEAST 2" EDGE DISTANCE AND SHALL BE CHAIRED TO PROVIDE ADEQUATE COVER.
 - THIS DETAIL SHOWS TYPICAL CONDITIONS. VERIFY REINFORCING STEEL PLACEMENT WITH ENGINEER FOR SPECIAL CASES OR WHEN DIMENSIONS EXCEED MAXIMUM DIMENSIONS SHOWN IN THIS DETAIL.
 - PROVIDE REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR, OR ROUND OPENINGS.



- NOTE:**
- REINFORCEMENT SHOWN OVER GIRDERS IS IN ADDITION TO THE TYPICAL SLAB REINFORCEMENT SPECIFIED ON THE DRAWINGS.

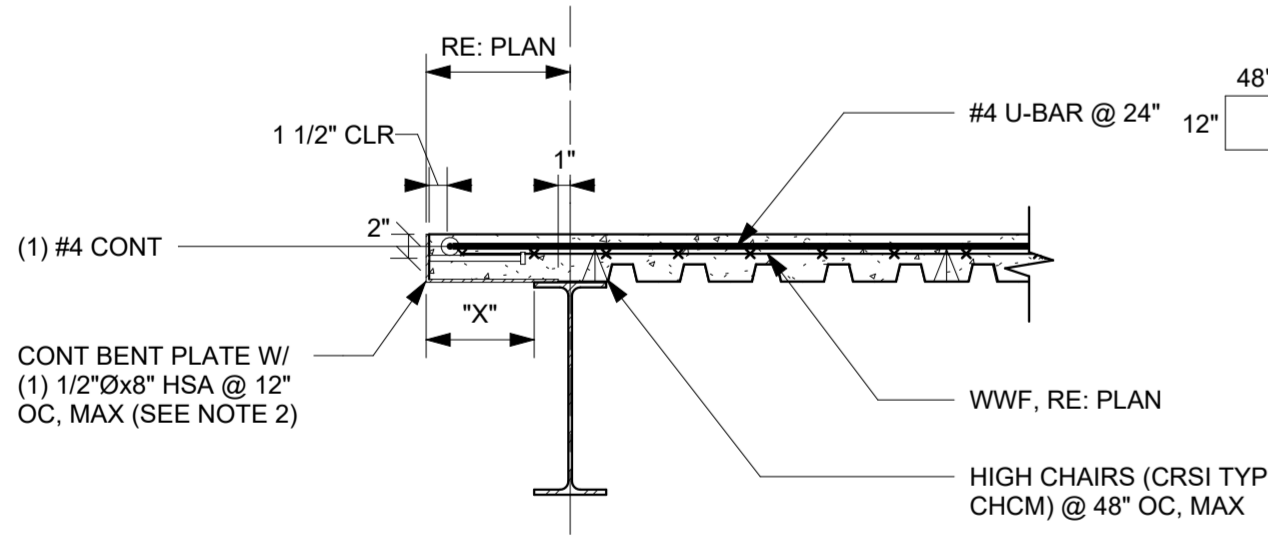
1 TYP COMPOSITE SLAB OPENINGS (SMALL)

3/4" = 1'-0"

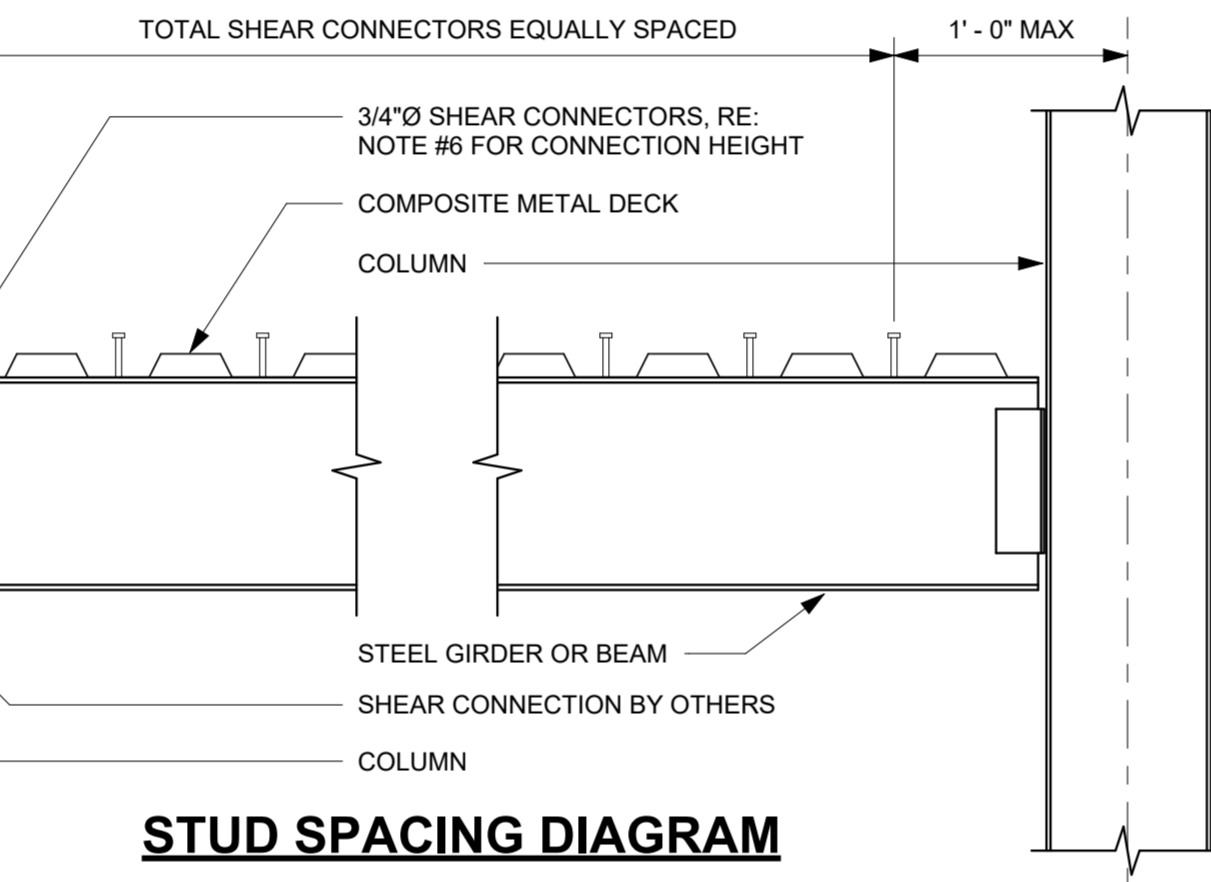


3 TYP COMPOSITE CONSTRUCTION DETAILS

3/4" = 1'-0"



- NOTES:**
- THIS DETAIL DOES NOT APPLY WHEN DISTANCE "X" EXCEEDS 1'-4".
 - TYPICAL 1/4" THICK PLATE FOR DIMENSION "X" ≤ 12". FOR "X" > 12" INCREASE PLATE THICKNESS TO 3/8".

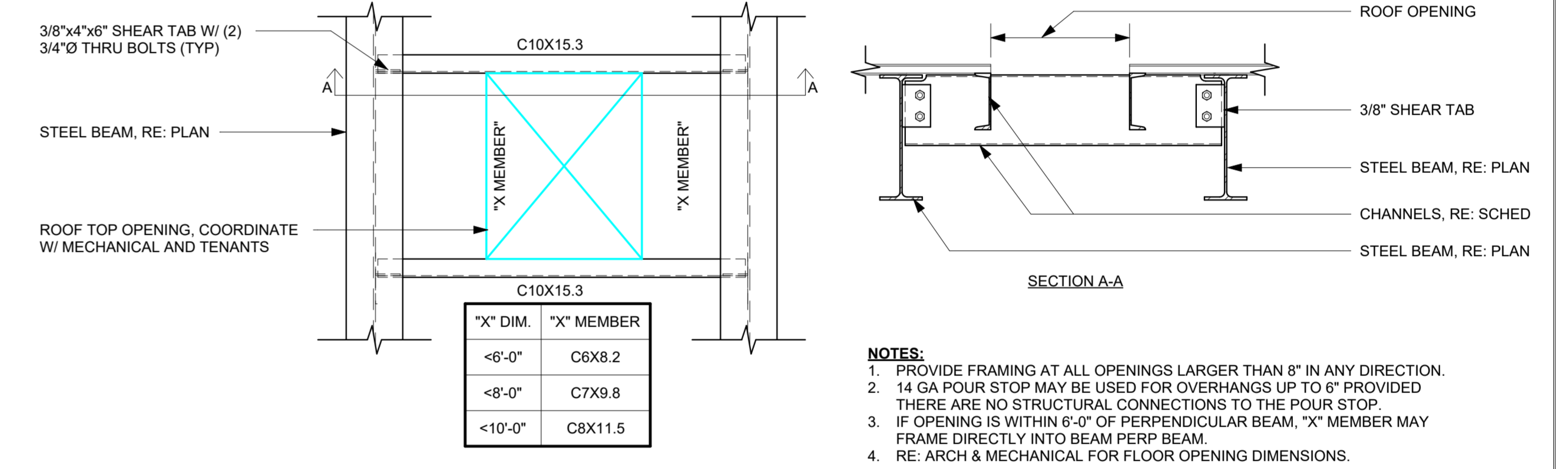


- NOTE:**
- ABOVE DIAGRAM SHOWS STUD PLACEMENT FOR ALL BEAMS AND GIRDERS

- COMPOSITE BEAM NOTES:**
- ON PLANS COMPOSITE BEAM SIZE IS FOLLOWED BY TOTAL NUMBER OF SHEAR CONNECTORS IN BRACKETS.
 - AT BEAMS, SHEAR CONNECTORS (STUDS) ARE EQUALLY SPACED BETWEEN ENDS. AT GIRDERS, THE FIRST NUMBER IS THE AMOUNT OF STUDS SPACED EQUALLY BETWEEN THE GIRDER END AND THE FIRST BEAM. THE SECOND NUMBER IS THE AMOUNT OF STUDS SPACED EQUALLY BETWEEN THE FIRST BEAM AND THE NEXT BEAM, ECT.
 - WELDED STUDS ARE APPLIED TO THE TOP FLANGE AND ARE INSTALLED AFTER THE METAL DECK IS IN PLACE (FIELD WELDED).
 - MINIMUM STUD SPACING SHALL BE 4 1/2" OC. PLACE STUDS IN PAIRS ACROSS LENGTH OF GIRDER WHEN REQD TO KEEP STUD SPACING EQUAL TO OR GREATER THAN 4 1/2" OC.
 - TOP SURFACE OF BEAM FLANGE TO RECEIVE STUDS SHALL BE LEFT UNPAINTED AND UNPRIMED.
 - CONNECTOR HEIGHT = DECK HEIGHT + 2" (4" MIN & 4 1/2" MAX).
 - WHERE DECK IS INTERRUPTED TO ALLOW STUD PLACEMENT, WELD DECK TO TOP FLANGE W/ 5/8" PUDDLE WELDS @ 6" OC.
 - BEAMS PERPENDICULAR & PARALLEL TO DECK SPAN SHALL HAVE A MAX STUD SPACING OF 2'-0" OC, UNCL.
 - STUDS SHALL BE PLACED DIRECTLY OVER THE BEAM WEB FOR BEAMS HAVING A FLANGE THICKNESS LESS THAN 0.30 IN.

2 ADDITIONAL REINF @ INTERIOR GIRDERS

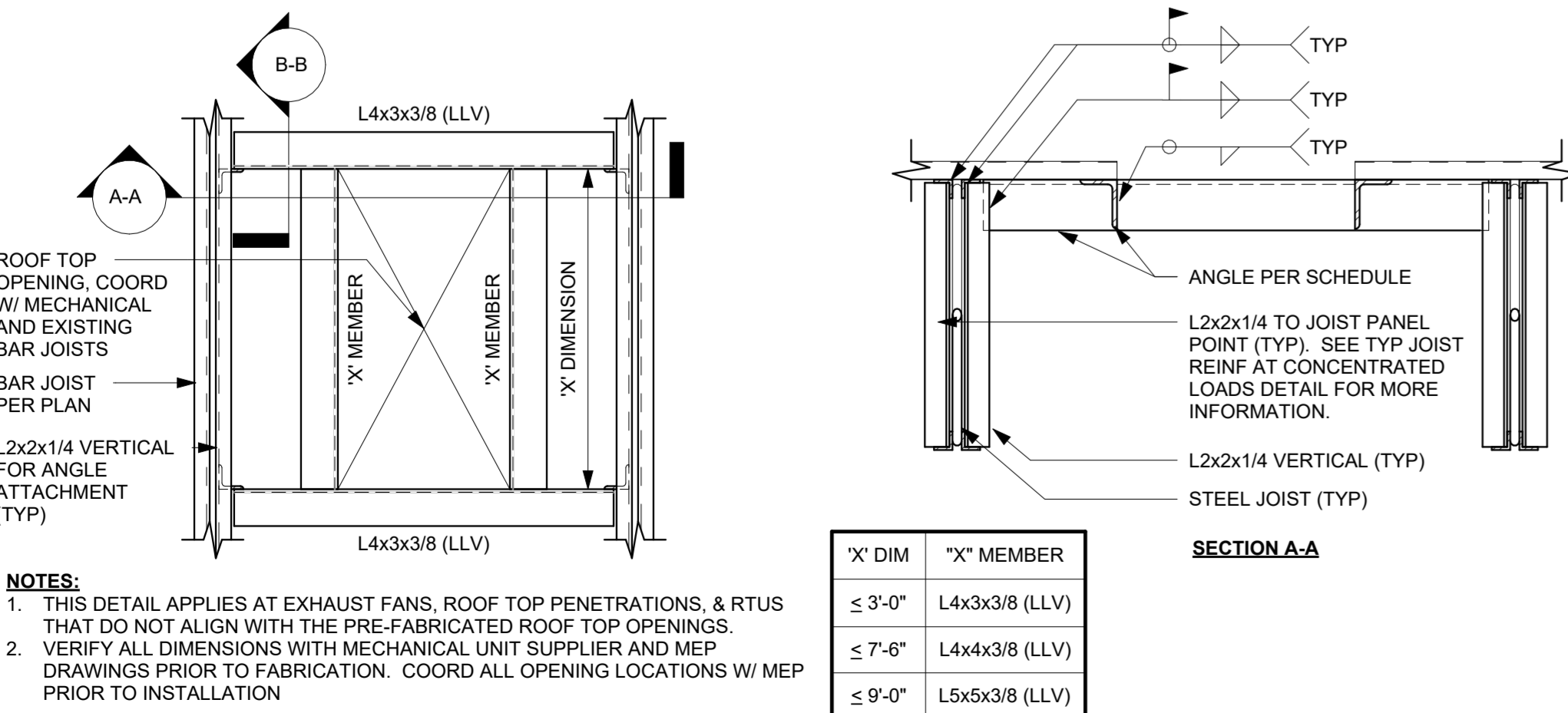
3/4" = 1'-0"



- NOTES:**
- PROVIDE FRAMING AT ALL OPENINGS LARGER THAN 8" IN ANY DIRECTION.
 - 14 GA POUR STOP MAY BE USED FOR OVERHANGS UP TO 6" PROVIDED THERE ARE NO STRUCTURAL CONNECTIONS TO THE POUR STOP.
 - IF OPENING IS WITHIN 6'-0" OF PERPENDICULAR BEAM, "X" MEMBER MAY FRAME DIRECTLY INTO BEAM PERP BEAM.
 - RE: ARCH & MECHANICAL FOR FLOOR OPENING DIMENSIONS.

5 TYP SLAB EDGE WITH BENT PLATE (PARALLEL)

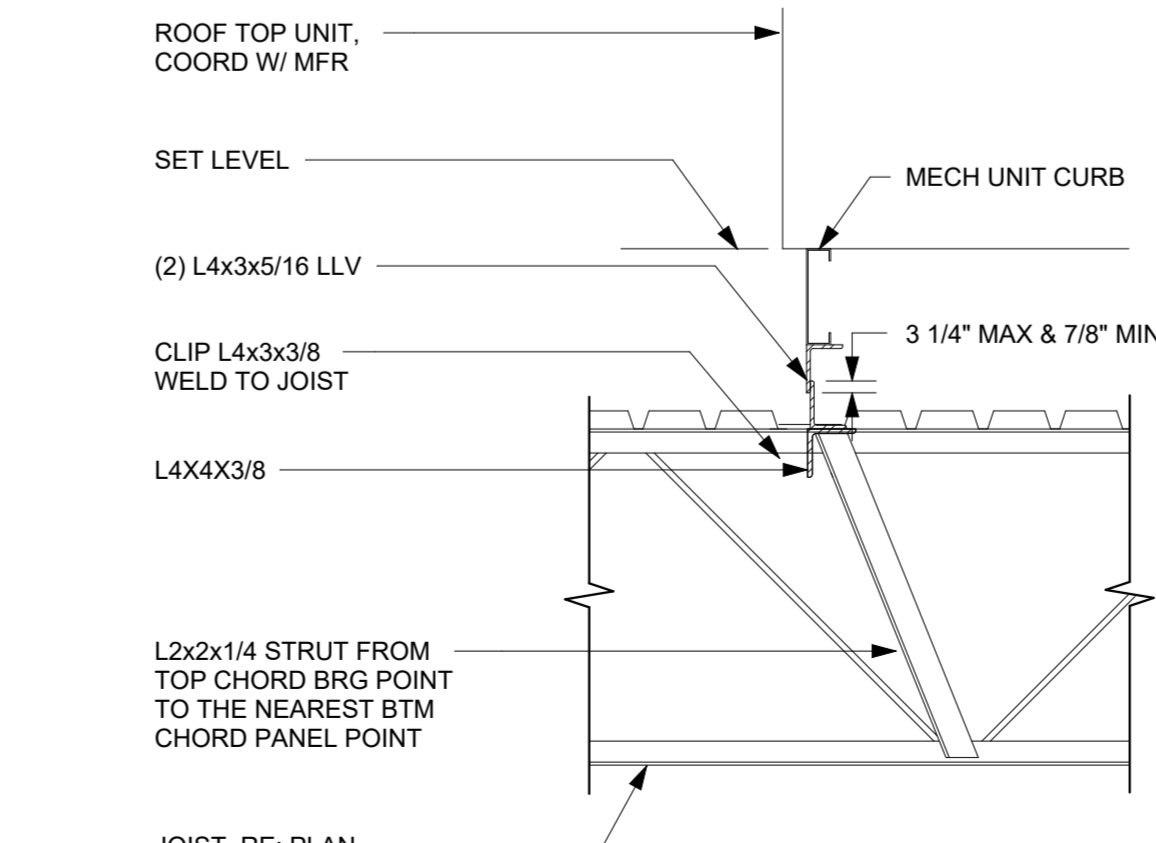
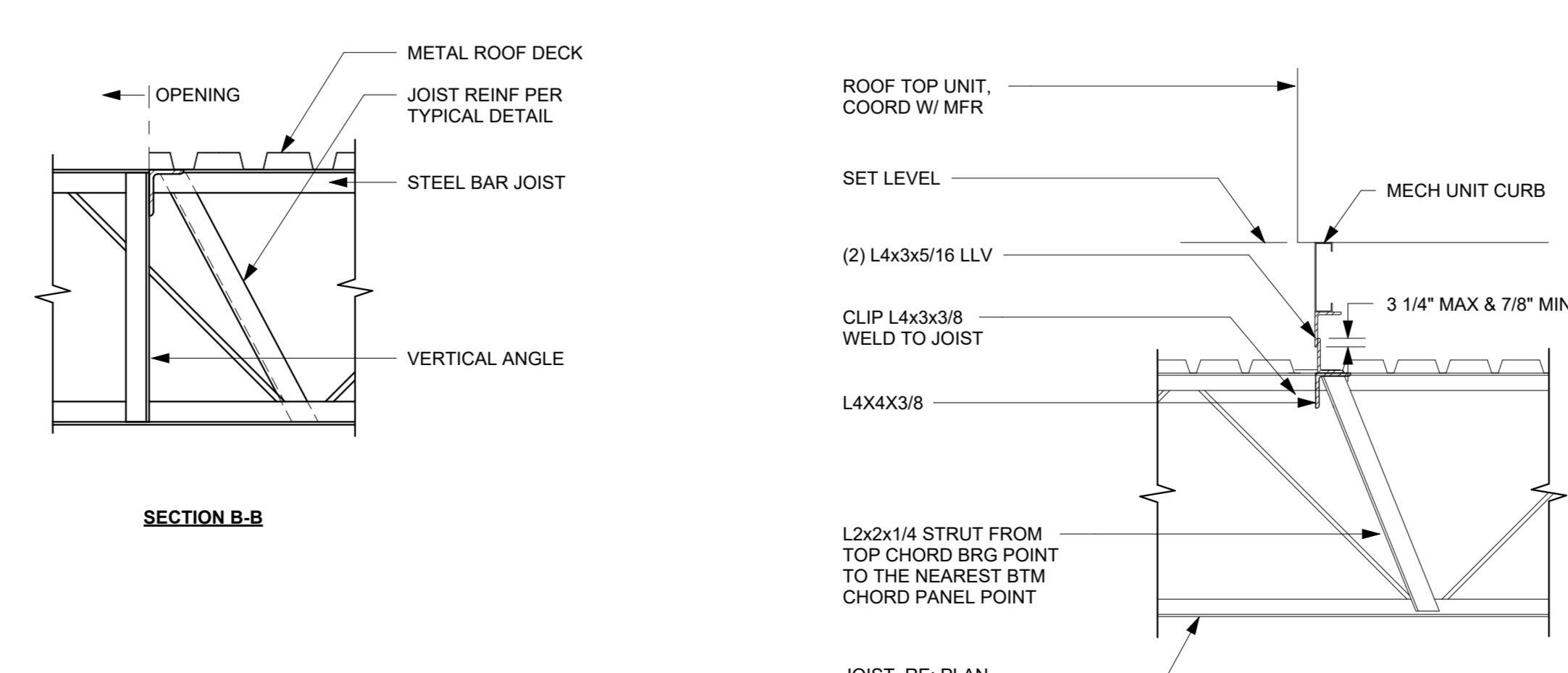
3/4" = 1'-0"



- NOTES:**
- THIS DETAIL APPLIES AT EXHAUST FANS, ROOF TOP PENETRATIONS, & RTUS THAT DO NOT ALIGN WITH THE PRE-FABRICATED ROOF TOP OPENINGS.
 - VERIFY ALL DIMENSIONS WITH MECHANICAL UNIT SUPPLIER AND MEP DRAWINGS PRIOR TO FABRICATION. COORD ALL OPENING LOCATIONS W/ MEP PRIOR TO INSTALLATION.

6 TYP SLAB EDGE WITH BENT PLATE (PERP.)

3/4" = 1'-0"

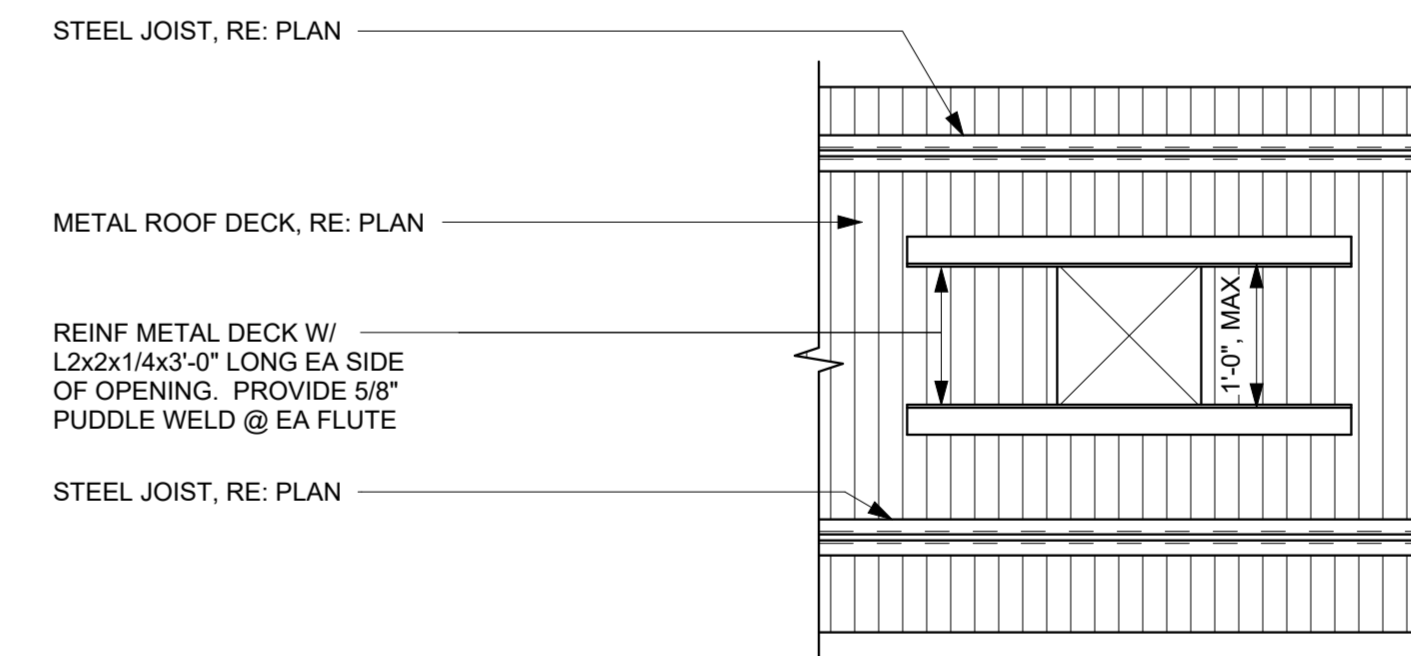


9 TYP STEEL RTU SUPPORT (SEISMIC)

3/4" = 1'-0"

4 TYP STEEL OPENINGS - FLOOR

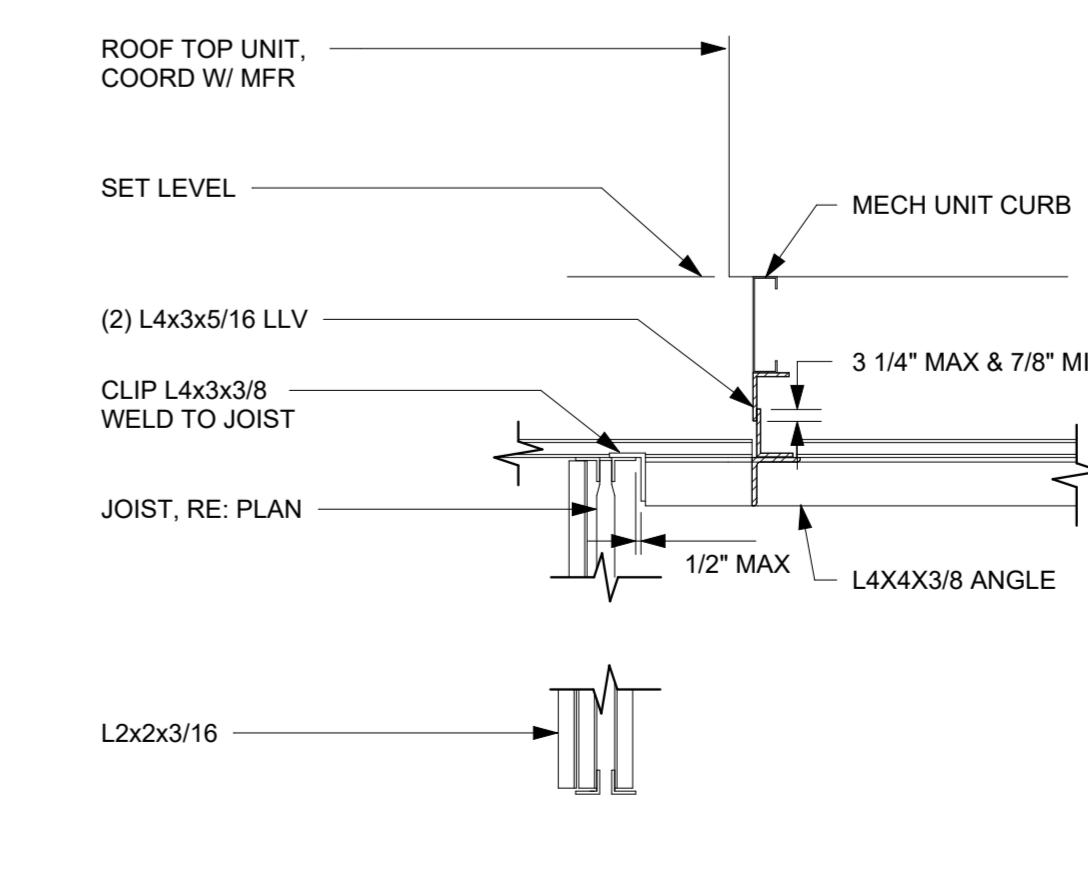
3/4" = 1'-0"



- NOTES:**
- DETAIL TYPICAL FOR OPENING SMALLER THAN 12" SQUARE BUT LARGER THAN 8" SQUARE.
 - IF OPENING IS UNDER ROOF TOP EQUIPMENT WEIGHING MORE THAN 100 LB.

7 TYP STEEL OPENINGS - ROOF TOP (SMALL)

3/4" = 1'-0"



8 TYP STEEL ROOF TOP OPENING FRAMING

1" = 1'-0"





| TYPE | CAPACITY | SINGLE SHEAR PLATE SIZE | # OF BOLTS | BEAM SIZE |
|------|----------|-------------------------|------------|------------------|
| 1 | 11 kips | L6x4x3/8 | 2 | W6, C6 |
| 2 | 21 kips | L3 1/2x3 1/2x5/16 | 2 | W8, W10, C8, C10 |
| 3 | 38 kips | L3 1/2x3 1/2x5/16 | 3 | W12, W14, C12 |
| 4 | 63 kips | L3 1/2x3 1/2x5/16 | 4 | W16, W18 |
| 5 | 109 kips | L3 1/2x3 1/2x5/16 | 5 | W21 |
| 6 | 148 kips | L3 1/2x3 1/2x5/16 | 6 | W24 |

NOTES:
 1. ALL BOLTS SHALL BE ASTM A325 BEARING TYPE BOLTS.
 2. ALL HOLES SHALL BE STANDARD HOLES.
 3. CONNECTION CAPACITIES ARE LRFD FACTORED CAPACITIES BASED ON THE SMALLEST COPED BEAM WEB THICKNESS OF EACH BEAM DEPTH.
 4. ANGLES MAY BE SHOP WELDED SHOP WELDED TO SUPPORTED BEAM AT CONTRACTOR'S OPTION. IF CONNECTION IS WELDED, DO NOT PROVIDE BOLT HOLES IN ANGLE LEGS TO RECEIVE WELDS.
 5. SINGLE CLIP ANGLE CONNECTIONS ARE NOT ALLOWED.
 6. BEAM-TO-BEAM SHOWN. BEAM-TO-COLUMN SIMILAR.

NOTE:
 CONNECTIONS PROVIDED HERE ARE FOR REPRESENTATION PURPOSES ONLY. ACTUAL CONNECTION REQUIREMENTS MAY VARY. COORD.

1 TYP STEEL SHEAR DOUBLE ANGLE CONN SCHED
 1" = 1'-0"

| TYPE | CAPACITY | SINGLE SHEAR PLATE SIZE | # OF BOLTS | BEAM SIZE |
|------|----------|-------------------------|------------|------------------|
| 1 | 10 kips | PL 1/2" x 6" x 3" | 2 | W6, C4, C6 |
| 2 | 30 kips | PL 3/8" x 4" x 5 1/2" | 2 | W8, W10, C8, C10 |
| 3 | 47 kips | PL 3/8" x 4" x 8 1/2" | 3 | W12, W14, C12 |
| 4 | 63 kips | PL 3/8" x 4" x 11 1/2" | 4 | W16, W18 |
| 5 | 79 kips | PL 3/8" x 4" x 15 1/2" | 5 | W21 |
| 6 | 95 kips | PL 3/8" x 4" x 19 1/2" | 6 | W24 |

NOTES:
 1. ALL HOLES SHALL BE STANDARD HOLES.
 2. ALL BOLTS SHALL BE ASTM A325 BEARING TYPE BOLTS.
 3. DO NOT COPE BEAMS AT STANDARD SHEAR TAB CONNECTIONS.
 4. ALL CONNECTION CAPACITIES ARE LRFD FACTORED CAPACITIES BASED ON THE SMALLEST BEAM WEB THICKNESS OF EACH BEAM DEPTH.

NOTE:
 CONNECTIONS PROVIDED HERE ARE FOR REPRESENTATION PURPOSES ONLY. ACTUAL CONNECTION REQUIREMENTS MAY VARY. COORD.

2 TYP STEEL SHEAR TAB CONN SCHED
 1" = 1'-0"

NOTES:
 1. SNUG-TIGHTENED BOLTED CONNECTION USING A325 BOLTS IN HORIZONTAL SHORT-SLOTTED HOLES. RESISTANCE BY BOLTS TO SHEAR SHALL BE BY SHEAR-BEARING.
 2. FABRICATOR SHALL ASSUME WORKPOINT TO BE LOCATED AT CENTERLINE OF COLUMN FOR VALUES OF 2x1" AND PROVIDE CONNECTION AS SPECIFIED IN DETAIL A. FABRICATOR SHALL ASSUME LOCATION OF WORKPOINT TO BE AT COLUMN FLANGE-WEB INTERSECTION AND PROVIDE CONNECTION AS SPECIFIED IN DETAIL B WHERE CLEARANCE DOES NOT EXIST FOR DETAIL A TO APPLY. FABRICATOR SHALL APPLY DETAIL C ONLY WHEN LOCATION OF WORKPOINT MUST BE LOCATED AT COLUMN CENTERLINE AND CLEARANCE DOES NOT EXIST FOR DETAIL A TO APPLY. LOCATION OF WORKPOINTS SHALL BE APPROVED BY THE ENGINEER PRIOR TO SHOP DRAWING SUBMITTAL. DETAIL C SHALL NOT BE USED FOR LOADS GREATER THAN 40 KIPS.

3. SINGLE PLATE CONNECTION MAY BE USED ONLY WHERE WORKPOINT IS AT INTERSECTION OF COLUMN FLANGE AND WEB.
 4. SINGLE BENT PLATES SHALL BE DESIGNED FOLLOWING PROCEDURES OUTLINED IN THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL.
 5. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.

3 TYP STEEL SKEWED SHEAR CONN. AT COLUMN
 3/4" = 1'-0"

NOTE:
 1. PROVIDE CJP WELD FOR FULL WIDTH OF COLUMN OR BEAM FLANGE, WHICHEVER IS SMALLER.

NOTE:
 1. PROVIDE CJP WELD FOR FULL WIDTH OF COLUMN OR BEAM FLANGE, WHICHEVER IS SMALLER.

4 TYP STEEL MOMENT CONN. - HSS COLUMNS
 3/4" = 1'-0"

NOTES:
 1. BEAM FLANGES SHALL BE CONNECTED TO COLUMN FLANGES USING COMPLETE JOINT-PENETRATION (CJP) GROOVE WELDS. BEAM FLANGE WELDS SHALL CONFORM TO THE REQUIREMENTS FOR DEMAND CRITICAL WELDS IN SECTION 7.3 AND APPENDIX W OF THE AISC SEISMIC PROVISIONS. ("THE AISC SEISMIC PROVISIONS" OR "SEISMIC PROVISIONS" AS USED HERE IN REFER TO: AISC 341-16 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS).
 2. WELD ACCESS HOLE GEOMETRY SHALL CONFORM TO THE REQUIREMENTS OF AISC 360-16 SECTION J1.6.

5 TYP STEEL MOMENT CONNECTIONS - RBS
 1" = 1'-0"

| BEAM SIZE | REDUCED BEAM SECTION (RBS) DIMENSIONS | | | LOADING CRITERIA | | |
|-----------|---------------------------------------|-----|-------|------------------|------|------|
| | a | b | c | R | V | M |
| W27X94 | 6" | 18" | 2.25" | 19.15' | 41.5 | 24.5 |
| W18X71 | 4" | 13" | 1.61" | 13.96' | 32 | 21.2 |
| W18X60 | 4" | 12" | 1.59" | 12.10' | 36.5 | 18 |
| W16X89 | 6" | 12" | 1.98" | 10.09' | 52.5 | 27 |
| W14X48 | 4" | 10" | 1.57" | 8.74' | 24 | 0 |
| W14X38 | 4" | 10" | 1.38" | 9.72' | 16 | 6 |
| W12X35 | 4" | 9" | 1.29" | 8.47' | 17 | 10 |

NOTE: LOADS IN TABLE ARE FACTORED LRFD VALUES. SHEAR (V) AND AXIAL (A) LOADS ARE IN UNITS OF KIPS, MOMENTS (M) ARE IN UNITS OF KIP-FT.

6 TYP STEEL STAIR BASE DETAIL
 1" = 1'-0"

NOTES:
 1. RE: GENERAL NOTES FOR DECK TYPE AND FASTENER PATTERN.
 2. ATTACHMENT SIDE LAPS SHALL BE #10 TEK SCREWS, U.N.O.
 3. ATTACHMENT AROUND OPENINGS SHALL BE WELDS/SCREWS PER SCHEDULE @ 6" OC.
 4. AT ROOF PERIMETER, PROVIDE WELDS/SCREWS PER SCHEDULE @ 6" OC.
 5. PROVIDE A MINIMUM END LAP OF 3" (TYP)
 6. DECK SHALL BEAR 2 1/2" MIN ON STRUCTURAL STEEL UNLESS NOTED OTHERWISE ON PLAN.
 7. METAL DECKING SHALL BE CONTINUOUS OVER AT LEAST TWO SUPPORTS.

7 TYP STEEL METAL DECK FASTENER PATTERN
 1/2" = 1'-0"

| COLUMN SIZE | "A" | "B" | "E" | PLATE THICK | BOLTS F1554 GR 105 | ANCHOR EMBED |
|-------------|-------|-------|--------|-------------|--------------------|--------------|
| W10 | 1'-8" | 1'-8" | 2 1/2" | 1 1/2" | (4) 1 1/2"Ø | 15" |
| W12 | 2'-1" | 2'-1" | 2 3/4" | 2" | (4) 1 1/2"Ø | 15" |
| W14 | 2'-3" | 2'-3" | 3 1/4" | 2" | (6) 2"Ø | 25" |

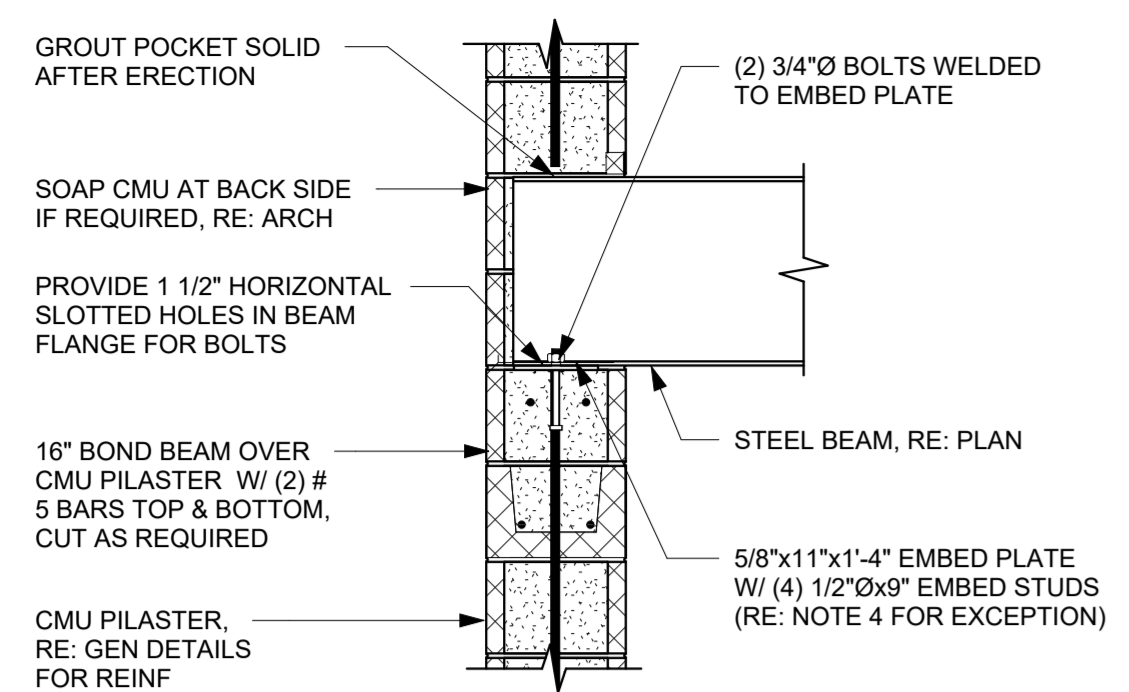
NOTES:
 1. BASEPLATE W/ 1" (MIN) GROUT, RE: SCHEDULE FOR PLATE AND BOLT SIZE
 2. COLUMN BLOCKOUT

8 TYP STEEL COLUMN BASEPLATE
 1" = 1'-0"

NOTE:
 1. EXTEND THICKENED EDGE 6" PAST STRINGER FACE.

9 TYP STEEL MOMENT CONNECTIONS - WIDE FLANGE COLUMNS
 3/4" = 1'-0"

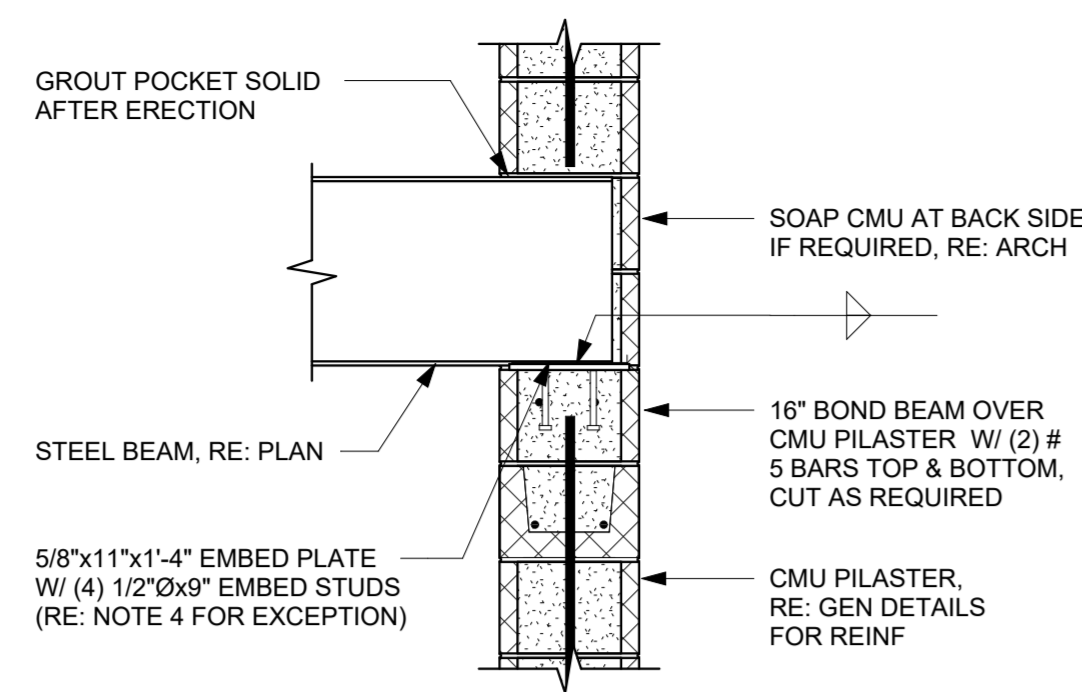




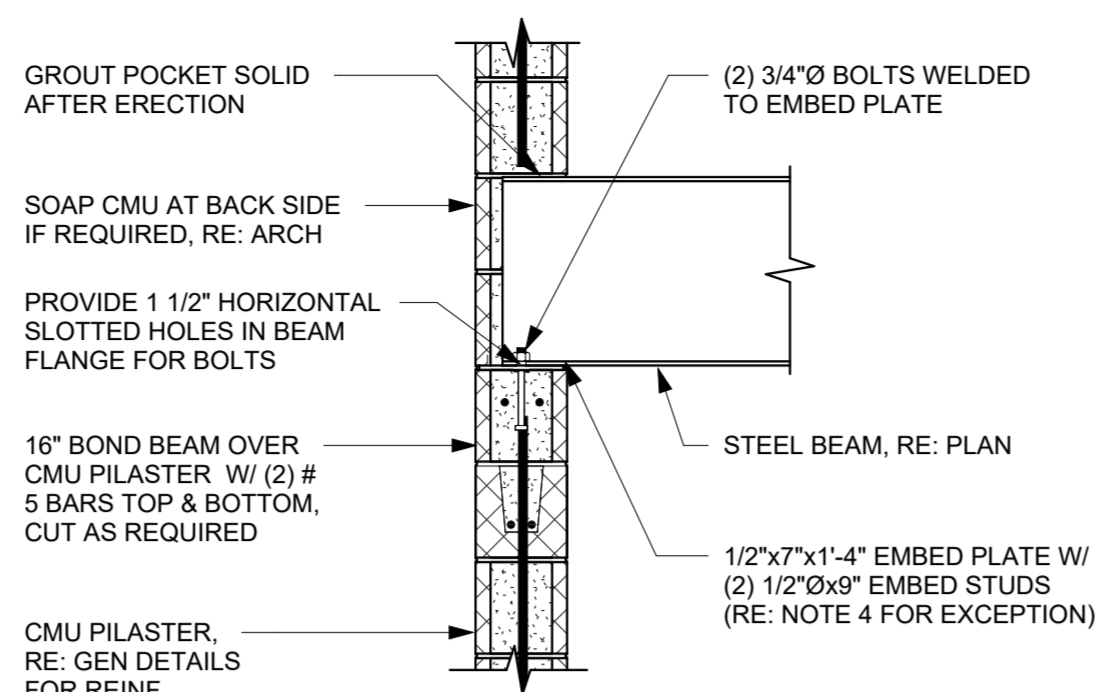
12" CMU SLIP CONNECTION

NOTES:

1. PROVIDE SIMILAR DETAIL WHEN BEAM IS IN-PLANE w/ CMU WALL.
2. IF BEAM BEARS ON MASONRY WALL EACH END, PROVIDE SLIP CONNECTION AT ONE END AND FIXED CONNECTION AT OTHER END. IF BEAM FRAMES TO STEEL COLUMN ON ONE END AND BEARS ON MASONRY AT OTHER END, PROVIDE SLIP CONNECTION AT END WHERE BEAM BEARS ON MASONRY.



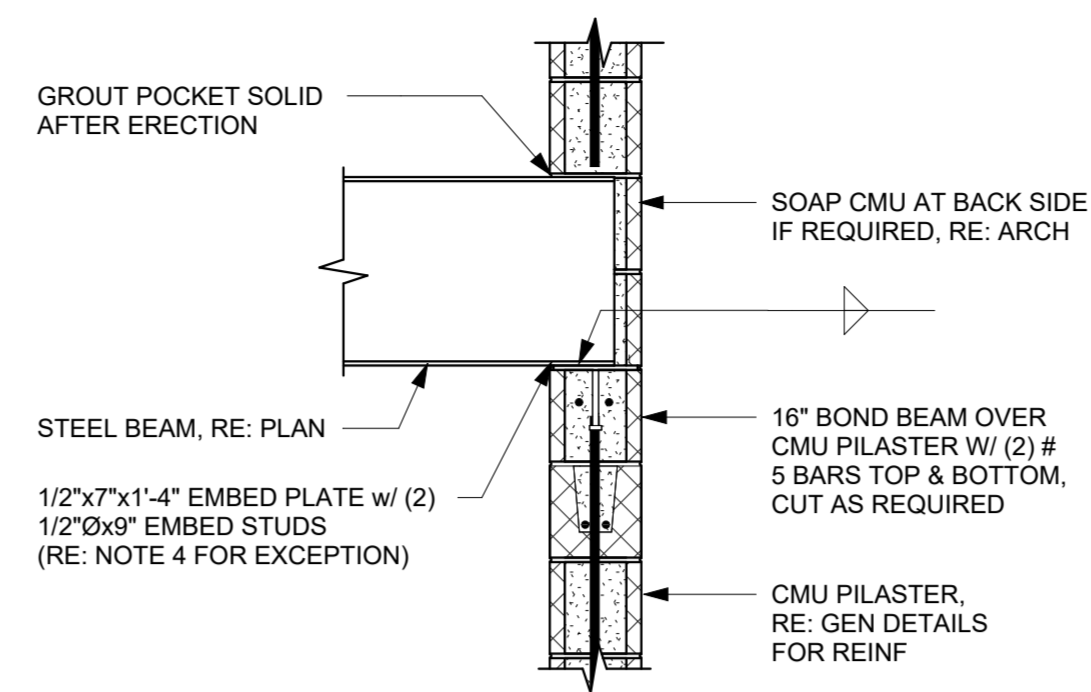
12" CMU FIXED CONNECTION



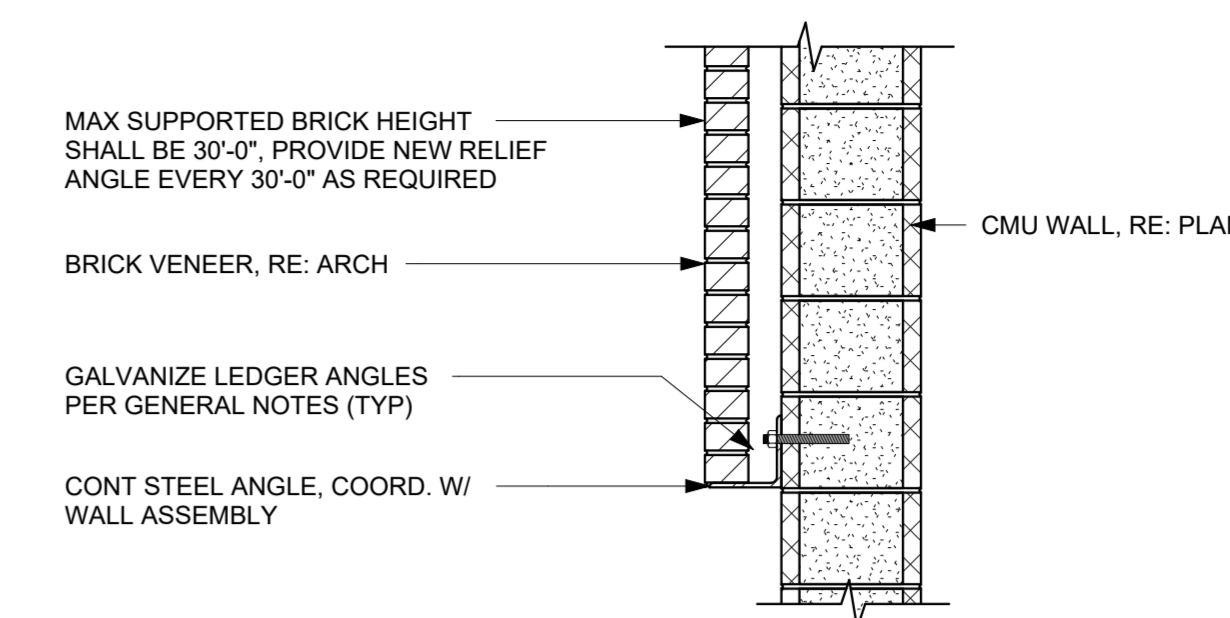
8" CMU SLIP CONNECTION

NOTES:

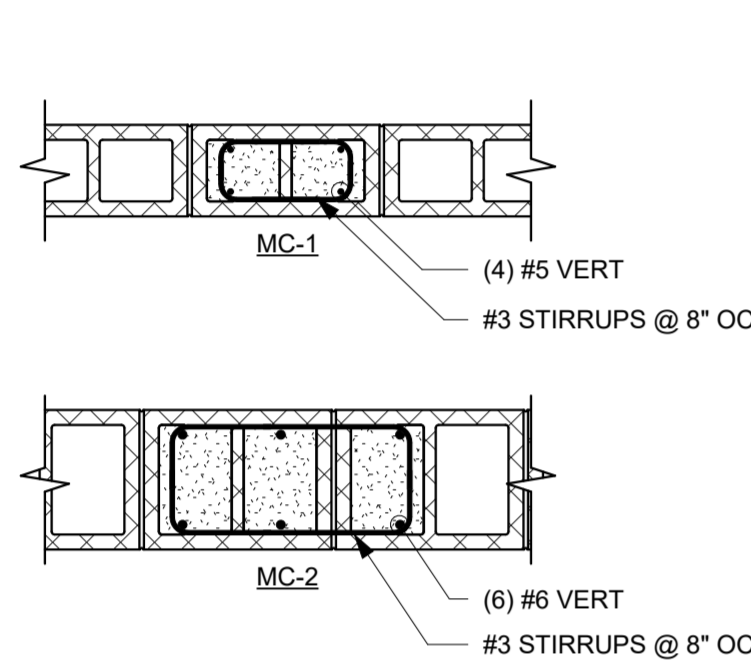
1. PROVIDE SIMILAR DETAIL WHEN BEAM IS IN-PLANE w/ CMU WALL.
2. IF BEAM BEARS ON MASONRY WALL EACH END, PROVIDE SLIP CONNECTION AT ONE END AND FIXED CONNECTION AT OTHER END. IF BEAM FRAMES TO STEEL COLUMN ON ONE END AND BEARS ON MASONRY AT OTHER END, PROVIDE SLIP CONNECTION AT END WHERE BEAM BEARS ON MASONRY.
3. PROVIDE AS A MIN M1 MASONRY COLUMN UNDER BEAM BEARING END UNO ON PLAN.



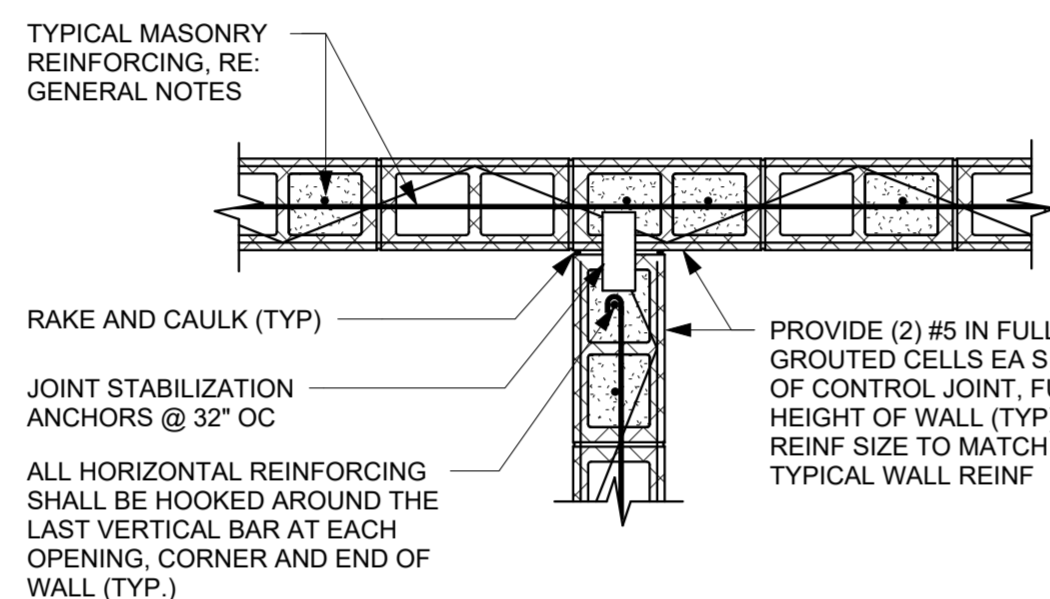
8" CMU FIXED CONNECTION



2 TYP MASONRY BRICK LEDGER ATTACHMENT
 3/4" = 1'-0"



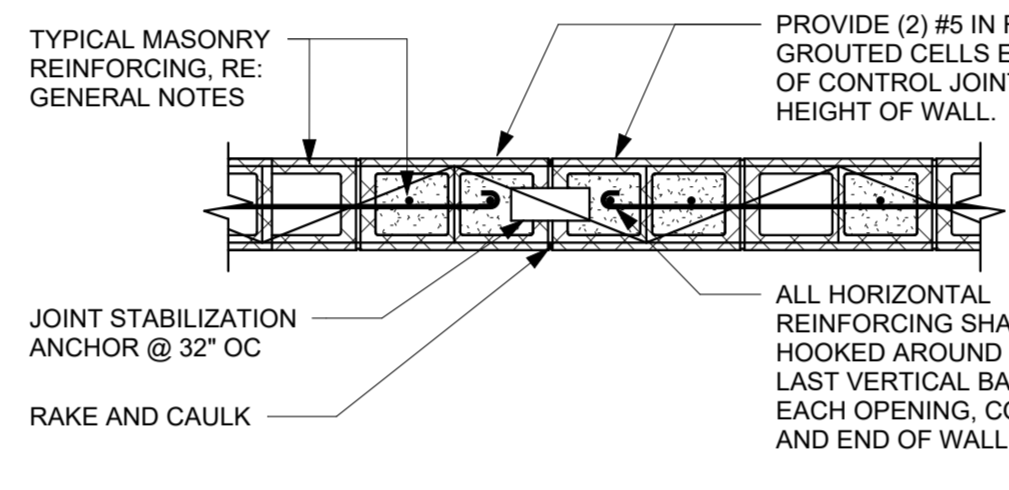
- NOTES:**
1. ALTERNATE ORIENTATION OF BLOCKS TO MAINTAIN RUNNING BOND PATTERN.



RAKE AND CAULK (TYP)

JOINT STABILIZATION ANCHORS @ 32" OC

ALL HORIZONTAL REINFORCING SHALL BE HOOKED AROUND THE LAST VERTICAL BAR AT EACH OPENING, CORNER AND END OF WALL (TYP.)

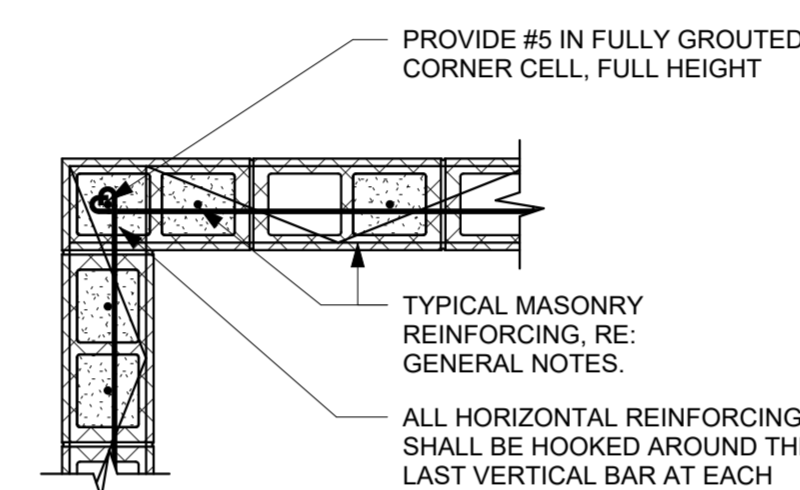


RAKE AND CAULK

JOINT STABILIZATION ANCHOR @ 32" OC

TYPICAL MASONRY REINFORCING, RE: GENERAL NOTES.

ALL HORIZONTAL REINFORCING SHALL BE HOOKED AROUND THE LAST VERTICAL BAR AT EACH OPENING, CORNER AND END OF WALL (TYP.)



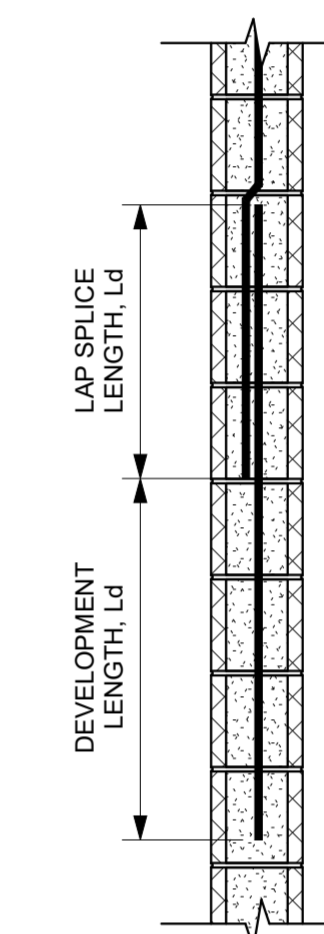
TYPICAL MASONRY REINFORCING, RE: GENERAL NOTES.

ALL HORIZONTAL REINFORCING SHALL BE HOOKED AROUND THE LAST VERTICAL BAR AT EACH OPENING, CORNER AND END OF WALL (TYP.)

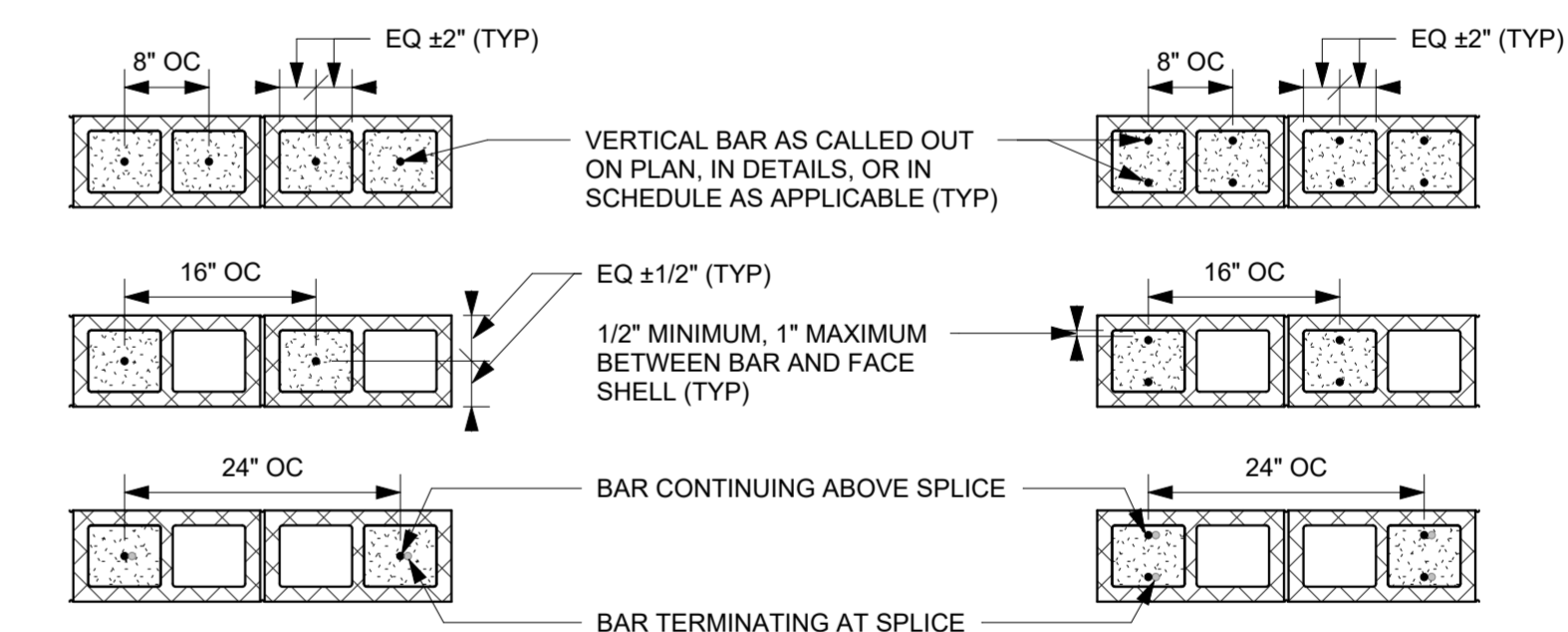
- NOTES:**
1. SPACE CONTROL JOINT FOR BEARING AND NON-BEARING WALLS AT APPROX 25'-0" OC, OR 1.5x WALL HEIGHT, WHICHEVER IS LESS.
 2. LOCATE CONTROL JOINTS PER ARCHITECTURAL DRAWINGS
 3. CONTROL JOINTS SHALL BE 8" MINIMUM FROM EDGE OF OPENINGS
 4. CONTROL JOINTS SHALL BE CONSTRUCTED AS CONTINUOUS VERTICAL HEAD JOINTS USING FULL AND HALF MASONRY UNITS.
 5. MORTAR SHALL BE RAKED BACK AT LEAST 1" IN DEPTH AND CAULKED PER ARCH DRAWINGS.
 6. CONTROL JOINTS SHALL NOT BE CONTINUOUS THROUGH BOND BEAMS.

| BAR SIZE | TENSION DEVELOPMENT LENGTH, L _d | | STD HOOK DEV LENGTH, L _h |
|----------|--|------------------------|-------------------------------------|
| | CENTERED | AT FACE ⁽¹⁾ | |
| #3 | 15 | 17 | 5 |
| #4 | 20 | 29 | 6 |
| #5 | 25 | 45 | 8 |
| #6 | 43 | 85 | 9 |
| #7 | 60 | 115 | 11 |
| #8 | 72 | 173 | 12 |

- NOTES:**
1. BASED ON 1 3/4" FROM FACE OF BAR TO FACE OF MASONRY. THIS MATCHES TYPICAL PLACEMENT DETAIL.
 2. ALL VALUES ARE IN INCHES.
 3. F_m = 1,500 PSI
 4. f_y = 52,000 PSI



5 TYP MASONRY DEV. & SPLICE LENGTH SCHED.
 3/4" = 1'-0"

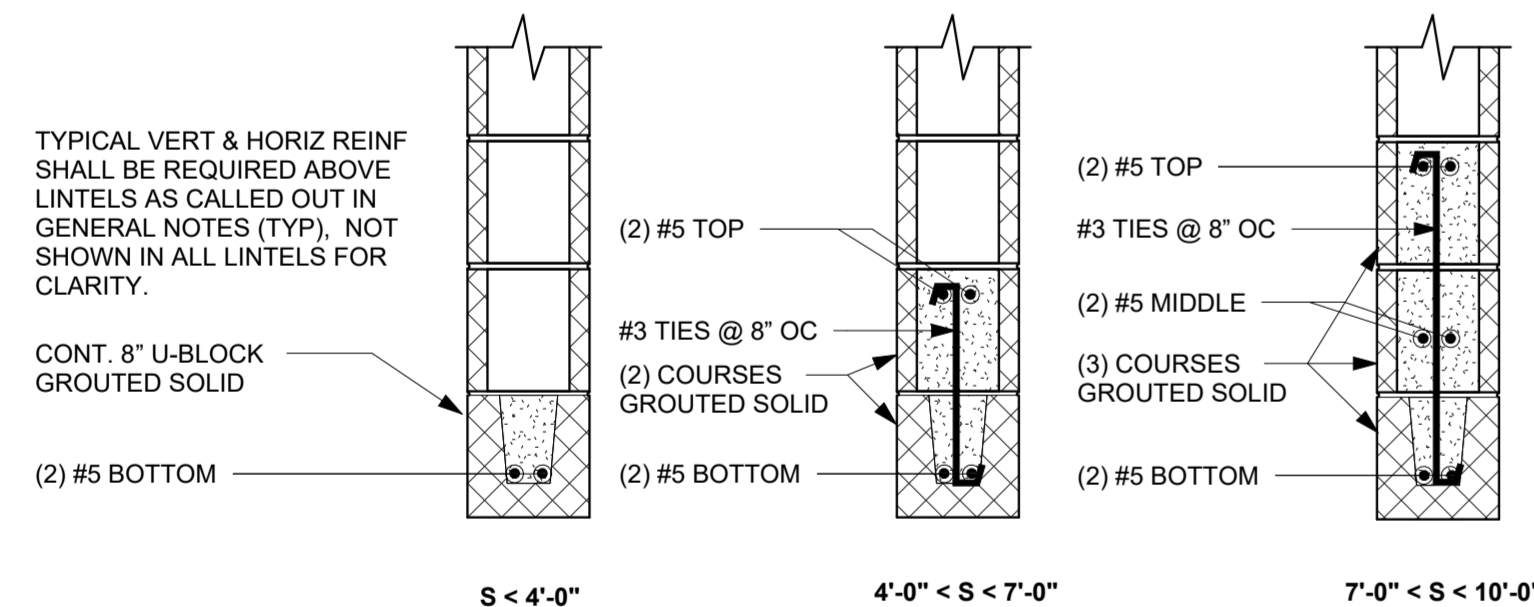


BARS NOT SPECIFIED "EACH FACE": 32", 40", AND 48" OC SPACING SIMILAR

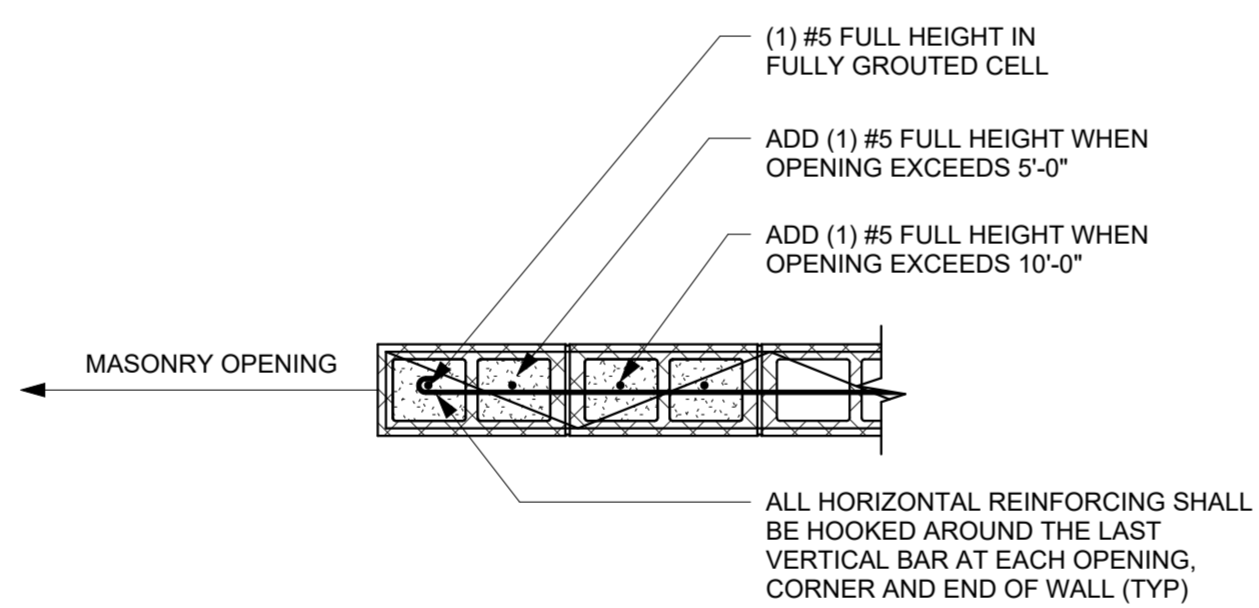
- NOTES:**
1. REINFORCEMENT MUST BE PLACED USING BAR POSITIONS THAT LOCATE THE BAR AS SPECIFIED ABOVE AND THAT PREVENT MOVEMENT DURING CONSTRUCTION.
 2. ALL BAR SPLICES SHALL BE A CONTACT LAP SPLICE WITH BARS ALIGNED PARALLEL TO THE WALL AS ILLUSTRATED ABOVE.
 3. NOTIFY ENGINEER PRIOR TO PLACING ANY REINFORCEMENT THAT FALLS OUTSIDE OF THE TOLERANCES SHOWN IN THIS DETAIL.

3 TYP MASONRY COLUMNS
 3/4" = 1'-0"

4 TYP MASONRY JOINT DETAILS
 3/4" = 1'-0"



- NOTES:**
1. MASONRY LINTELS SHALL BEAR 8" MIN ON EA SIDE OF OPENING (TYP).
 2. VERTICAL JAM STEEL SHALL BE CONT THROUGH LINTEL BEARING (TYP).
 3. RE: MASONRY OPENING DETAIL FOR JAM REINF REQUIREMENTS.
 4. LINTELS MUST BE PROPERLY SHORED UNTIL GROUT IS CURED.
 5. ALL BLOCK SHALL BE 8" CMU (TYP, UNO).
 6. VERTICAL WALL REINFORCING TO EXTEND INTO BOND BEAM LINTEL AS SHOWN (TYP).

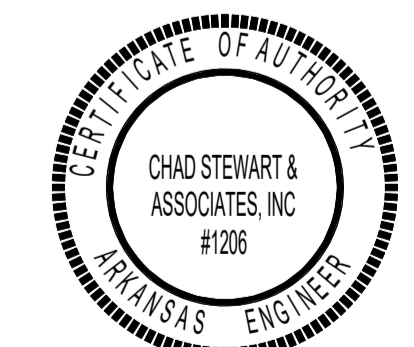


- NOTES:**
1. OPENING REINFORCEMENT SHOWN IN THIS DETAIL IS TYPICAL AT EACH SIDE OF ALL OPENINGS UNLESS NOTED OTHERWISE ON PLAN.
 2. SIZE OF OPENING REINFORCING IS TO MATCH SIZE OF TYPICAL WALL REINFORCING CALLED OUT ON PLAN OR IN TYPICAL MASONRY WALL REINFORCING SCHEDULE AS APPLICABLE.
 3. OPENING REINFORCING IS TO BE FULL HEIGHT OF WALL.

6 TYP MASONRY LINTELS
 1" = 1'-0"

7 TYP MASONRY OPENING REINFORCEMENT
 3/4" = 1'-0"

8 TYP MASONRY VERTICAL BAR PLACEMENT
 3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

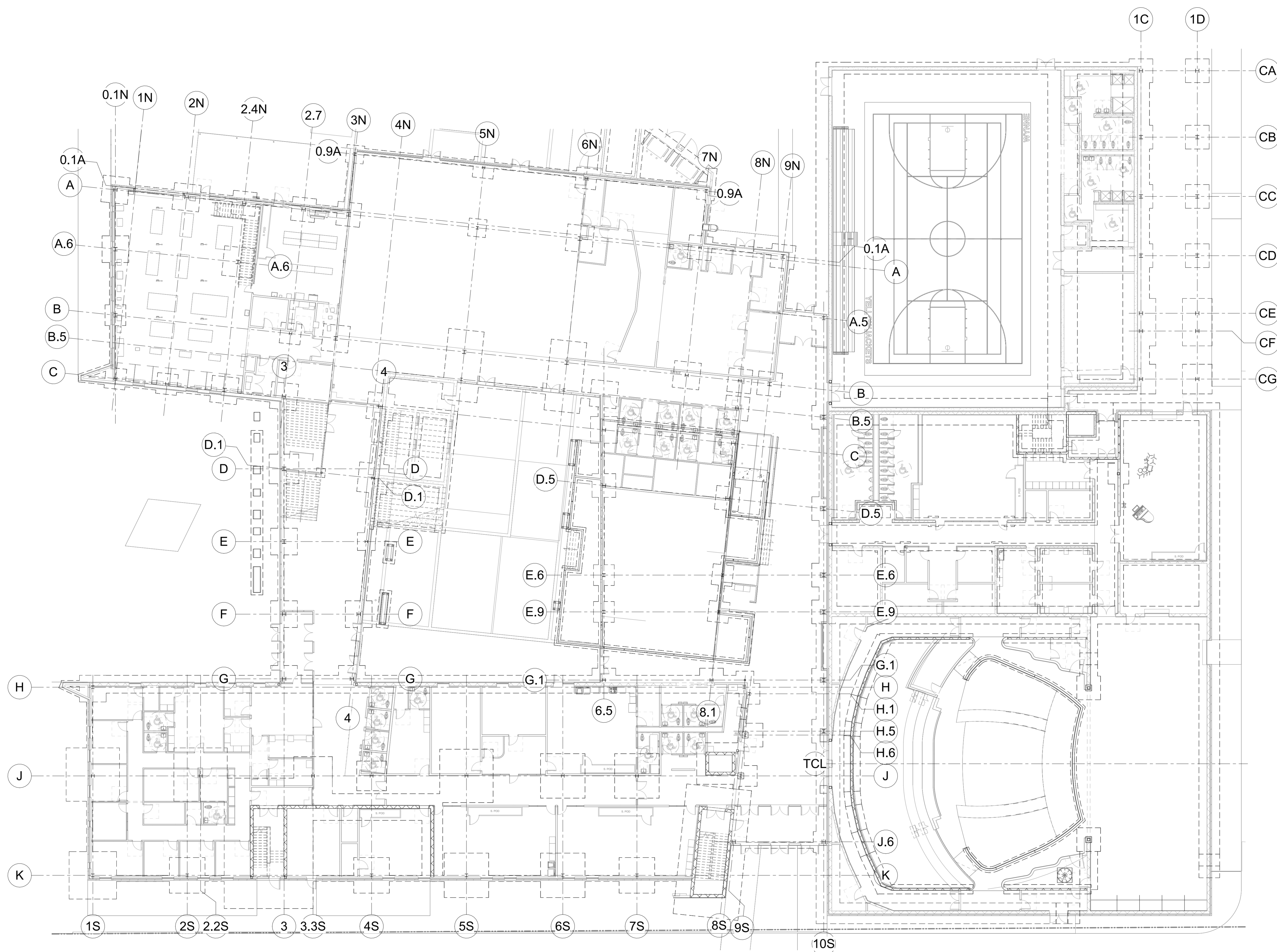
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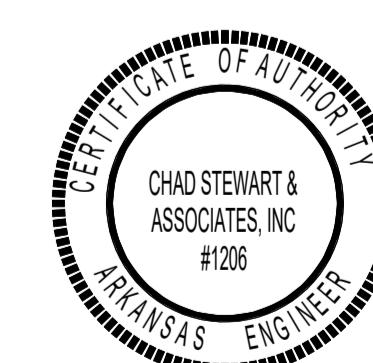
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



1 FOUNDATION PLAN - OVERALL
1/16" = 1'-0"



SHEET TITLE
FOUNDATION PLAN -
OVERALL

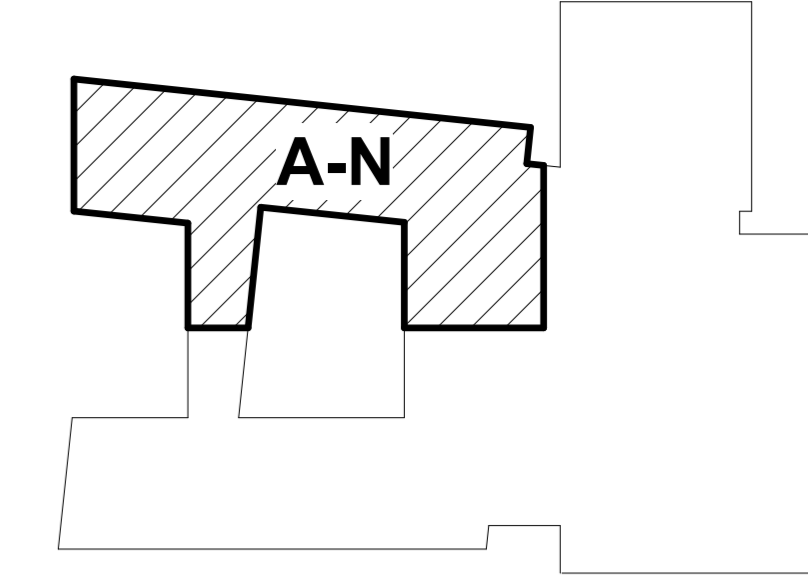
DATE

17.10.24

SHEET NUMBER

S101

KEYPLAN



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 Little Rock, AR 72201
 501.378.0878 office
 www.polkstanleywilcox.com

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- FOUNDATION PLAN NOTES:**
- ELEVATIONS ARE BASED ON TOP OF SLAB DATUM EL. 0'-0". COORD. W/ CIVIL FOR FINAL GRADING ELEVATIONS.
 - TYPICAL TOP OF FOOTING ELEVATION = -1'-4" (TYP UNO).
 - FOR DESIGN CRITERIA, GENERAL NOTES, AND TYPICAL DETAILS, RE: GENERAL NOTES & DETAILS SHEET.
 - FOR ELEVATIONS, WALL SECTIONS, AND DIMENSIONS NOT SHOWN, RE: ARCHITECTURAL DRAWINGS.
 - RE: GENERAL DETAILS FOR TYPICAL SLAB-ON-GRADE CONTROL & CONSTRUCTION JOINT DETAILS. NOTE THAT CONTROL JOINTS ARE NOT SHOWN ON PLAN FOR CLARITY. RE: SPECIFICATIONS FOR GENERAL REQUIREMENTS. JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 12'-0" EACH WAY.
 - RE: GENERAL DETAILS FOR TYPICAL FOOTING CORNER BAR REQUIREMENTS.
 - RE: GENERAL DETAILS FOR TYPICAL FOOTING STEP REQUIREMENTS.

PROJECT NAME
 WSD - NEW SENIOR HIGH SCHOOL

LOCATION
 800 E JACKSON AVE
 WYNNE AR 72396

PROJECT NUMBER
 -

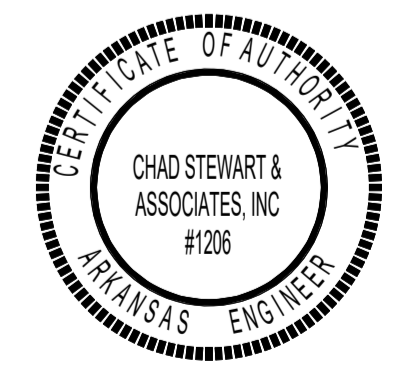
DEVELOPER/OWNER
 WYNNE SCHOOL DISTRICT

INFORMATION



1 FOUNDATION PLAN - AREA A NORTH
 1/8" = 1'-0"

CSA PROJECT NO. 24122
 Chad Stewart & Associates, Inc.
 9720 Village Circle Lakeland, TN 38002
 Phone 901-260-7850 CSAEngineering.com



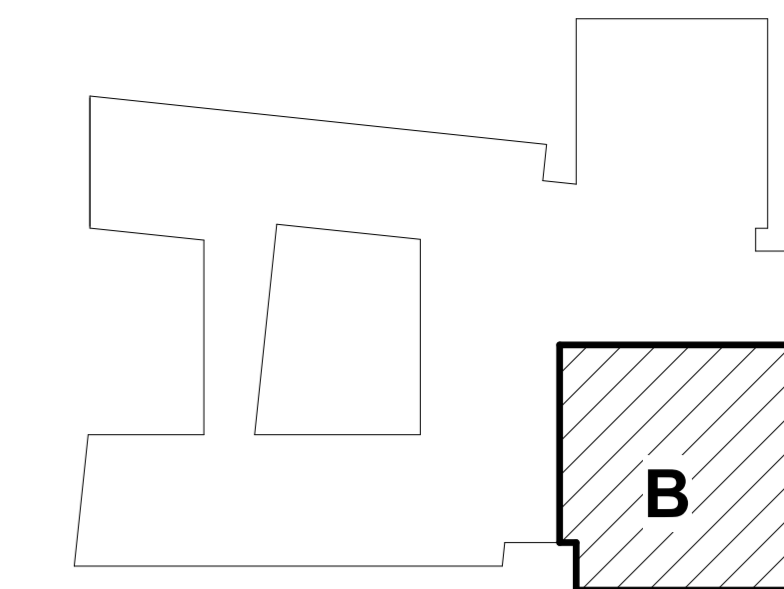
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 FOUNDATION PLAN - AREA A NORTH

DATE
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PROJECT NAME

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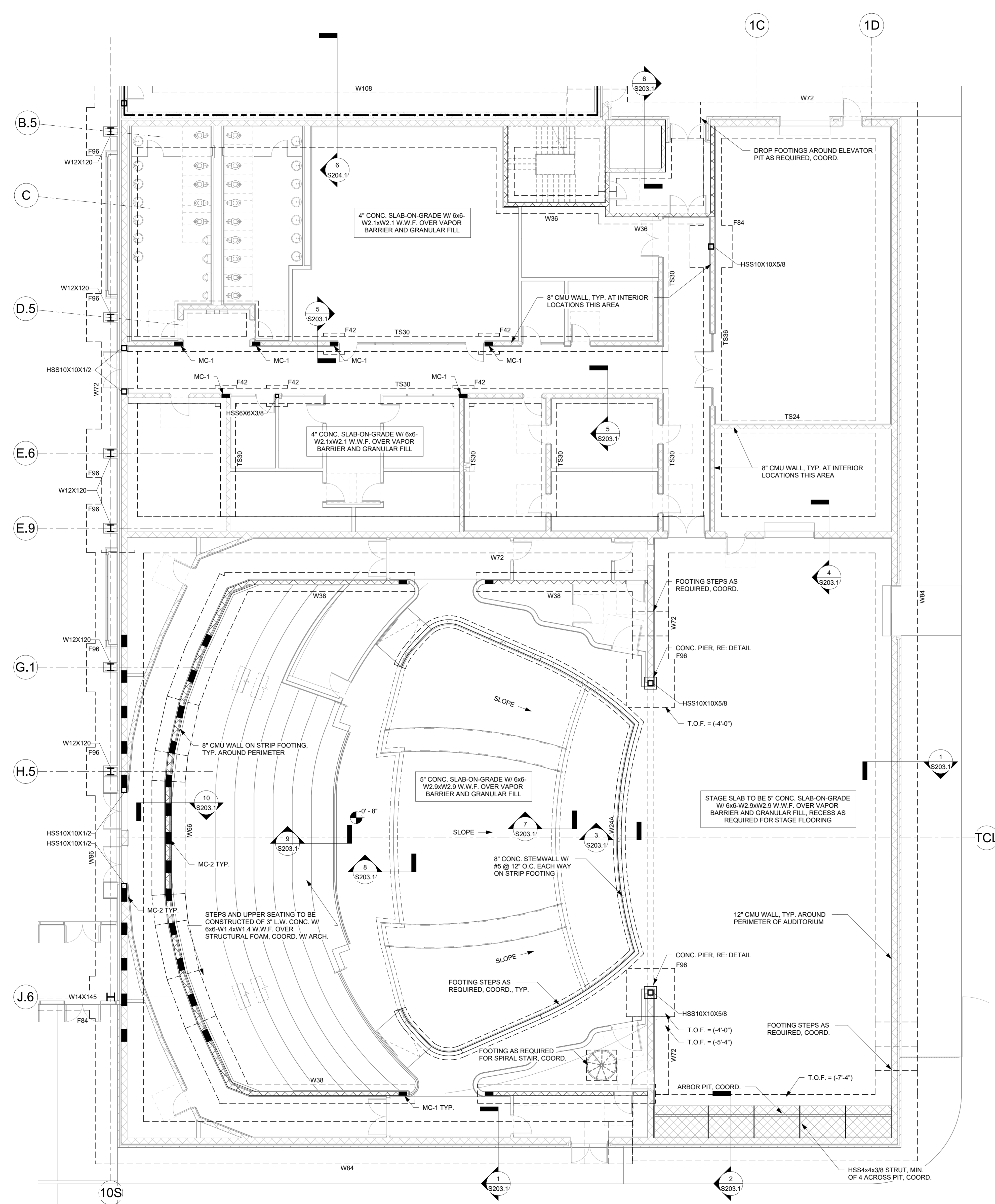
PROJECT NUMBER

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DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



- FOUNDATION PLAN NOTES:**
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 - RE: GENERAL DETAILS FOR TYPICAL FOOTING CORNER BAR REQUIREMENTS.
 - RE: GENERAL DETAILS FOR TYPICAL FOOTING STEP REQUIREMENTS.

1 FOUNDATION PLAN - AREA B
1/8" = 1'-0"

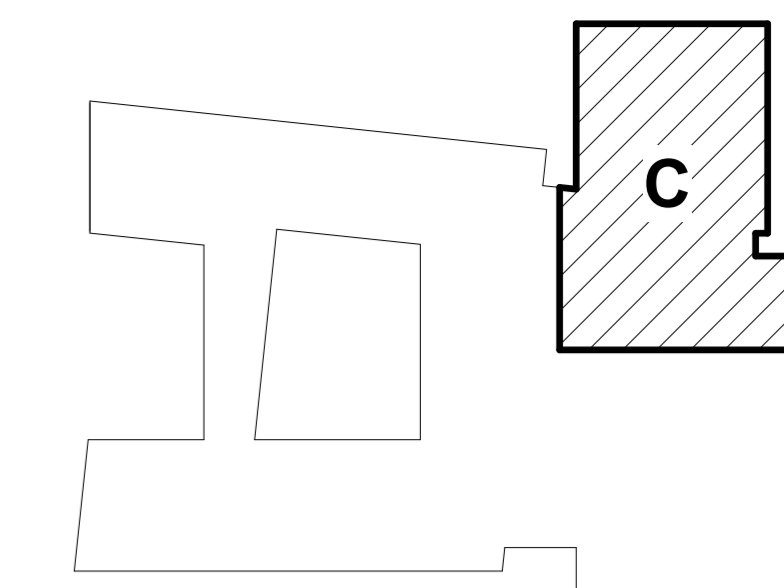
SHEET TITLE
FOUNDATION PLAN - AREA B

DATE
17.10.24

SHEET NUMBER

S101.3

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PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

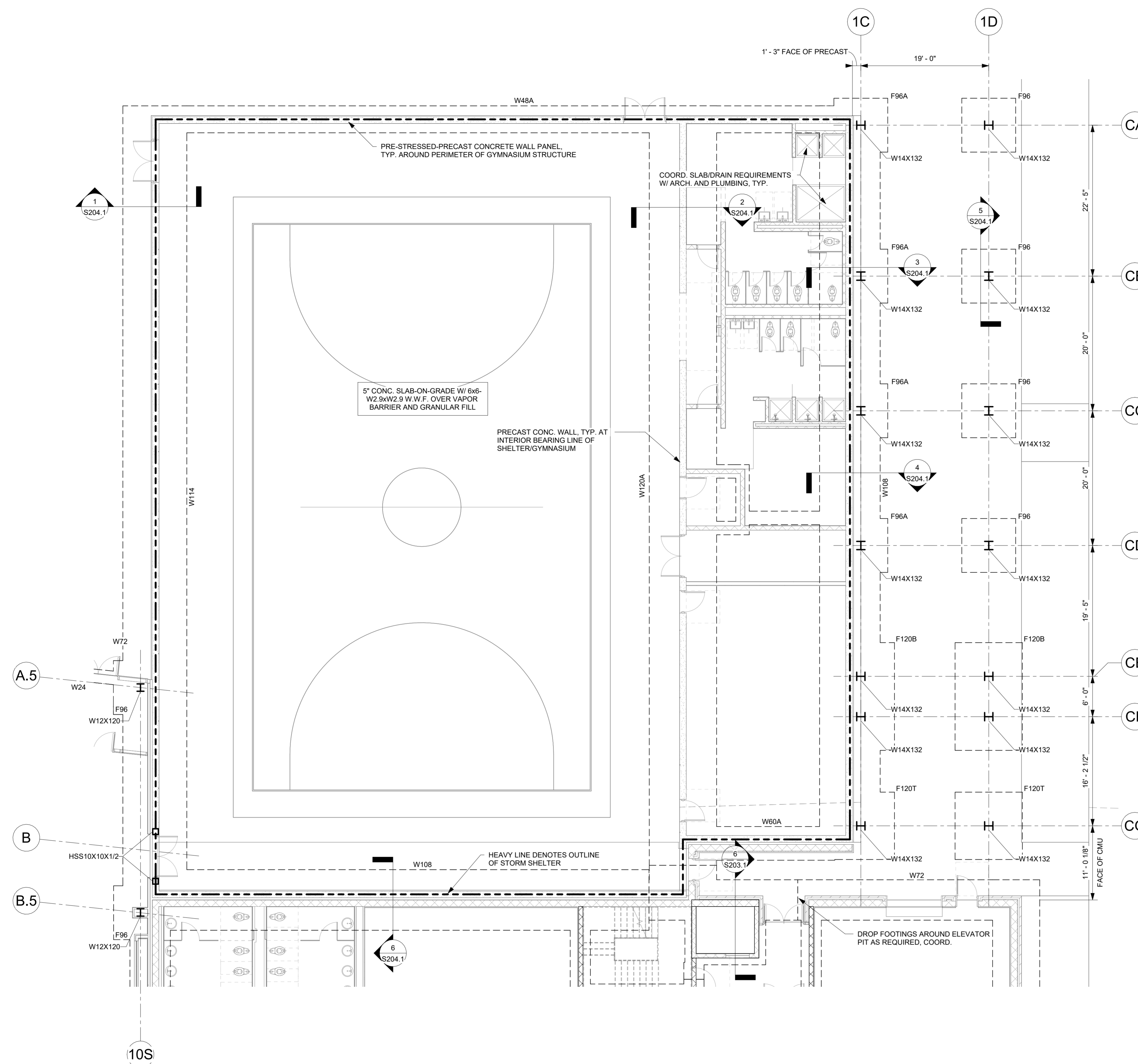
PROJECT NUMBER

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DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



FOUNDATION PLAN NOTES:

- ELEVATIONS ARE BASED ON TOP OF SLAB DATUM EL 0'-0". COORD. W/ CIVIL FOR FINAL GRADING ELEVATIONS.
- TYPICAL TOP OF FOOTING ELEVATION = -1'-4" (TYP UNO).
- FOR DESIGN CRITERIA, GENERAL NOTES, AND TYPICAL DETAILS. RE: GENERAL NOTES & DETAILS SHEET.
- FOR ELEVATIONS, WALL SECTIONS, AND DIMENSIONS NOT SHOWN. RE: ARCHITECTURAL DRAWINGS.
- RE: GENERAL DETAILS FOR TYPICAL SLAB-ON-GRADE CONTROL & CONSTRUCTION JOINT DETAILS. NOTE THAT CONTROL JOINTS ARE NOT SHOWN ON PLAN FOR CLARITY. RE: SPECIFICATIONS FOR GENERAL REQUIREMENTS. JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 12'-0" EACH WAY.
- RE: GENERAL DETAILS FOR TYPICAL FOOTING CORNER BAR REQUIREMENTS.
- RE: GENERAL DETAILS FOR TYPICAL FOOTING STEP REQUIREMENTS.

1 FOUNDATION PLAN - AREA C
 1/8" = 1'-0"



SHEET TITLE
 FOUNDATION PLAN - AREA C

DATE
 17.10.24

SHEET NUMBER

S101.4



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

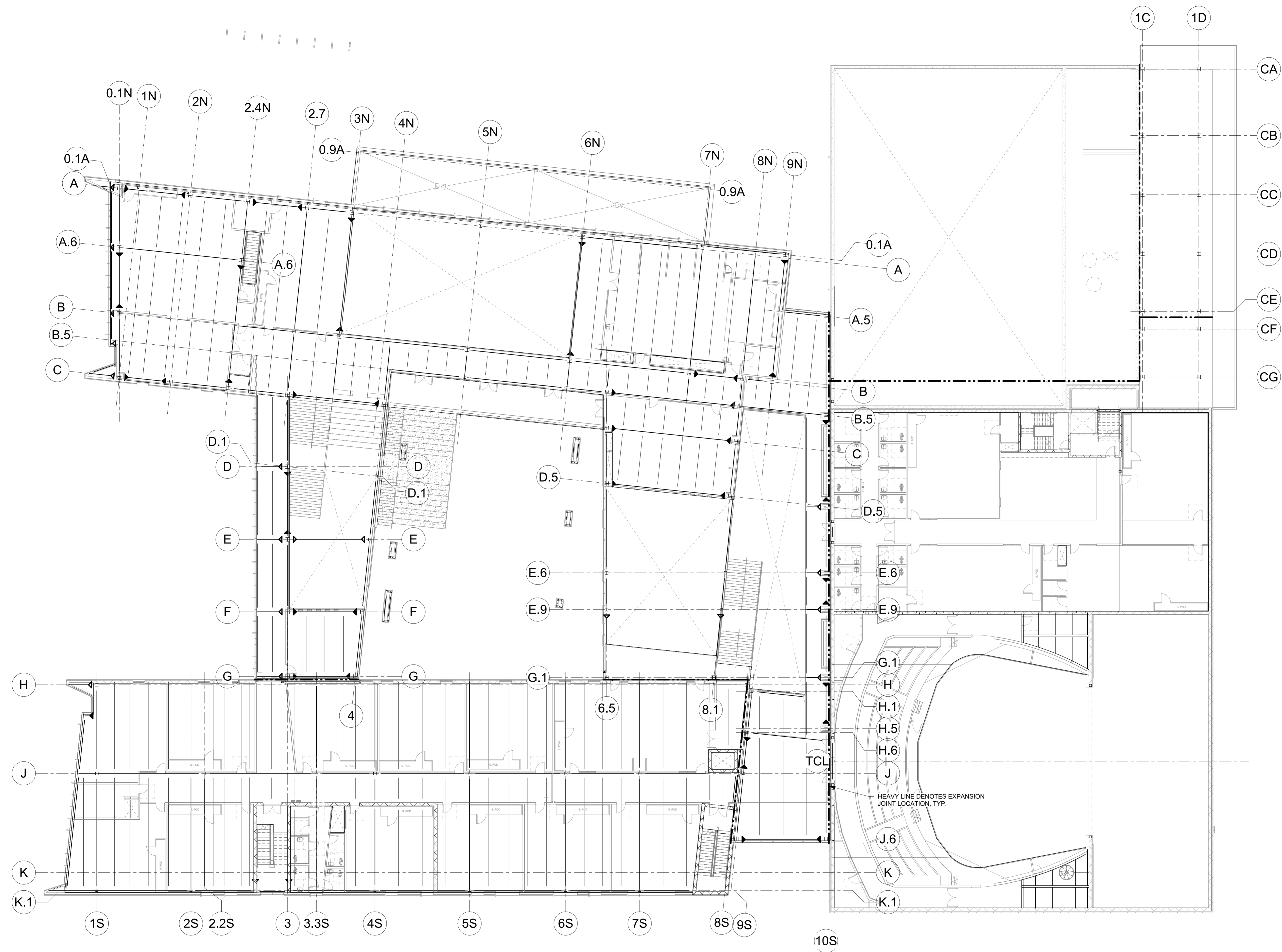
PROJECT NUMBER

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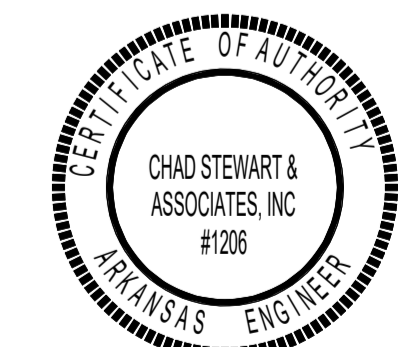
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



1 SECOND FLOOR FRAMING PLAN - OVERALL
1/16" = 1'-0"



SHEET TITLE

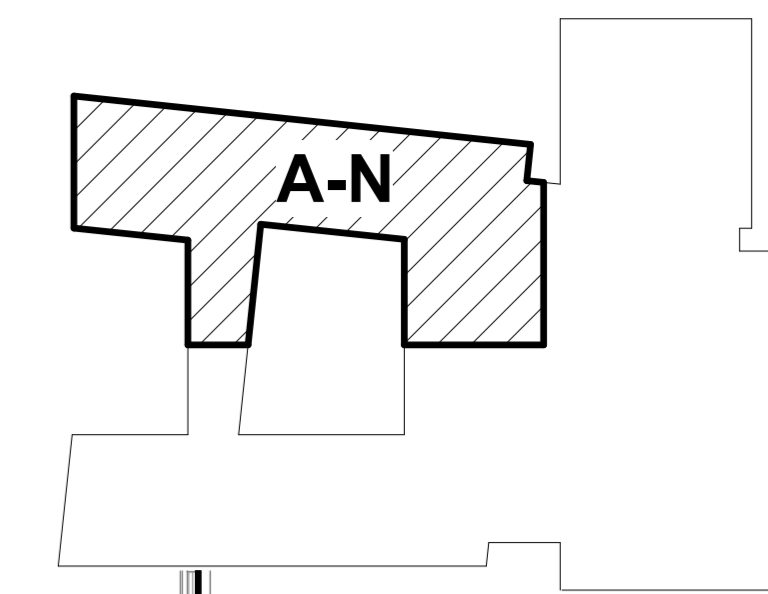
SECOND FLOOR
FRAMING PLAN -
OVERALL

DATE

17.10.24

SHEET NUMBER

S102



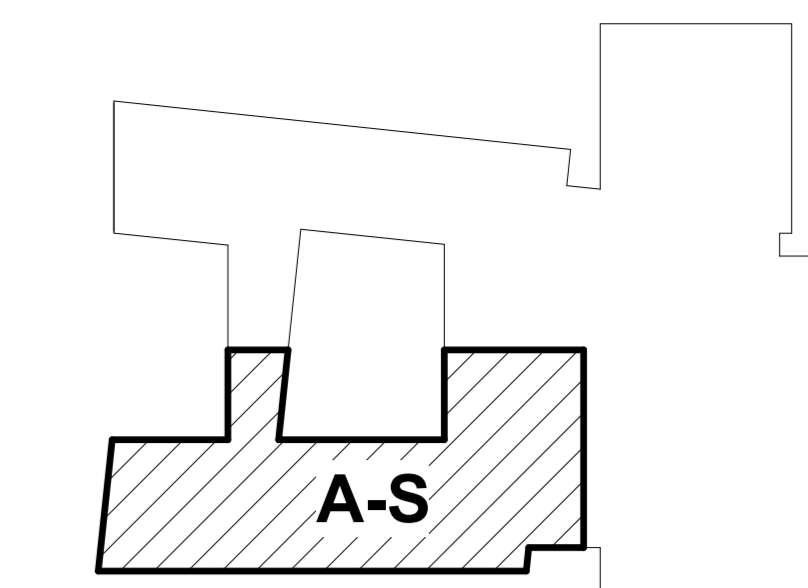
SECOND FLOOR FRAMING NOTES:

- TOP OF SECOND FLOOR PLAN = 20' - 0" AFF.
- TOP OF SECOND FLOOR STEEL IS 5" BELOW SLAB. T1STEEL = 19' - 7" AFF UNO. SECOND FLOOR SLAB SHALL BE 5" L.W. CONCRETE SLAB OVER 2" VLI 16 GA COMPOSITE DECK W/ (1) LAYER OF 6X6-W2.9XW2.9 WWF (5" TOTAL THICKNESS)
- (X) DENOTES NUMBER OF SHEAR STUDS REQUIRED, RE: GENERAL DETAILS FOR TYPICAL COMPOSITE DETAILS.
- BEAMS NOT INDICATING NUMBER OF SHEAR STUDS REQUIRED SHALL HAVE AT MIN STUDS PLACED ALONG BEAMS AT A MAX SPACING OF 2'-0" OC UNLESS SPECIFICALLY NOTES AS (X).
- "XX K" DENOTES FACTORED (LRFD) BEAM REACTION FOR SIZING BEAM CONNECTION. REACTION SHOWN SHALL BE APPLIED TO EACH END, NOT DIVIDED BY TWO. REACTION NOT LISTED SHALL BE ASSUMED TO BE 10K OR THE CONNECTION IS AS DETAILED ON THESE DRAWINGS.
- C = "X" DENOTES CAMBER REQUIRED IN STEEL BEAM. BEAMS AND GIRDERS HAVE BEEN CAMBERED.
- BEAM CAMBER SHALL BE INDUCED SUCH THAT THE BEAM HAS A CURVED PROFILE APPROXIMATING A PARABOLA. BEAMS CAMBERED BY FORCE, OR HEAT, AT A SINGLE LOCATION SHALL NOT BE PERMITTED.
- CONCRETE SCREED SUPPORTS SHALL BE PLACED ON BEAMS TO FOLLOW THE STEEL PROFILE SO AS TO GIVE A UNIFORM SLAB THICKNESS AT THE BEAMS. WHERE THE SLAB DEFLECTS TO BELOW FINISH FLOOR LEVEL, THE SLAB SHALL BE THICKENED (UP TO 1" MAX AT THE CENTER OF THE BEAM TO PROVIDE A LEVEL FLOOR). WHERE THE SLAB IS ABOVE FINISH FLOOR LEVEL THE SLAB SHALL REMAIN ABOVE FINISH FLOOR LEVEL SO THAT THE SLAB HAS THE MINIMUM THICKNESS SPECIFIED.
- THE GAUGE OF THE METAL FLOOR DECK SHOWN IS THE MINIMUM ALLOWED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING DECK SHORING AS REQUIRED TO AVOID OVER STRESSING THE DECK DURING PLACEMENT OF CONCRETE.
- FLOOR DECK SHALL HAVE (3) SPANS UNLESS APPROVED BY THE ENGINEER. IF APPROVED, MAXIMUM UNSHORED SPANS SHALL BE AS FOLLOWS: (GREATER SPANS SHALL BE SHORED):
 SINGLE SPAN = 11'-0"
 DOUBLE SPAN = 13'-1"
- CONSTRUCTION JOINT LOCATION AND CONCRETE PLACEMENT PROCEDURE IS VERY IMPORTANT AND SHALL BE DISCUSSED WITH THE STRUCTURAL ENGINEER PRIOR TO POURING THE SLAB.
- CONTRACTOR SHOULD NOTE THAT DECK AND BEAM DEFLECTION WILL INCREASE THE ESTIMATED CONCRETE QUANTITY.
- RE: GENERAL DETAILS FOR ADDITIONAL METAL DECK REQUIREMENTS.
- RE: GENERAL DETAILS FOR TYPICAL FLOOR DECK ATTACHMENT PATTERN.
- ALL FLOOR OPENING DIMENSIONS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SYMBOL DENOTES MOMENT CONNECTIONS DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.
- SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.

COLUMN TO TIE INTO CMU WALL W/ #5 BARS @ 4'-0" O.C. INTO BOND BEAM, MIN. 8" EMBED.

1 SECOND FLOOR FRAMING PLAN - AREA A NORTH
 1/8" = 1'-0"

KEYPLAN



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PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

PROJECT NUMBER

-

DEVELOPER/OWNER

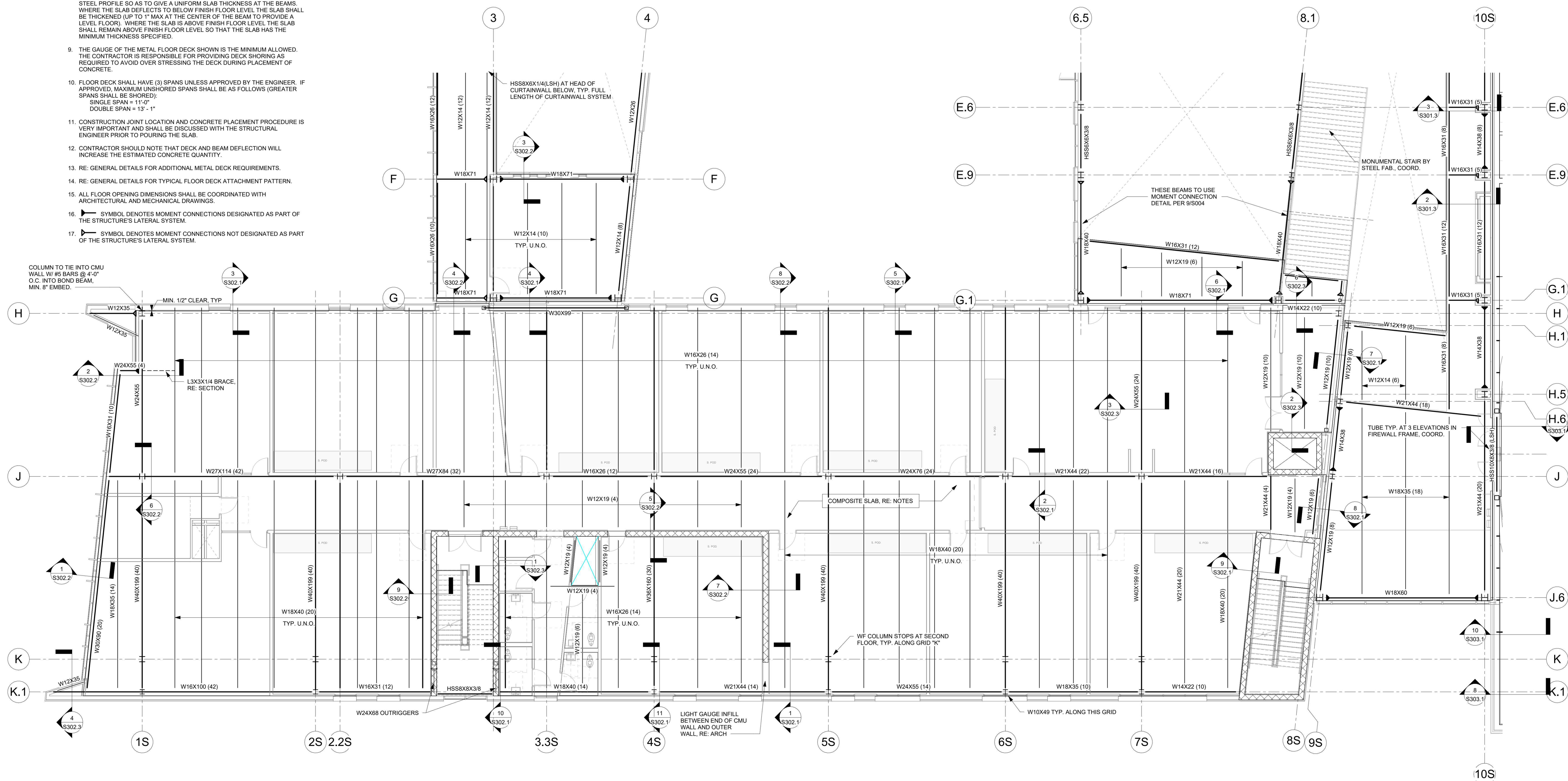
WYNNE SCHOOL DISTRICT

INFORMATION



SECOND FLOOR FRAMING NOTES:

- TOP OF SECOND FLOOR PLAN = 2'-0" - 0" AFF.
- TOP OF SECOND FLOOR STEEL IS 5" BELOW SLAB. 1" STEEL = 19" - 7" AFF UNO. SECOND FLOOR SLAB SHALL 5" L.W. CONCRETE SLAB OVER 2" VLI 16 GA COMPOSITE DECK W/ (1) LAYER OF 6X6:W2.9XW2.9 WWF (5" TOTAL THICKNESS)
- (X) DENOTES NUMBER OF SHEAR STUDS REQUIRED, RE: GENERAL DETAILS FOR TYPICAL COMPOSITE DETAILS.
- BEAMS NOT INDICATING NUMBER OF SHEAR STUDS REQUIRED SHALL HAVE AT MIN STUDS PLACED ALONG BEAMS AT A MAX SPACING OF 2'-0" OC UNLESS SPECIFICALLY NOTES AS [0].
- "XX K" DENOTES FACTORED (LRFD) BEAM REACTION FOR SIZING BEAM CONNECTION. REACTION SHOWN SHALL BE APPLIED TO EACH END, NOT DIVIDED BY TWO. REACTION NOT LISTED SHALL BE ASSUMED TO BE 10K OR THE CONNECTION IS AS DETAILED ON THESE DRAWINGS.
- C = "X" DENOTES CAMBER REQUIRED IN STEEL BEAM. BEAMS AND GIRDERS HAVE BEEN CAMBERED.
- BEAM CAMBER SHALL BE INDUCED SUCH THAT THE BEAM HAS A CURVED PROFILE APPROXIMATING A PARABOLA. BEAMS CAMBERED BY FORCE, OR HEAT, AT A SINGLE LOCATION SHALL NOT BE PERMITTED.
- CONCRETE SCREED SUPPORTS SHALL BE PLACED ON BEAMS TO FOLLOW THE STEEL PROFILE SO AS TO GIVE A UNIFORM SLAB THICKNESS AT THE BEAMS. WHERE THE SLAB DEFLECTS TO BELOW FINISH FLOOR LEVEL THE SLAB SHALL BE THICKENED (UP TO 1" MAX AT THE CENTER OF THE BEAM TO PROVIDE A LEVEL FLOOR). WHERE THE SLAB IS ABOVE FINISH FLOOR LEVEL THE SLAB SHALL REMAIN ABOVE FINISH FLOOR LEVEL SO THAT THE SLAB HAS THE MINIMUM THICKNESS SPECIFIED.
- THE GAUGE OF THE METAL FLOOR DECK SHOWN IS THE MINIMUM ALLOWED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING DECK SHORING AS REQUIRED TO AVOID OVER STRESSING THE DECK DURING PLACEMENT OF CONCRETE.
- FLOOR DECK SHALL HAVE (3) SPANS UNLESS APPROVED BY THE ENGINEER. IF APPROVED, MAXIMUM UNSHORED SPANS SHALL BE AS FOLLOWS (GREATER SPANS SHALL BE SHORED):
SINGLE SPAN = 11'-0"
DOUBLE SPAN = 13' - 1"
- CONSTRUCTION JOINT LOCATION AND CONCRETE PLACEMENT PROCEDURE IS VERY IMPORTANT AND SHALL BE DISCUSSED WITH THE STRUCTURAL ENGINEER PRIOR TO POURING THE SLAB.
- CONTRACTOR SHOULD NOTE THAT DECK AND BEAM DEFLECTION WILL INCREASE THE ESTIMATED CONCRETE QUANTITY.
- RE: GENERAL DETAILS FOR ADDITIONAL METAL DECK REQUIREMENTS.
- RE: GENERAL DETAILS FOR TYPICAL FLOOR DECK ATTACHMENT PATTERN.
- ALL FLOOR OPENING DIMENSIONS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SYMBOL DENOTES MOMENT CONNECTIONS DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.
- SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.



1 SECOND FLOOR FRAMING PLAN - AREA A SOUTH
1/8" = 1'-0"

SHEET TITLE
SECOND FLOOR
FRAMING PLAN - AREA
A SOUTH

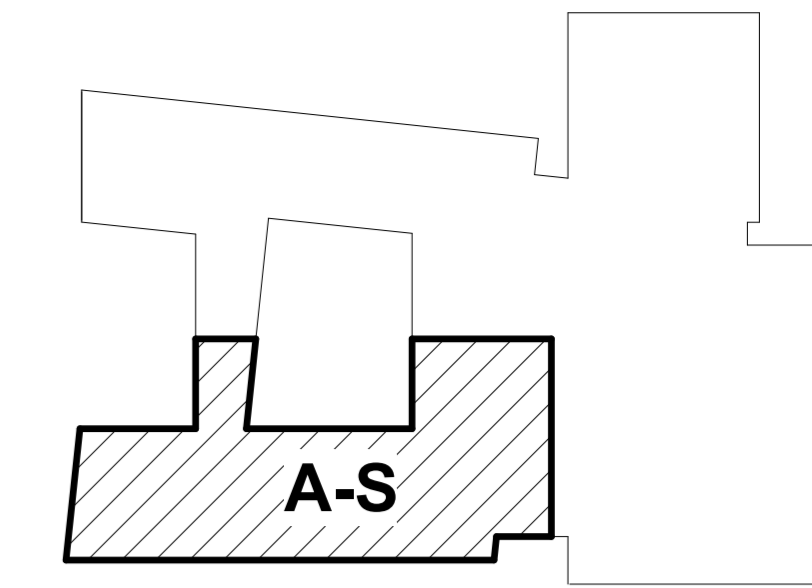
DATE

17.10.24

SHEET NUMBER

S102.2

KEYPLAN



ARCH 1010

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**POLK
STANLEY
WILCOX**

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Little Rock, AR 72201
501.378.0878 office
www.polkstanleywilcox.com

CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



SHEET TITLE

SECOND FLOOR DIAPHRAGM CHORD REINF. PLAN - AREA A SOUTH

DATE
17.10.24

SHEET NUMBER

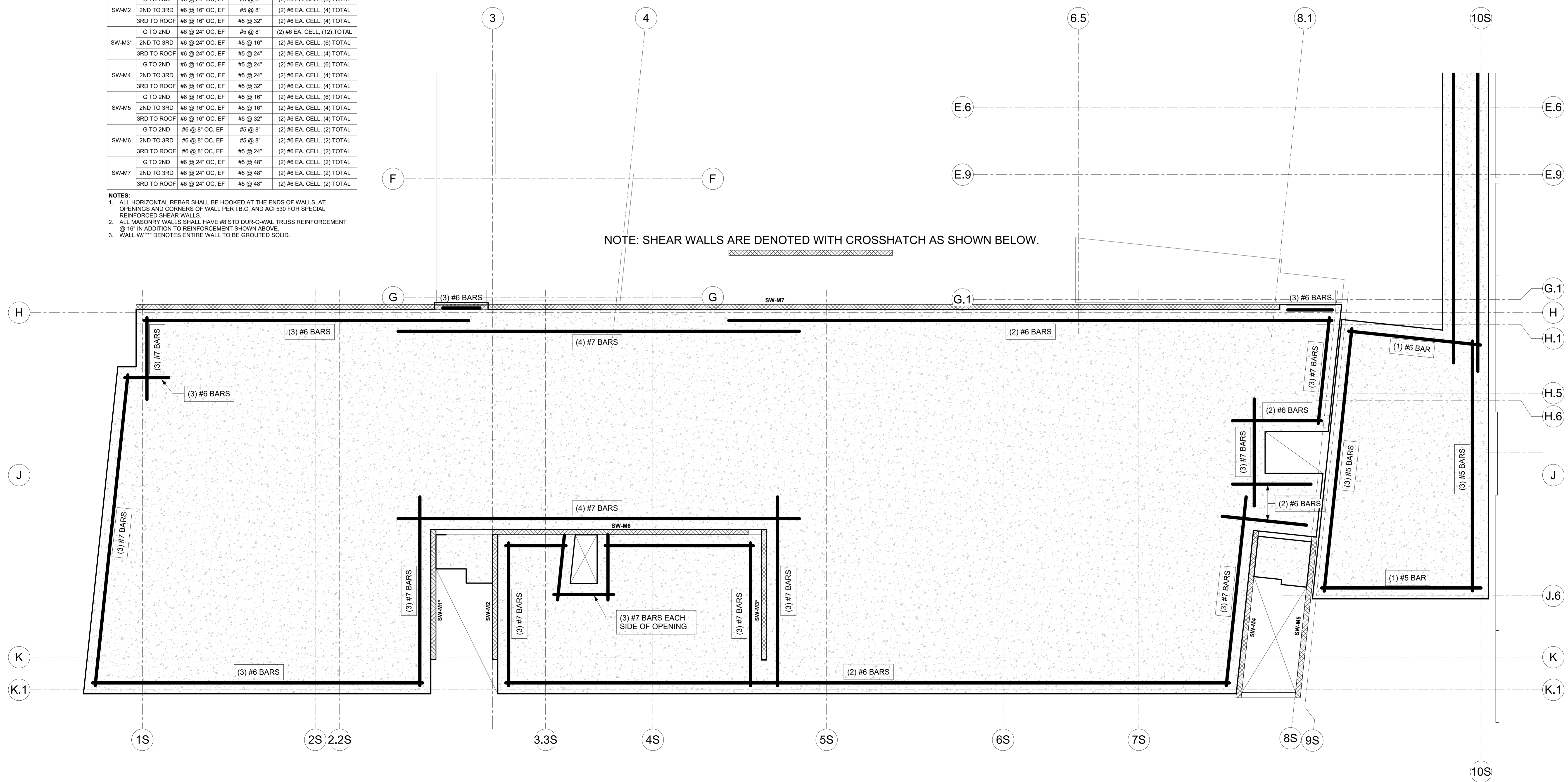
S102.2A

MASONRY SHEARWALL REINFORCEMENT SCHEDULE

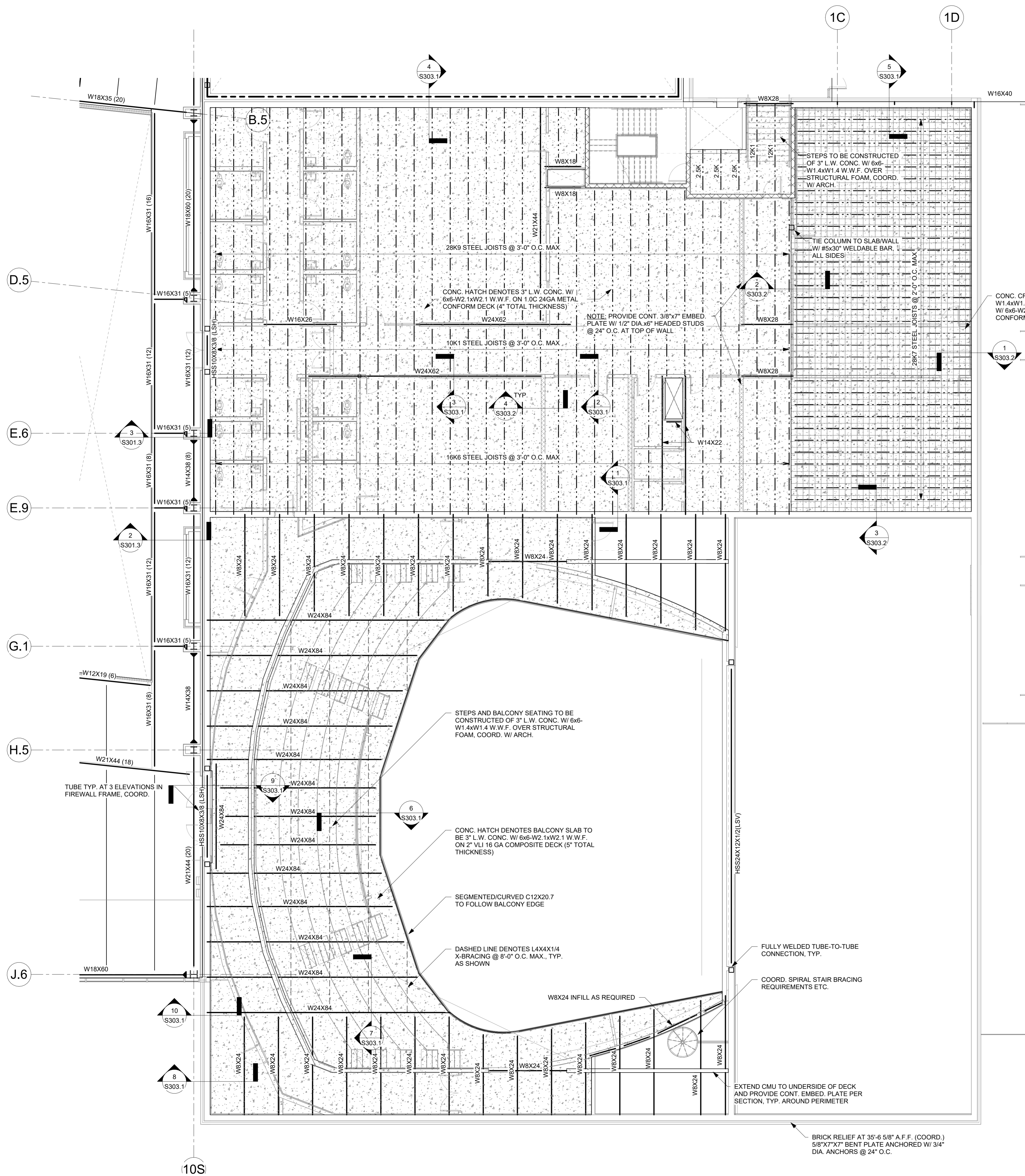
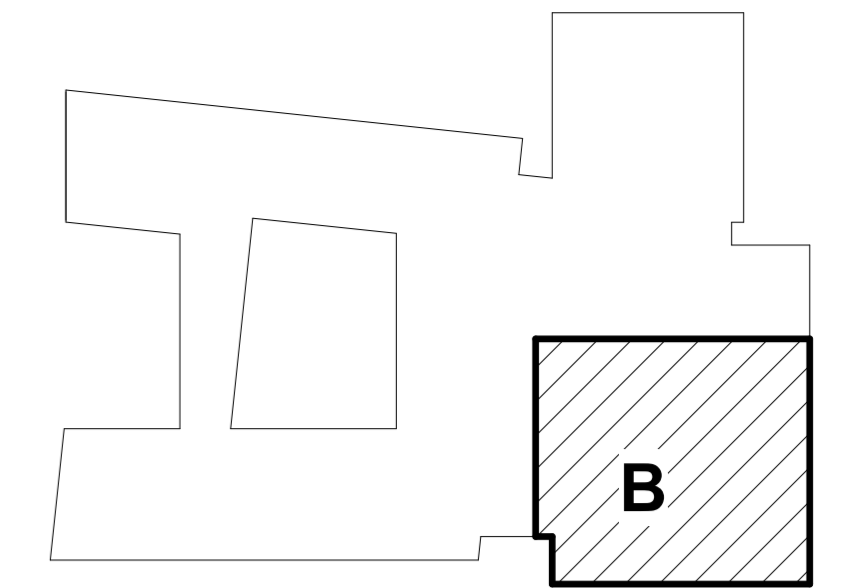
| TAG | LEVEL | REINFORCEMENT | | |
|--------|-------------|-----------------|------------|-----------------------------|
| | | VERTICAL | HORIZONTAL | END CELLS REINF. |
| SW-M1* | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (16) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M2 | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (8) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M3* | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (12) TOTAL |
| | 2ND TO 3RD | #6 @ 24" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (6) TOTAL |
| | 3RD TO ROOF | #6 @ 24" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M4 | G TO 2ND | #6 @ 16" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (6) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M5 | G TO 2ND | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (6) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M6 | G TO 2ND | #6 @ 8" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (2) TOTAL |
| | 2ND TO 3RD | #6 @ 8" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (2) TOTAL |
| | 3RD TO ROOF | #6 @ 8" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (2) TOTAL |
| SW-M7 | G TO 2ND | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |
| | 2ND TO 3RD | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |
| | 3RD TO ROOF | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |

- NOTES:
- ALL HORIZONTAL REBAR SHALL BE HOOKED AT THE ENDS OF WALLS, AT OPENINGS AND CORNERS OF WALL PER I.B.C. AND ACI 530 FOR SPECIAL REINFORCED SHEAR WALLS.
 - ALL MASONRY WALLS SHALL HAVE #8 STD DUR-O-WAL TRUSS REINFORCEMENT @ 16" IN ADDITION TO REINFORCEMENT SHOWN ABOVE.
 - WALL W-*** DENOTES ENTIRE WALL TO BE GROUTED SOLID.

NOTE: SHEAR WALLS ARE DENOTED WITH CROSSHATCH AS SHOWN BELOW.



1 - AREA A SOUTH
1/8" = 1'-0"

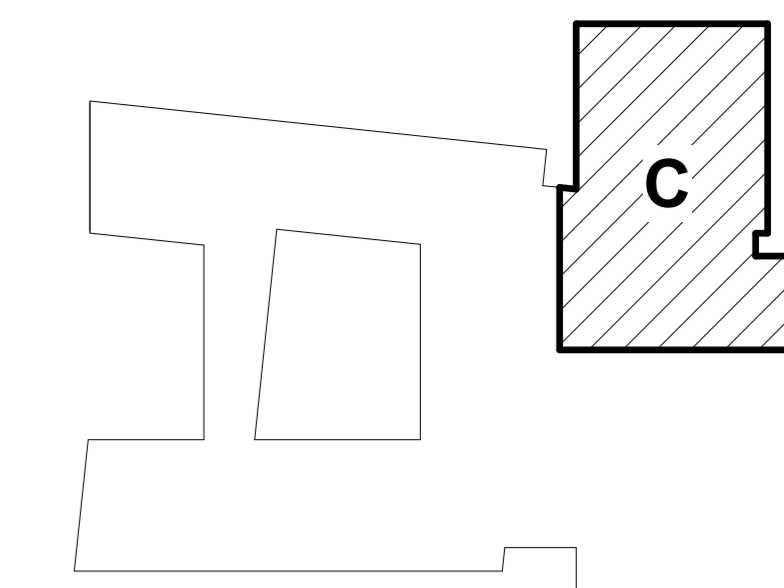


SECOND FLOOR FRAMING NOTES:

- TOP OF SECOND FLOOR PLAN = 20' - 0" AFF.
- TOP OF SECOND FLOOR STEEL IS 5" BELOW SLAB. 1/STEEL = 19' - 7" AFF UNO. SECOND FLOOR SLAB SHALL BE 3" L.W. CONC. SLAB OVER 2" VLI 16 GA COMPOSITE DECK W/ (1) LAYER OF 6#6-W2.9W.W.F. (5" TOTAL THICKNESS)
- (X) DENOTES NUMBER OF SHEAR STUDS REQUIRED, RE: GENERAL DETAILS FOR TYPICAL COMPOSITE DETAILS.
- BEAMS NOT INDICATING NUMBER OF SHEAR STUDS REQUIRED SHALL HAVE AT MIN STUDS PLACED ALONG BEAMS AT A MAX SPACING OF 2'-0" OC UNLESS SPECIFICALLY NOTES AS (0).
- "XX K" DENOTES FACTORED (LRFD) BEAM REACTION FOR SIZING BEAM CONNECTION. REACTION SHOWN SHALL BE APPLIED TO EACH END, NOT DIVIDED BY TWO. REACTION NOT LISTED SHALL BE ASSUMED TO BE 10K OR THE CONNECTION IS AS DETAILED ON THESE DRAWINGS.
- C = "X" DENOTES CAMBER REQUIRED IN STEEL BEAM. BEAMS AND GIRDERS HAVE BEEN CAMBERED.
- BEAM CAMBER SHALL BE INDUCED SUCH THAT THE BEAM HAS A CURVED PROFILE APPROXIMATING A PARABOLA. BEAMS CAMBERED BY FORCE, OR HEAT, AT A SINGLE LOCATION SHALL NOT BE PERMITTED.
- CONCRETE SCREED SUPPORTS SHALL BE PLACED ON BEAMS TO FOLLOW THE STEEL PROFILE SO AS TO GIVE A UNIFORM SLAB THICKNESS AT THE BEAMS. WHERE THE SLAB DEFLECTS TO BELOW FINISH FLOOR LEVEL THE SLAB SHALL BE THICKENED UP TO 1" MAX AT THE CENTER OF THE BEAM TO PROVIDE A LEVEL FLOOR. WHERE THE SLAB IS ABOVE FINISH FLOOR LEVEL THE SLAB SHALL REMAIN ABOVE FINISH FLOOR LEVEL SO THAT THE SLAB HAS THE MINIMUM THICKNESS SPECIFIED.
- THE GAUGE OF THE METAL FLOOR DECK SHOWN IS THE MINIMUM ALLOWED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING DECK SHORING AS REQUIRED TO AVOID OVER STRESSING THE DECK DURING PLACEMENT OF CONCRETE.
- FLOOR DECK SHALL HAVE (3) SPANS UNLESS APPROVED BY THE ENGINEER. IF APPROVED, MAXIMUM UNSHORED SPANS SHALL BE AS FOLLOWS (GREATER SPANS SHALL BE SHORED):
SINGLE SPAN = 11'-0"
DOUBLE SPAN = 13' - 1"
- CONSTRUCTION JOINT LOCATION AND CONCRETE PLACEMENT PROCEDURE IS VERY IMPORTANT AND SHALL BE DISCUSSED WITH THE STRUCTURAL ENGINEER PRIOR TO POURING THE SLAB.
- CONTRACTOR SHOULD NOTE THAT DECK AND BEAM DEFLECTION WILL INCREASE THE ESTIMATED CONCRETE QUANTITY.
- RE: GENERAL DETAILS FOR ADDITIONAL METAL DECK REQUIREMENTS.
- RE: GENERAL DETAILS FOR TYPICAL FLOOR DECK ATTACHMENT PATTERN.
- ALL FLOOR OPENING DIMENSIONS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SYMBOL DENOTES MOMENT CONNECTIONS DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.
- SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.

1 SECOND FLOOR FRAMING PLAN - AREA B
1/8" = 1'-0"

KEYPLAN



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CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

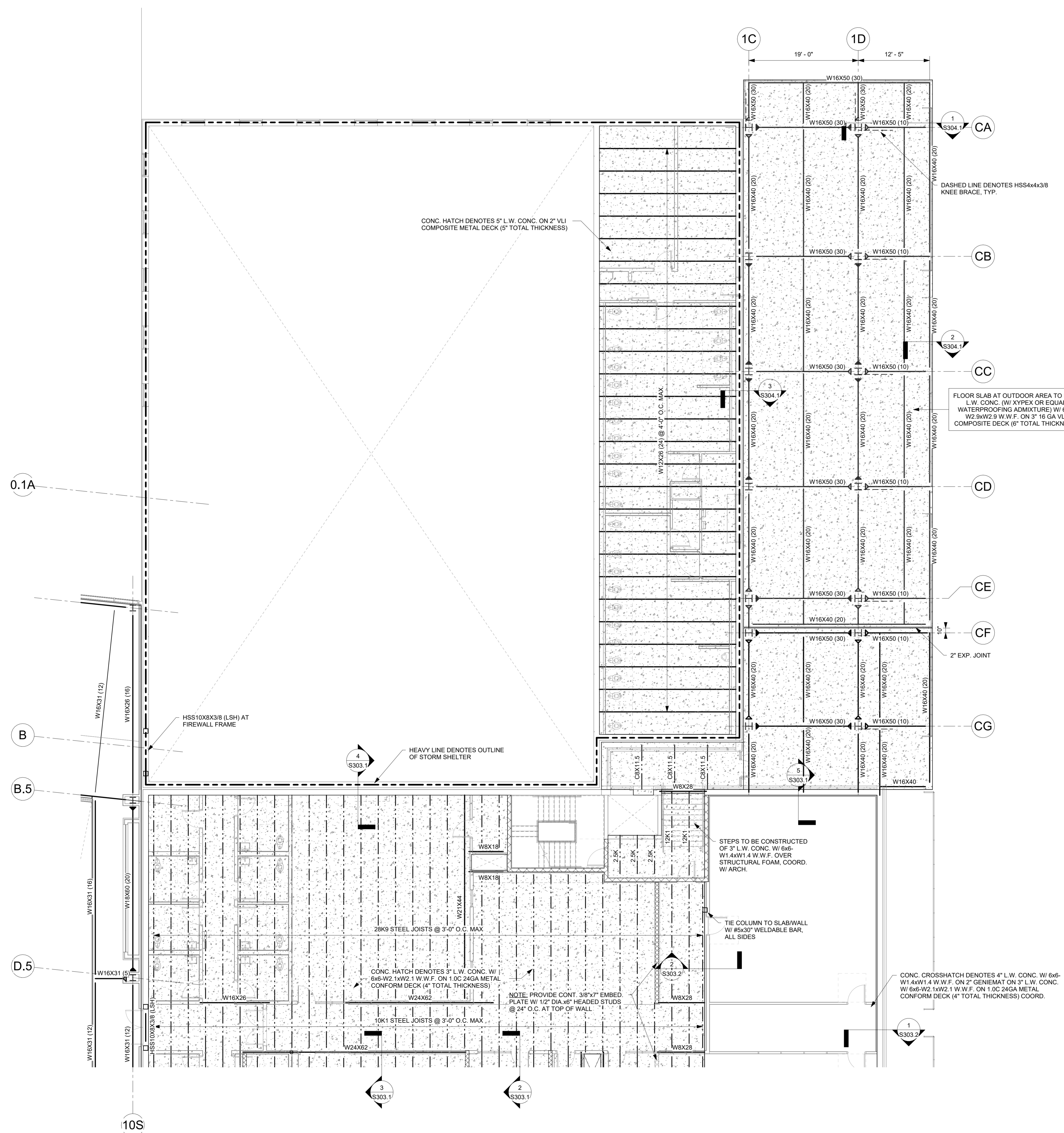
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



- SECOND FLOOR TERRACE FRAMING NOTES:**
- TOP OF SECOND FLOOR PLAN = 26' - 0" AFF.
 - TOP OF SECOND FLOOR STEEL IS 5' BELOW SLAB. 1" STEEL = 19' - 7" AFF UNO. SECOND FLOOR SLAB SHALL BE 5" L.W. CONCRETE SLAB OVER 2" VLI 16 GA COMPOSITE DECK W/ (1) LAYER OF 6x6 W2.9 W.W.F. (6" TOTAL THICKNESS)
 - (X) DENOTES NUMBER OF SHEAR STUDS REQUIRED, RE. GENERAL DETAILS FOR TYPICAL COMPOSITE DETAILS.
 - BEAMS NOT INDICATING NUMBER OF SHEAR STUDS REQUIRED SHALL HAVE AT MIN STUDS PLACED ALONG BEAMS AT A MAX SPACING OF 2'-0" OC UNLESS SPECIFICALLY NOTES AS (0).
 - "XX K" DENOTES FACTORED (LRFD) BEAM REACTION FOR SIZING BEAM CONNECTION. REACTION SHOWN SHALL BE APPLIED TO EACH END, NOT DIVIDED BY TWO. REACTION NOT LISTED SHALL BE ASSUMED TO BE 10K OR THE CONNECTION IS AS DETAILED ON THESE DRAWINGS.
 - C = "X" DENOTES CAMBER REQUIRED IN STEEL BEAM. BEAMS AND GIRDERS HAVE BEEN CAMBERED.
 - BEAM CAMBER SHALL BE INDUCED SUCH THAT THE BEAM HAS A CURVED PROFILE APPROXIMATING A PARABOLA. BEAMS CAMBERED BY FORCE, OR HEAT, AT A SINGLE LOCATION SHALL NOT BE PERMITTED.
 - CONCRETE SCREED SUPPORTS SHALL BE PLACED ON BEAMS TO FOLLOW THE STEEL PROFILE SO AS TO GIVE A UNIFORM SLAB THICKNESS AT THE BEAMS. WHERE THE SLAB DEFLECTS TO BELOW FINISH FLOOR LEVEL THE SLAB SHALL BE THICKENED (UP TO 1" MAX AT THE CENTER OF THE BEAM TO PROVIDE A LEVEL FLOOR). WHERE THE SLAB IS ABOVE FINISH FLOOR LEVEL THE SLAB SHALL REMAIN ABOVE FINISH FLOOR LEVEL SO THAT THE SLAB HAS THE MINIMUM THICKNESS SPECIFIED.
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 - FLOOR DECK SHALL HAVE (3) SPANS UNLESS APPROVED BY THE ENGINEER. IF APPROVED, MAXIMUM UNSHORED SPANS SHALL BE AS FOLLOWS (GREATER SPANS SHALL BE SHORED):
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 - SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.

1 SECOND FLOOR FRAMING PLAN - AREA C
 1/8" = 1'-0"



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

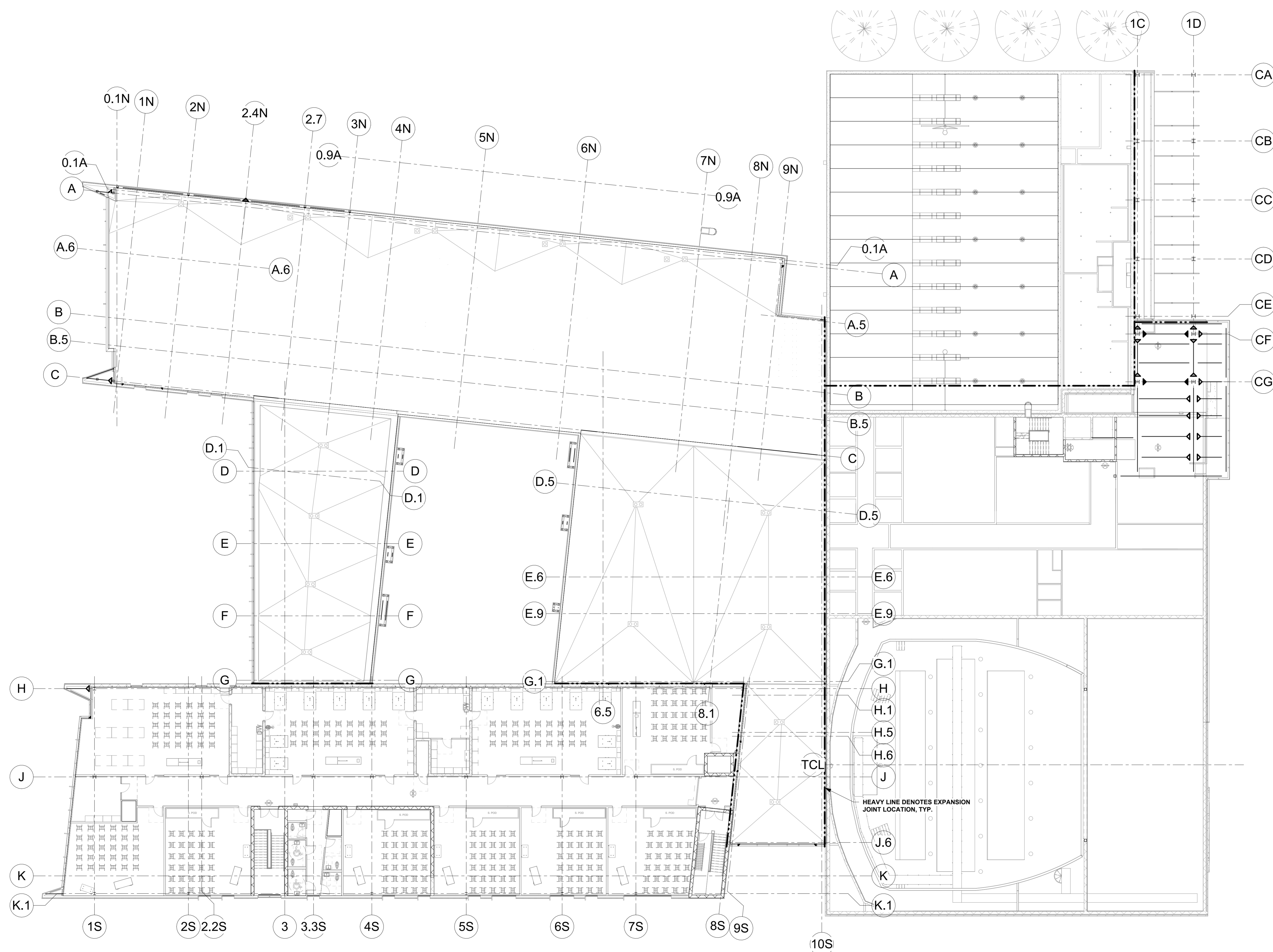
PROJECT NUMBER

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DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



1 THIRD FLOOR FRAMING PLAN - OVERALL
1/16" = 1'-0"

SHEET TITLE

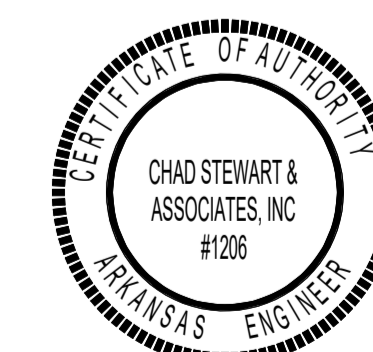
THIRD FLOOR FRAMING PLAN - OVERALL

DATE

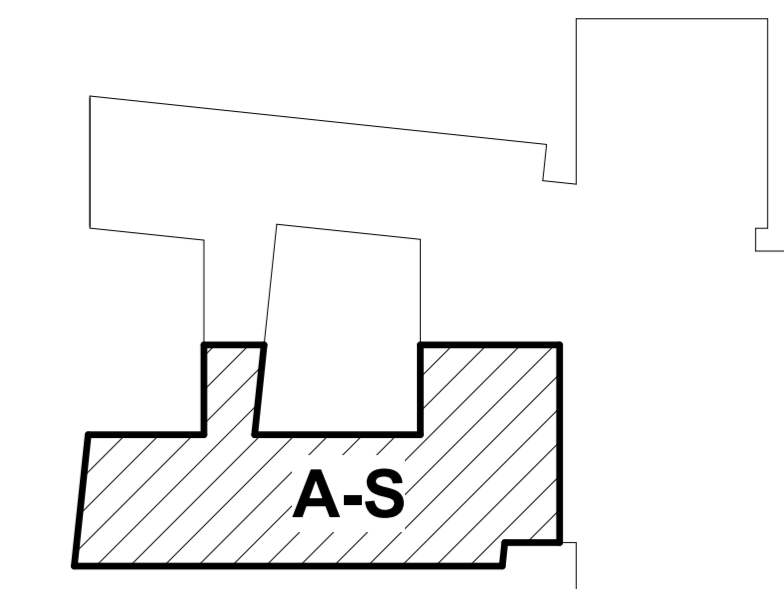
17.10.24

SHEET NUMBER

S103



KEYPLAN



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CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

PROJECT NUMBER

-

DEVELOPER/OWNER

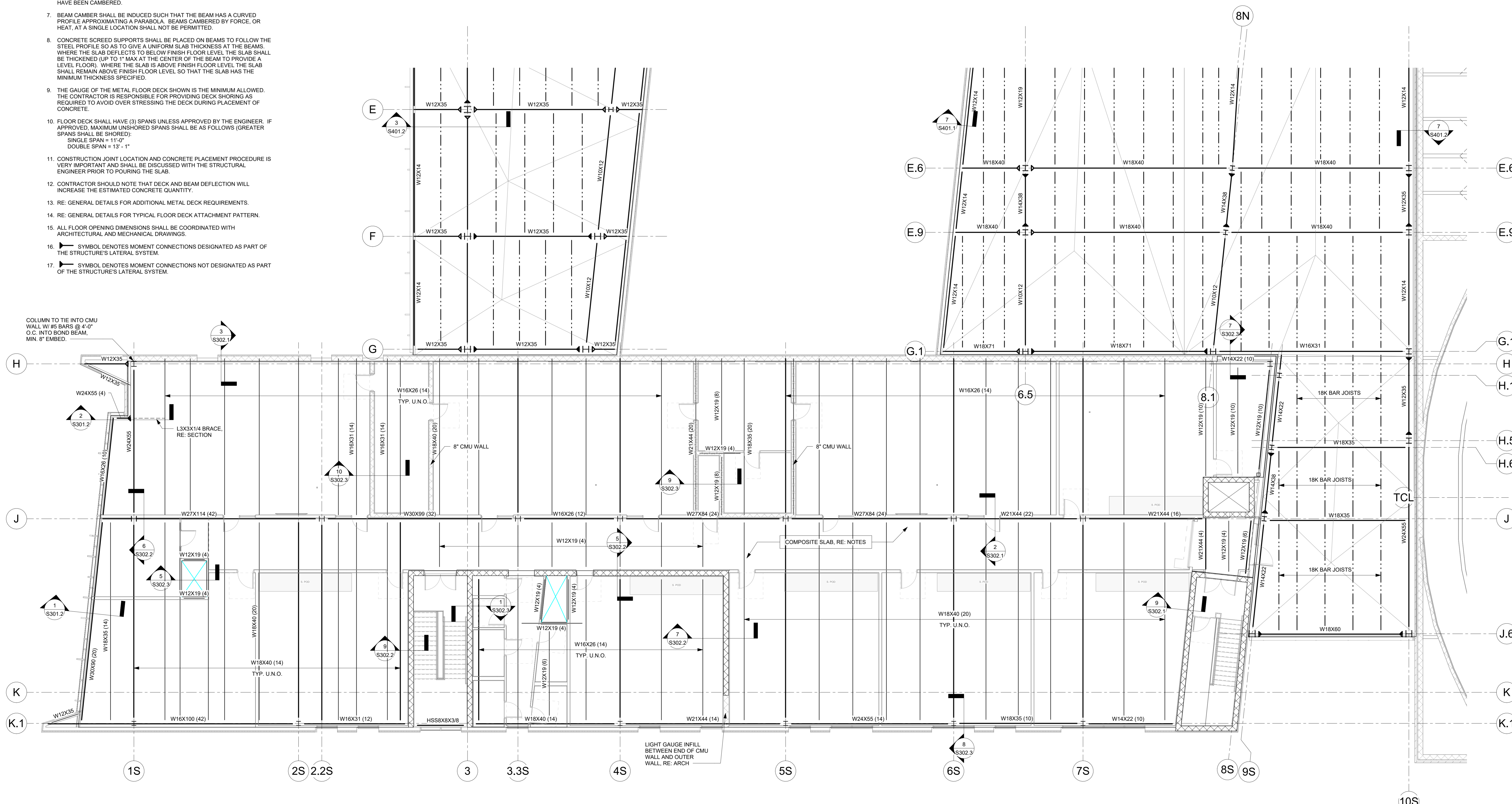
WYNNE SCHOOL DISTRICT

INFORMATION



THIRD FLOOR FRAMING NOTES:

- TOP OF THIRD FLOOR PLAN = 36' - 0" AFF.
- TOP OF SECOND FLOOR STEEL IS 5" BELOW SLAB. (TISEL = 35' - 7" AFF UNO. SECOND FLOOR SLAB SHALL 5" L.W. CONCRETE SLAB OVER 2" VLI 16 GA COMPOSITE DECK W/ (1) LAYER OF 6X6-W2.9XW2.9 WWF (5" TOTAL THICKNESS))
- (X) DENOTES NUMBER OF SHEAR STUDS REQUIRED, RE: GENERAL DETAILS FOR TYPICAL COMPOSITE DETAILS.
- BEAMS NOT INDICATING NUMBER OF SHEAR STUDS REQUIRED SHALL HAVE AT MIN STUDS PLACED ALONG BEAMS AT A MAX SPACING OF 2'-0" OC UNLESS SPECIFICALLY NOTES AS [0].
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SINGLE SPAN = 11'-0"
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- SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.



COLUMN TO TIE INTO CMU WALL W/ #5 BARS @ 4'-0" O.C. INTO BOND BEAM, MIN. 8" EMBED.

1 THIRD FLOOR FRAMING PLAN - AREA A SOUTH

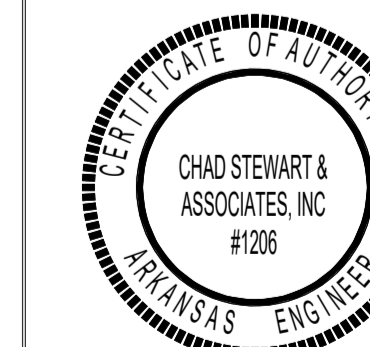
1/8" = 1'-0"

SHEET TITLE
THIRD FLOOR FRAMING PLAN - AREA A SOUTH

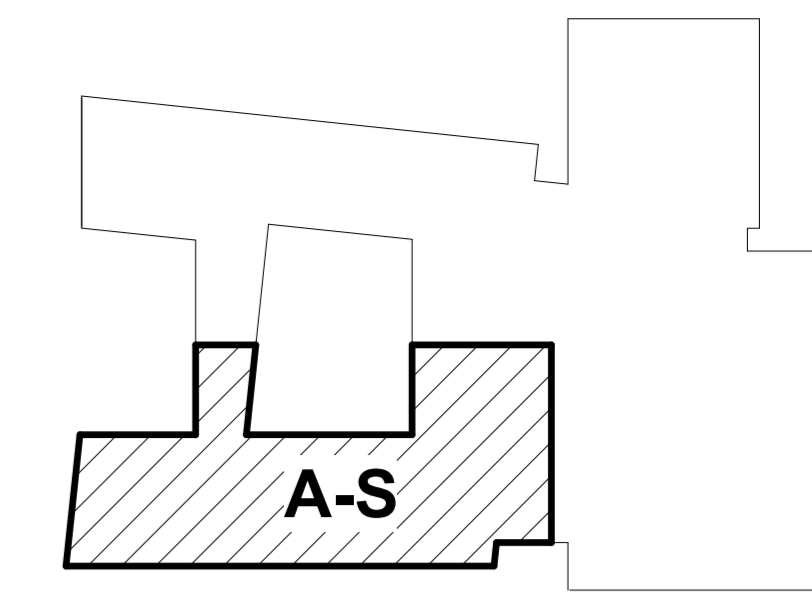
DATE
17.10.24

SHEET NUMBER

S103.1



KEYPLAN



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION

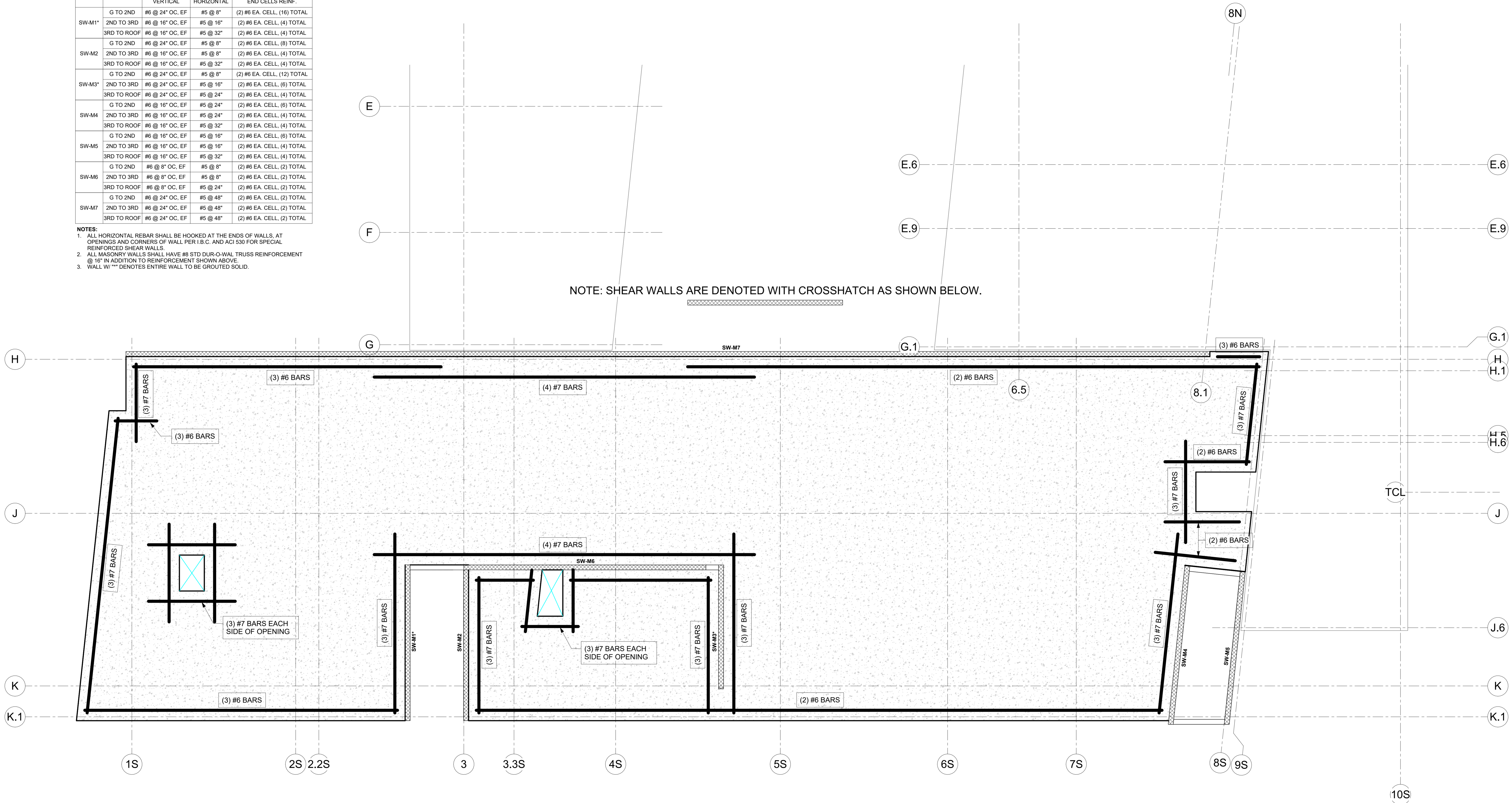


MASONRY SHEARWALL REINFORCEMENT SCHEDULE

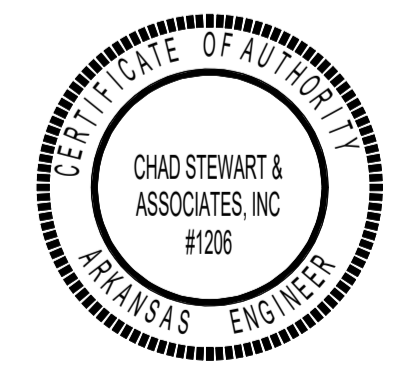
| TAG | LEVEL | REINFORCEMENT | | |
|--------|-------------|-----------------|------------|-----------------------------|
| | | VERTICAL | HORIZONTAL | END CELLS REINF. |
| SW-M1* | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (16) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M2 | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (8) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M3* | G TO 2ND | #6 @ 24" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (12) TOTAL |
| | 2ND TO 3RD | #6 @ 24" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (6) TOTAL |
| | 3RD TO ROOF | #6 @ 24" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M4 | G TO 2ND | #6 @ 16" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (6) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M5 | G TO 2ND | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (6) TOTAL |
| | 2ND TO 3RD | #6 @ 16" OC, EF | #5 @ 16" | (2) #6 EA. CELL, (4) TOTAL |
| | 3RD TO ROOF | #6 @ 16" OC, EF | #5 @ 32" | (2) #6 EA. CELL, (4) TOTAL |
| SW-M6 | G TO 2ND | #6 @ 8" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (2) TOTAL |
| | 2ND TO 3RD | #6 @ 8" OC, EF | #5 @ 8" | (2) #6 EA. CELL, (2) TOTAL |
| | 3RD TO ROOF | #6 @ 8" OC, EF | #5 @ 24" | (2) #6 EA. CELL, (2) TOTAL |
| SW-M7 | G TO 2ND | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |
| | 2ND TO 3RD | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |
| | 3RD TO ROOF | #6 @ 24" OC, EF | #5 @ 48" | (2) #6 EA. CELL, (2) TOTAL |

- NOTES:
- ALL HORIZONTAL REBAR SHALL BE HOOKED AT THE ENDS OF WALLS, AT OPENINGS AND CORNERS OF WALL PER I.B.C. AND ACI 530 FOR SPECIAL REINFORCED SHEAR WALLS.
 - ALL MASONRY WALLS SHALL HAVE #8 STD DUR-O-WAL TRUSS REINFORCEMENT @ 16" IN ADDITION TO REINFORCEMENT SHOWN ABOVE.
 - WALL W/ "" DENOTES ENTIRE WALL TO BE GROUTED SOLID.

NOTE: SHEAR WALLS ARE DENOTED WITH CROSSHATCH AS SHOWN BELOW.



THIRD FLOOR DIAPHRAGM CHORD REINF. PLAN - AREA A SOUTH
 1/8" = 1'-0"



SHEET TITLE

THIRD FLOOR DIAPHRAGM CHORD REINF. PLAN - AREA A SOUTH

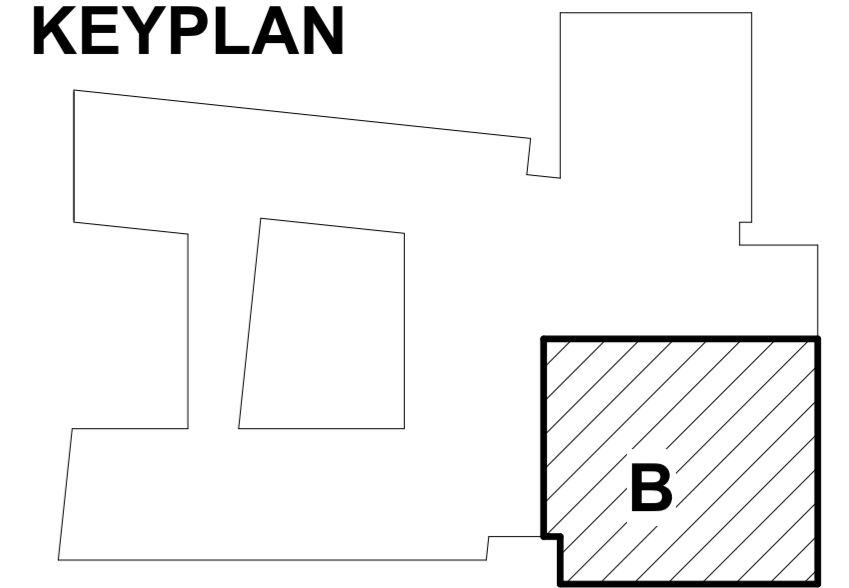
DATE

17.10.24

SHEET NUMBER

S103.1A

KEYPLAN



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CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

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WYNNE AR 72396

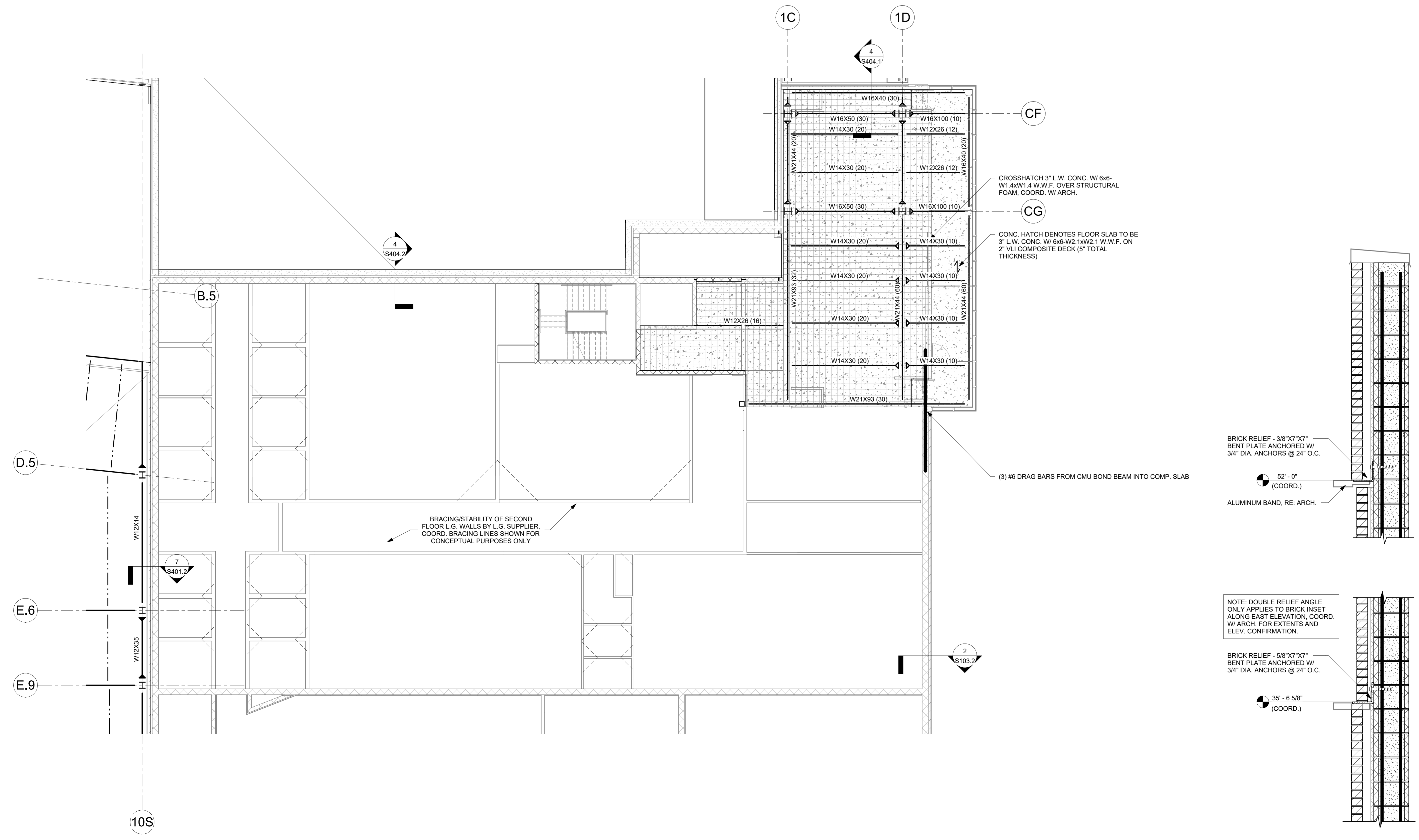
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



1 THIRD FLOOR FRAMING PLAN - AREA B
1/8" = 1'-0"

2 BRICK RELIEF
3/4" = 1'-0"

SHEET TITLE
THIRD FLOOR
FRAMING PLAN - AREA B

DATE
17.10.24

SHEET NUMBER

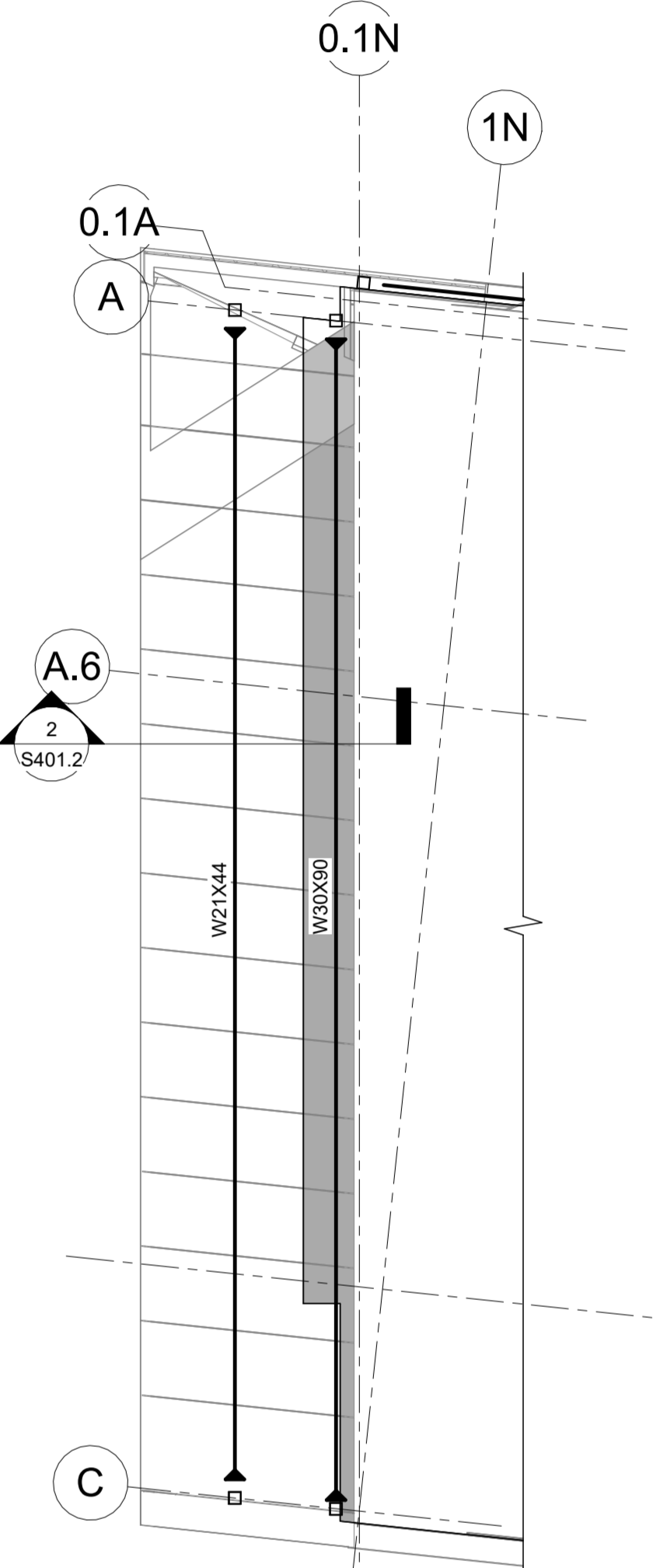
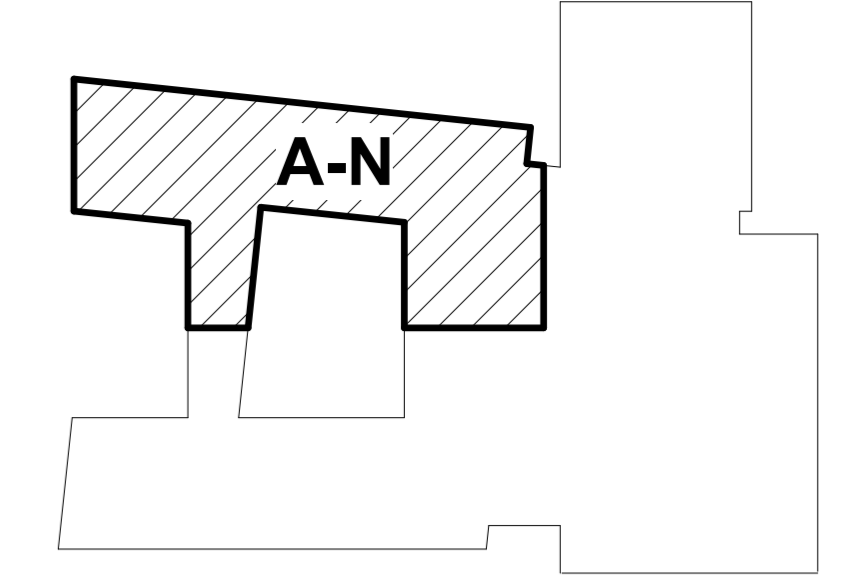
S103.2



- ROOF FRAMING NOTES:**
- ELEVATIONS ARE BASED ON TOP OF SLAB DATUM EL. 0'-0". COORD. ACTUAL FINISHED FLOOR ELEVATION.
 - FOR DESIGN CRITERIA, GENERAL NOTES, AND TYPICAL DETAILS, RE: GENERAL NOTES & DETAIL SHEETS.
 - FOR ELEVATIONS, WALL SECTIONS, AND DIMENSIONS NOT SHOWN, RE: ARCHITECTURAL DRAWINGS.
 - TYPICAL METAL ROOF DECK SHALL BE 1.58 METAL DECKING.
 - RE: GENERAL DETAILS FOR TYPICAL ROOF OPENING DETAILS.
 - RE: GENERAL DETAILS FOR TYPICAL ROOF TOP UNIT SUPPORT DETAILS.
 - SYMBOL DENOTES MOMENT CONNECTIONS DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.
 - SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.

1 ROOF FRAMING PLAN - AREA A NORTH
1/8" = 1'-0"

KEYPLAN



2 AREA A NORTH - FOIL
1/8" = 1'-0"

ARCH 1010

676 Marshall Ave., Suite 101
Memphis, TN 38103
901.497.6563
www.arch1010.com

POLK STANLEY WILCOX

801 South Spring Street
Little Rock, AR 72201
501.378.0878 office
www.polkstanleywilcox.com

CONSULTANT / SEAL



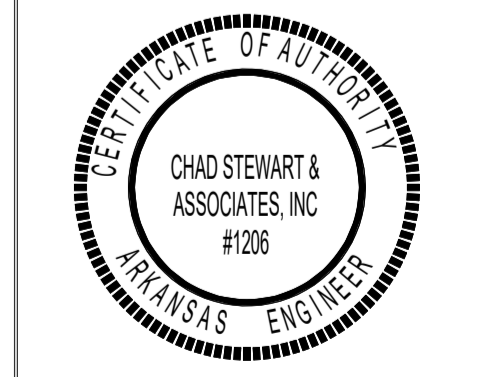
PROJECT NAME
WSD - NEW SENIOR HIGH SCHOOL

LOCATION
800 E JACKSON AVE
WYNNE AR 72396

PROJECT NUMBER
-

DEVELOPER/OWNER
WYNNE SCHOOL DISTRICT

INFORMATION

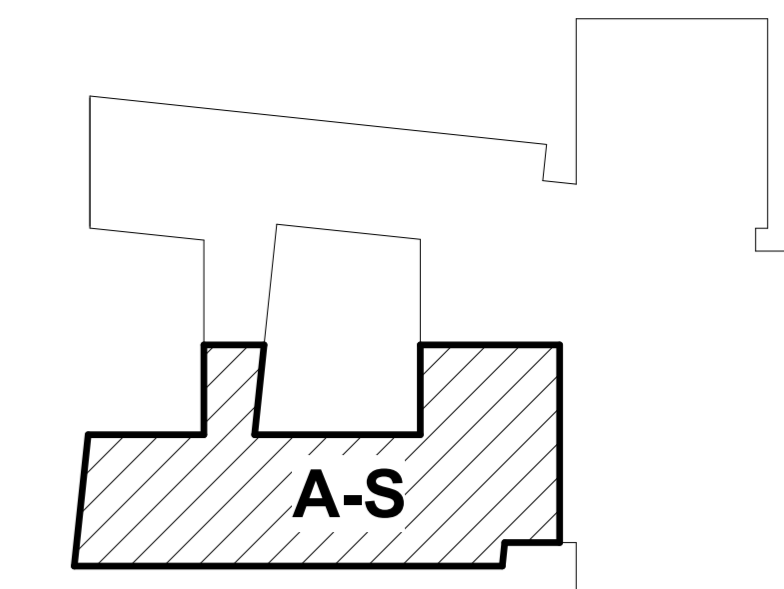


SHEET TITLE
ROOF FRAMING PLAN - AREA A NORTH

DATE
17.10.24

SHEET NUMBER

KEYPLAN



ARCH 1010

676 Marshall Ave. Suite 101
Memphis, TN 38103
901.431.6563
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STANLEY
WILCOX**

801 South Spring Street
Little Rock, AR 72201
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www.polkstanleywilcox.com

CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

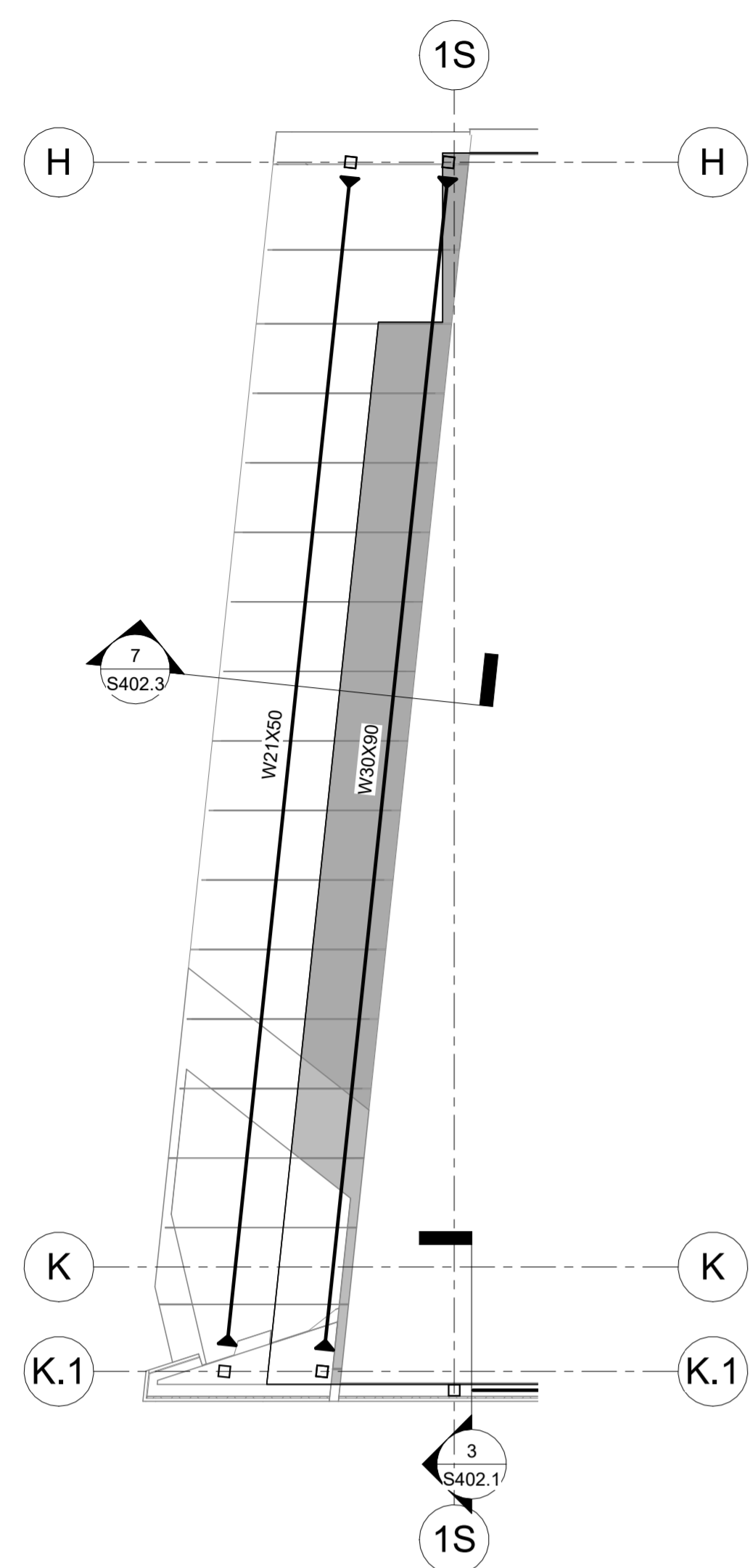
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

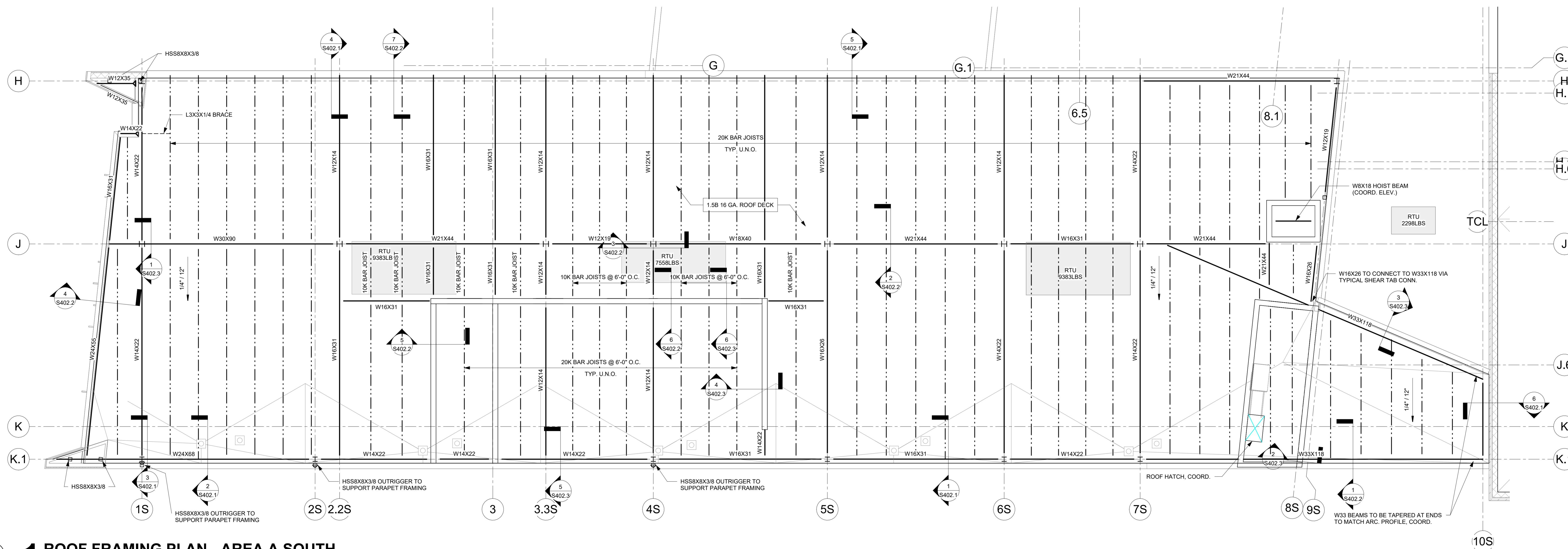
INFORMATION



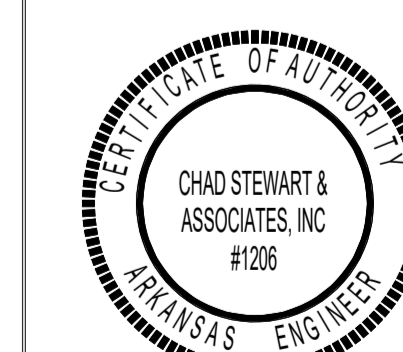
ROOF FRAMING NOTES:

- ELEVATIONS ARE BASED ON TOP OF SLAB DATUM EL 0'-0". COORD. ACTUAL FINISHED FLOOR ELEVATION.
- FOR DESIGN CRITERIA, GENERAL NOTES, AND TYPICAL DETAILS, RE: GENERAL NOTES & DETAIL SHEETS.
- FOR ELEVATIONS, WALL SECTIONS, AND DIMENSIONS NOT SHOWN, RE: ARCHITECTURAL DRAWINGS.
- TYPICAL METAL ROOF DECK SHALL BE 1.5B METAL DECKING.
- RE: GENERAL DETAILS FOR TYPICAL ROOF OPENING DETAILS.
- RE: GENERAL DETAILS FOR TYPICAL ROOF TOP UNIT SUPPORT DETAILS.
- SYMBOL DENOTES MOMENT CONNECTIONS DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.
- SYMBOL DENOTES MOMENT CONNECTIONS NOT DESIGNATED AS PART OF THE STRUCTURE'S LATERAL SYSTEM.

2 AREA A SOUTH - FOIL
1/8" = 1'-0"



1 ROOF FRAMING PLAN - AREA A SOUTH
1/8" = 1'-0"



SHEET TITLE
ROOF FRAMING PLAN - AREA A SOUTH

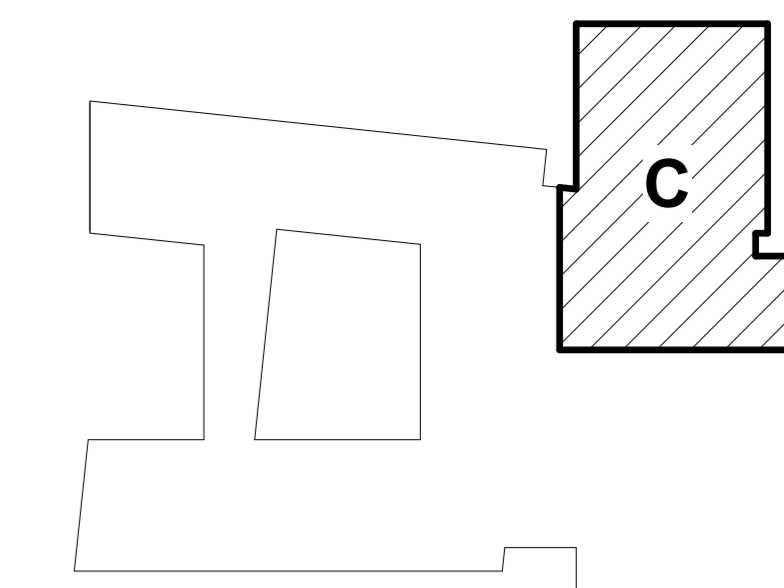
DATE

17.10.24

SHEET NUMBER

S104.2

KEYPLAN



ARCH 1010

676 Marshall Ave., Suite 101
 Memphis, TN 38103
 901.497.6563
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**POLK
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801 South Spring Street
 Little Rock, AR 72201
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 www.polkstanleywilcox.com

CONSULTANT / SEAL



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

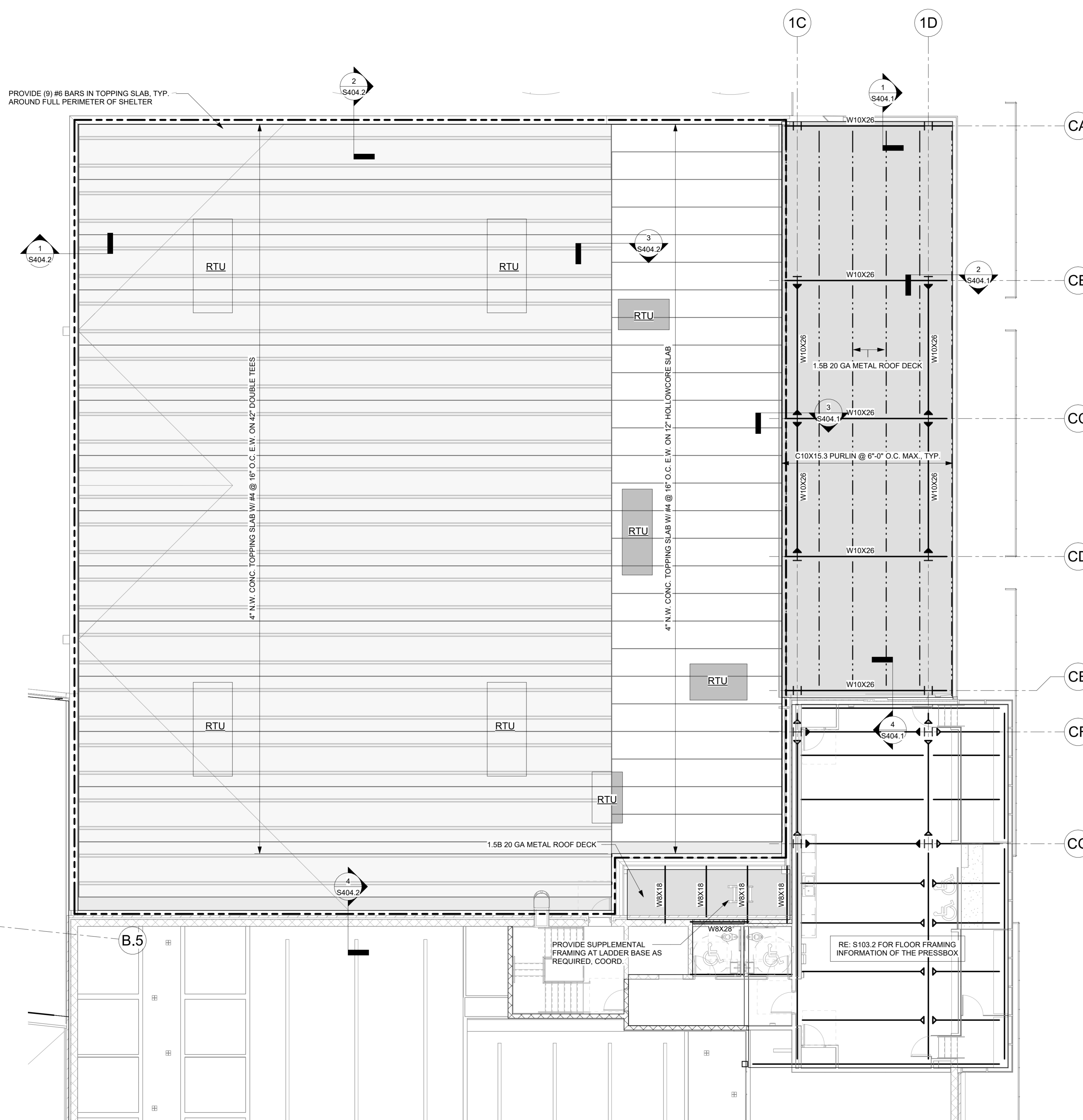
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



1 ROOF FRAMING PLAN - AREA C
 1/8" = 1'-0"

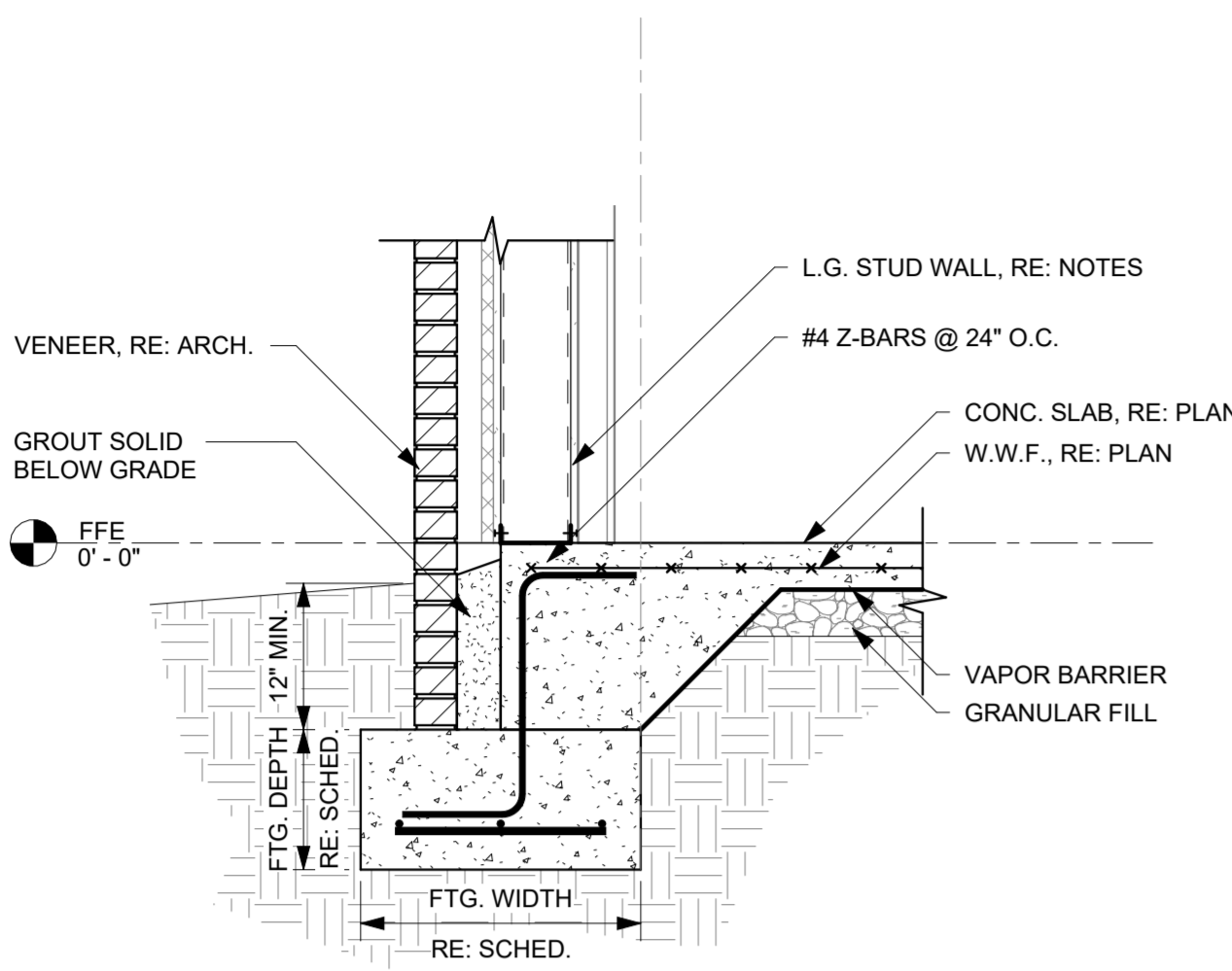
SHEET TITLE
 ROOF FRAMING PLAN -
 AREA C

DATE
 17.10.24

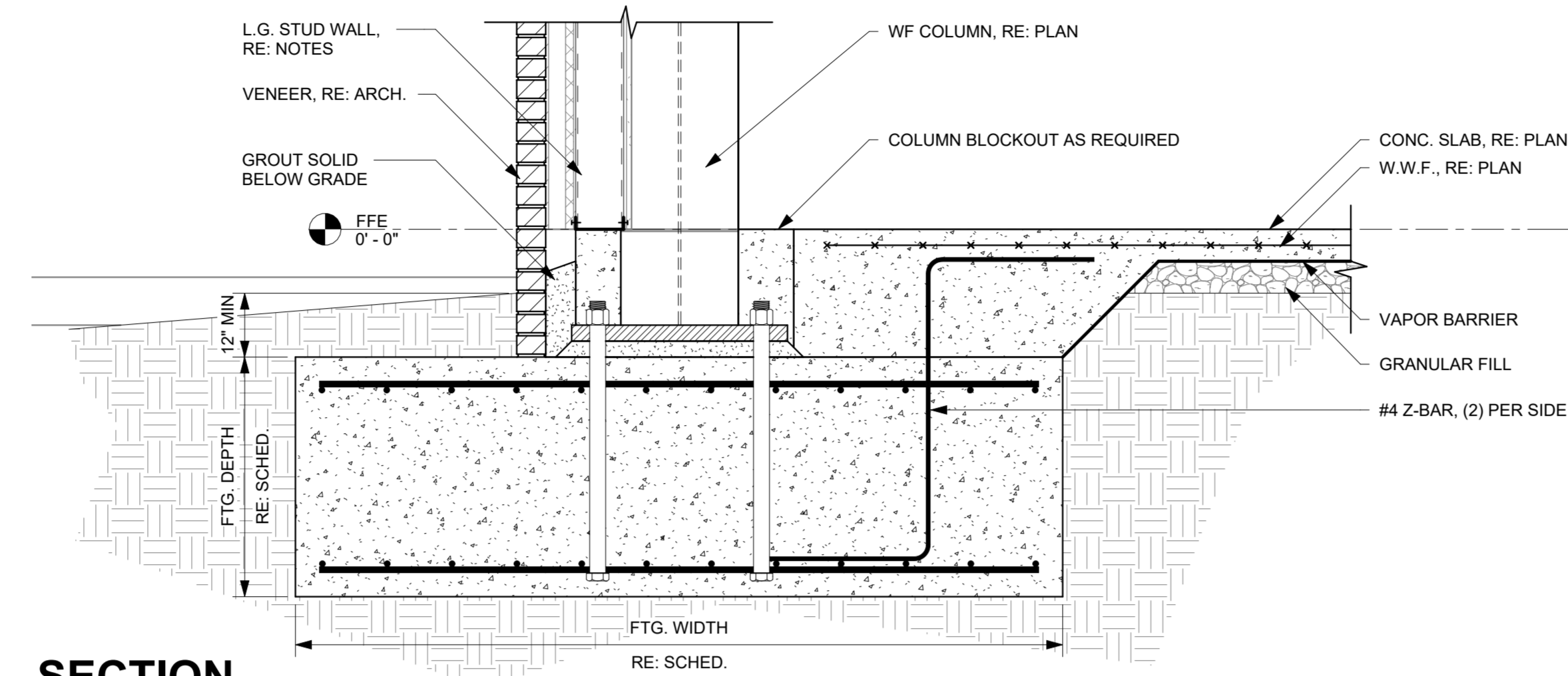
SHEET NUMBER

S104.4

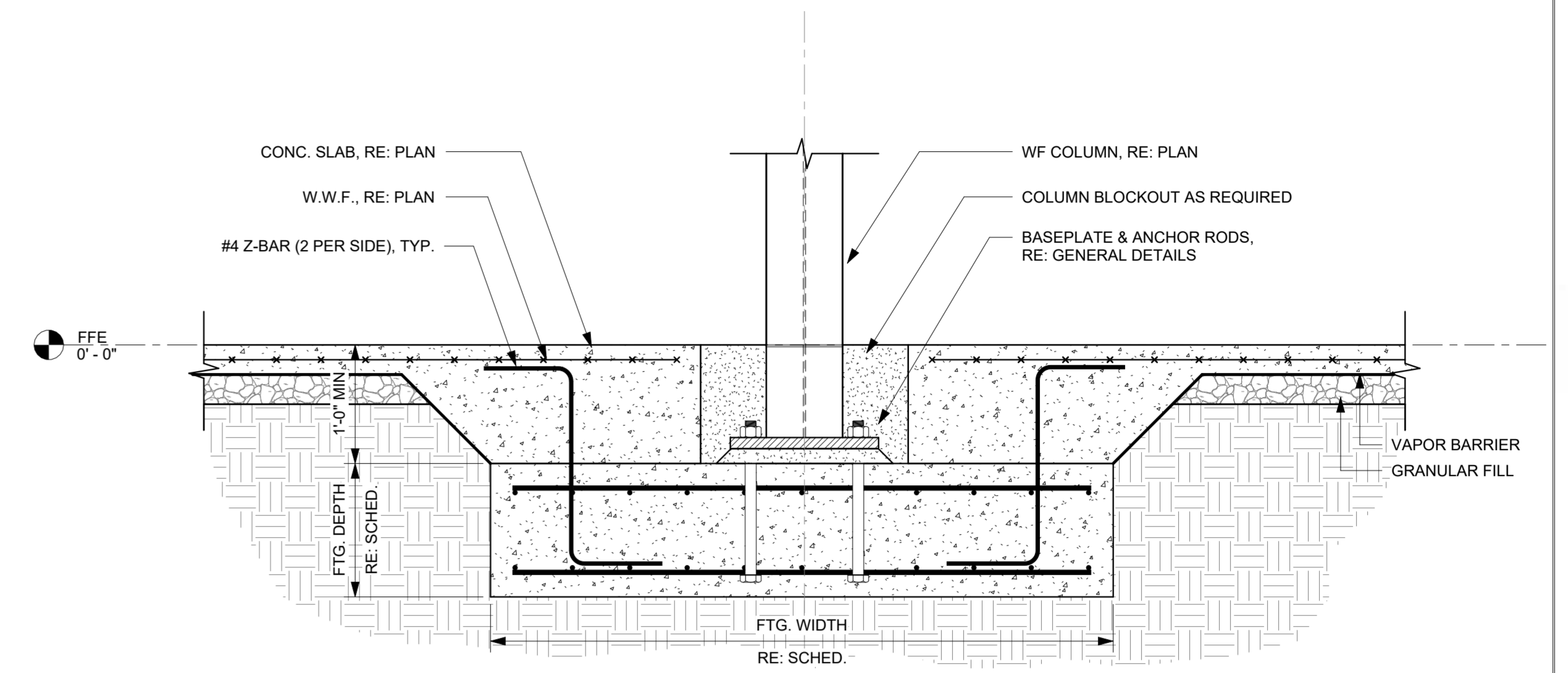




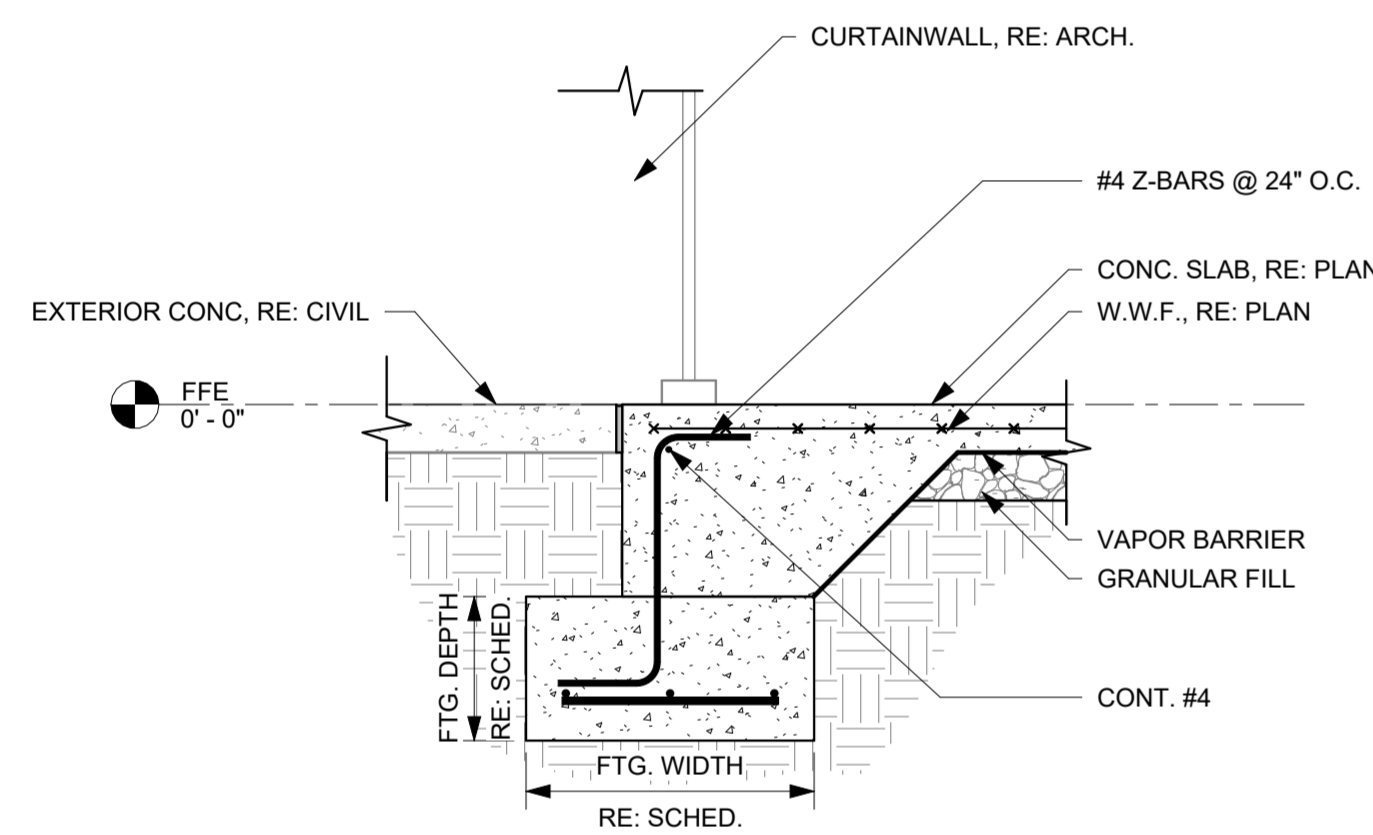
1 FDS - SECTION
 3/4" = 1'-0"



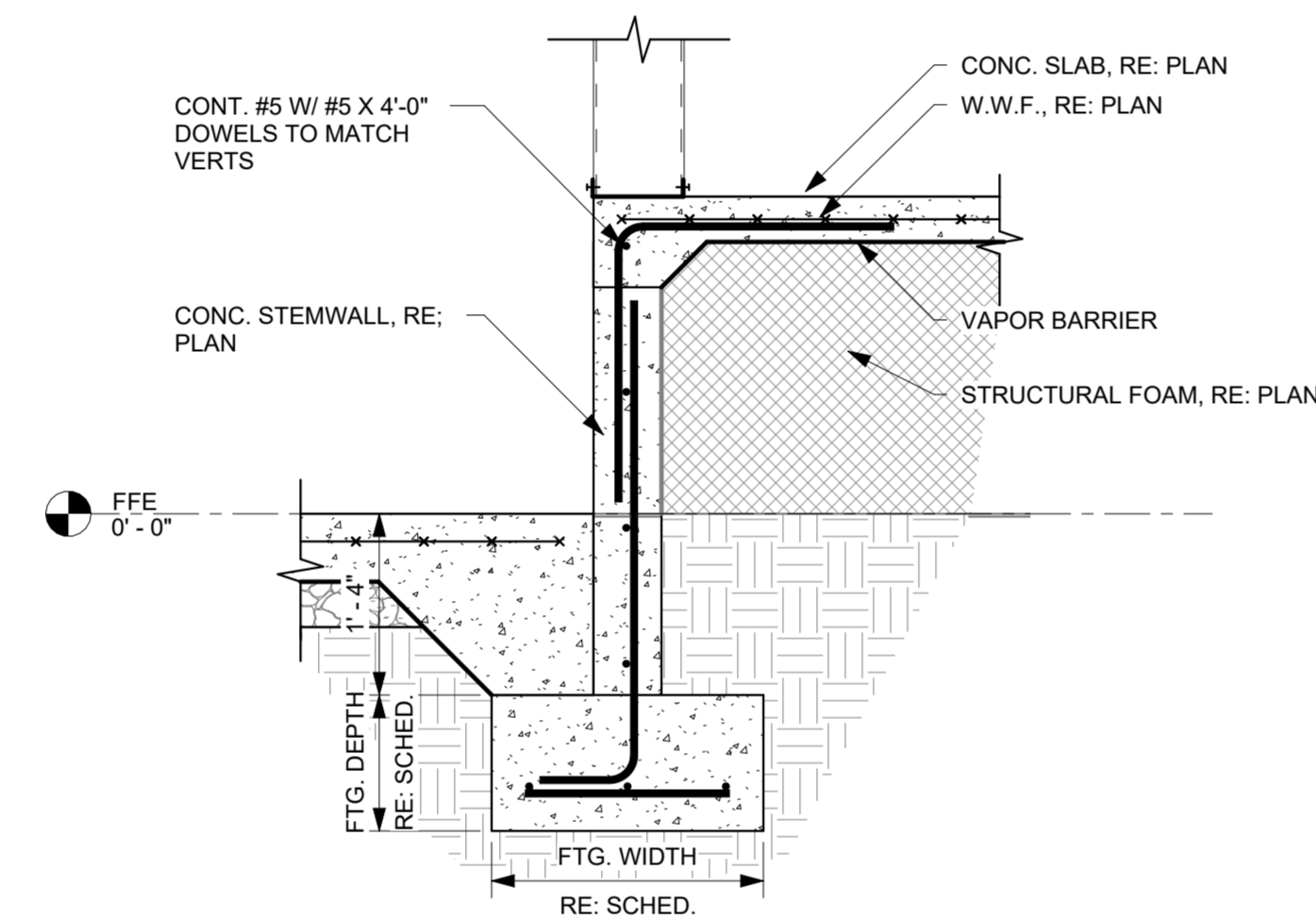
2 FDS - SECTION
 3/4" = 1'-0"



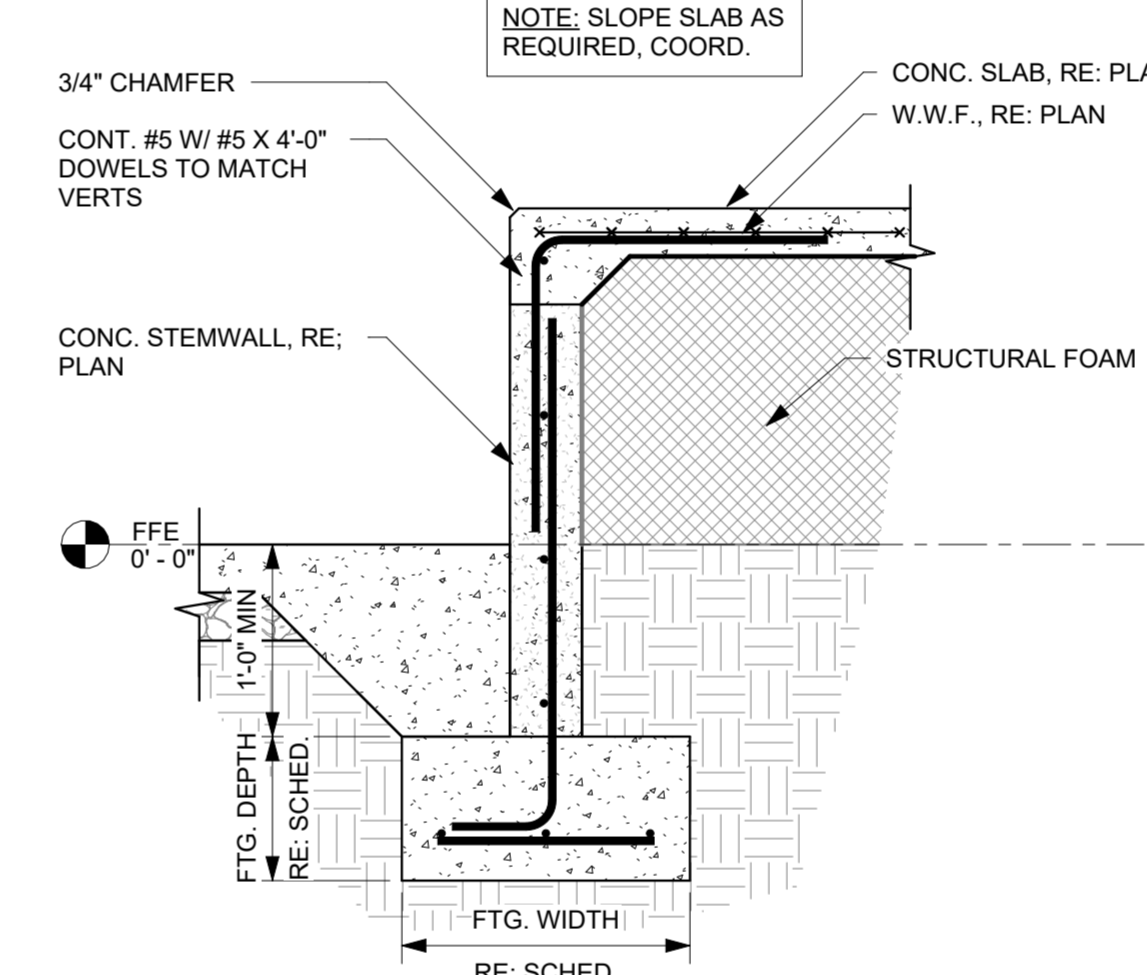
3 FDS - SECTION
 3/4" = 1'-0"



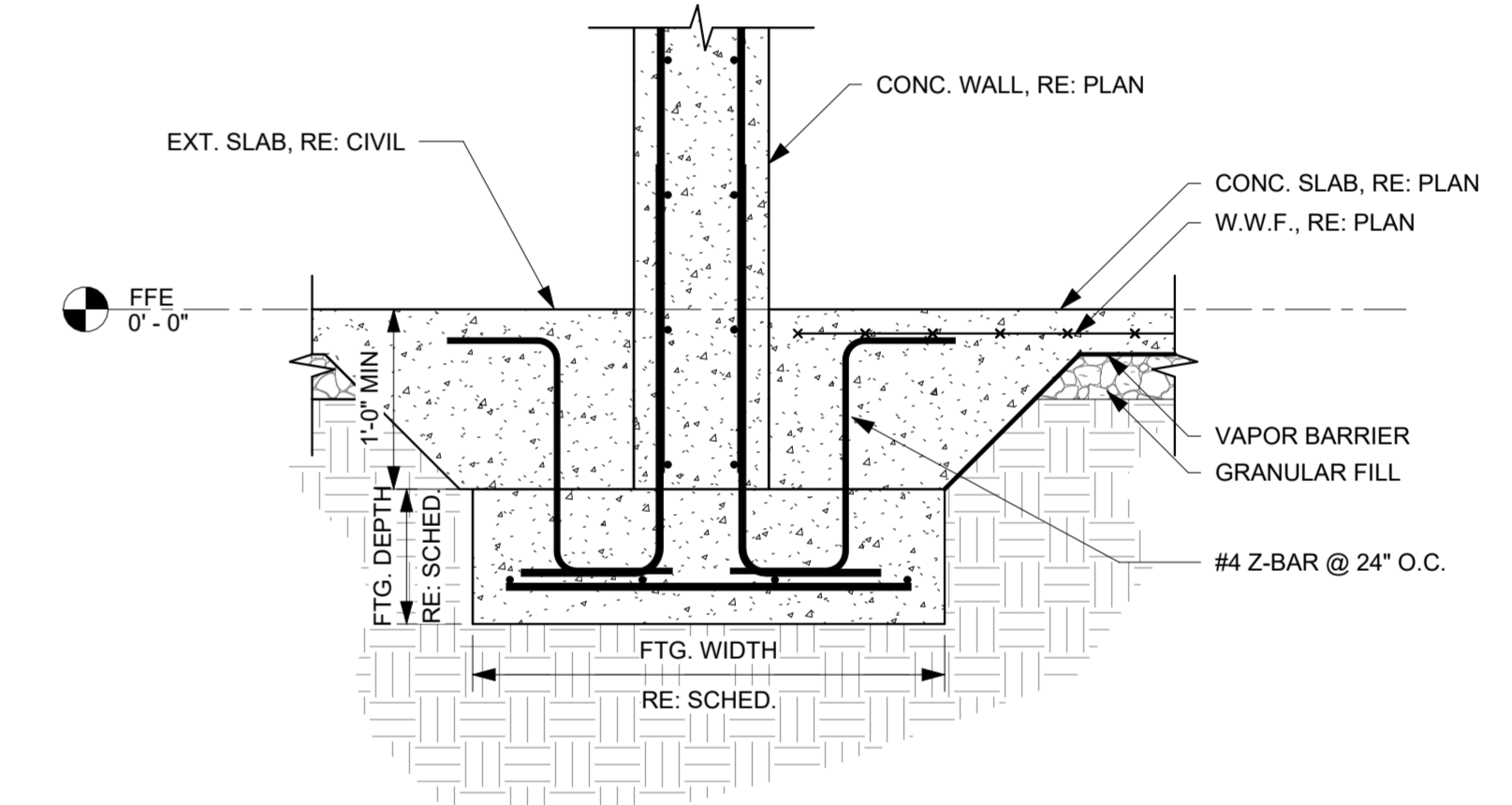
4 FDS - SECTION
 3/4" = 1'-0"



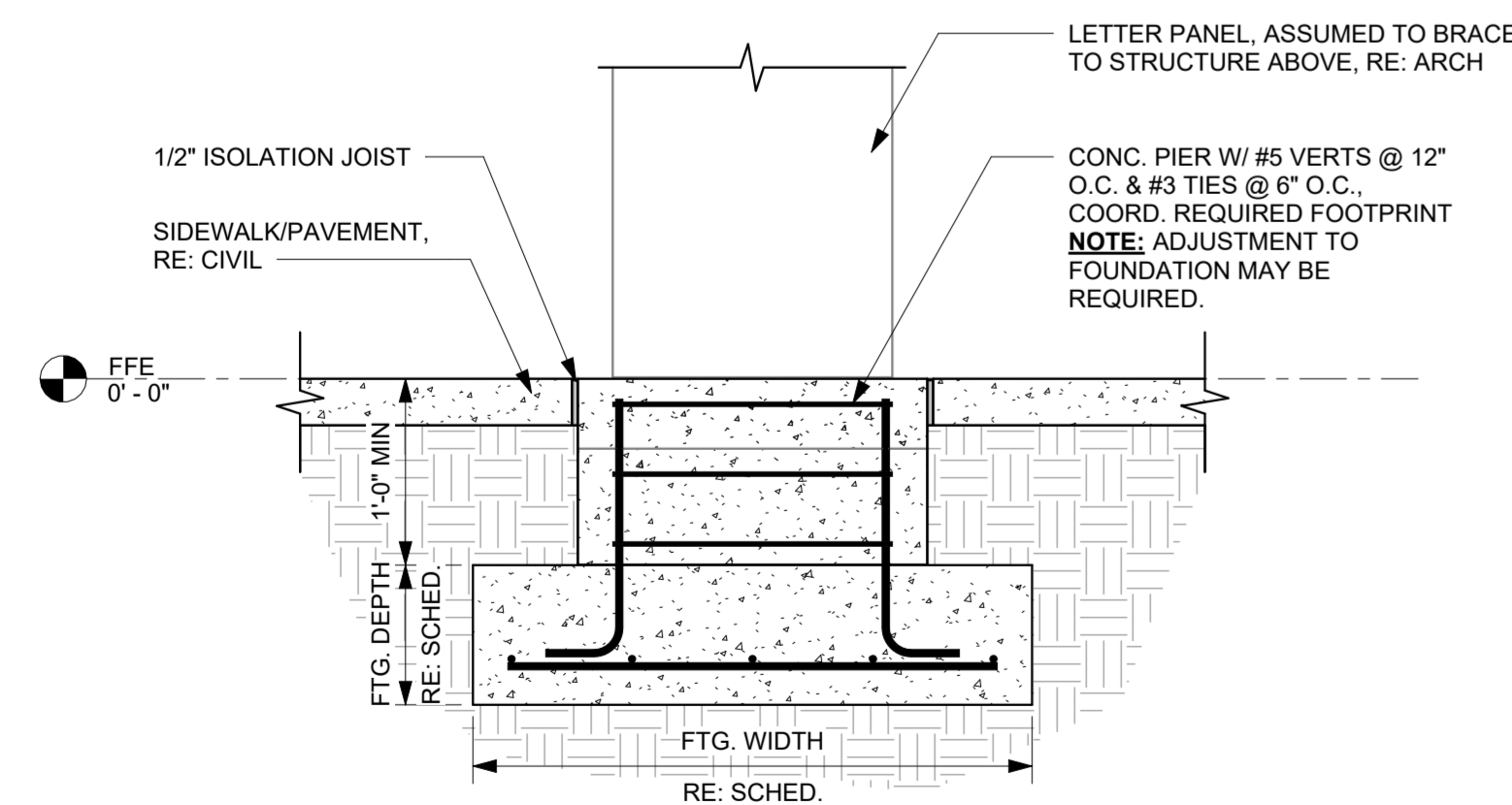
5 FDS - SECTION
 3/4" = 1'-0"



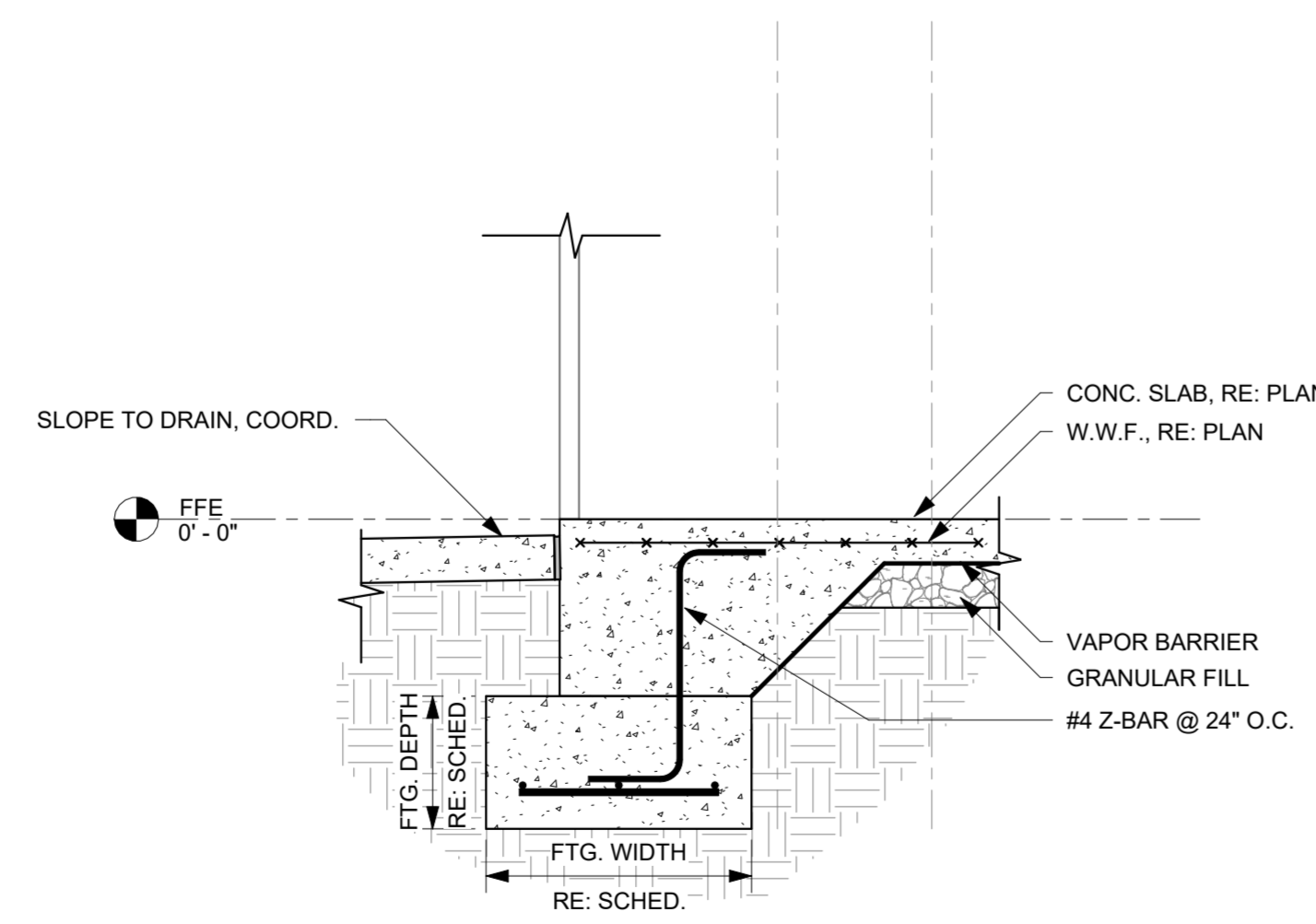
6 FDS - SECTION
 3/4" = 1'-0"



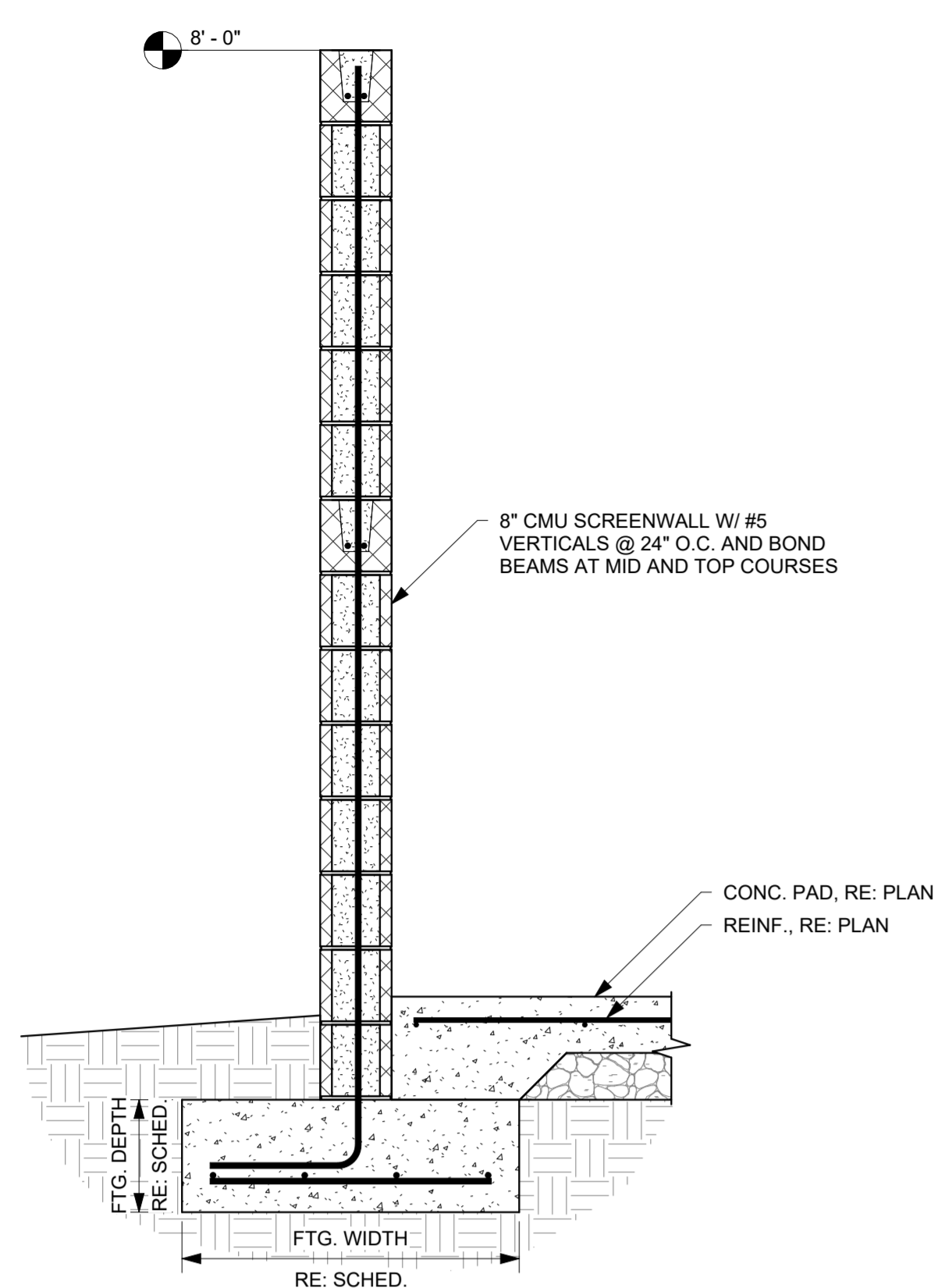
7 FDS - SECTION
 3/4" = 1'-0"



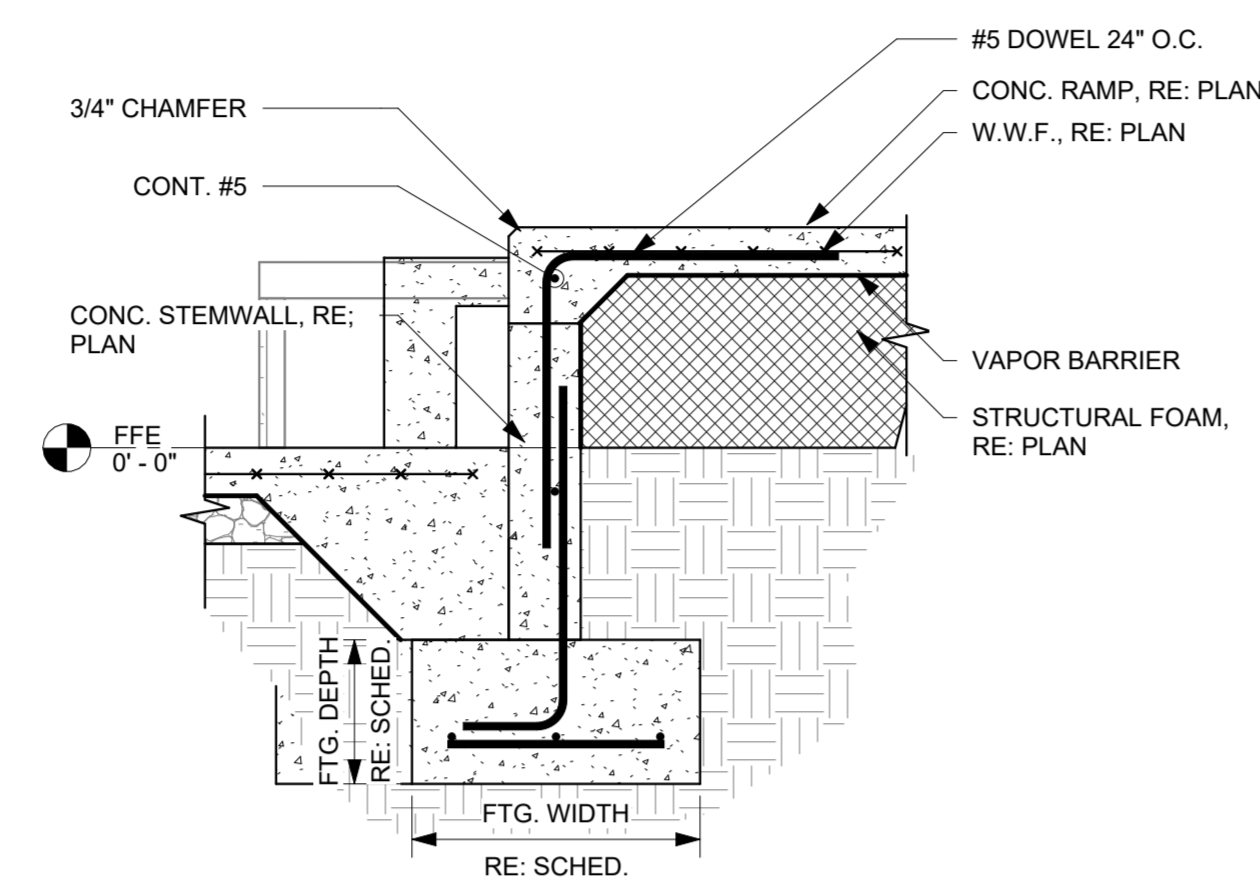
8 FDS - SECTION
 3/4" = 1'-0"



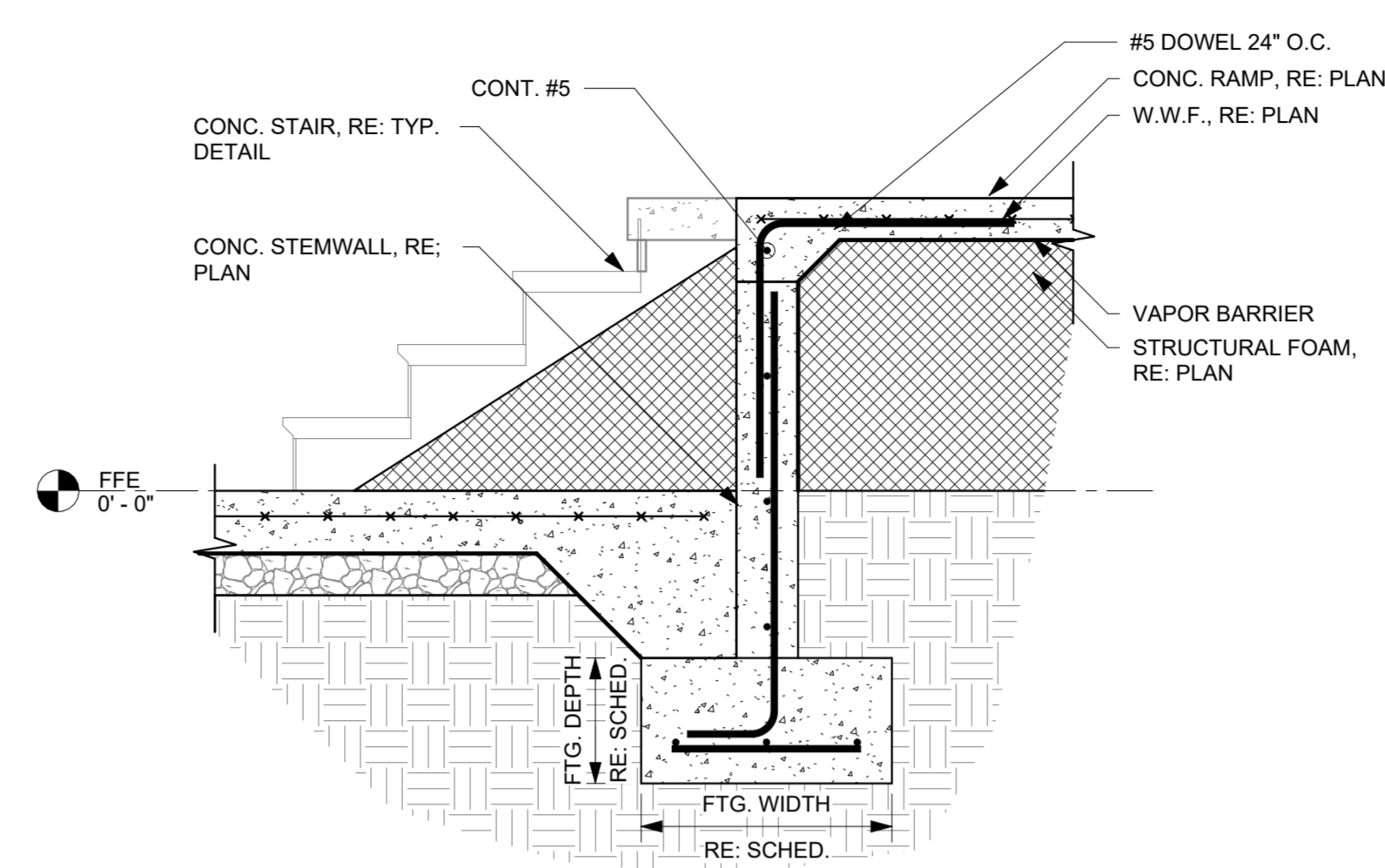
9 FDS - SECTION
 3/4" = 1'-0"



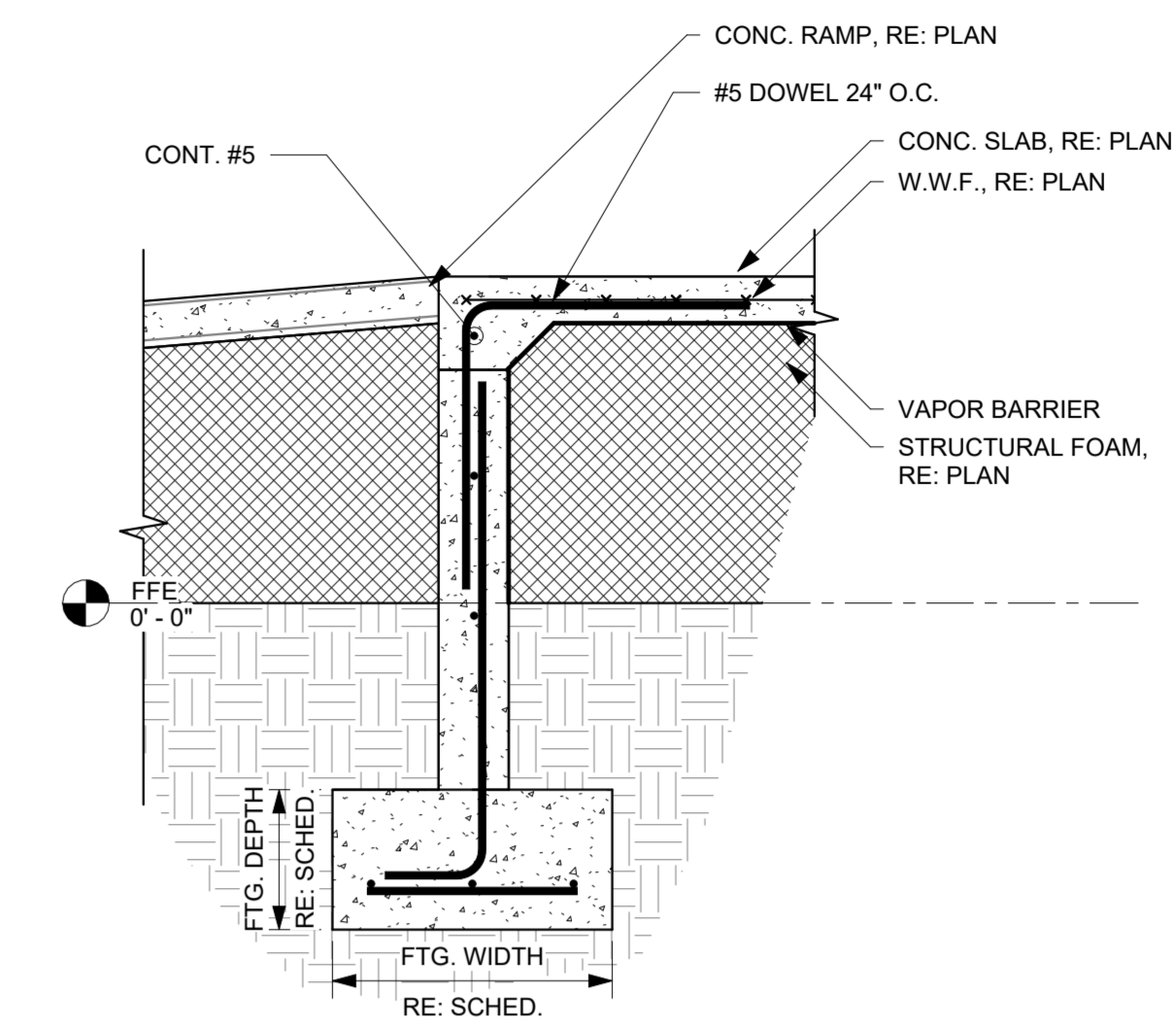
1 FDS - SECTION
3/4" = 1'-0"



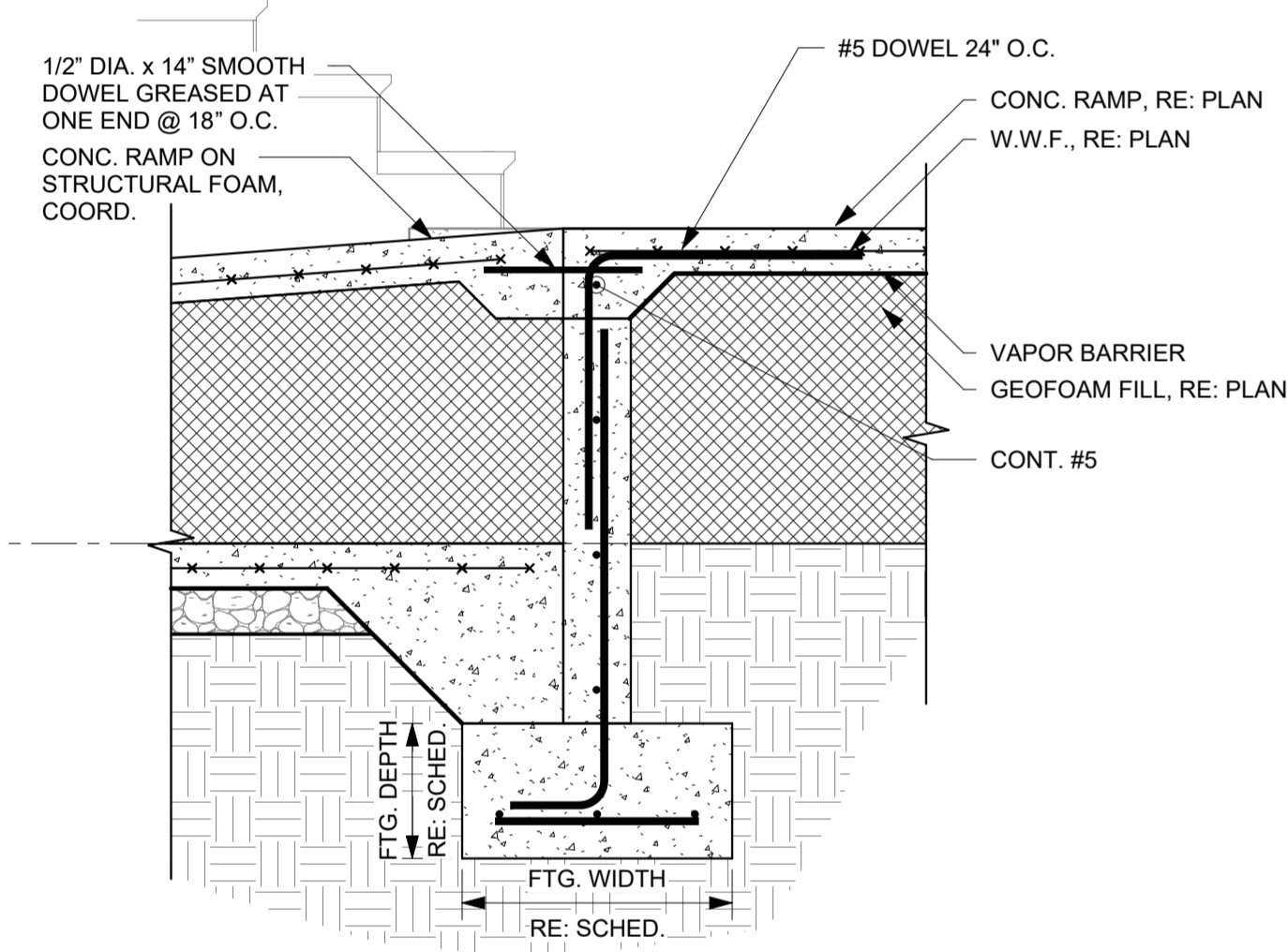
2 FDS - SECTION
3/4" = 1'-0"



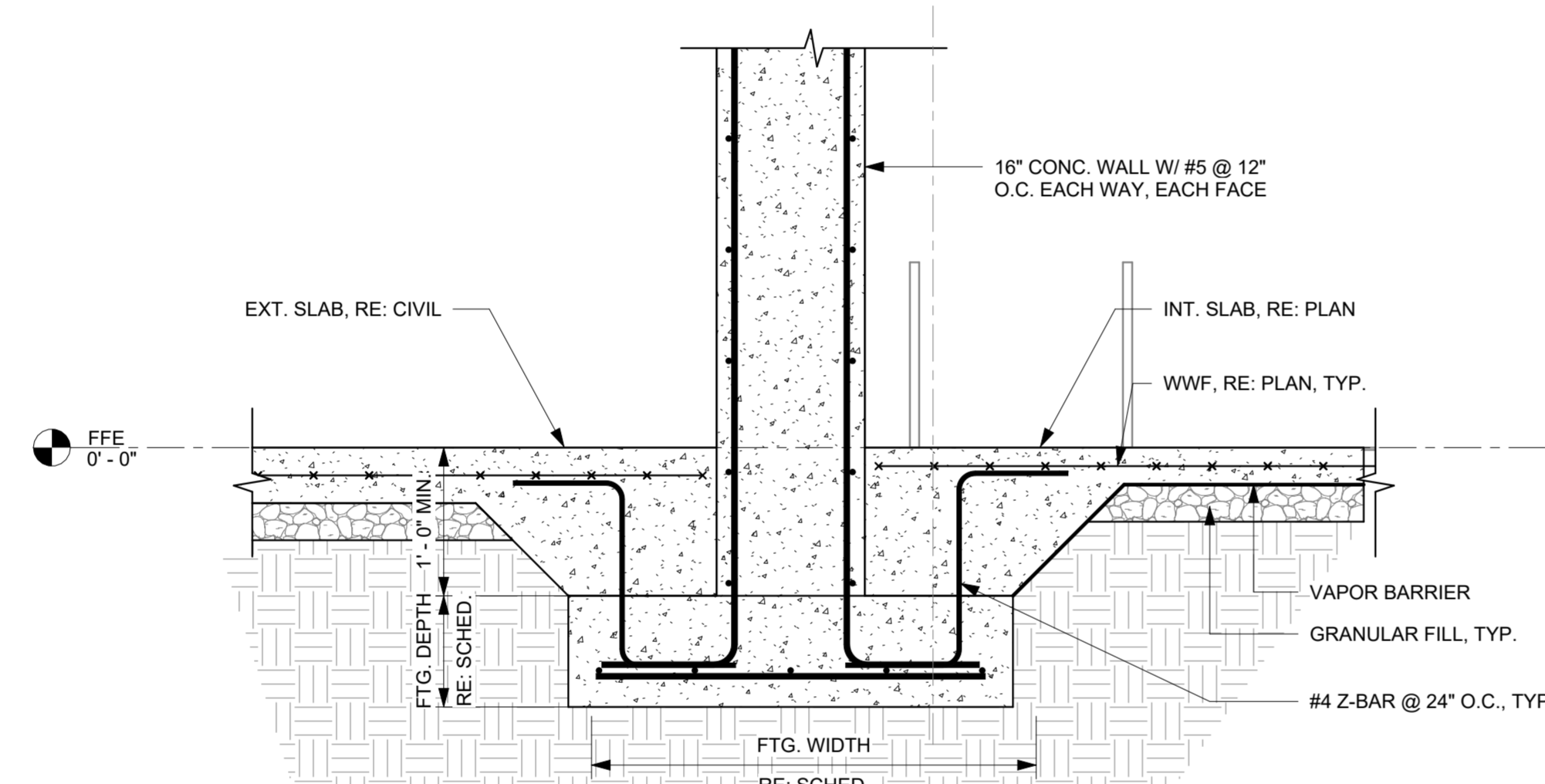
3 FDS - SECTION
3/4" = 1'-0"



4 FDS - SECTION
3/4" = 1'-0"



5 FDS - SECTION
3/4" = 1'-0"



6 FDS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

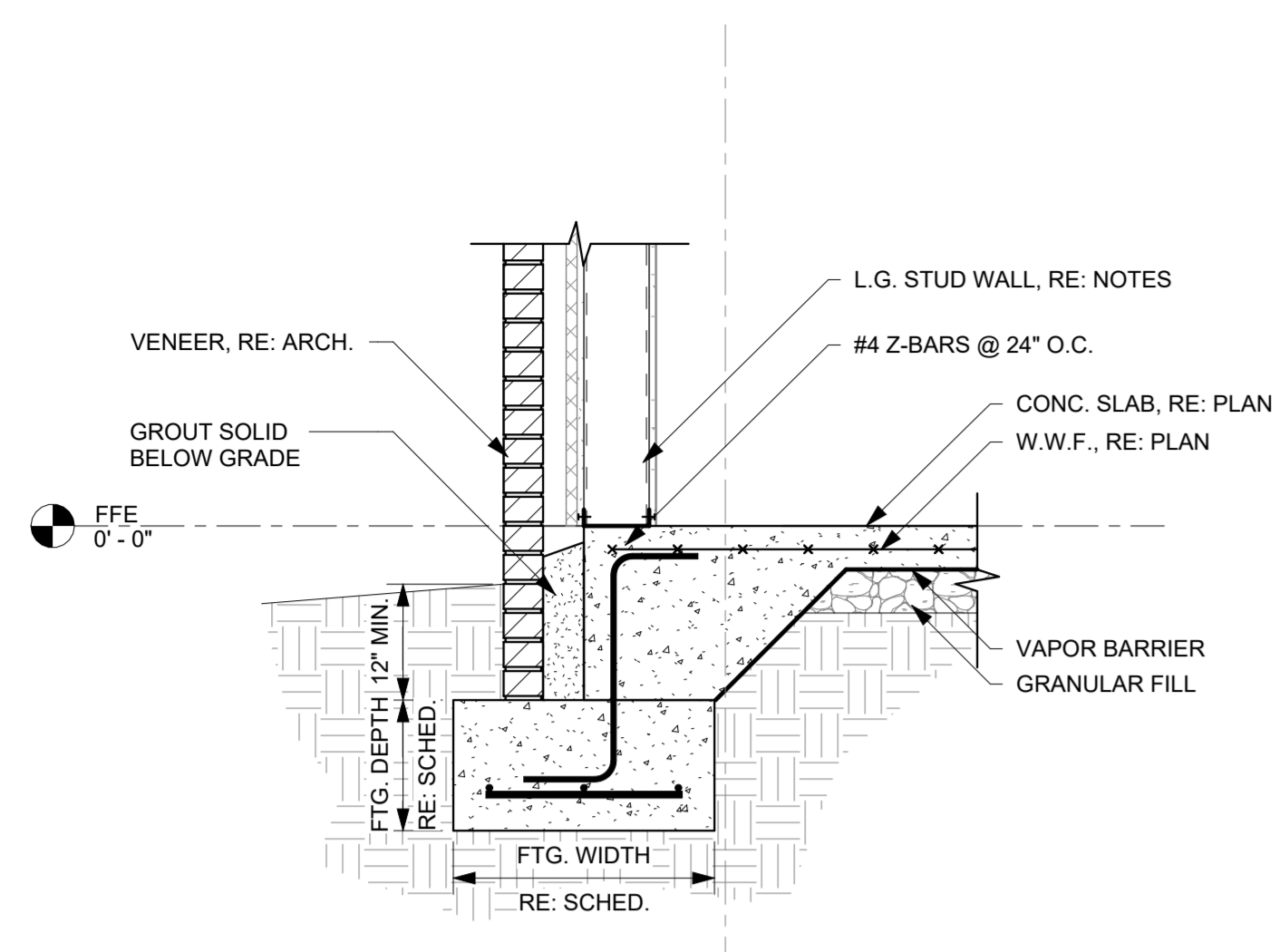
PROJECT NUMBER

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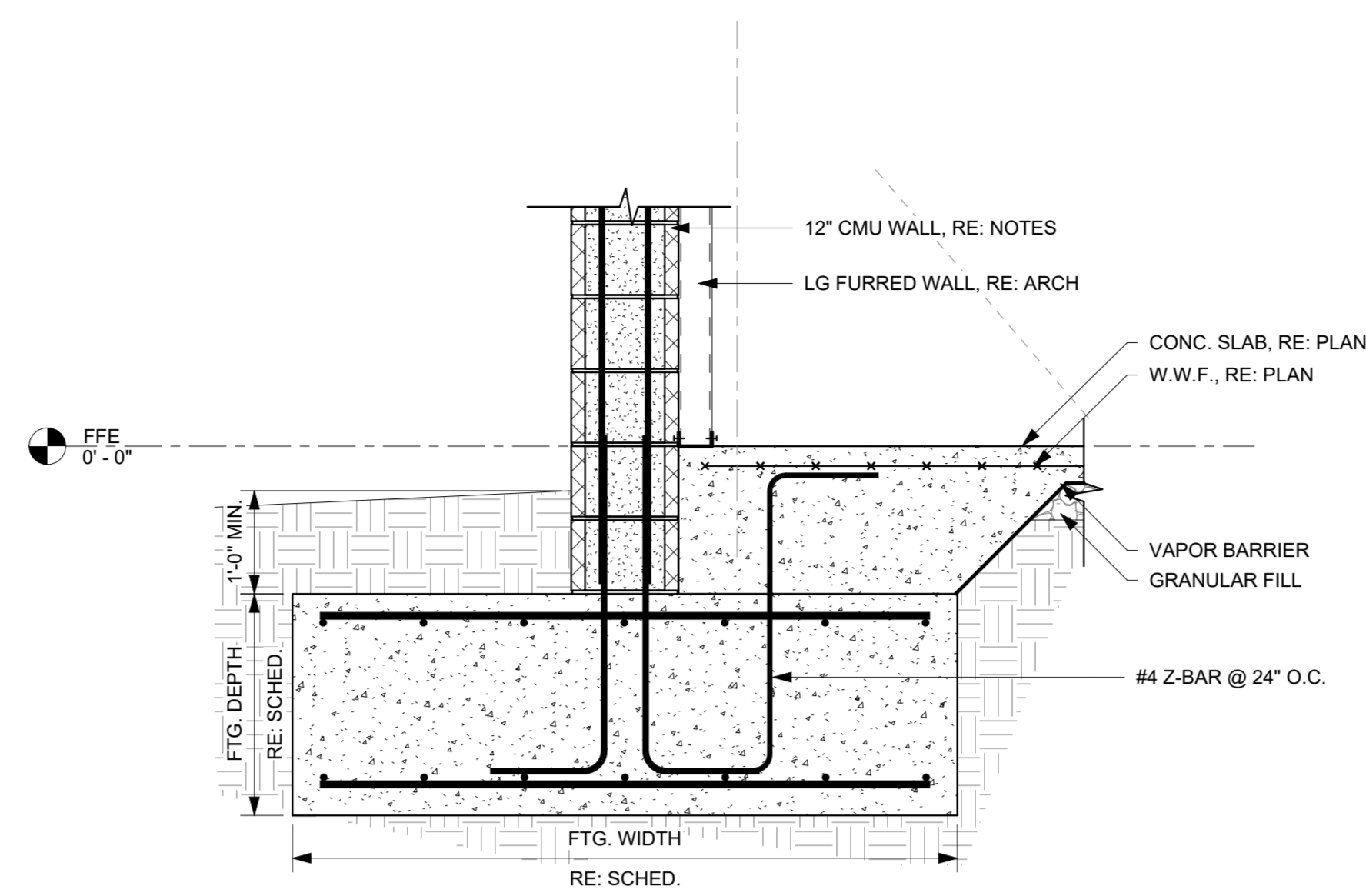
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

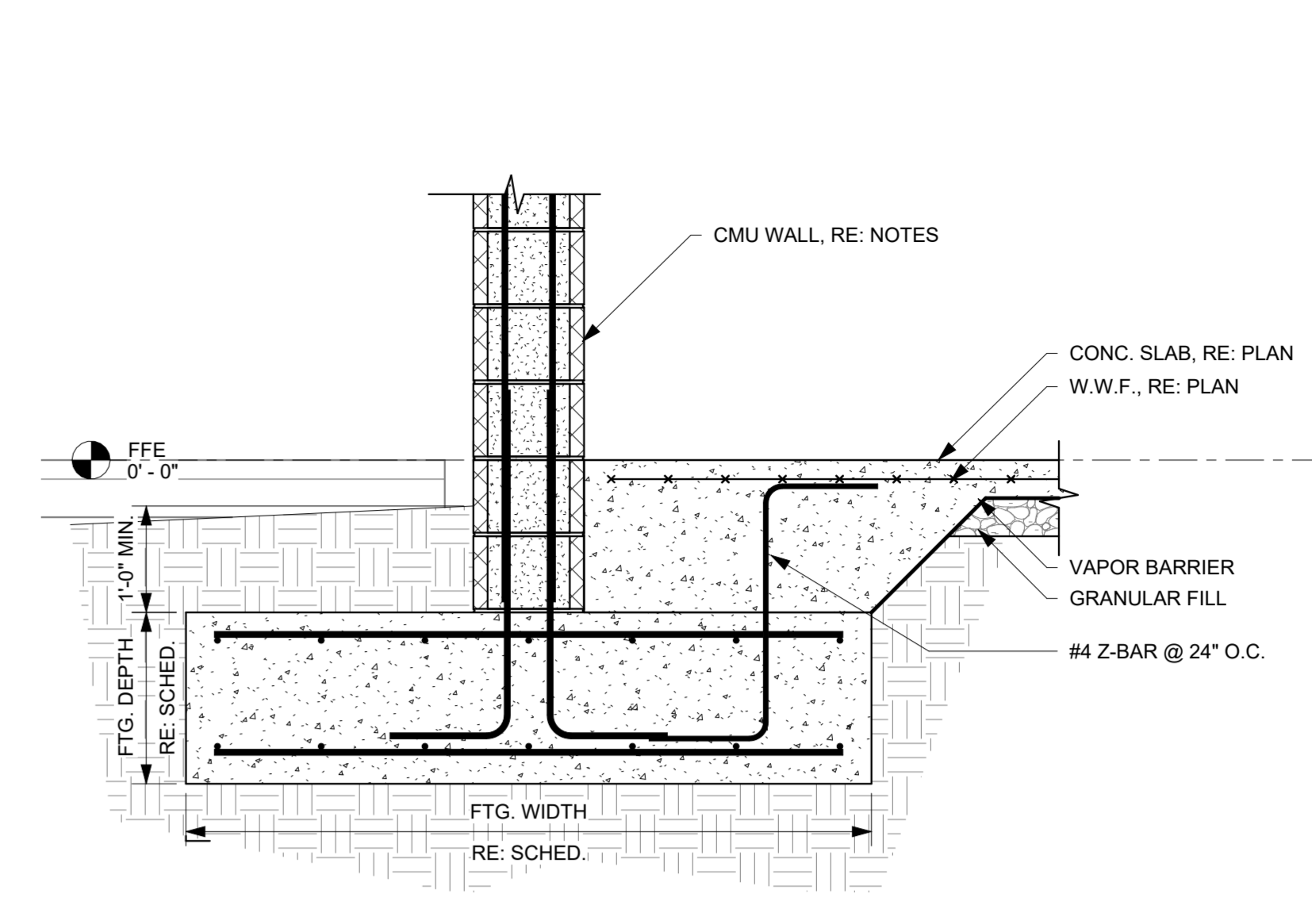
INFORMATION



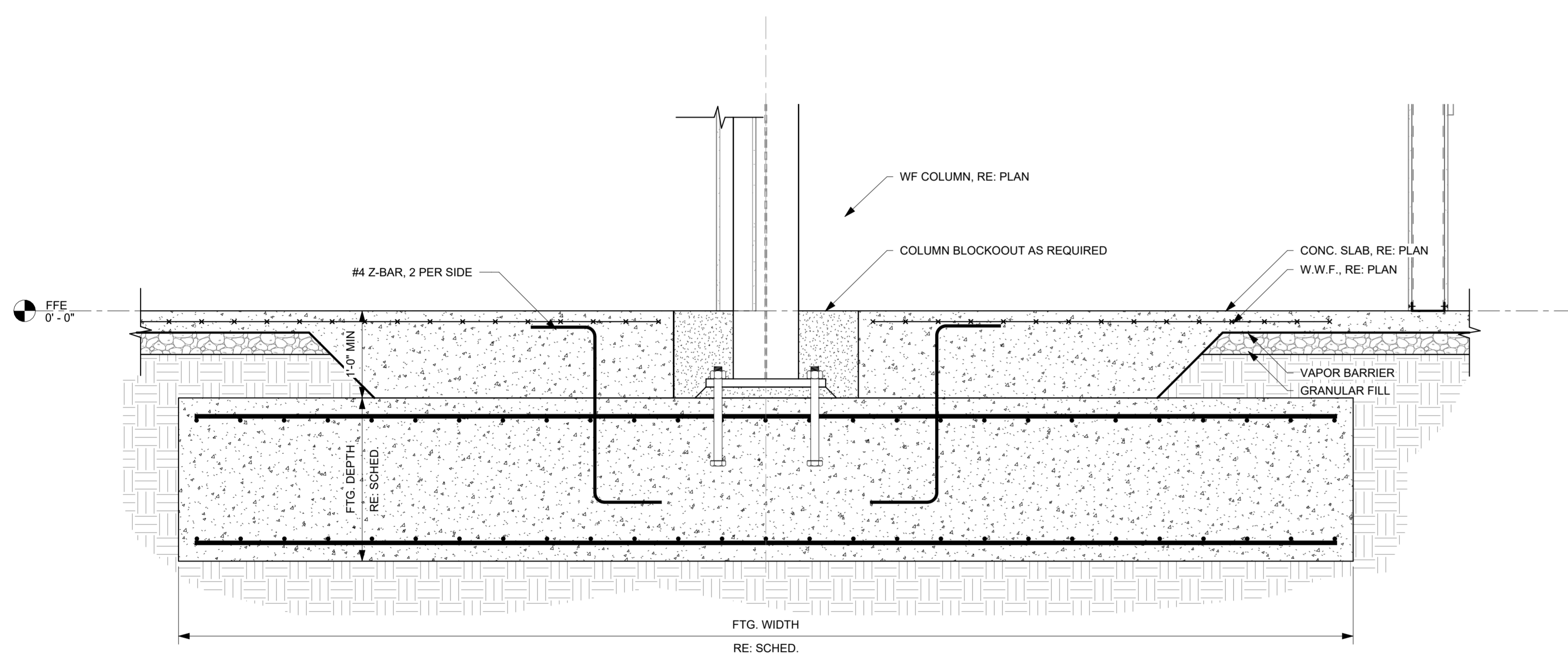
1 FDS - SECTION
3/4" = 1'-0"



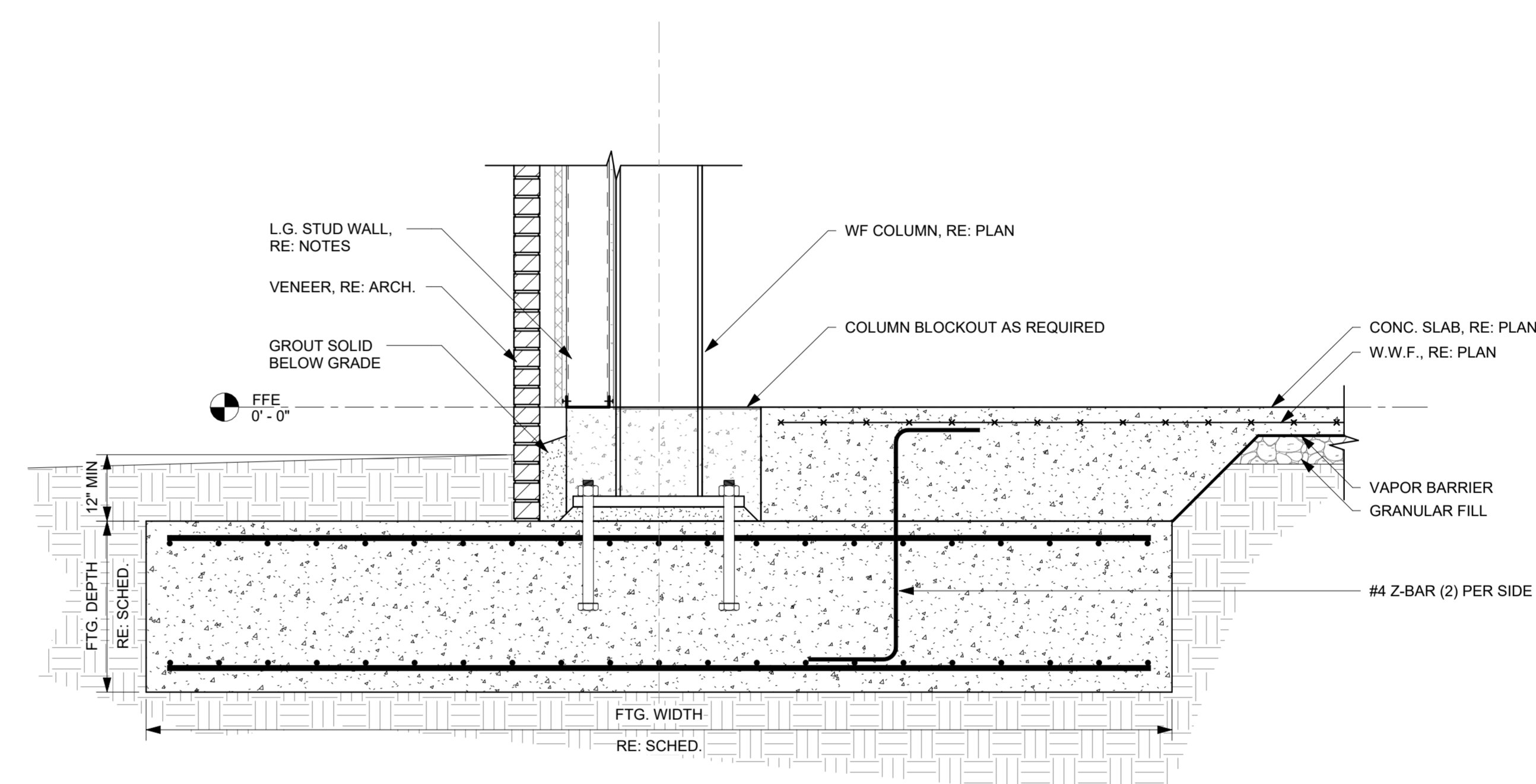
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3/4" = 1'-0"



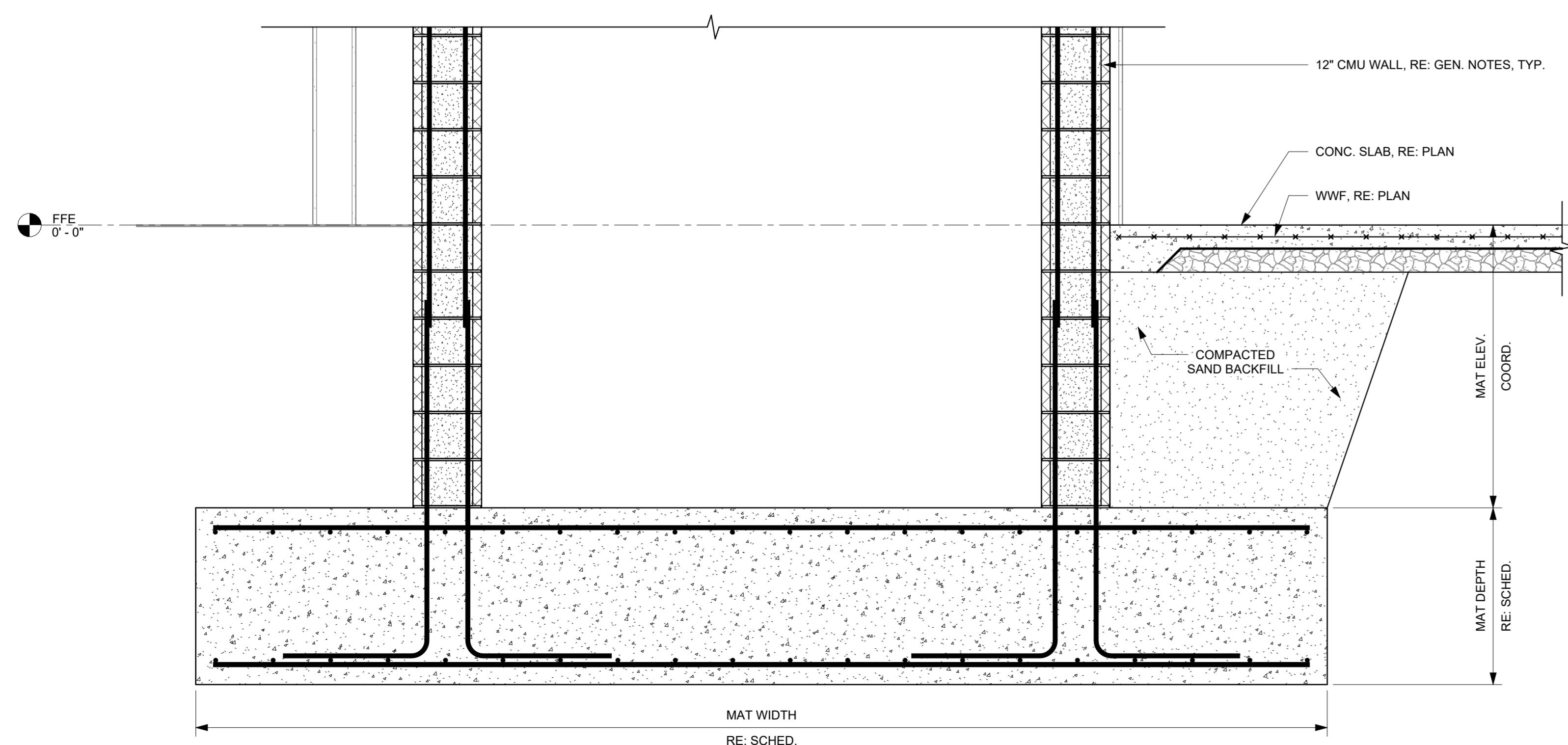
5 FDS - SECTION
3/4" = 1'-0"



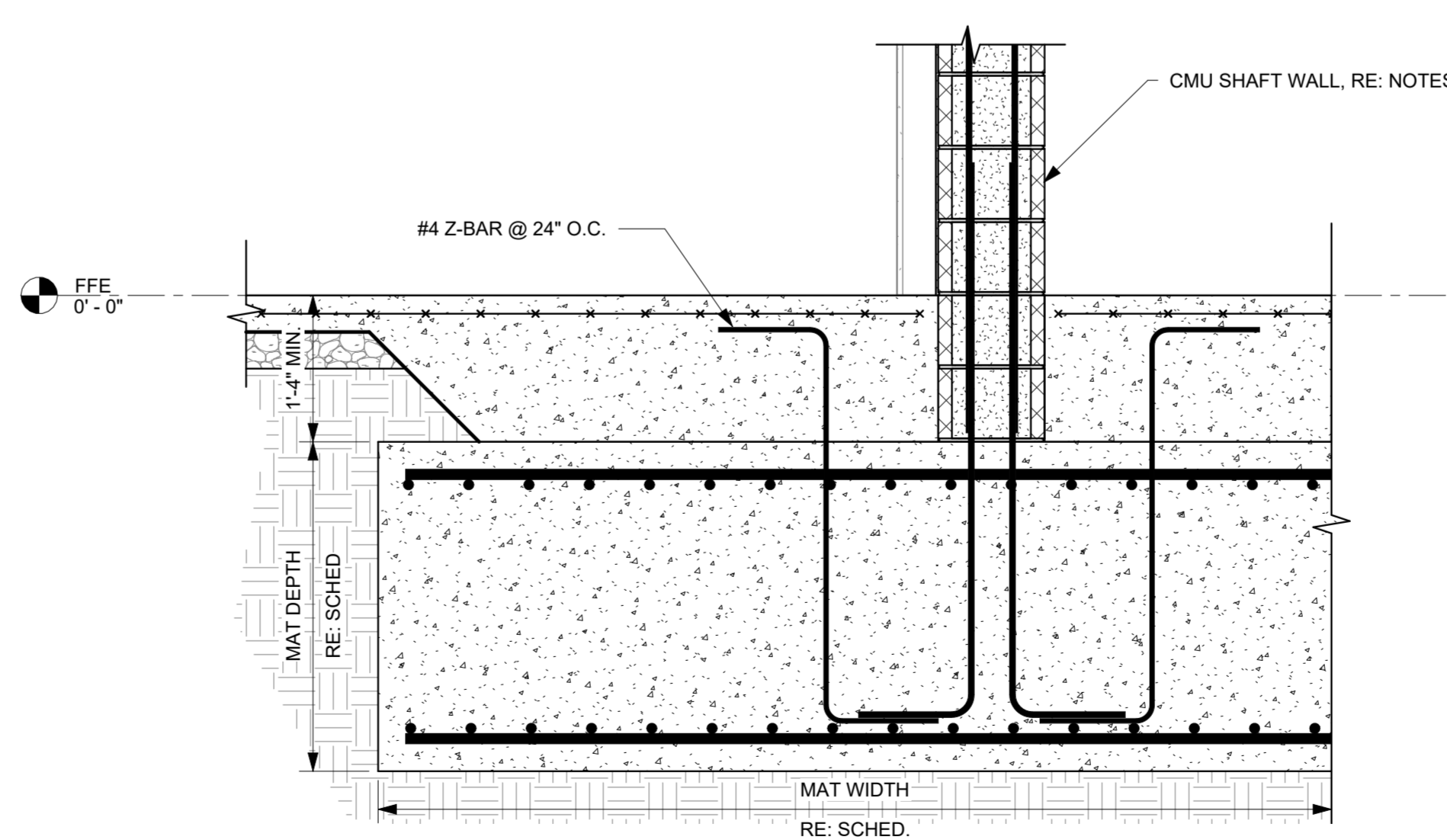
4 FDS - SECTION
3/4" = 1'-0"



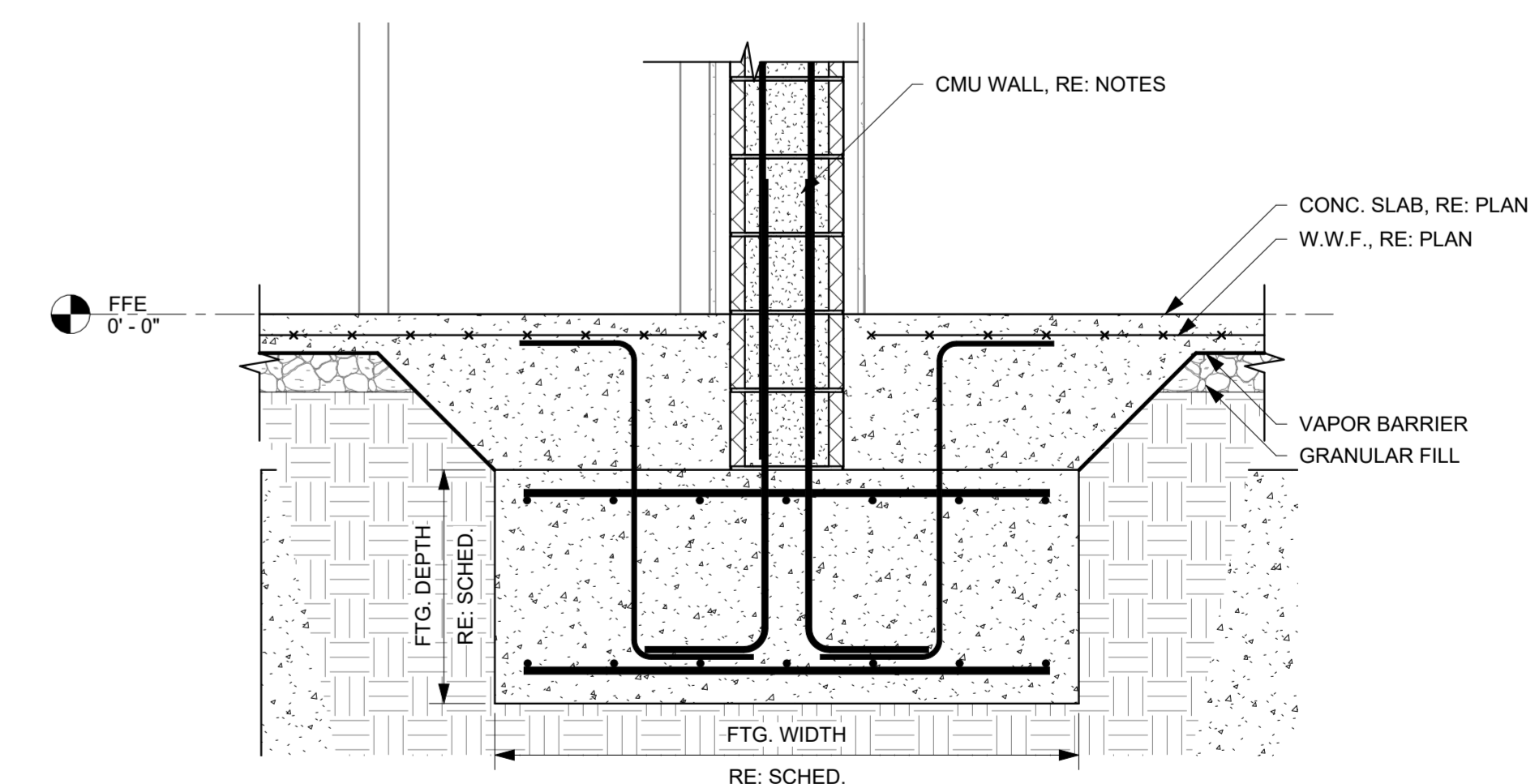
3 FDS - SECTION
3/4" = 1'-0"



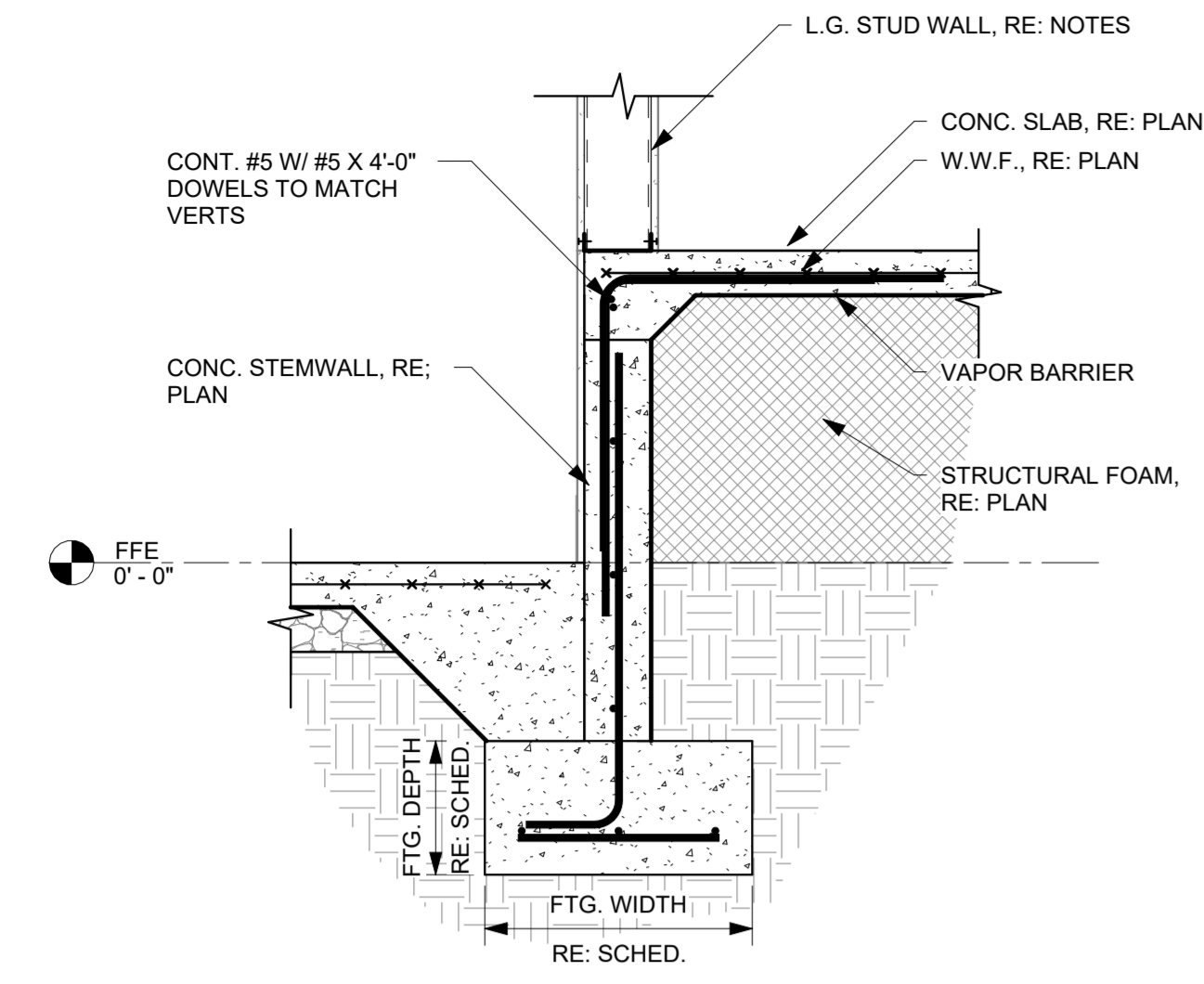
7 FDS - SECTION
3/4" = 1'-0"



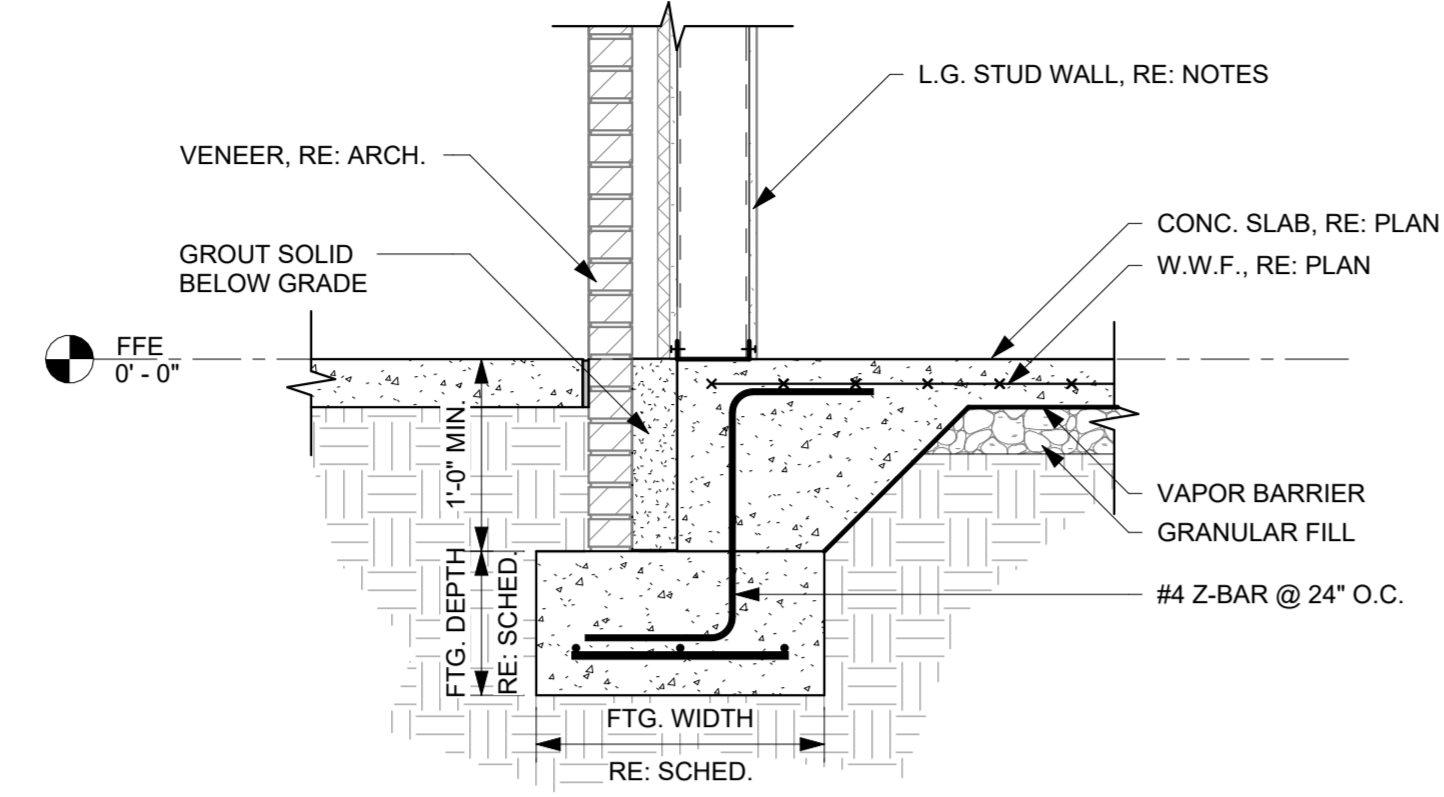
6 FDS - SECTION
3/4" = 1'-0"



8 FDS - SECTION
3/4" = 1'-0"



1 FDS - SECTION
3/4" = 1'-0"



2 FDS - SECTION
3/4" = 1'-0"



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

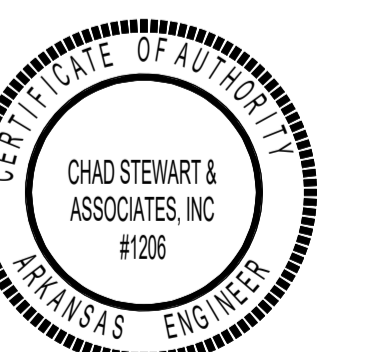
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION

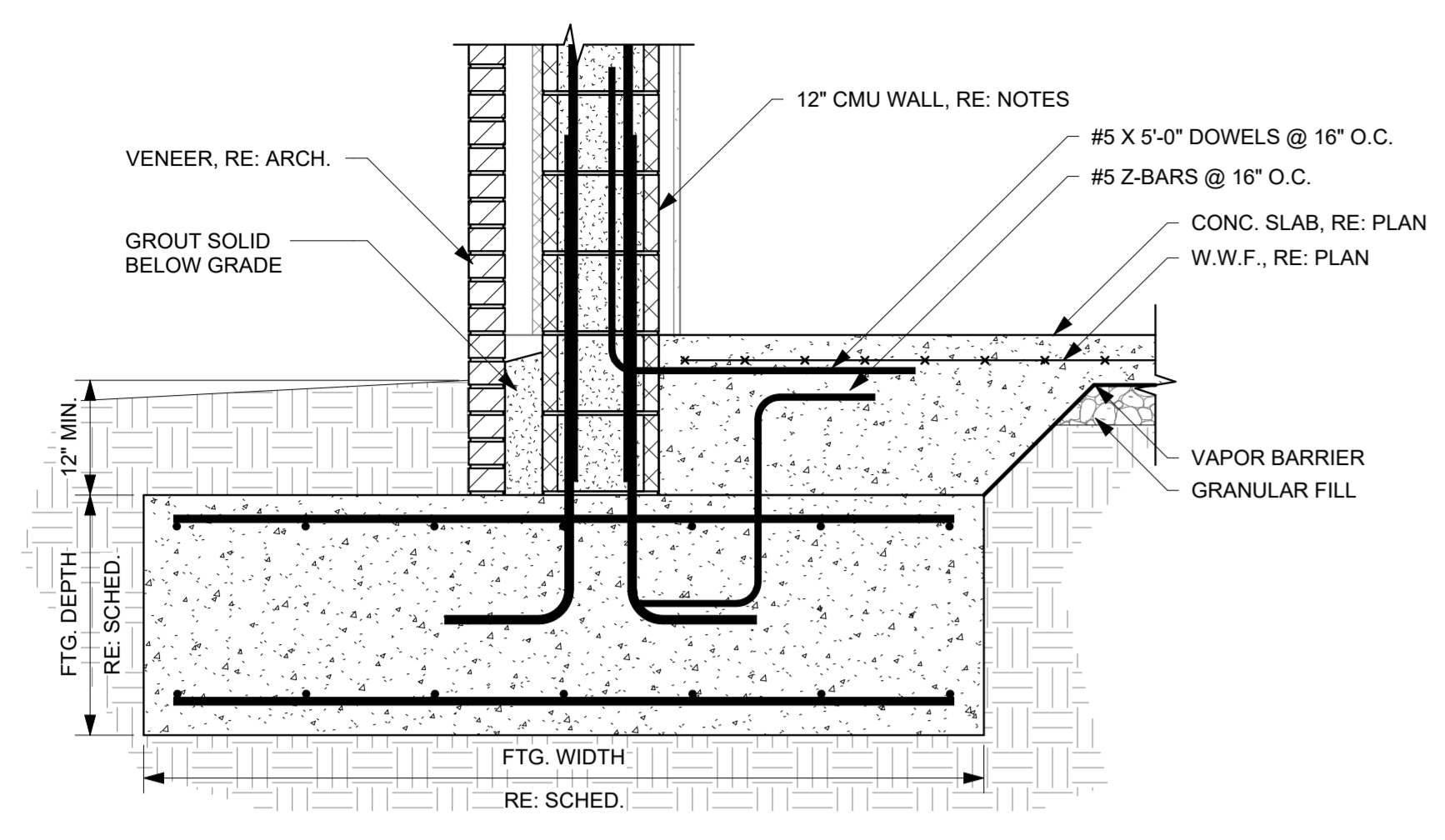


SHEET TITLE
FOUNDATION SECTIONS - AREA A SOUTH

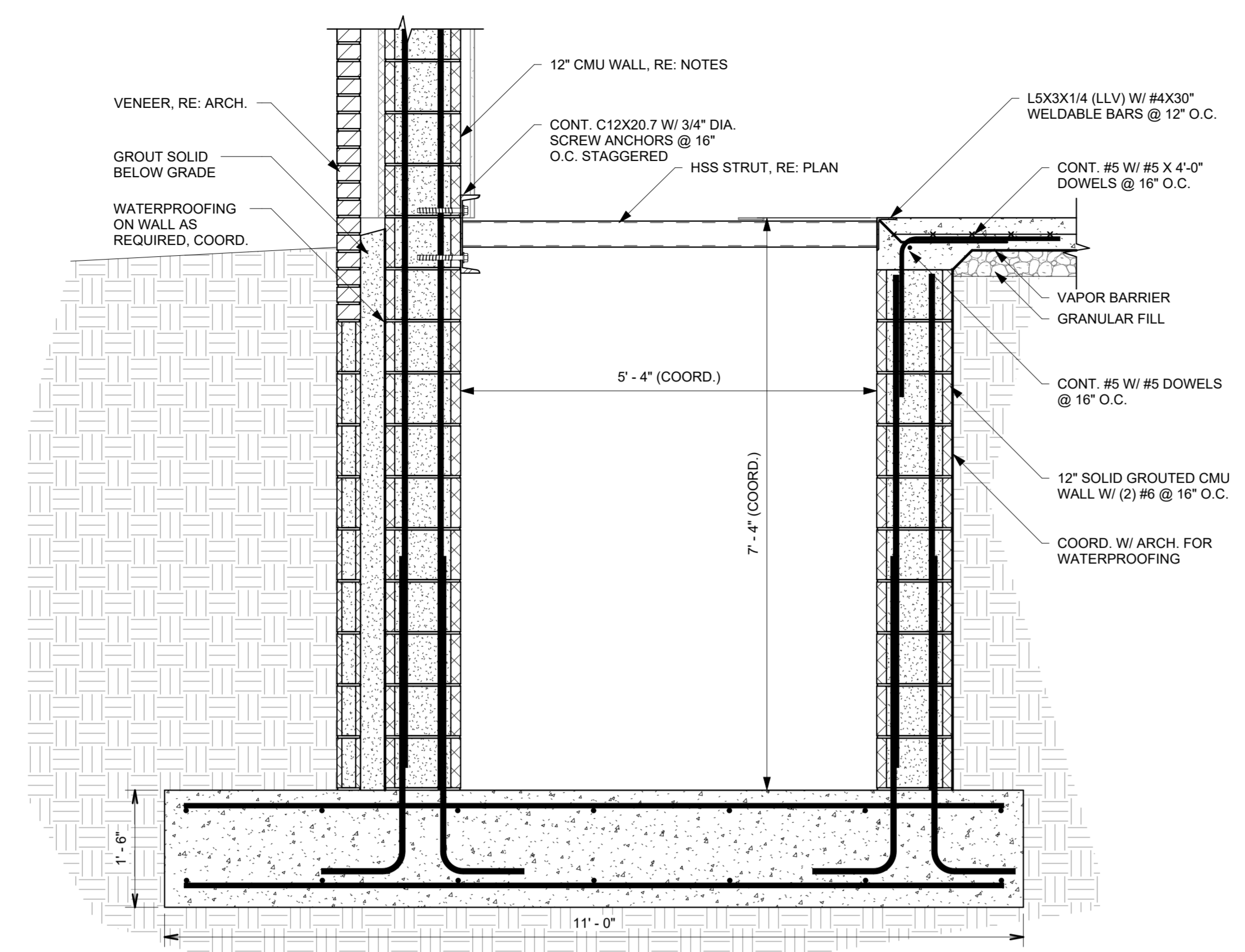
DATE
17.10.24

SHEET NUMBER

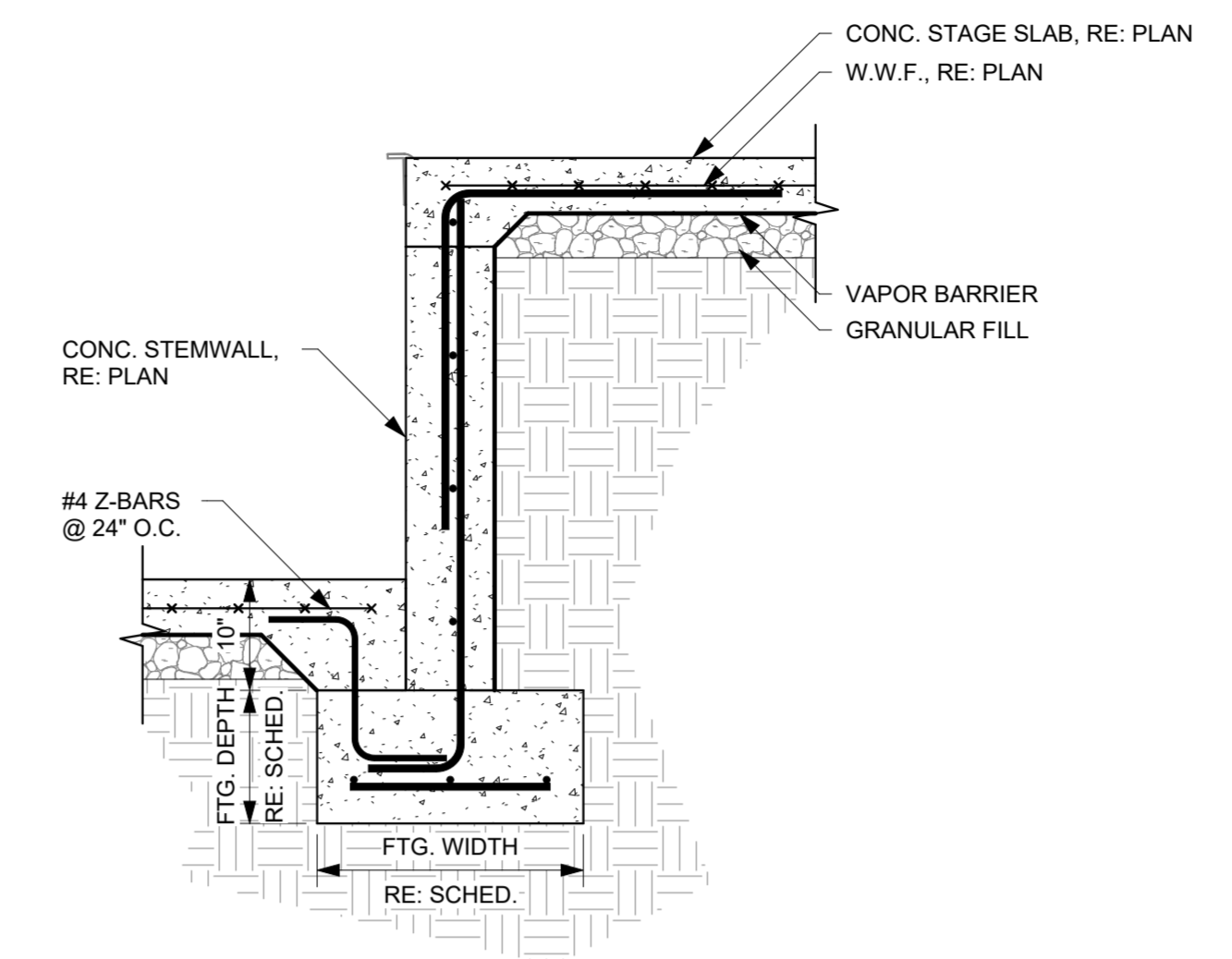
S202.2



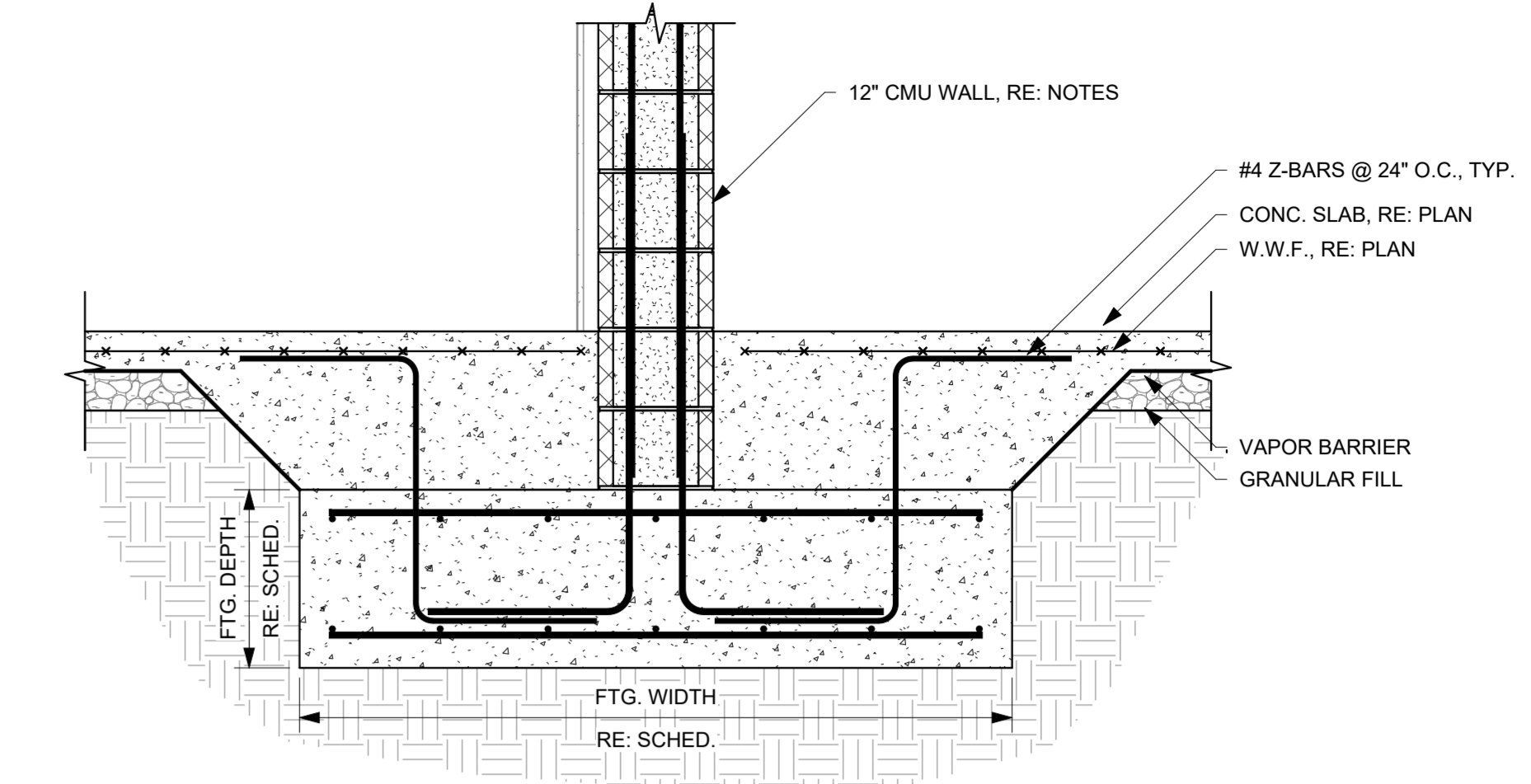
1 FDS - SECTION
3/4" = 1'-0"



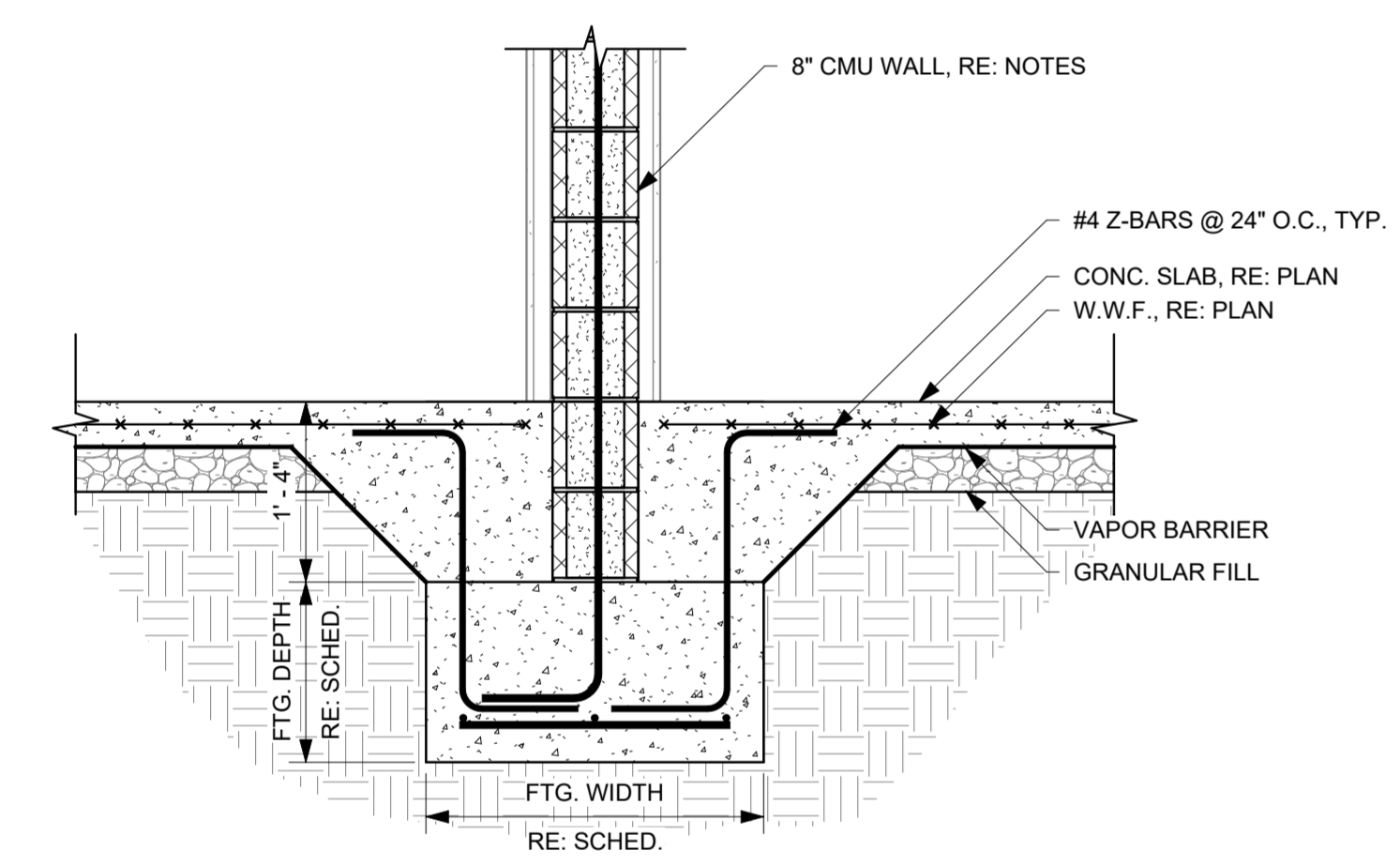
2 FDS - SECTION
3/4" = 1'-0"



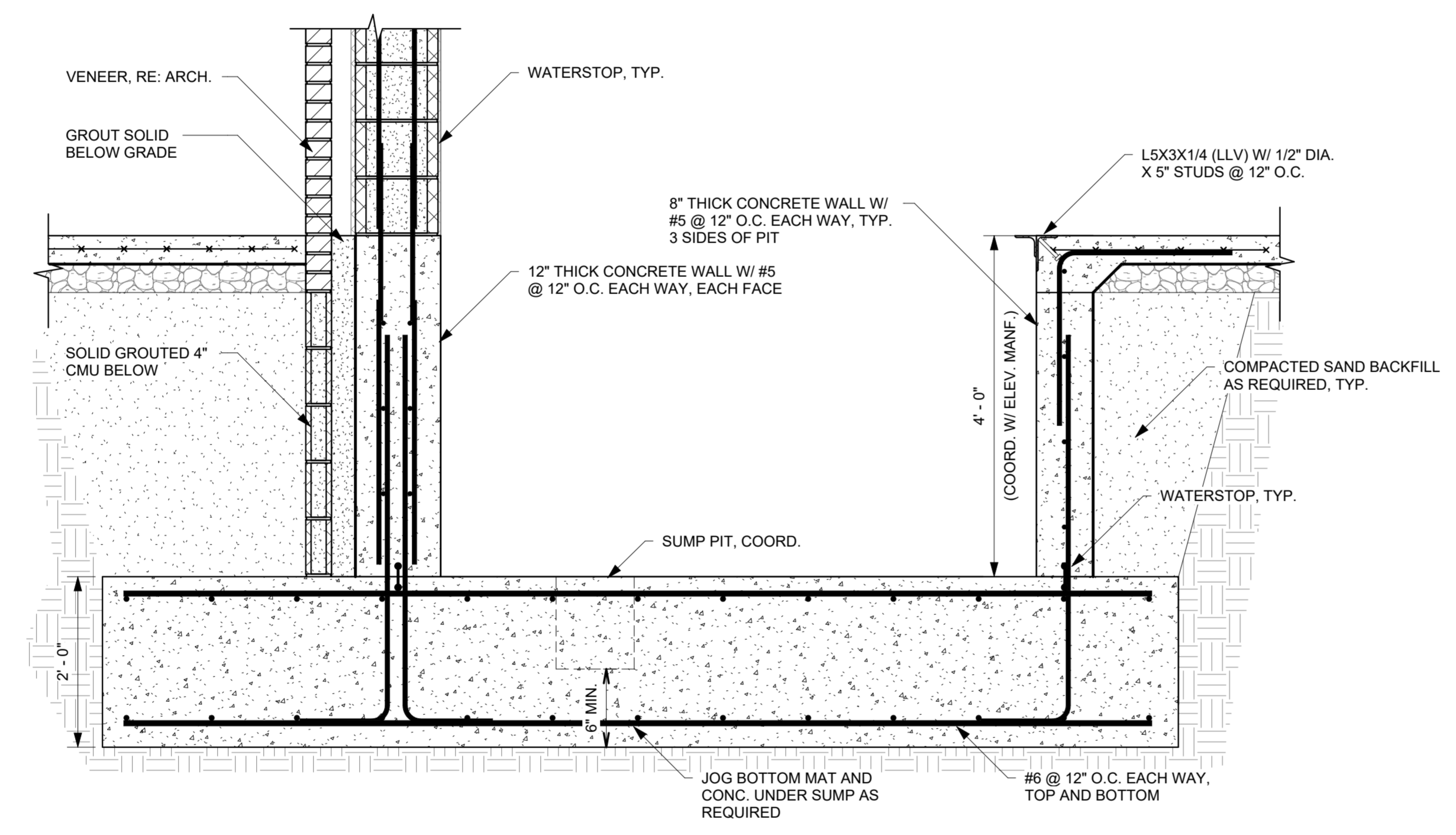
3 FDS - SECTION
3/4" = 1'-0"



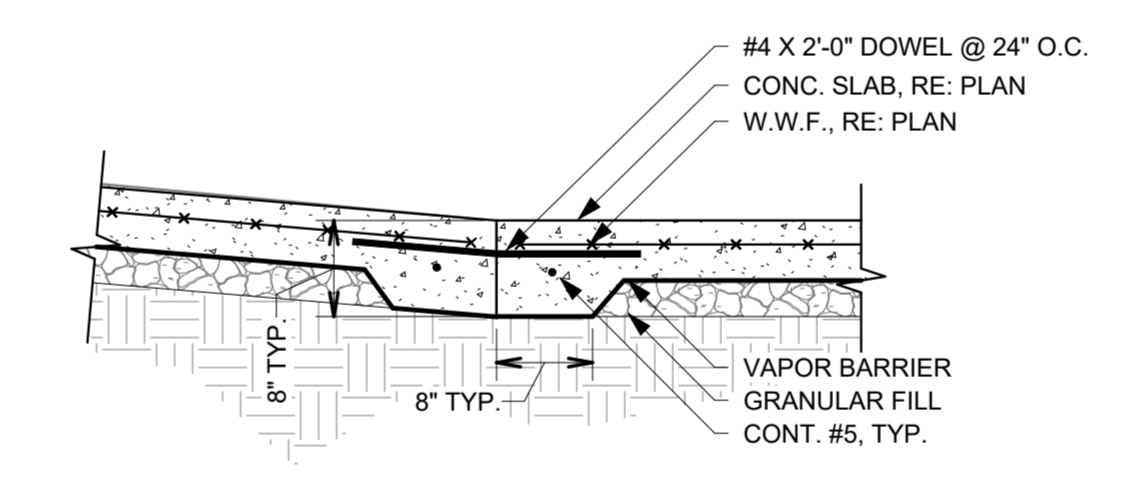
4 FDS - SECTION
3/4" = 1'-0"



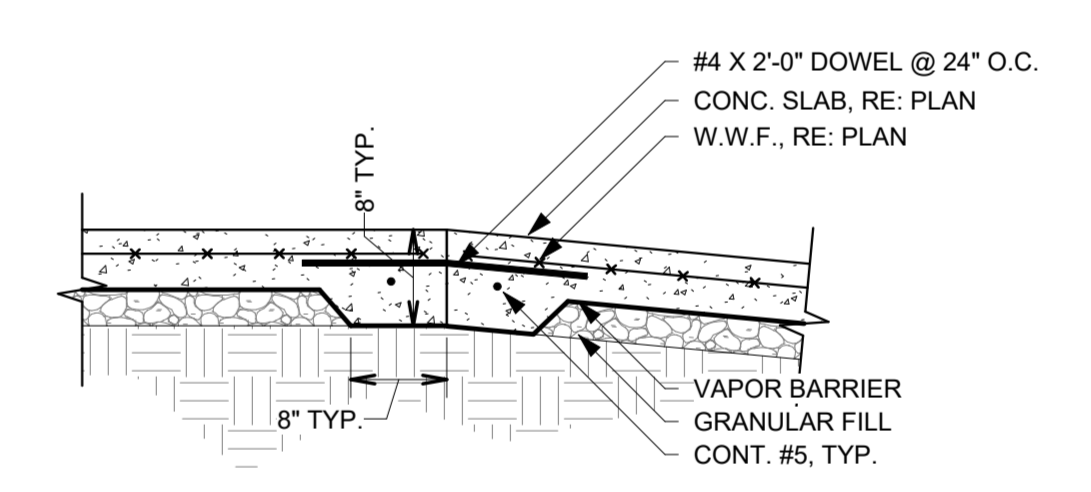
5 FDS - SECTION
3/4" = 1'-0"



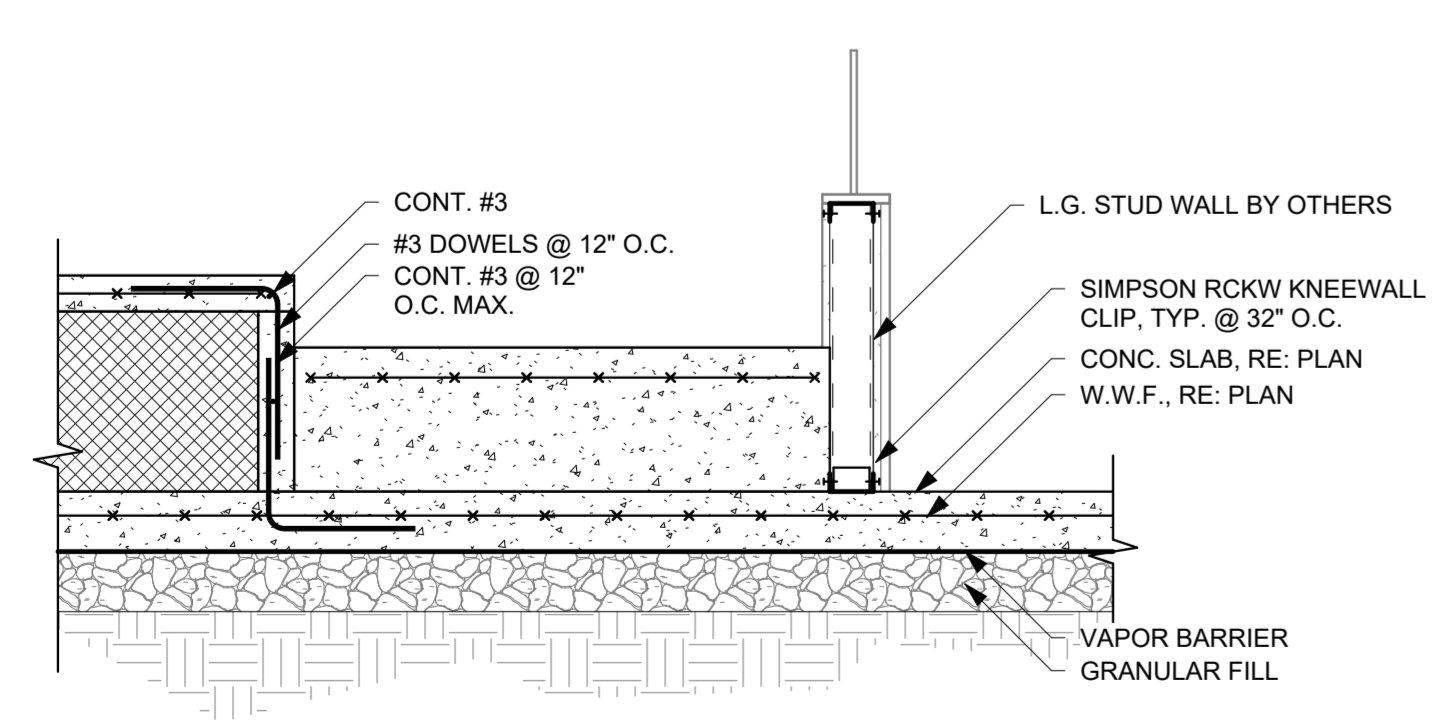
6 FDS - SECTION
3/4" = 1'-0"



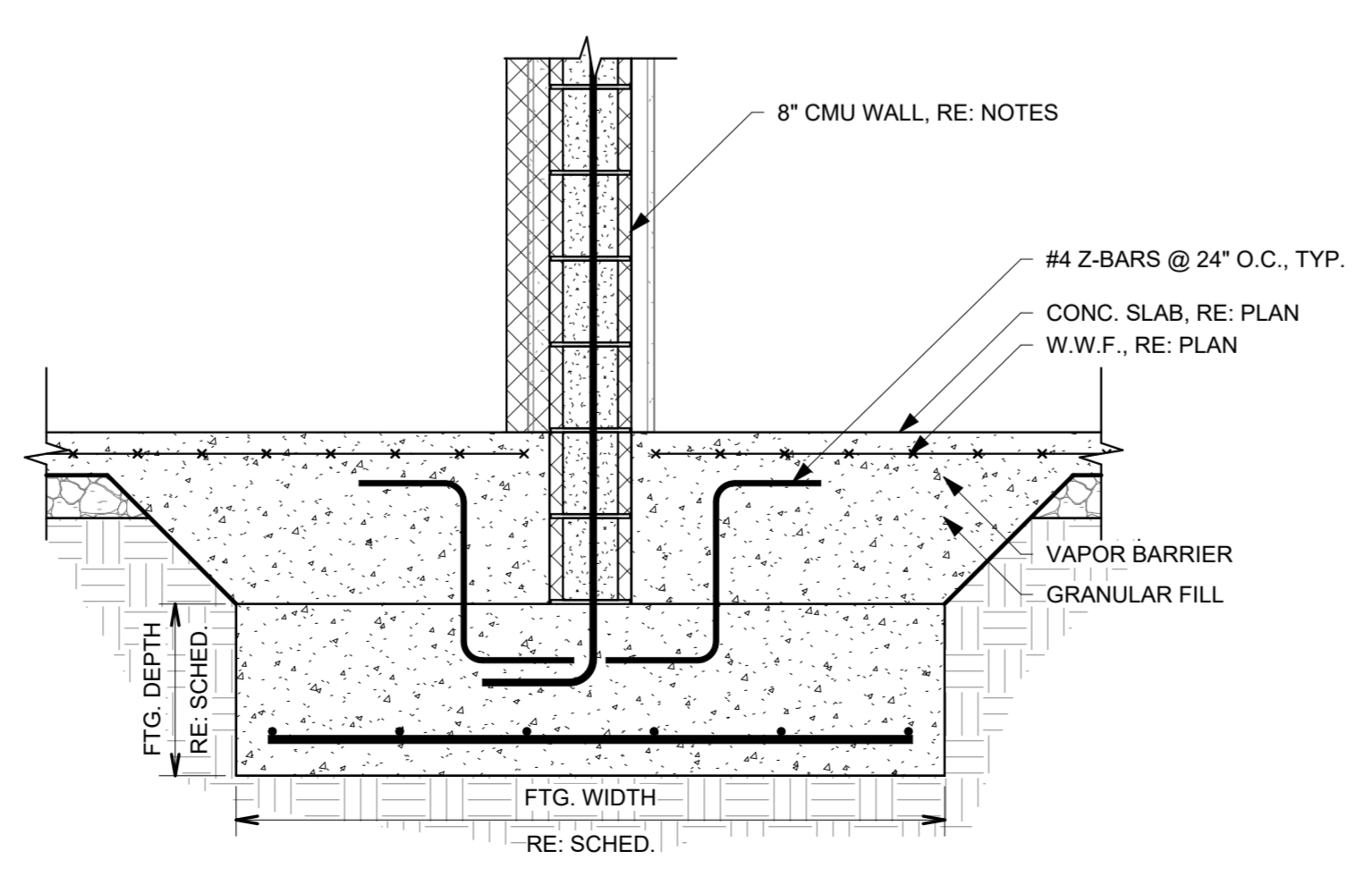
7 FDS - SECTION
3/4" = 1'-0"



8 FDS - SECTION
3/4" = 1'-0"



9 FDS - SECTION
3/4" = 1'-0"



10 FDS - SECTION
3/4" = 1'-0"



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

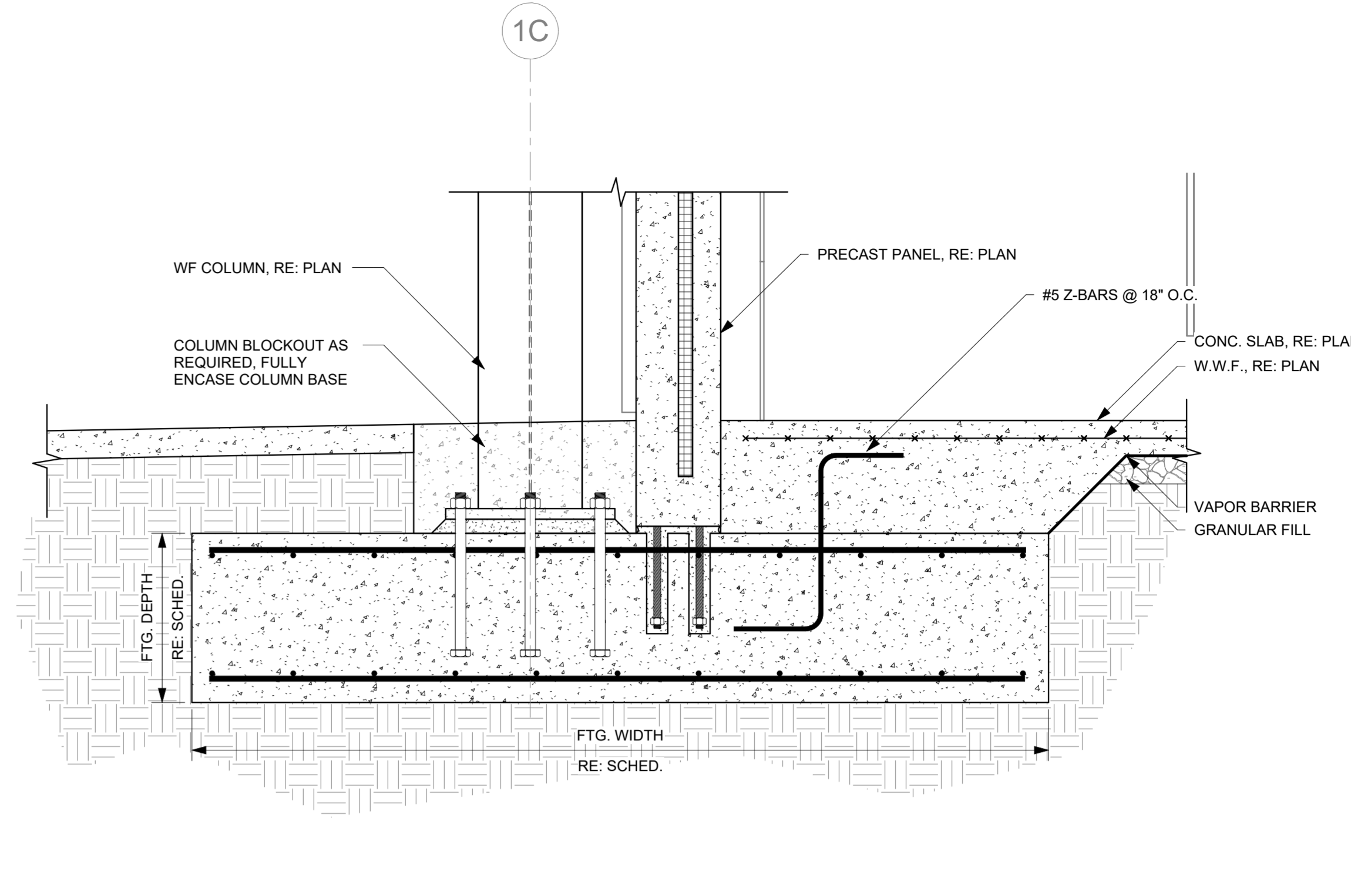
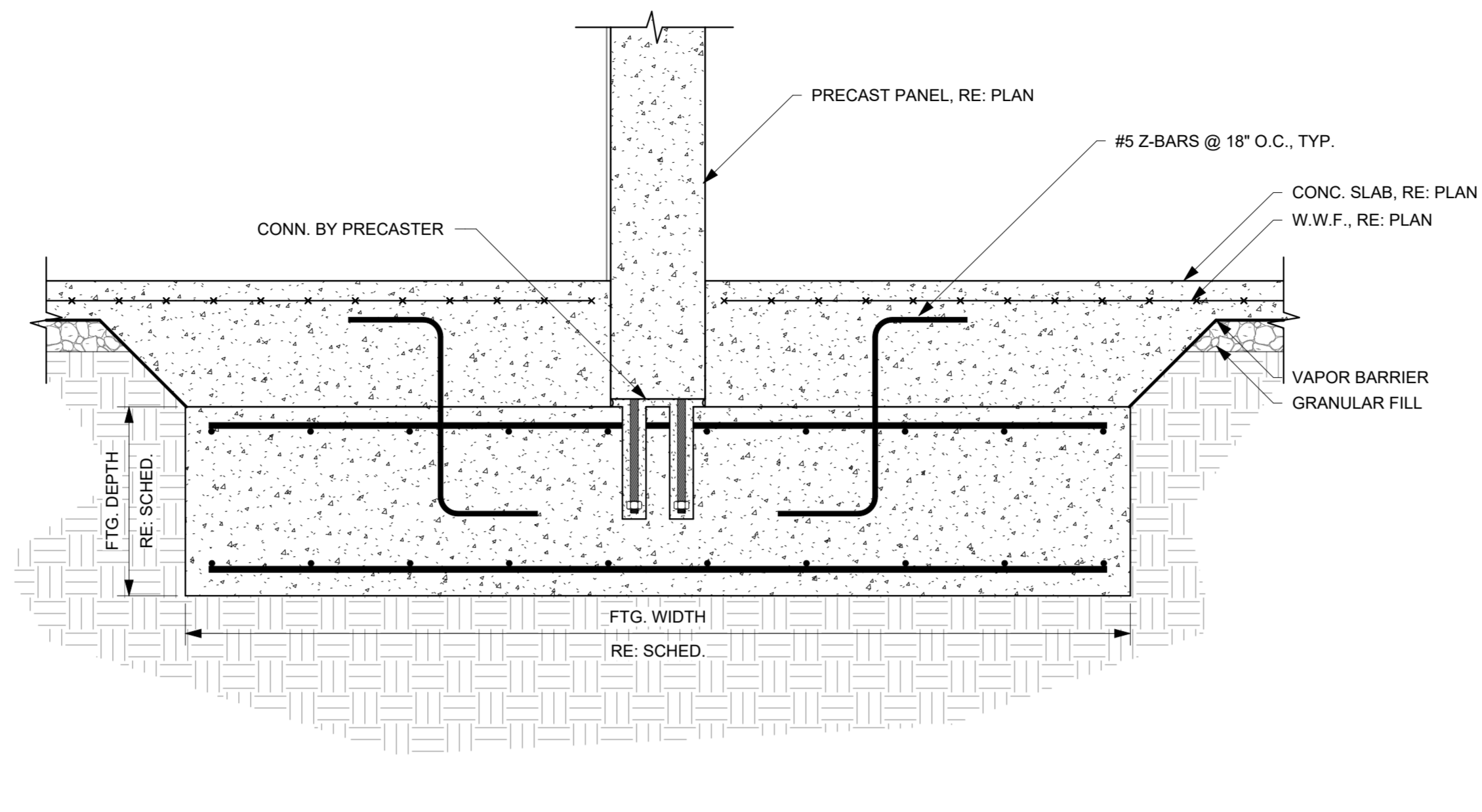
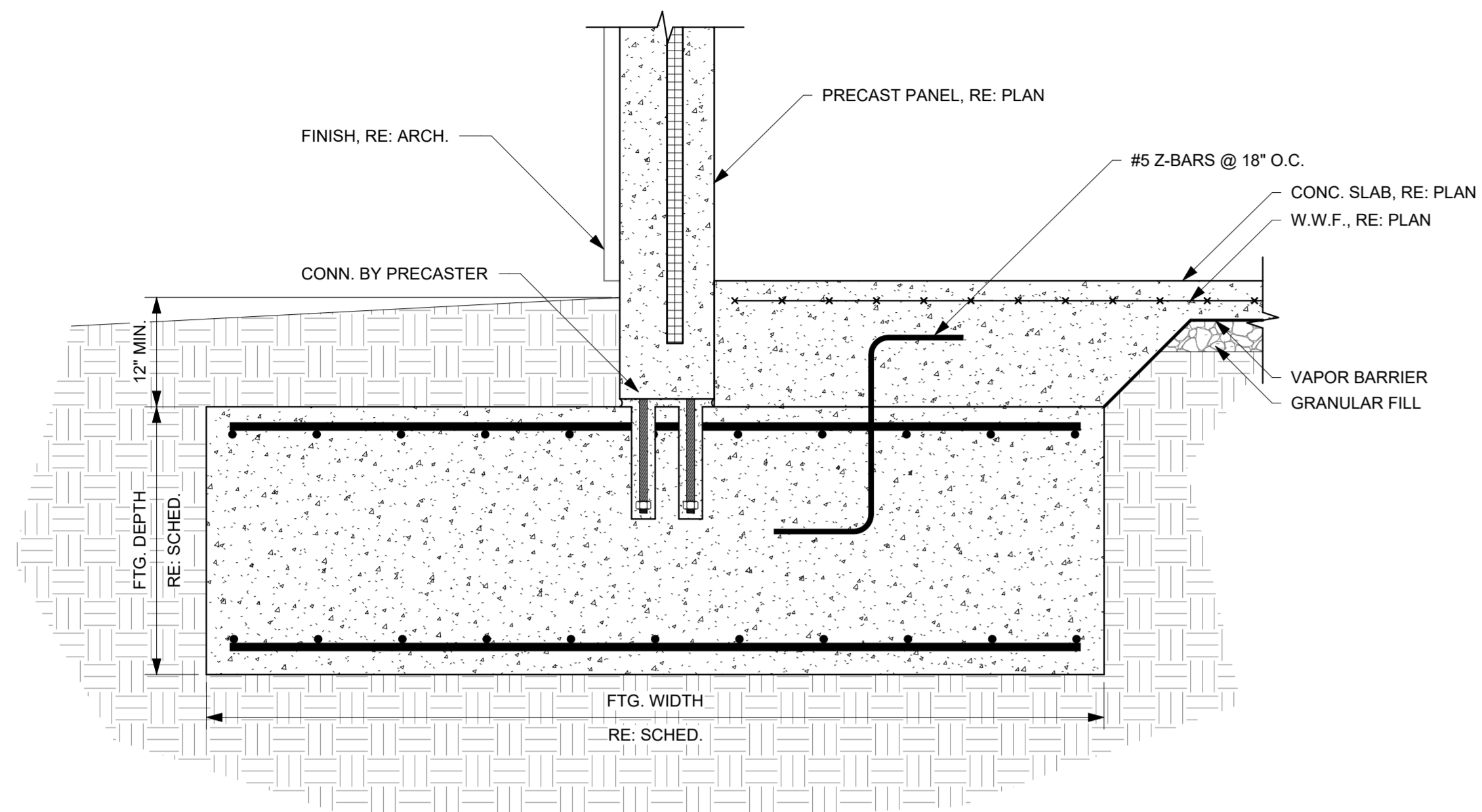
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

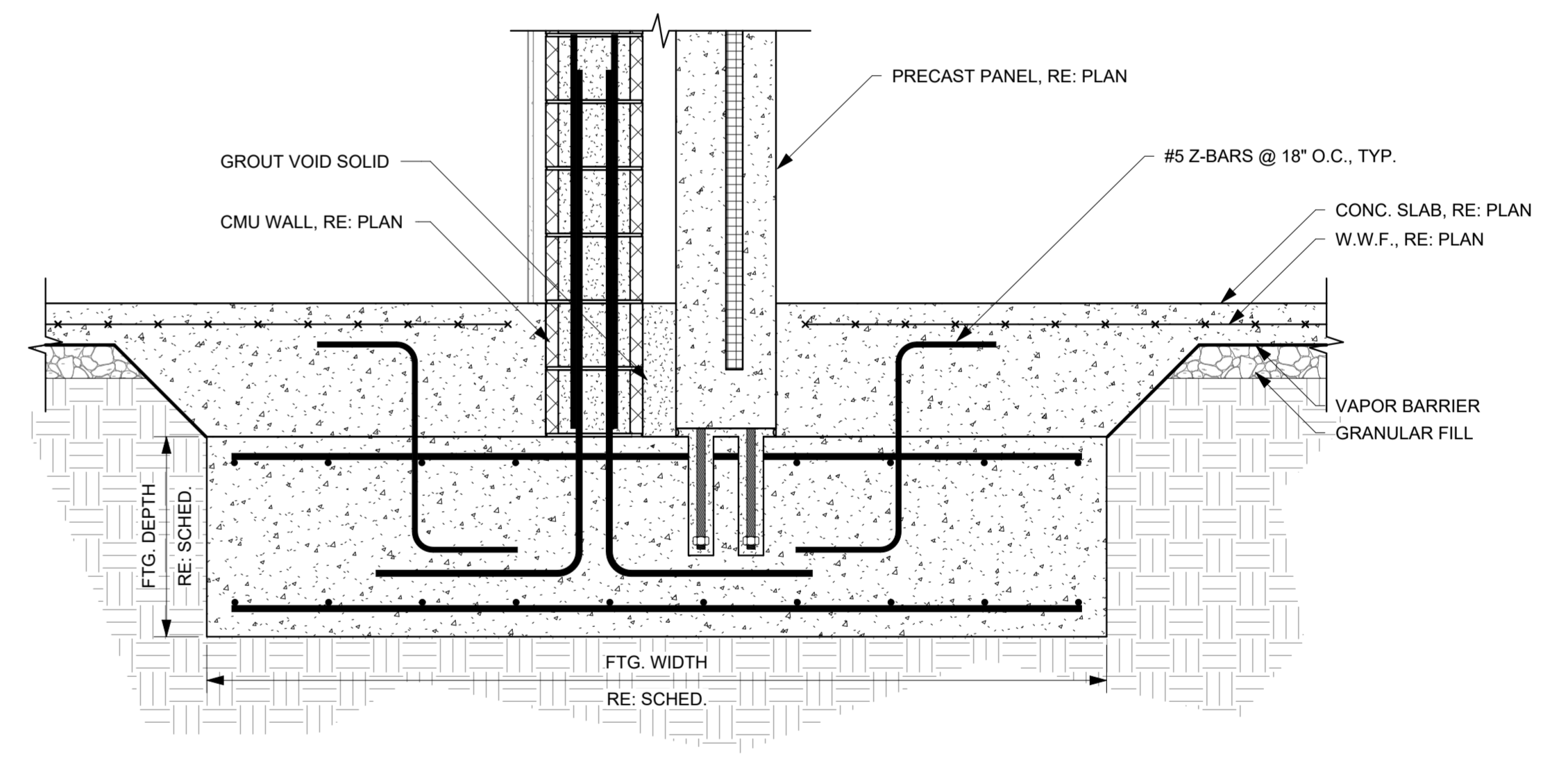
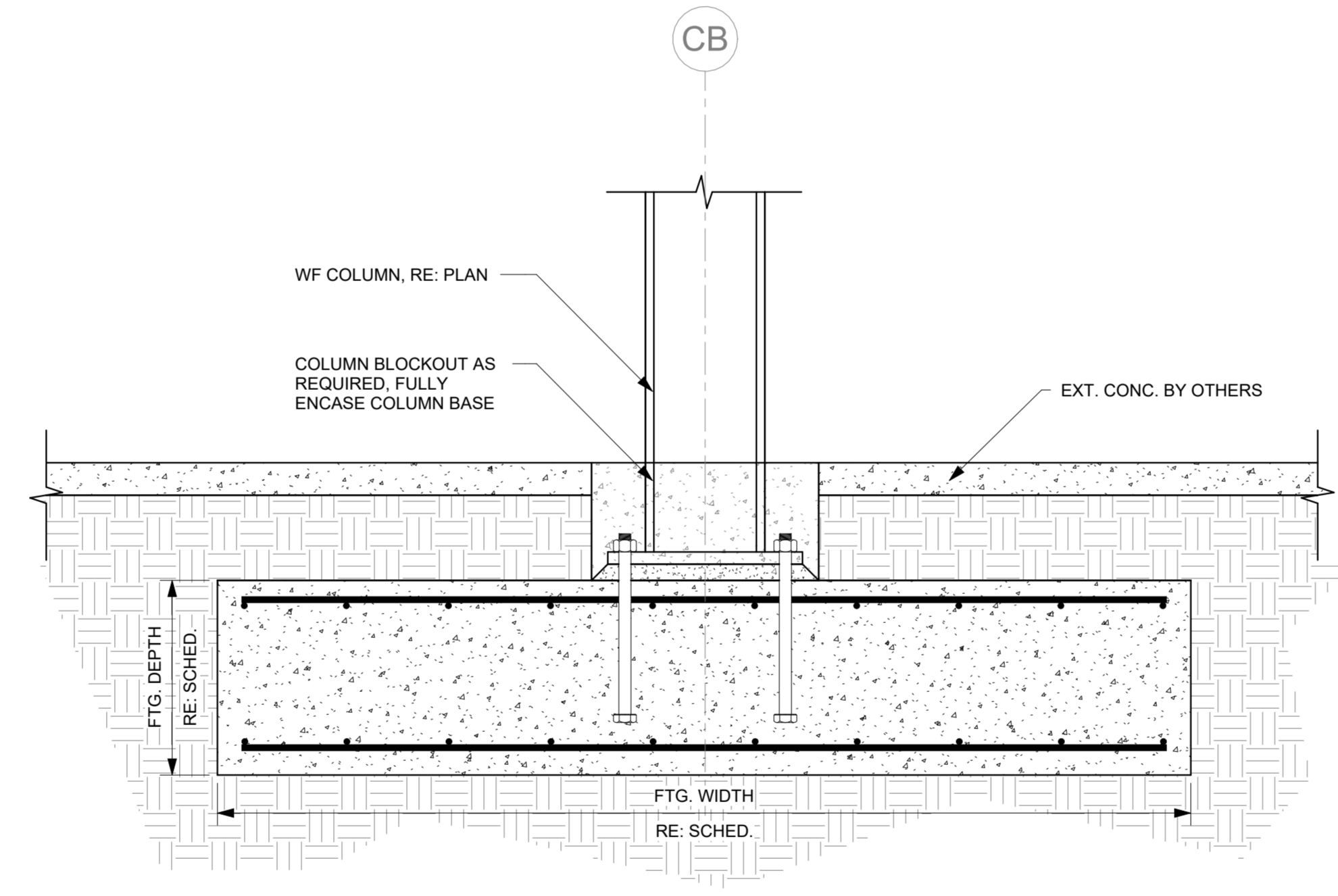
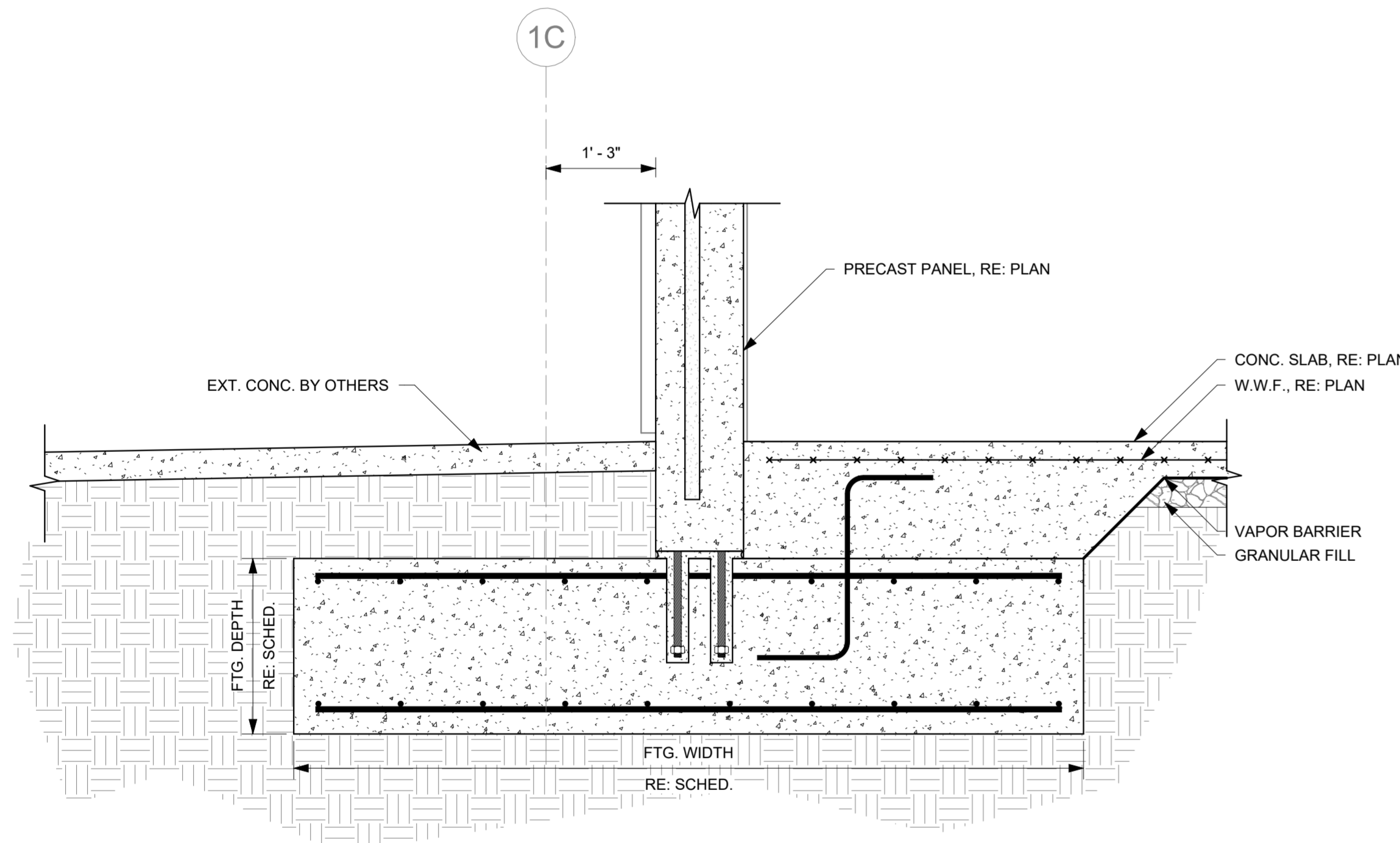
INFORMATION



1 FDS - SECTION
3/4" = 1'-0"

2 FDS - SECTION
3/4" = 1'-0"

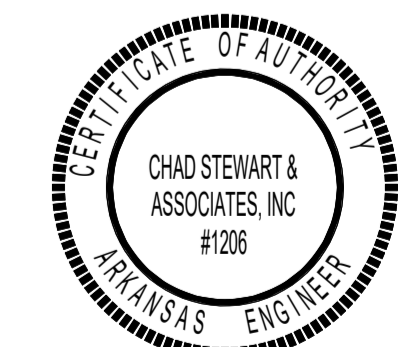
3 FDS - SECTION
3/4" = 1'-0"



4 FDS - SECTION
3/4" = 1'-0"

5 FDS - SECTION
3/4" = 1'-0"

6 FDS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

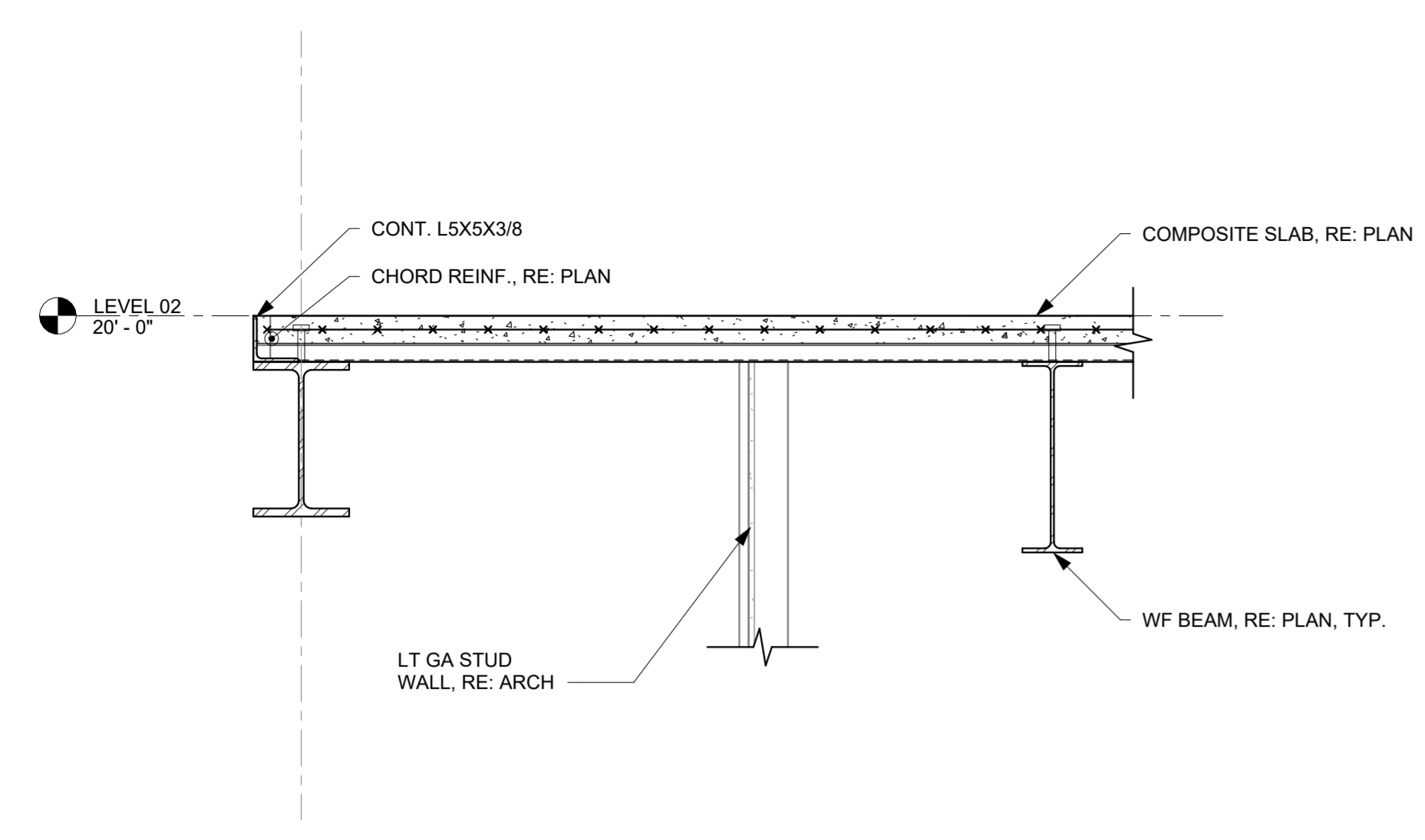
PROJECT NUMBER

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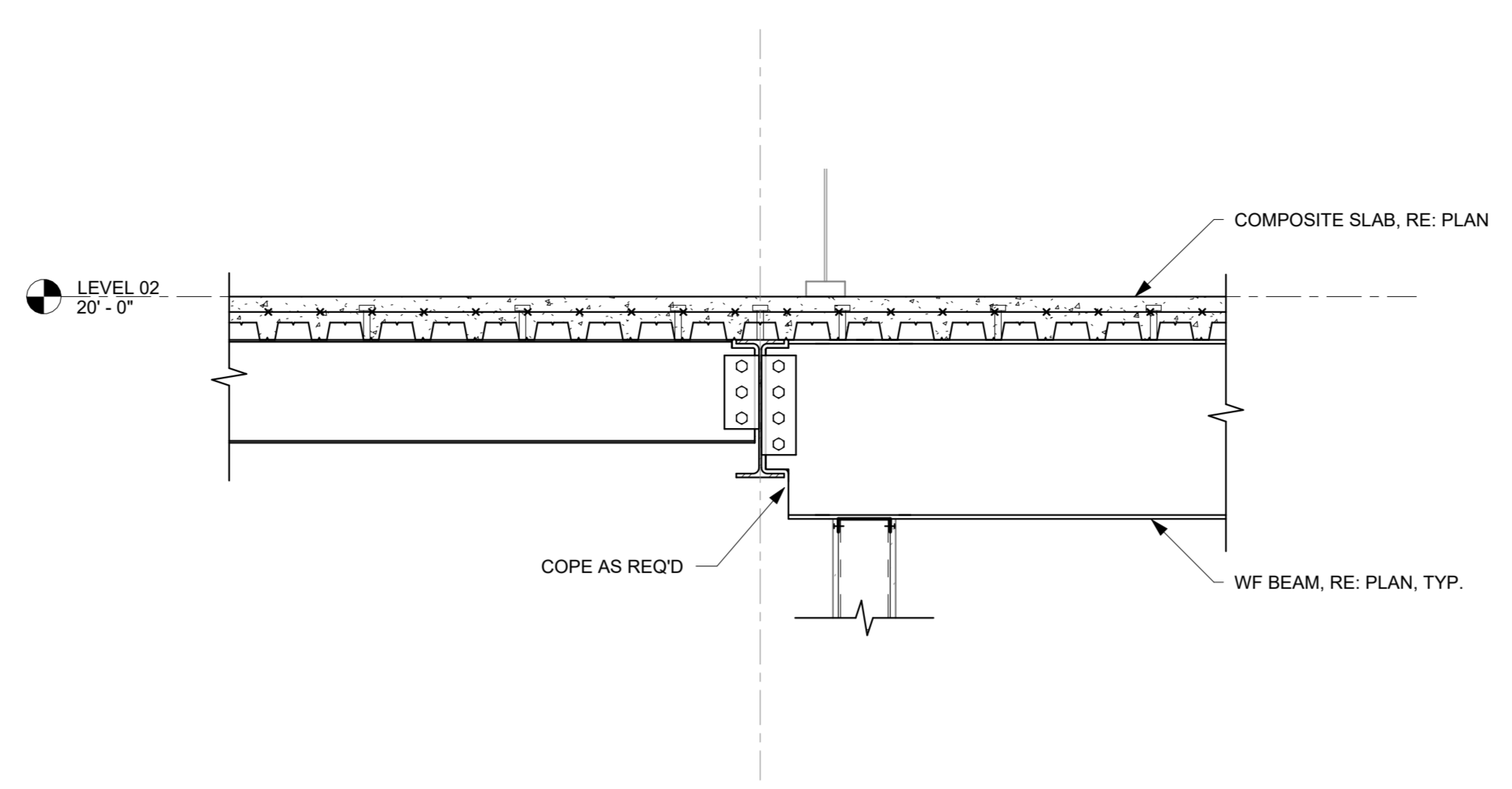
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

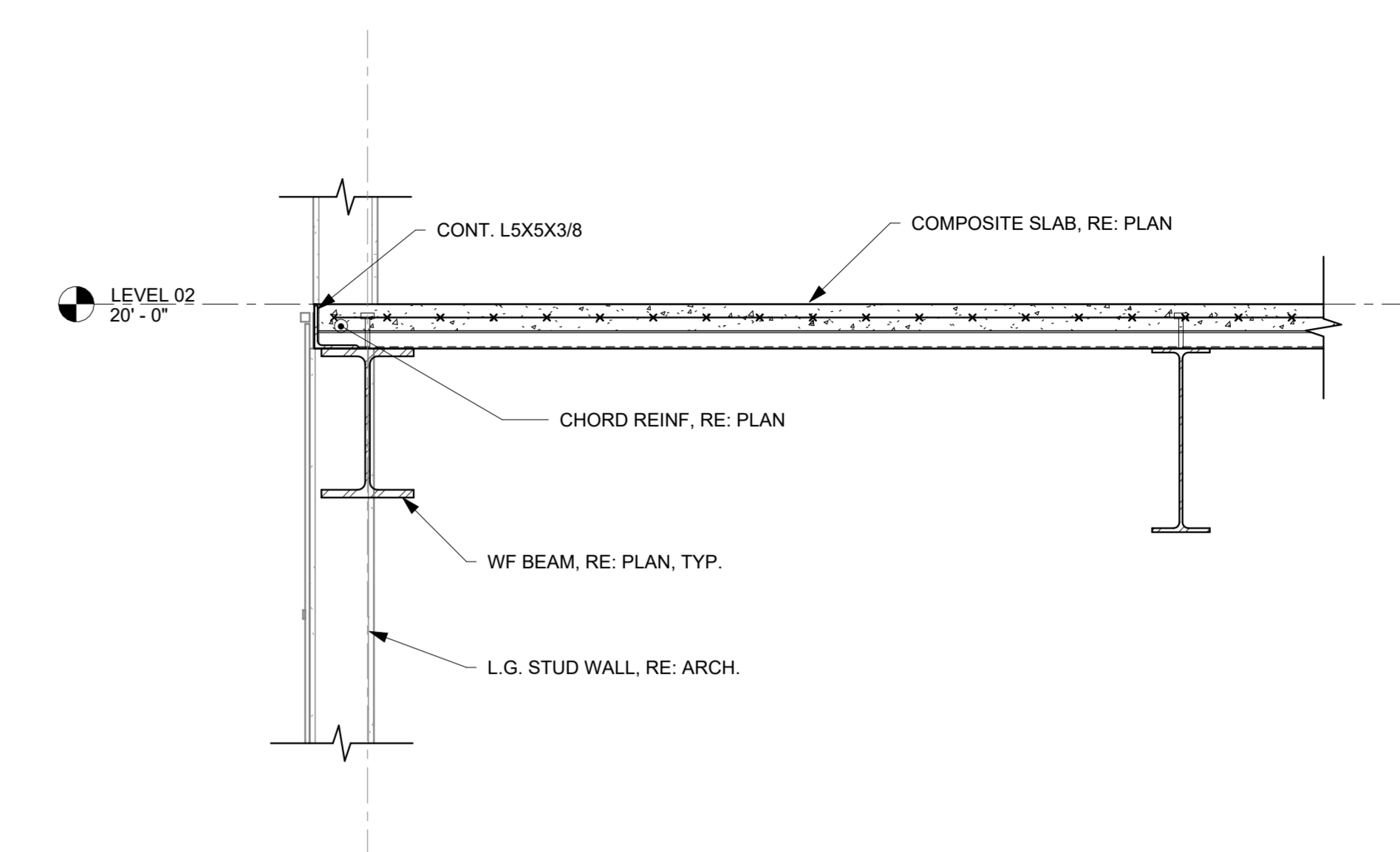
INFORMATION



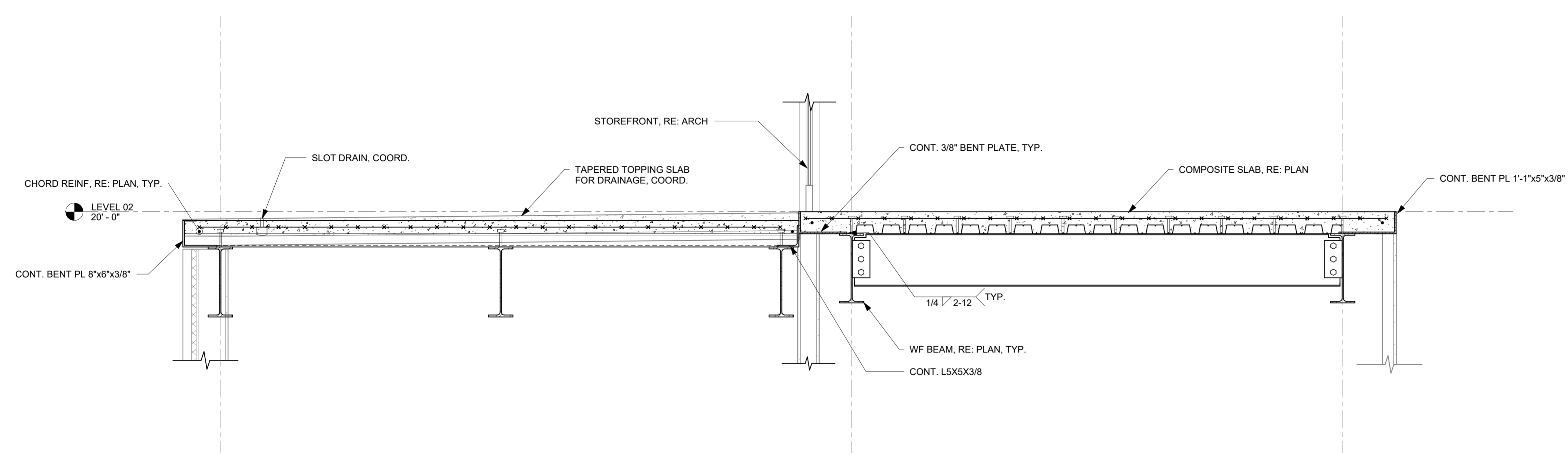
1 FFS - SECTION
3/4" = 1'-0"



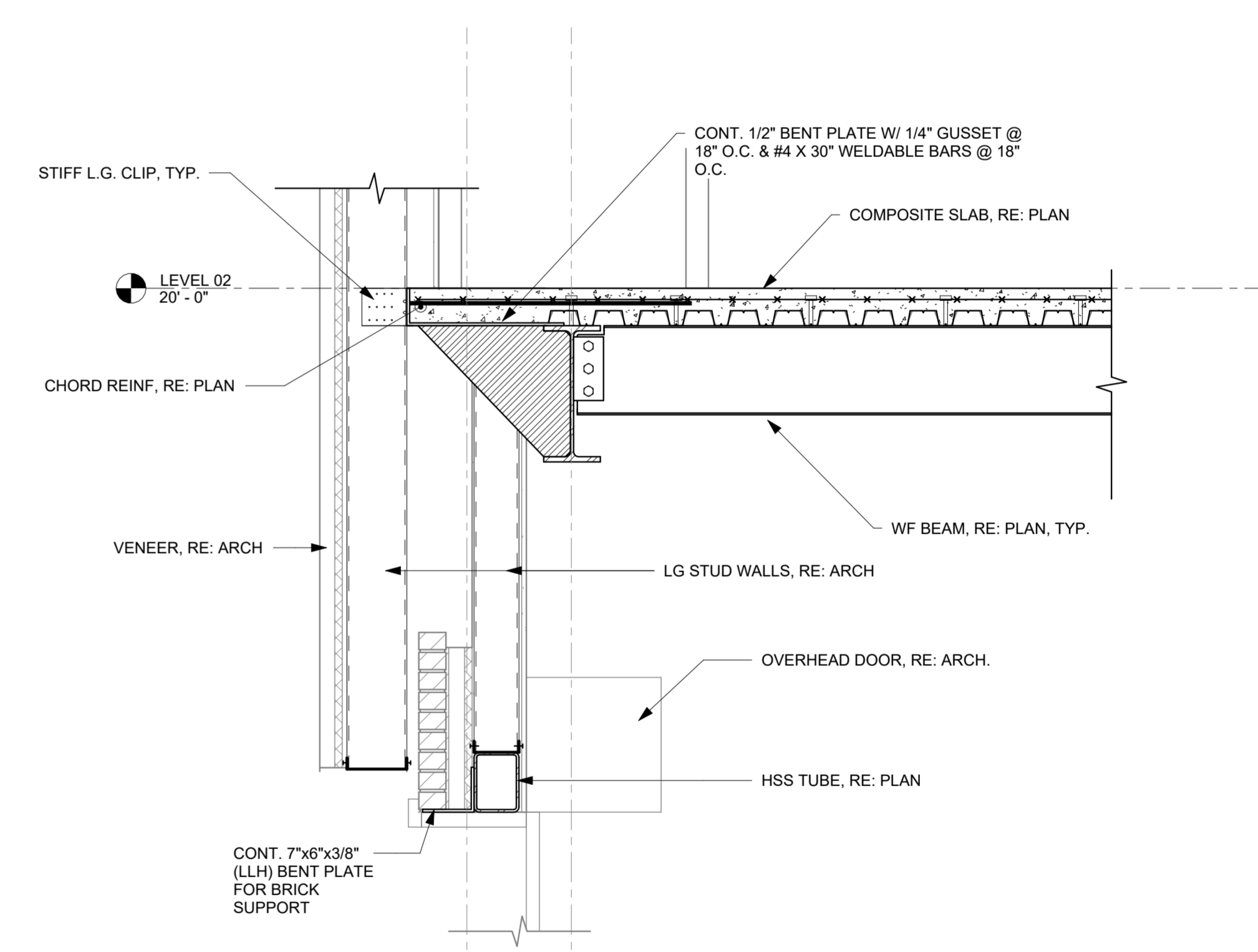
2 FFS - SECTION
3/4" = 1'-0"



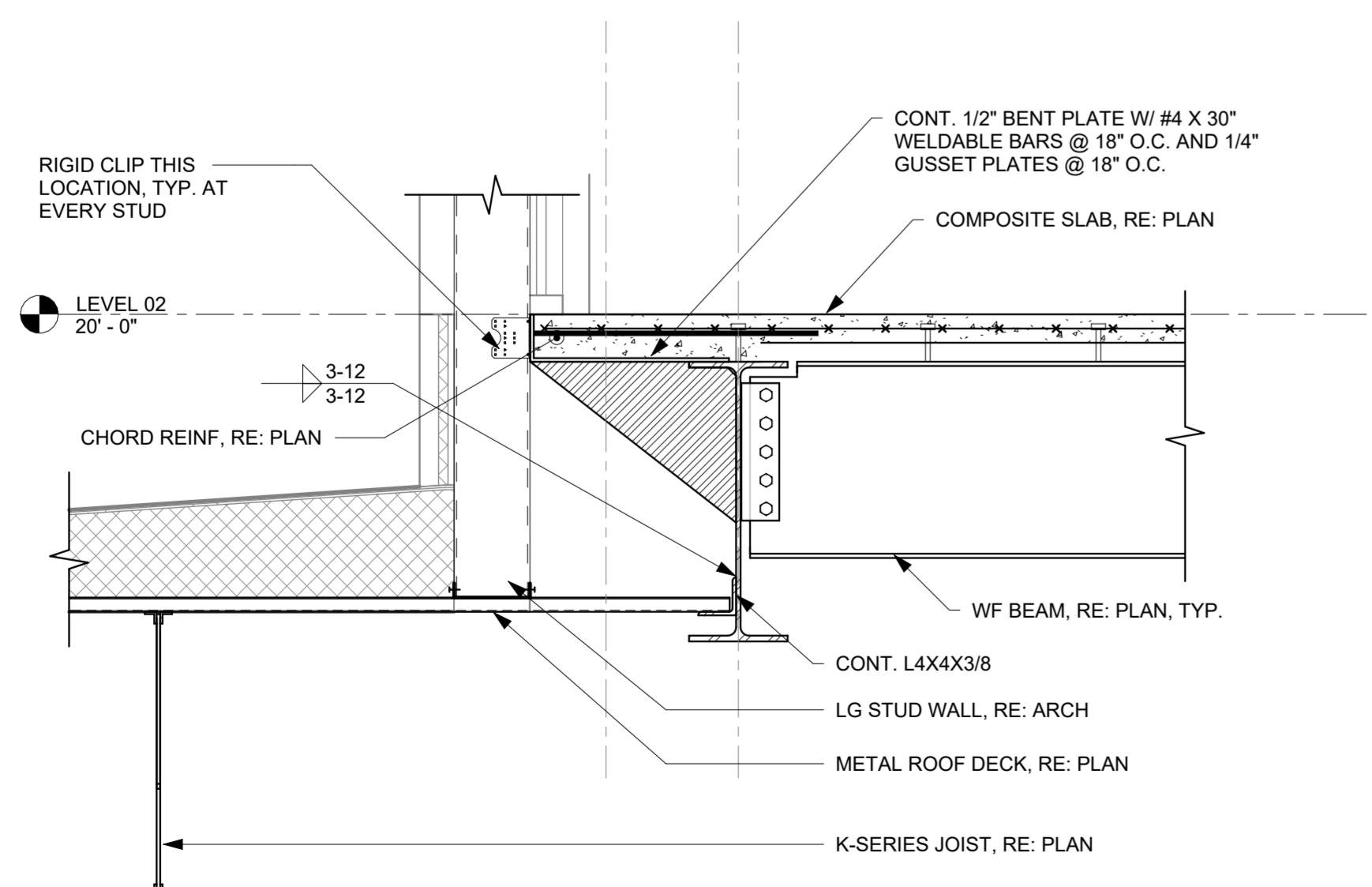
3 FFS - SECTION
3/4" = 1'-0"



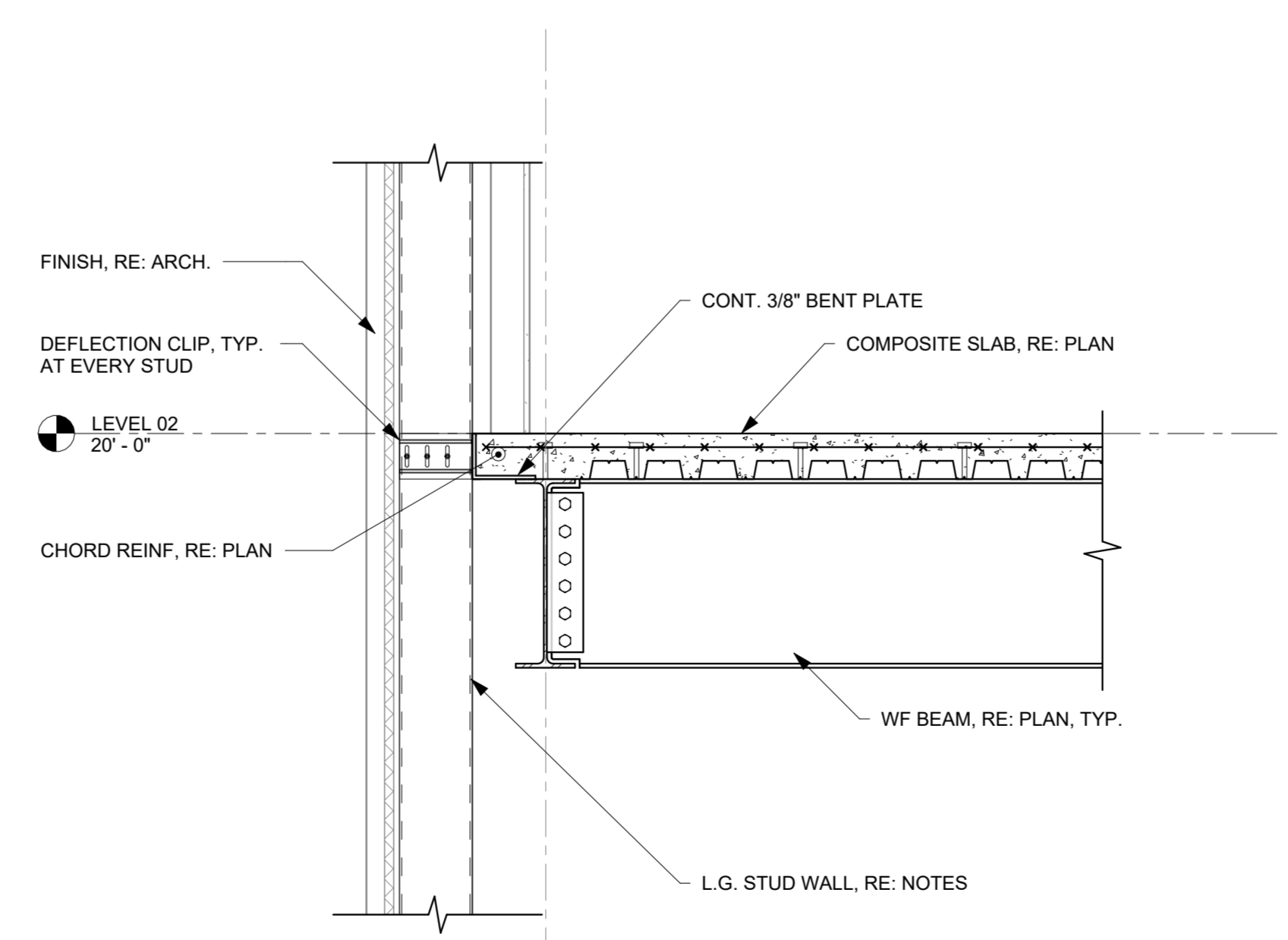
4 FFS - SECTION
3/4" = 1'-0"



5 FFS - SECTION
3/4" = 1'-0"



6 FFS - SECTION
3/4" = 1'-0"



7 FFS - SECTION
3/4" = 1'-0"



PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

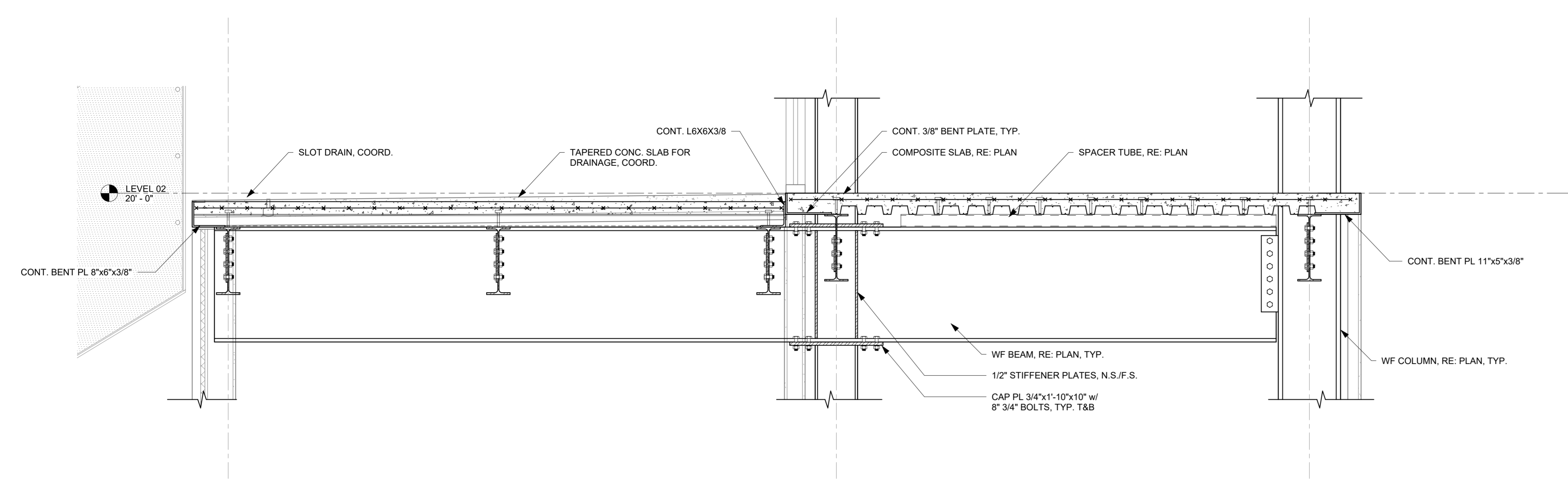
PROJECT NUMBER

-

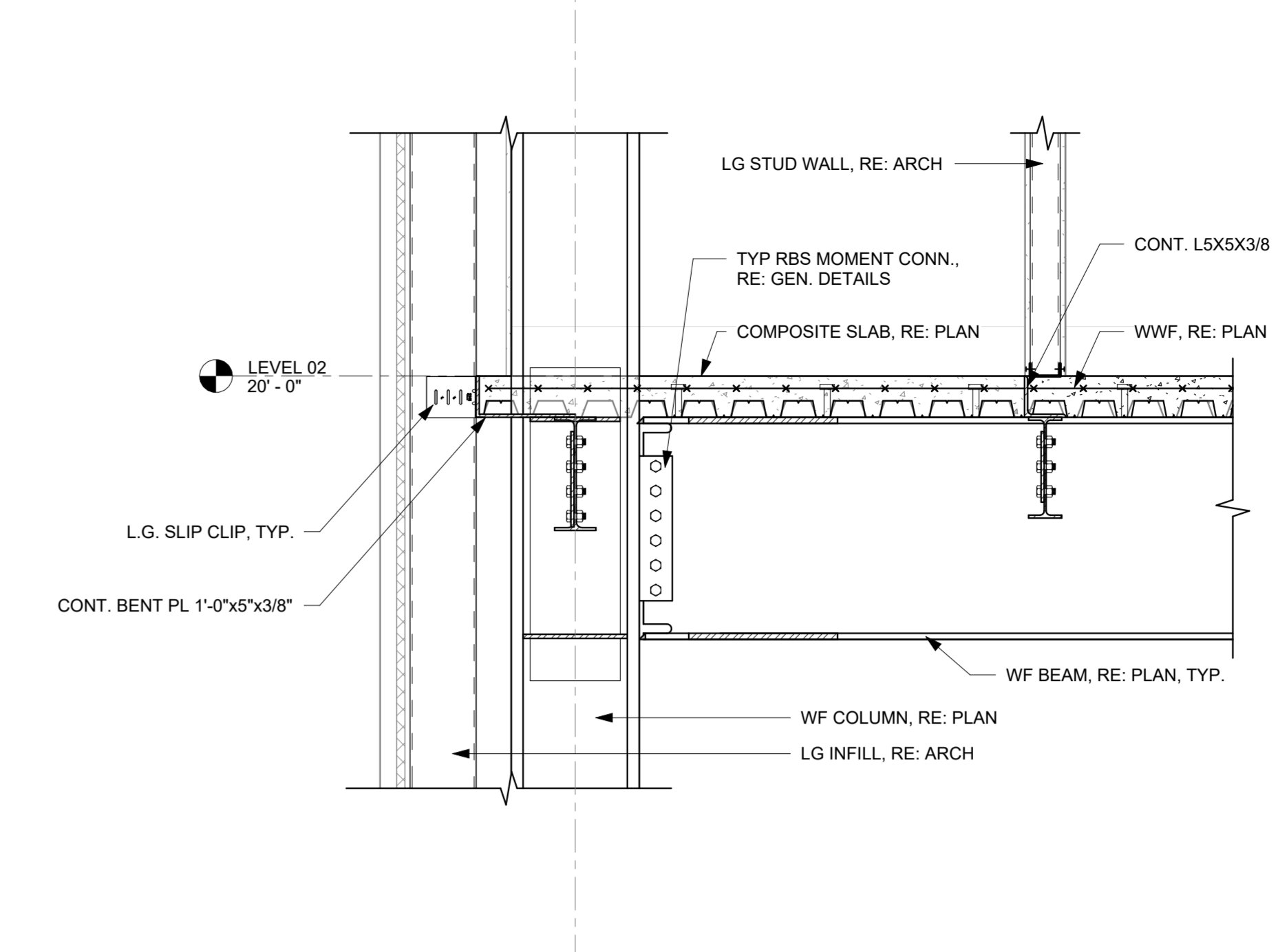
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

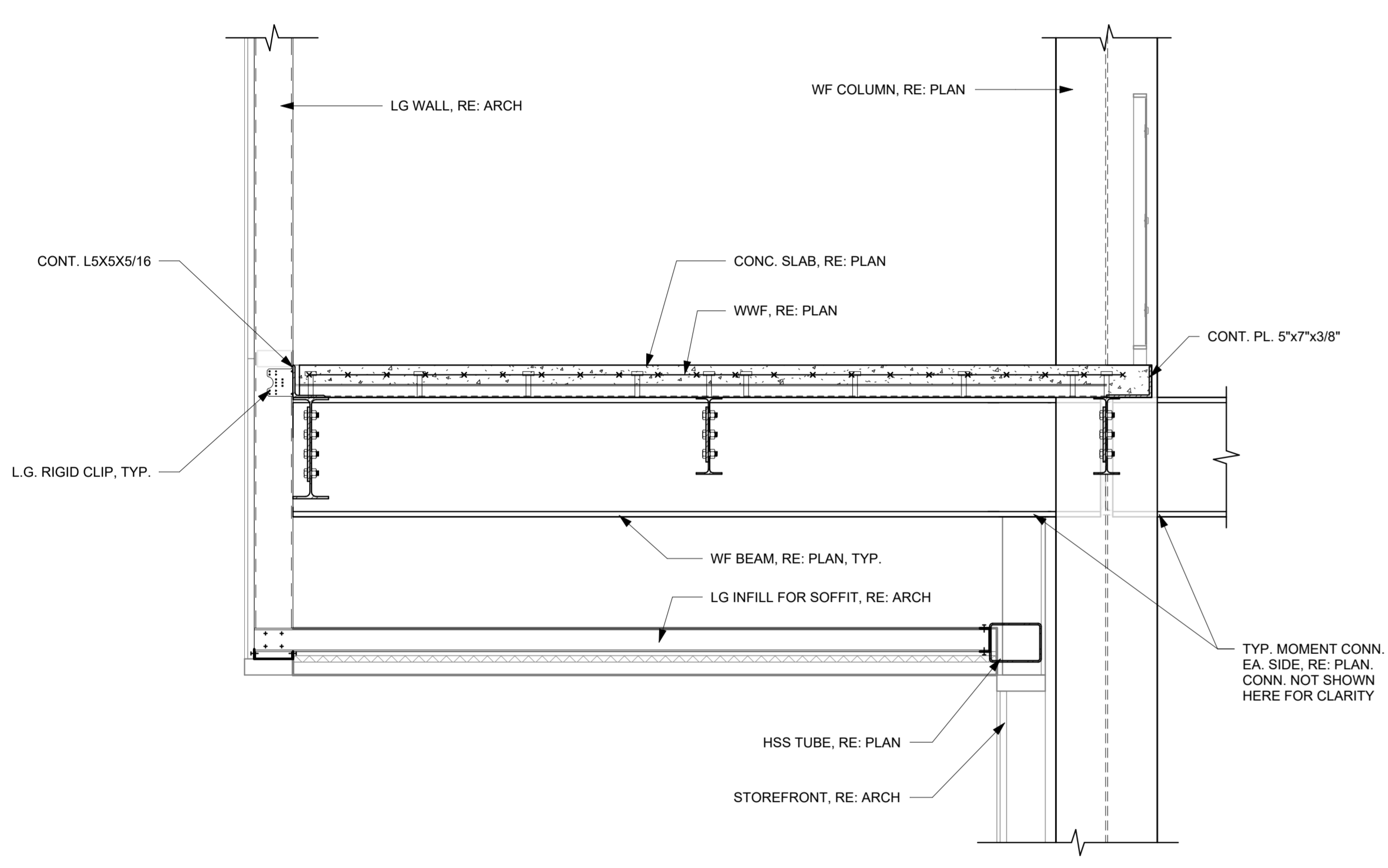
INFORMATION



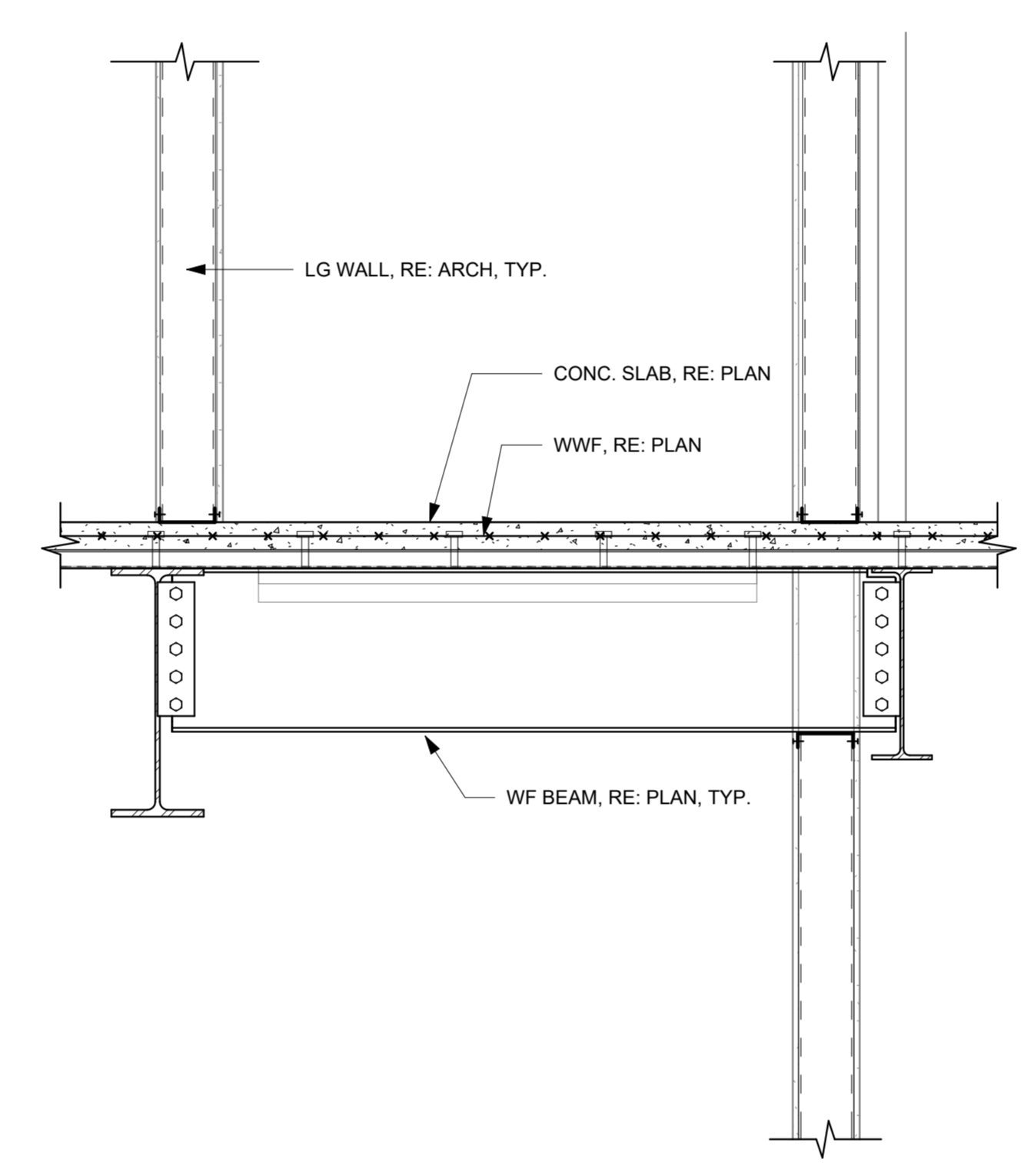
1 FFS - SECTION
3/4" = 1'-0"



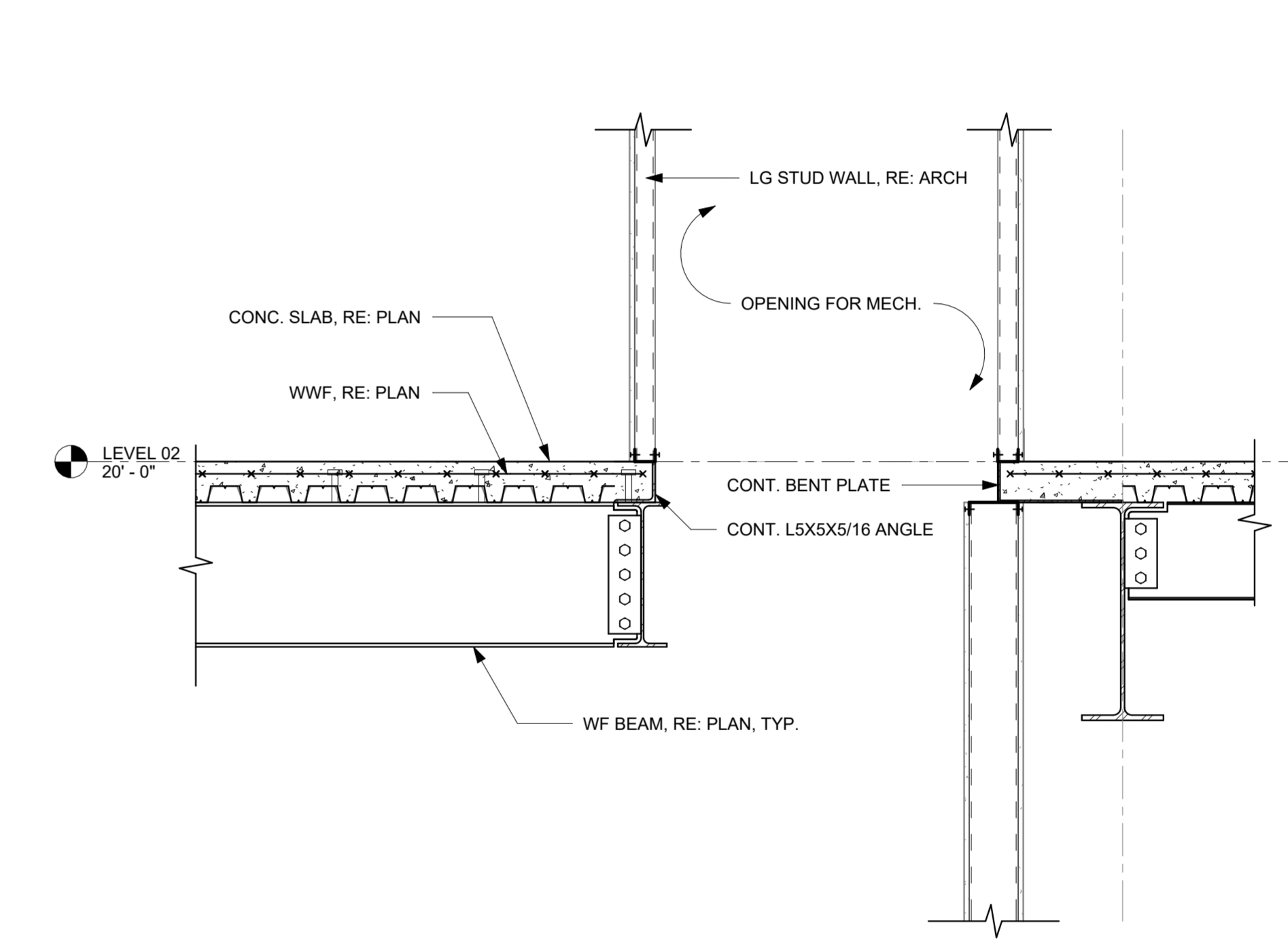
2 FFS - SECTION
3/4" = 1'-0"



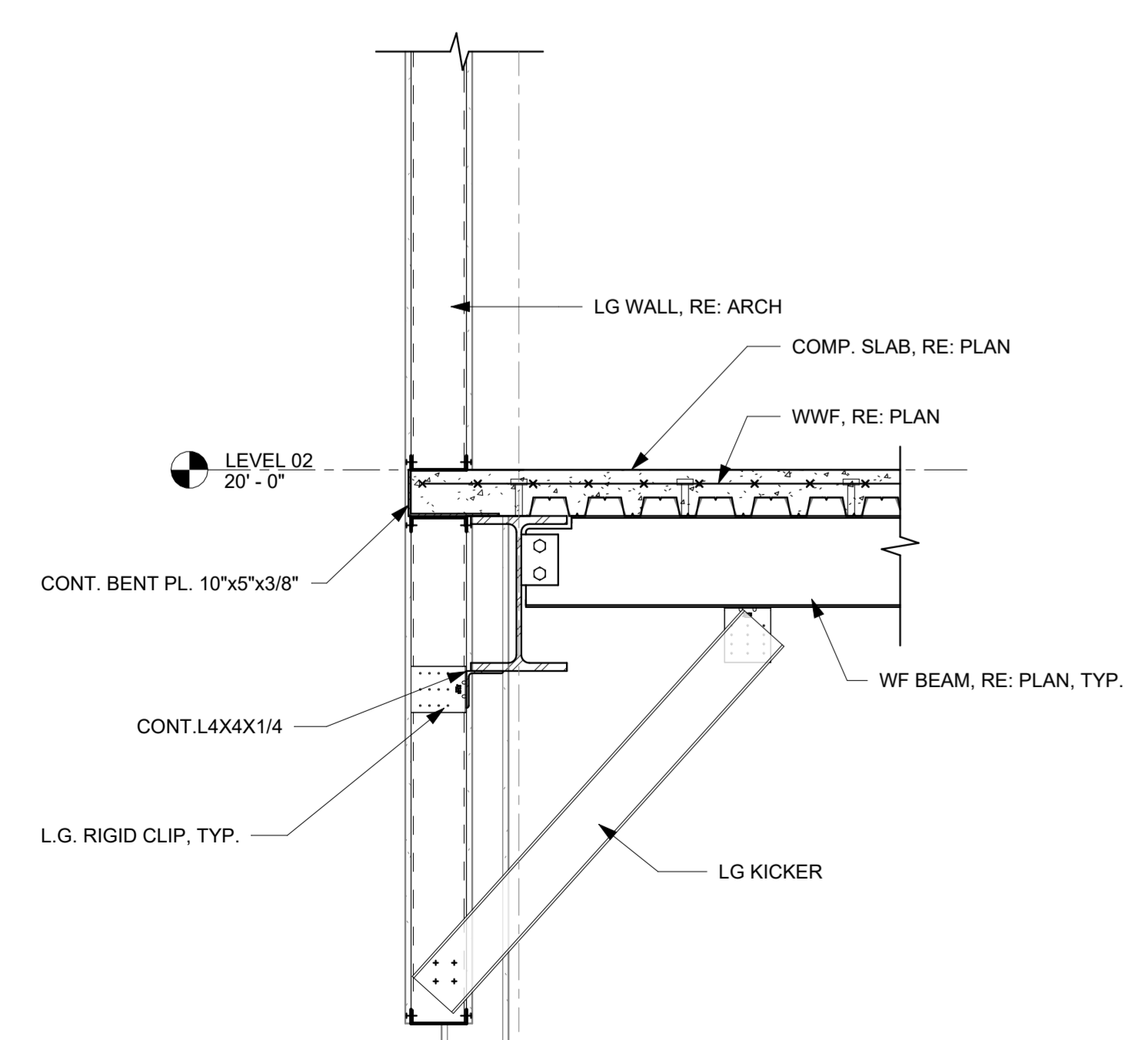
3 FFS - SECTION
3/4" = 1'-0"



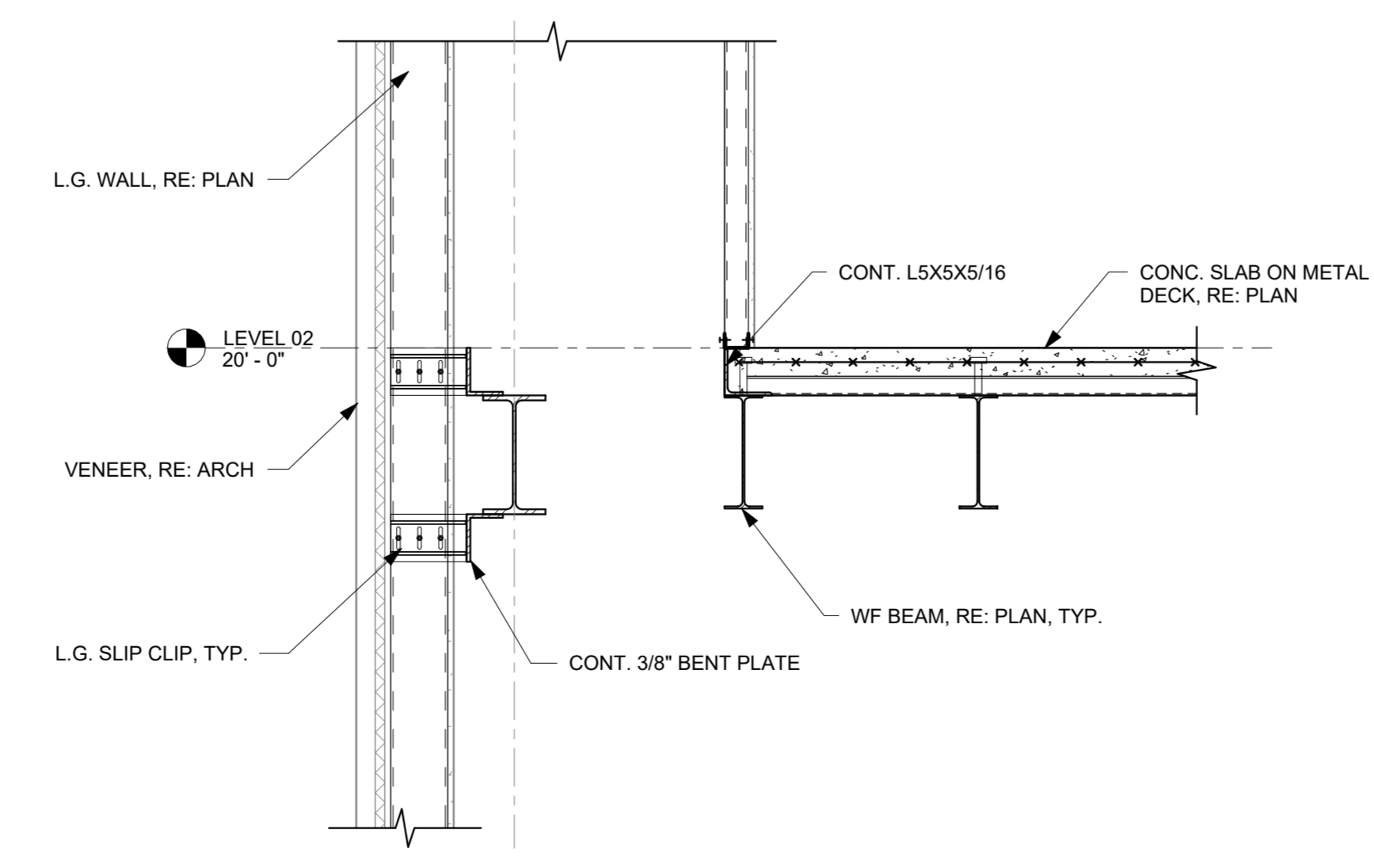
4 FFS - SECTION
3/4" = 1'-0"



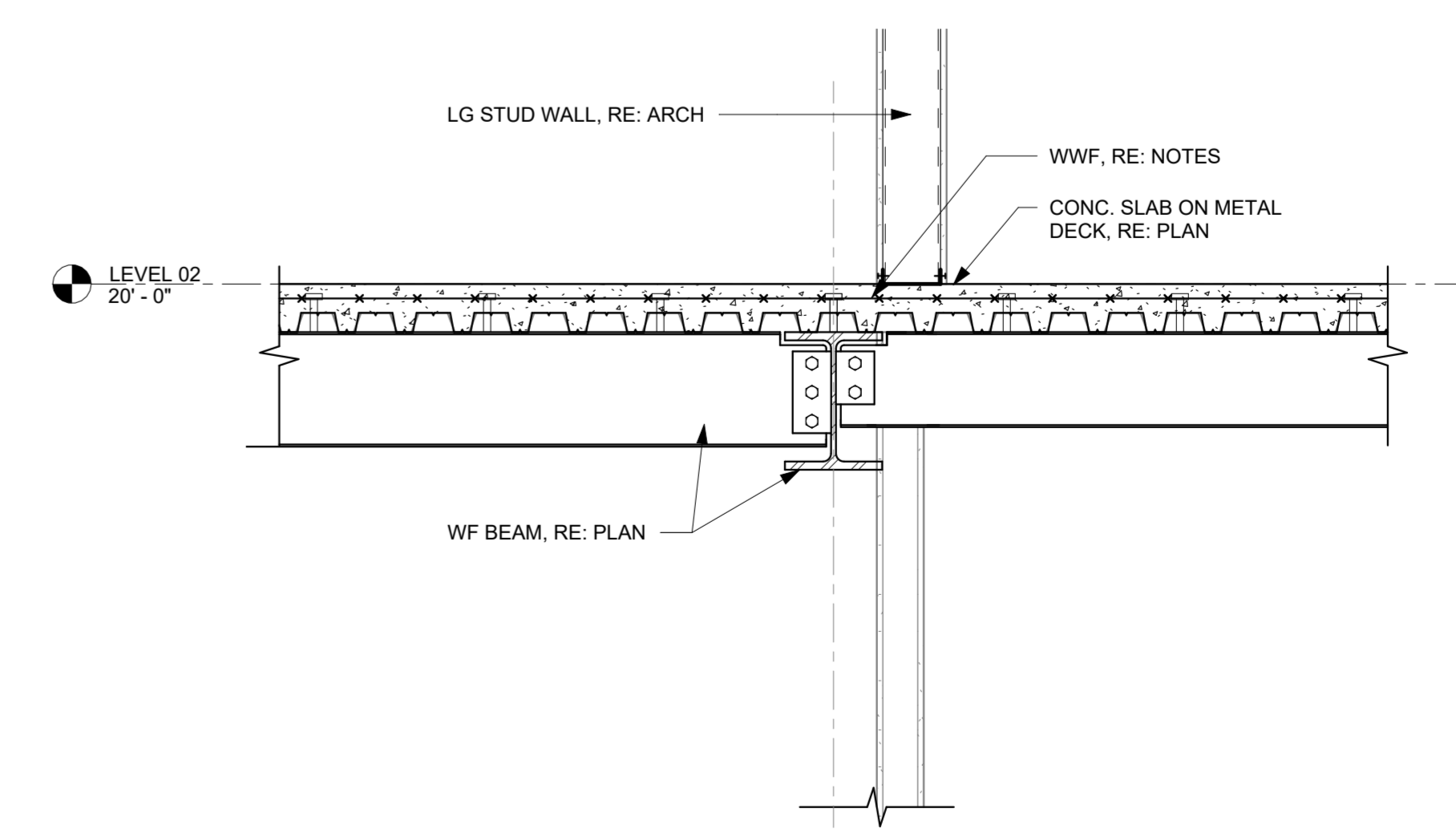
5 FFS - SECTION
3/4" = 1'-0"



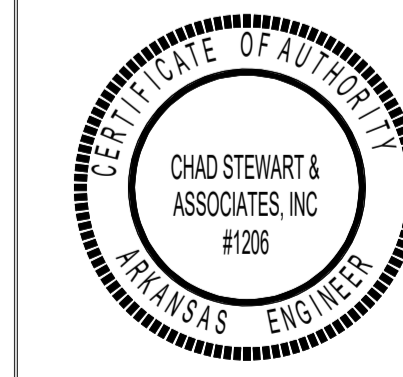
6 FFS - SECTION
3/4" = 1'-0"



7 FFS - SECTION
3/4" = 1'-0"



8 FFS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

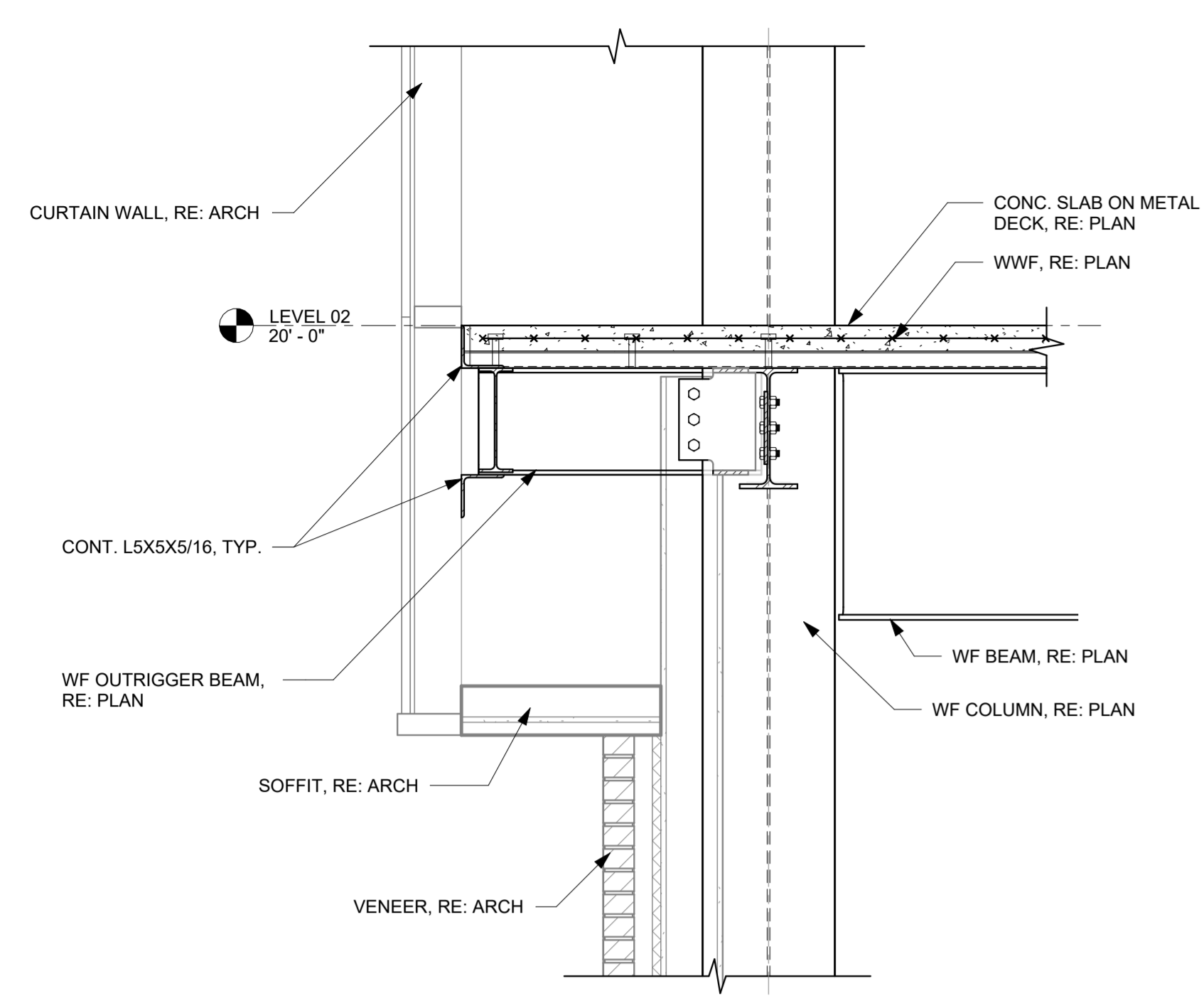
PROJECT NUMBER

-

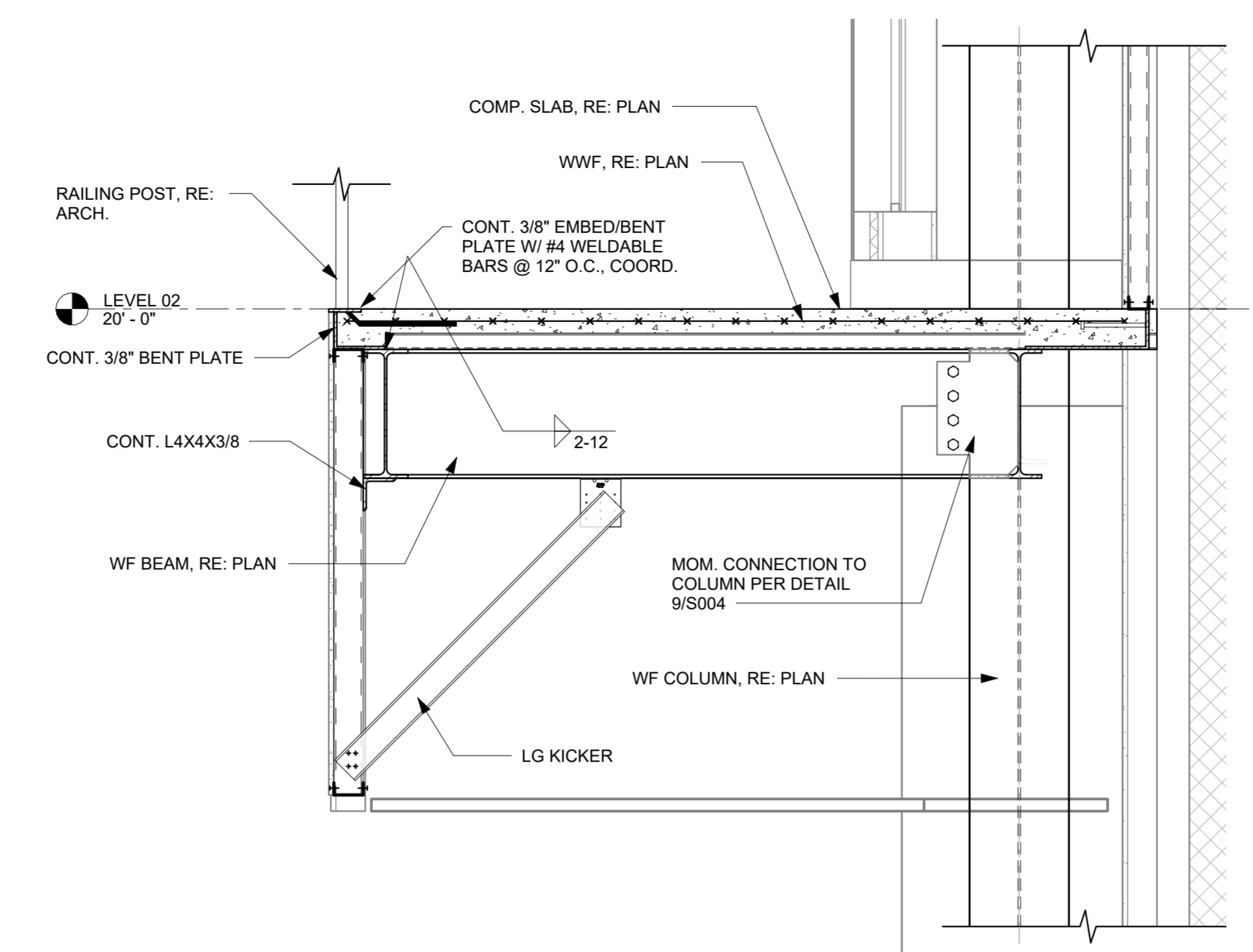
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

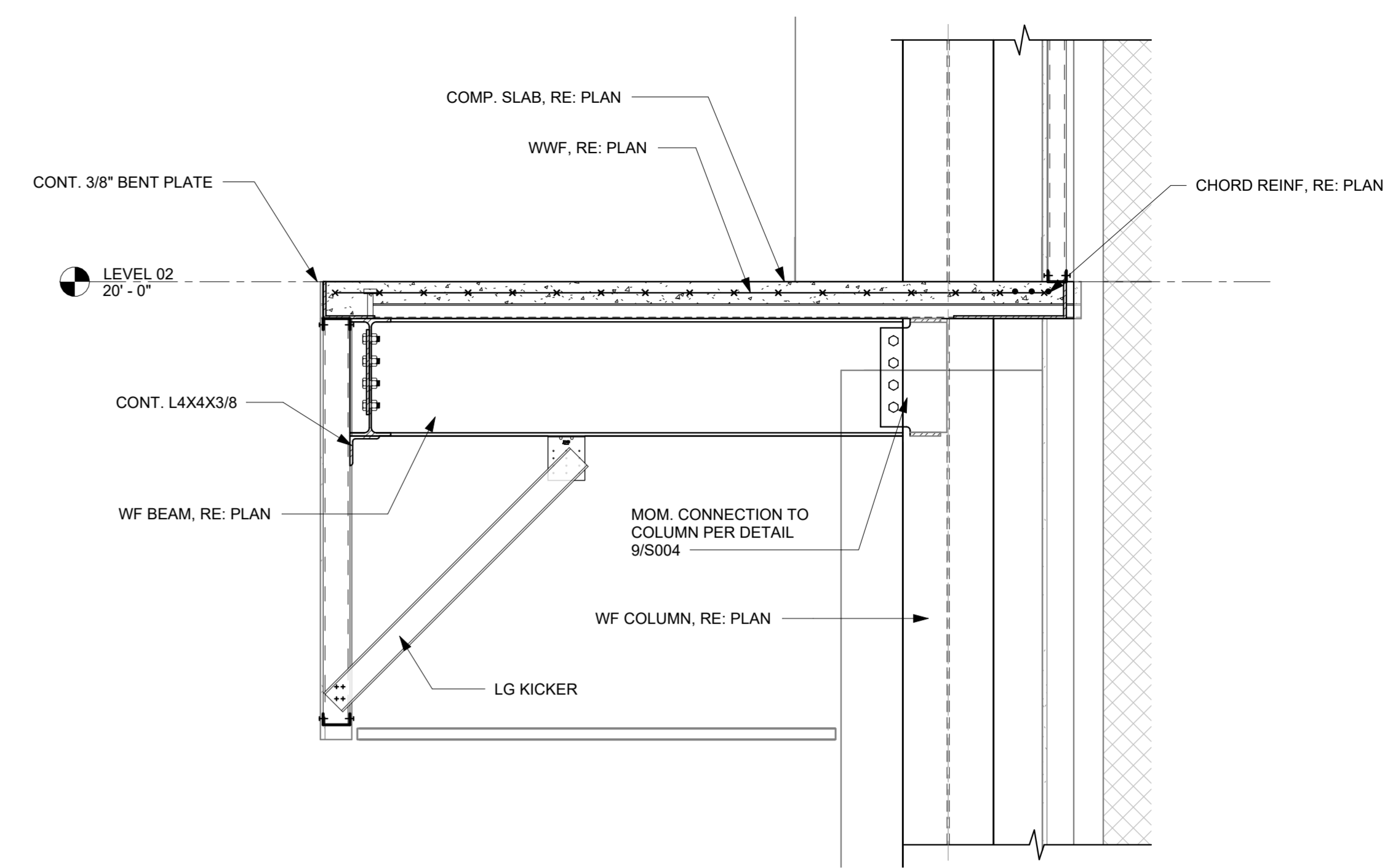
INFORMATION



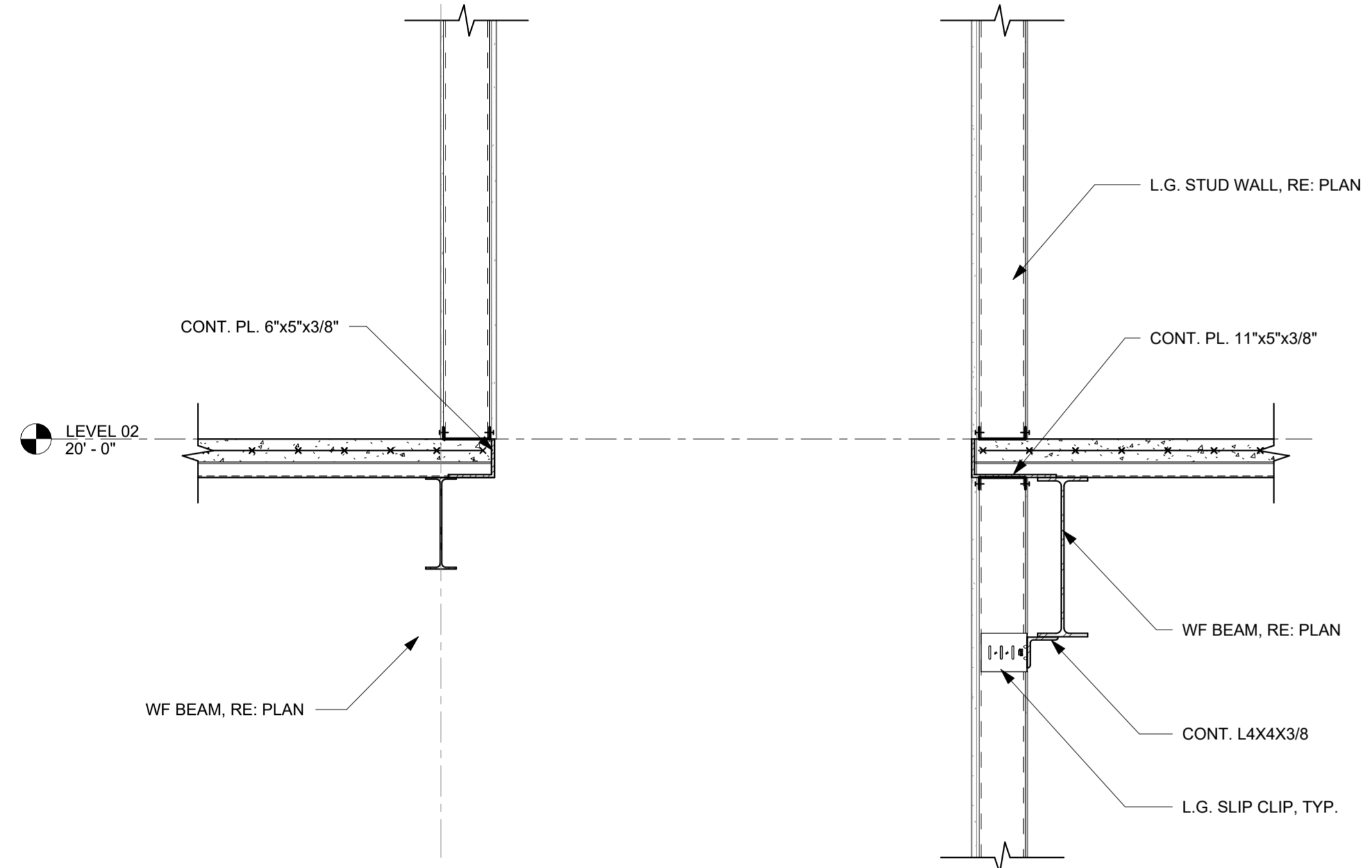
1 FFS - SECTION
3/4" = 1'-0"



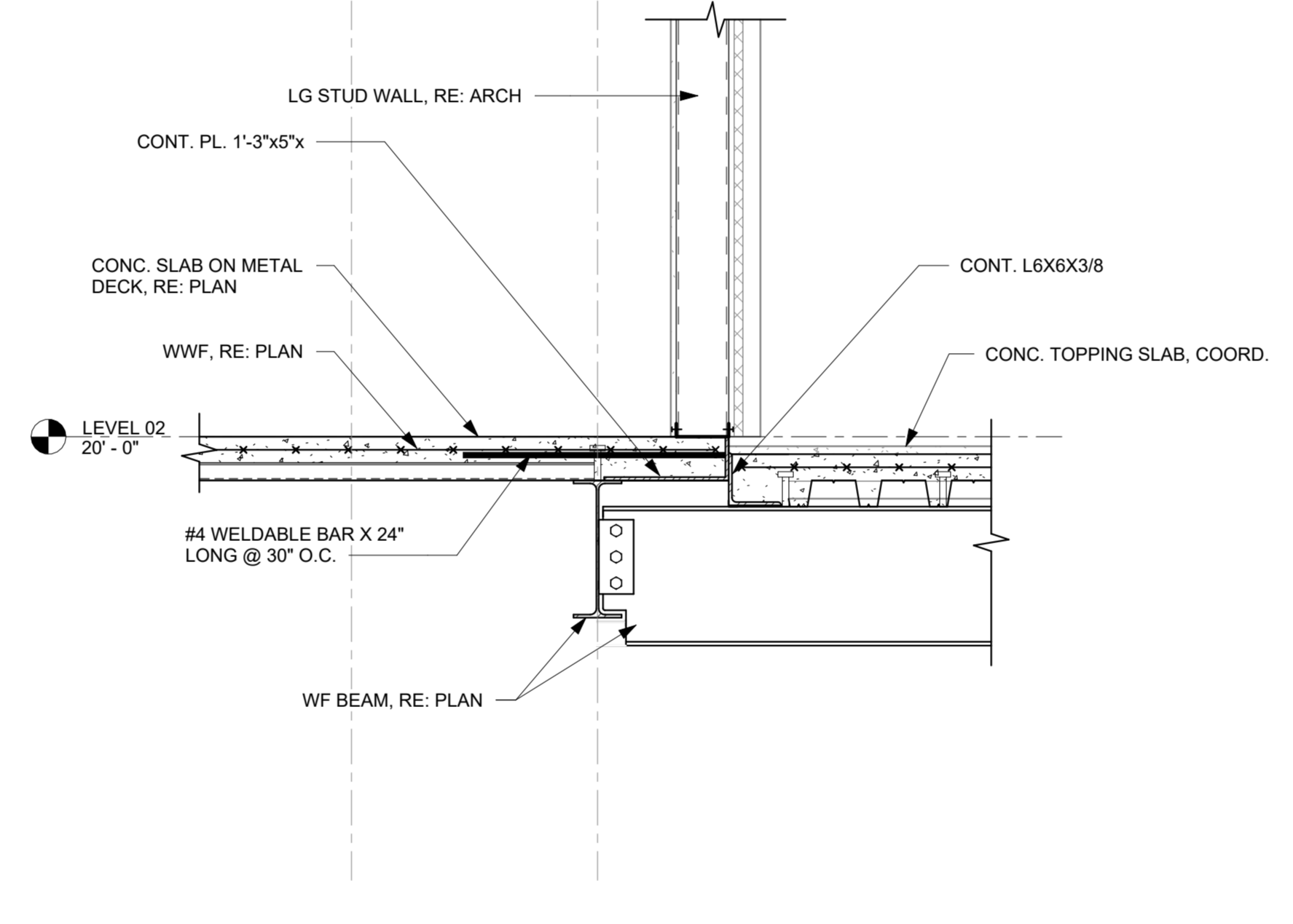
2 FFS - SECTION
3/4" = 1'-0"



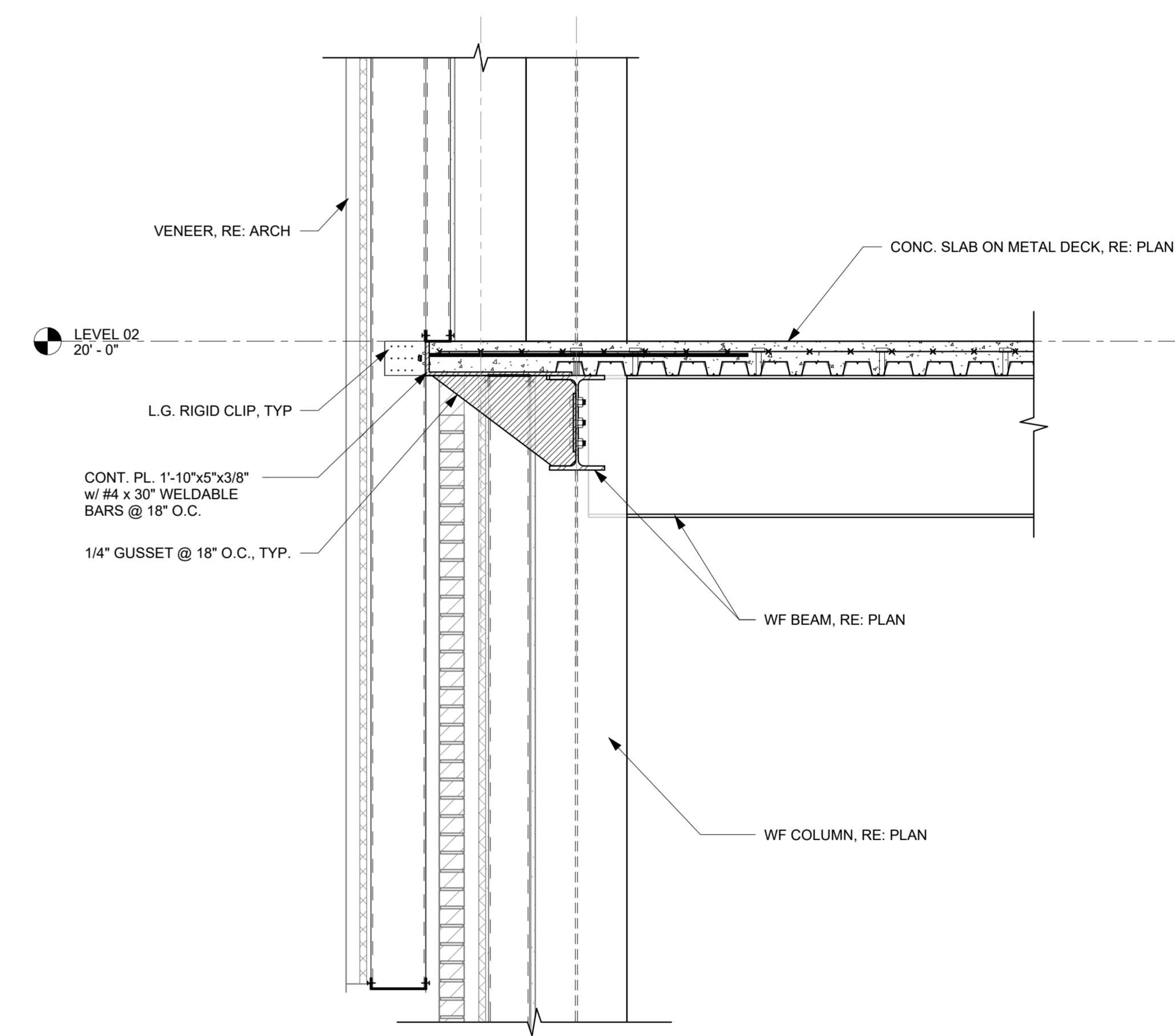
3 FFS - SECTION
3/4" = 1'-0"



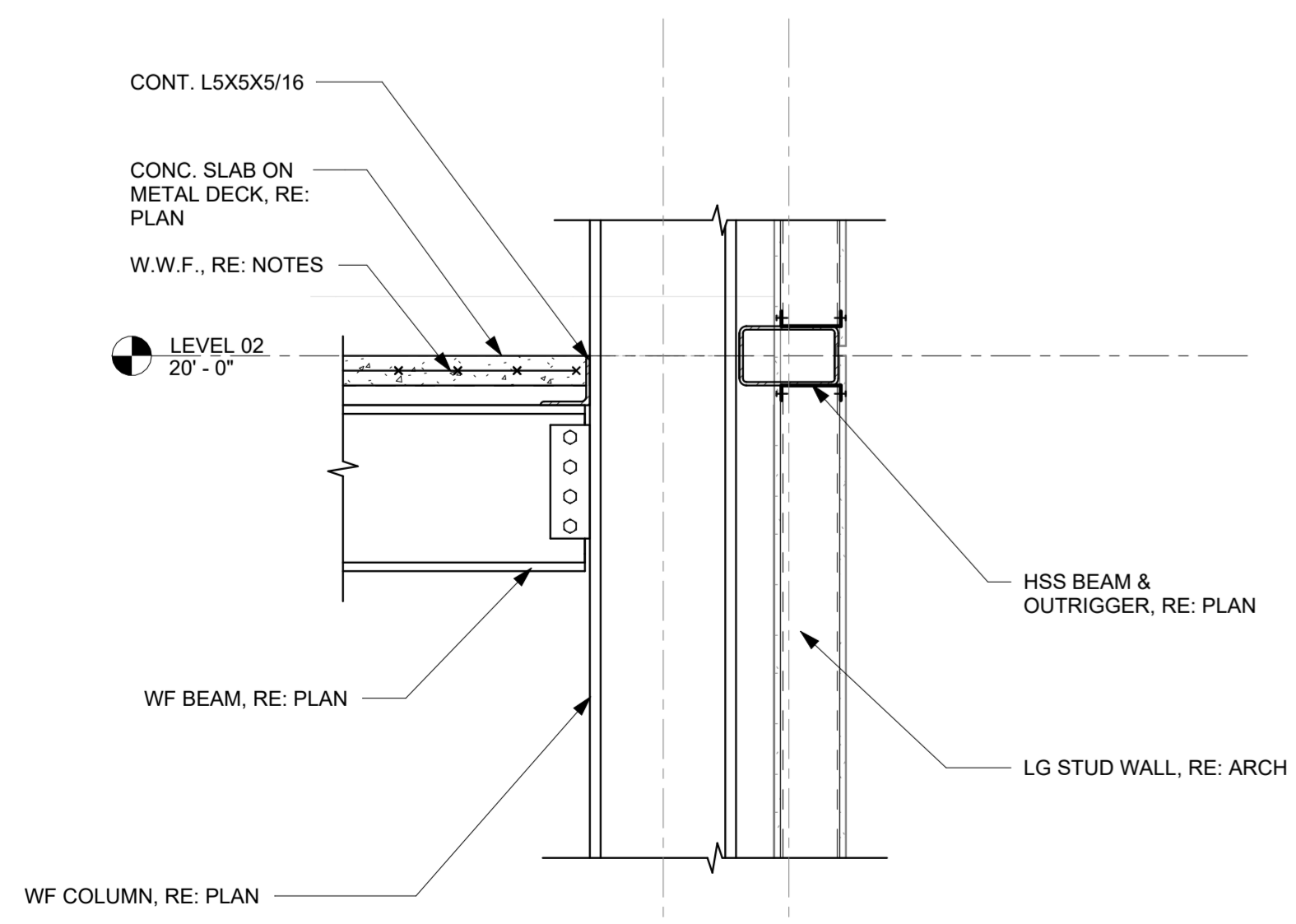
4 FFS - SECTION
3/4" = 1'-0"



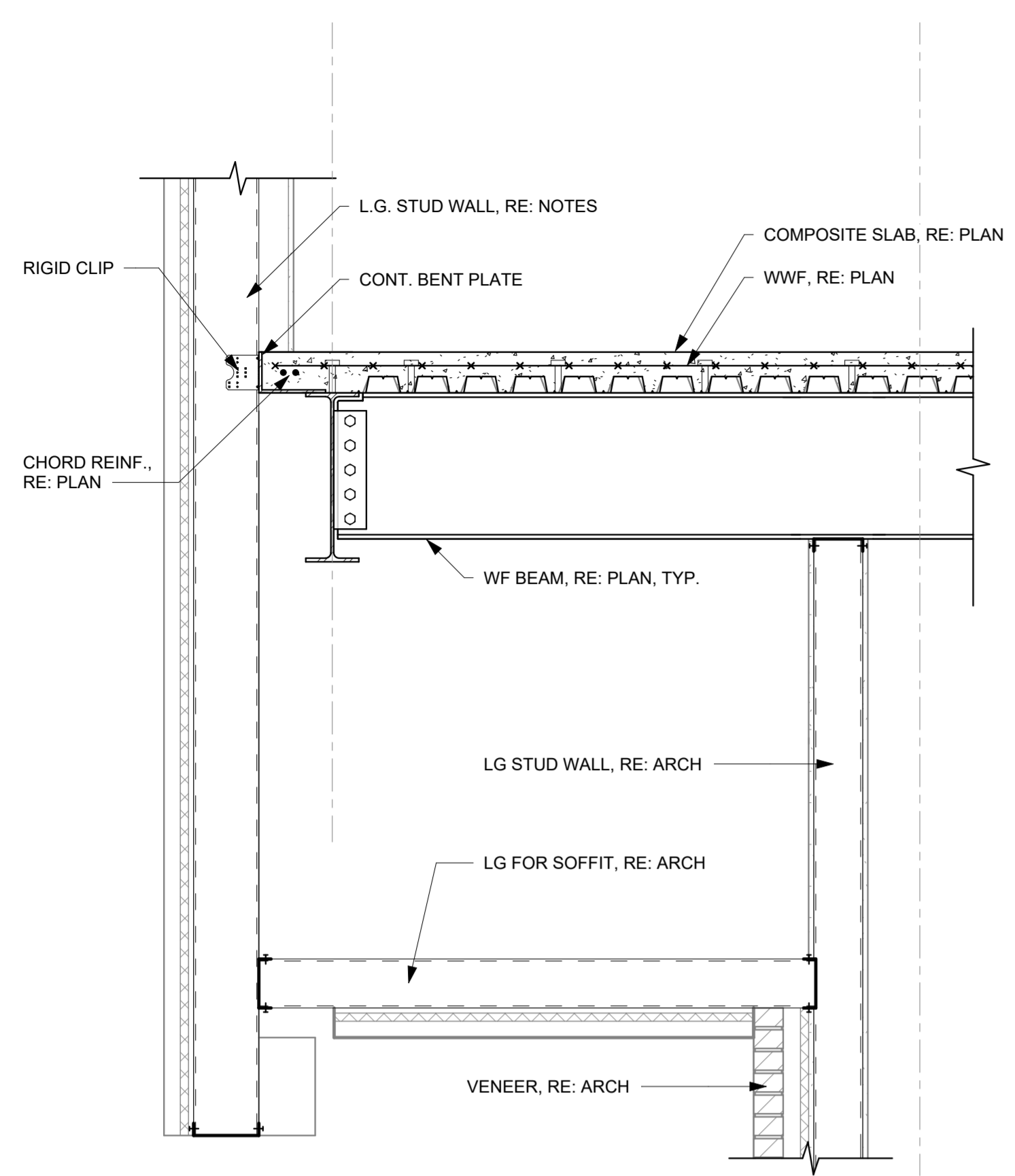
5 FFS - SECTION
3/4" = 1'-0"



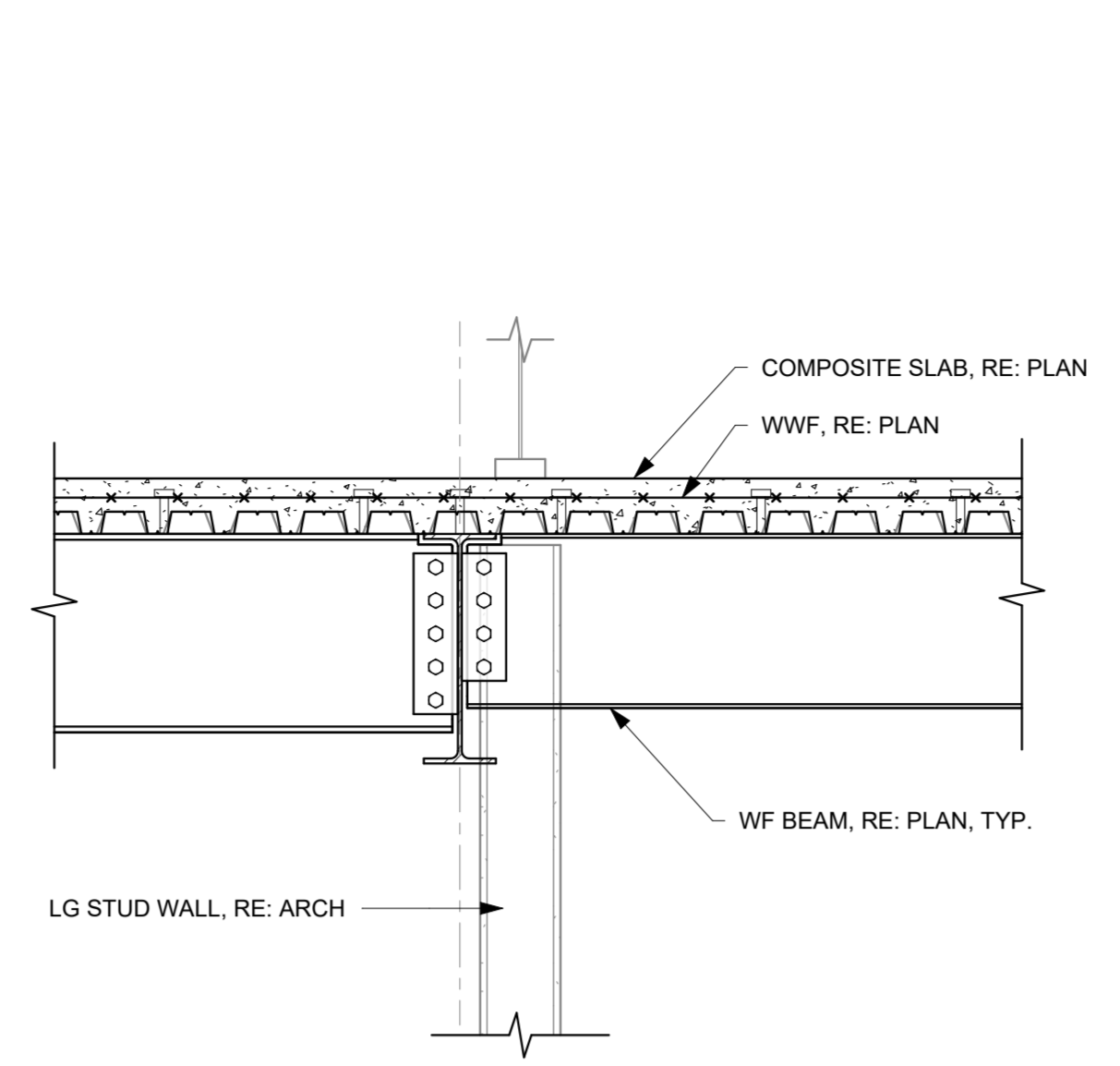
6 FFS - SECTION
3/4" = 1'-0"



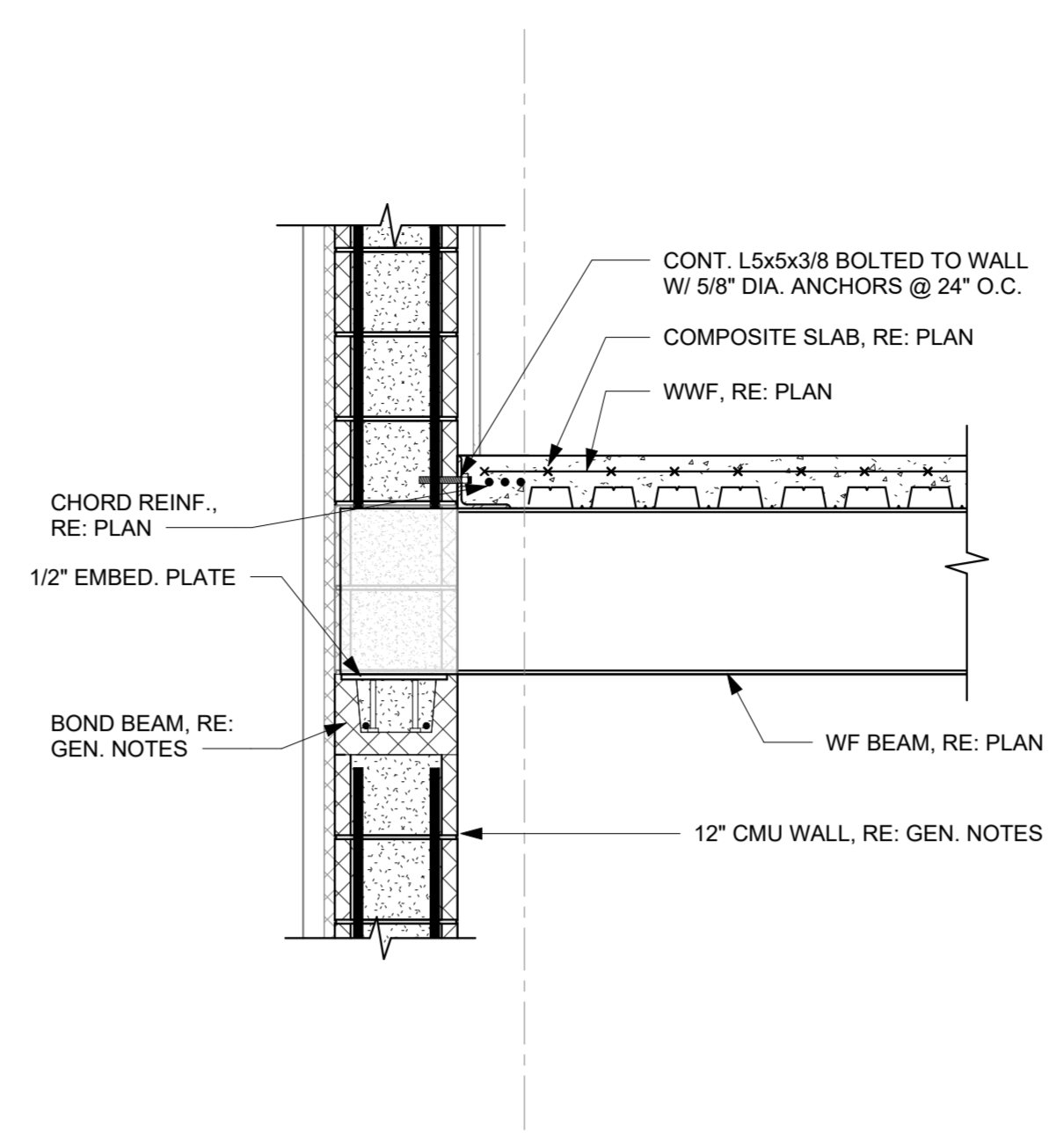
7 FFS - SECTION 180
3/4" = 1'-0"



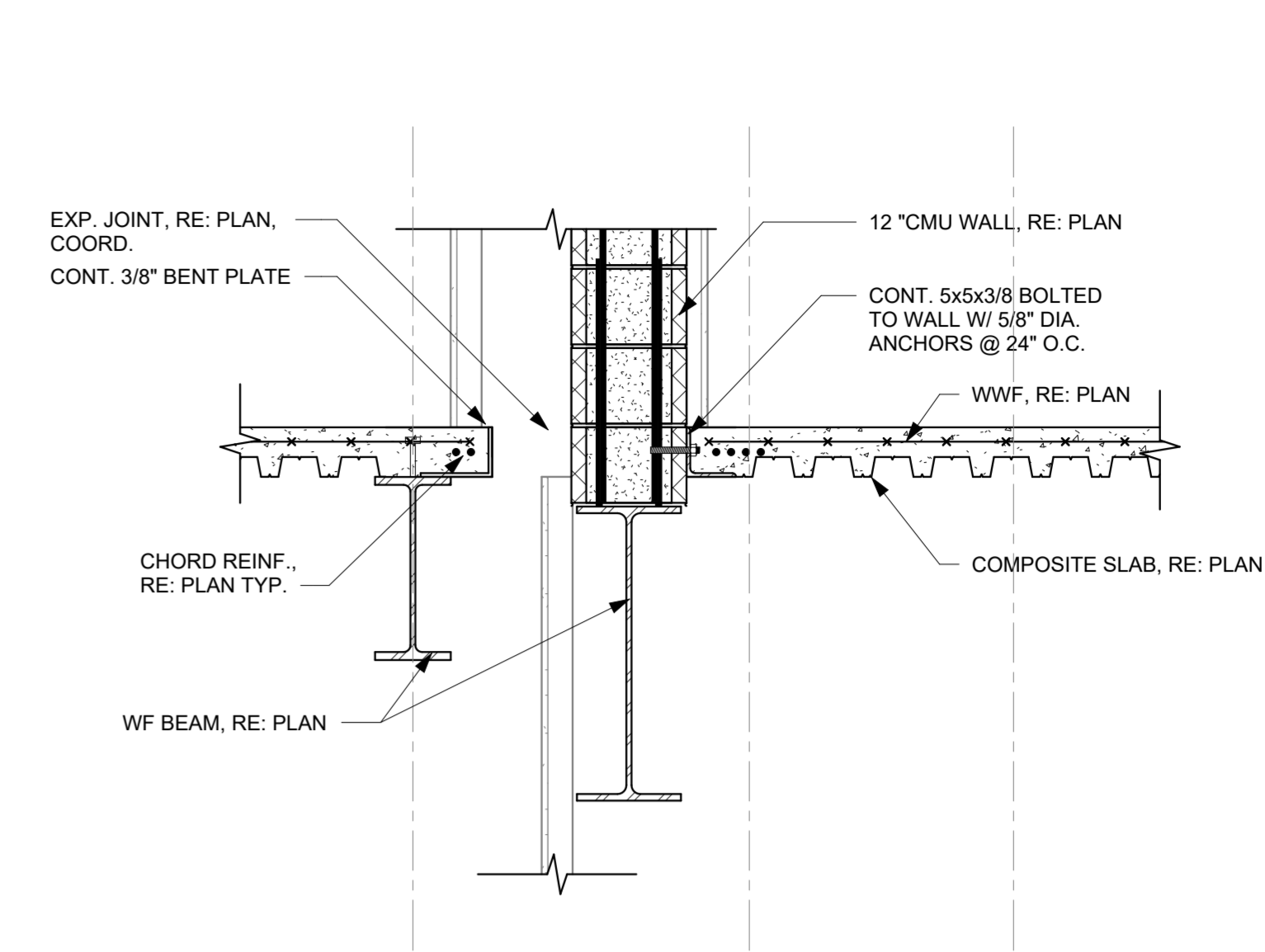
1 FFS - SECTION
3/4" = 1'-0"



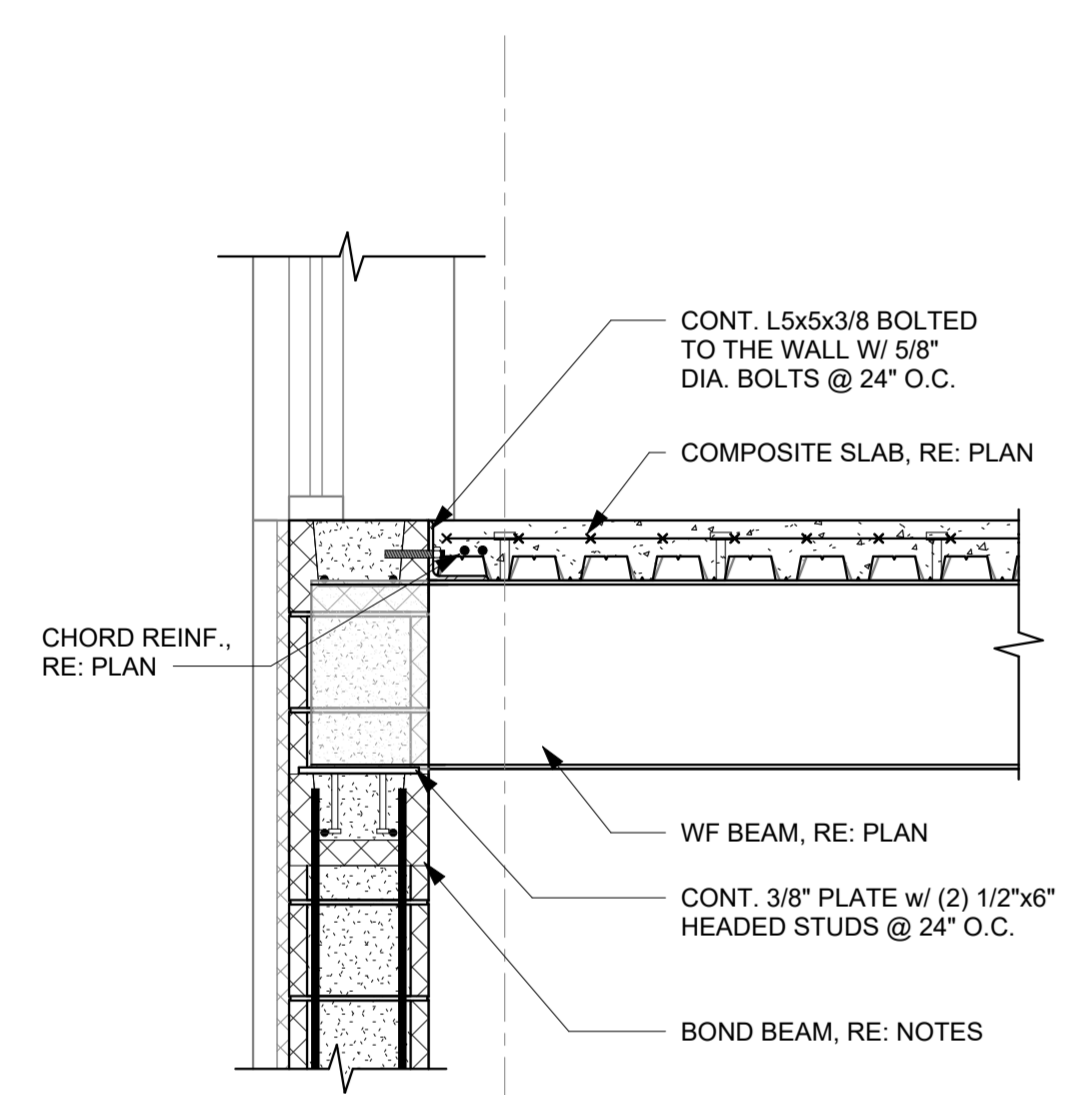
2 FFS - SECTION
3/4" = 1'-0"



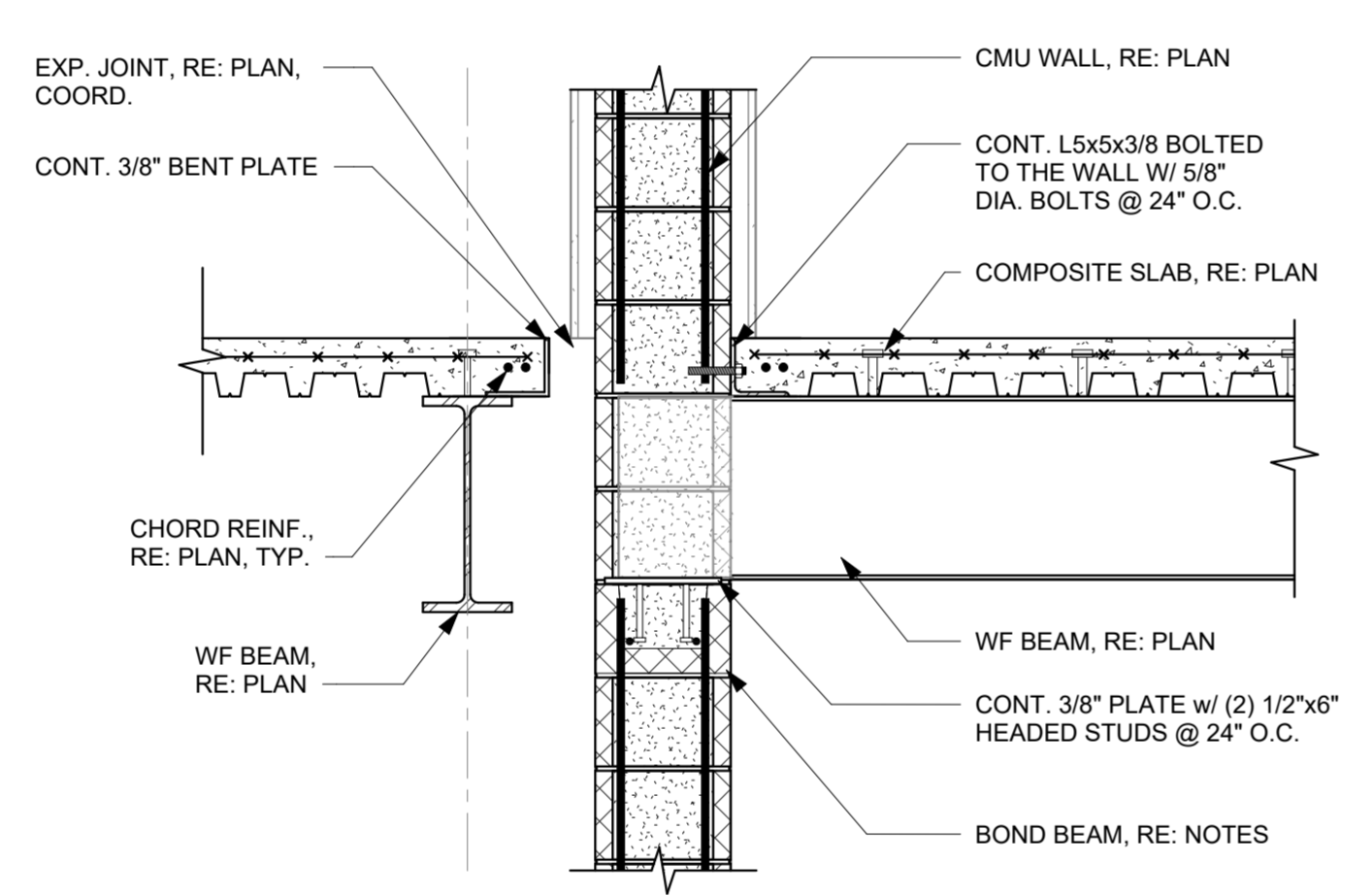
3 FFS - SECTION
3/4" = 1'-0"



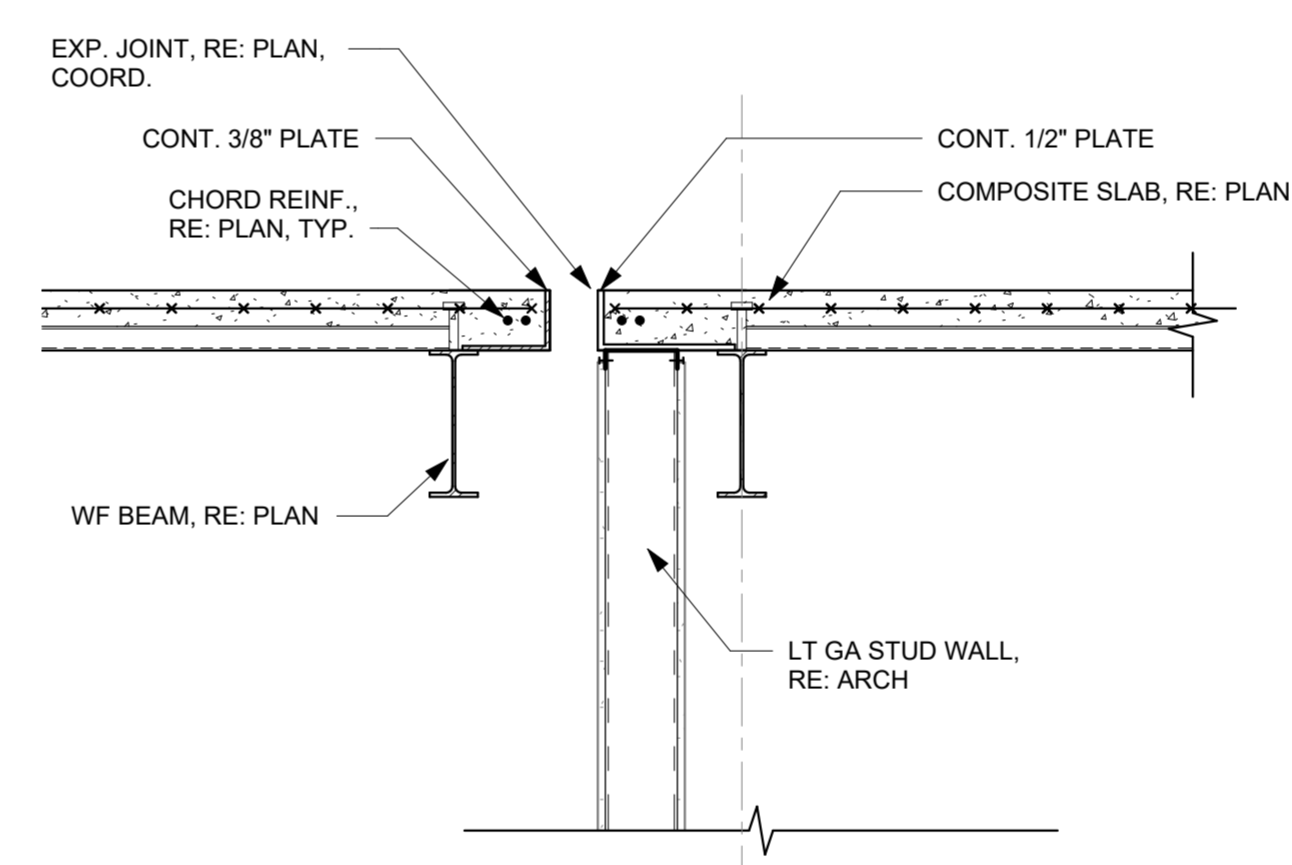
4 FFS - SECTION
3/4" = 1'-0"



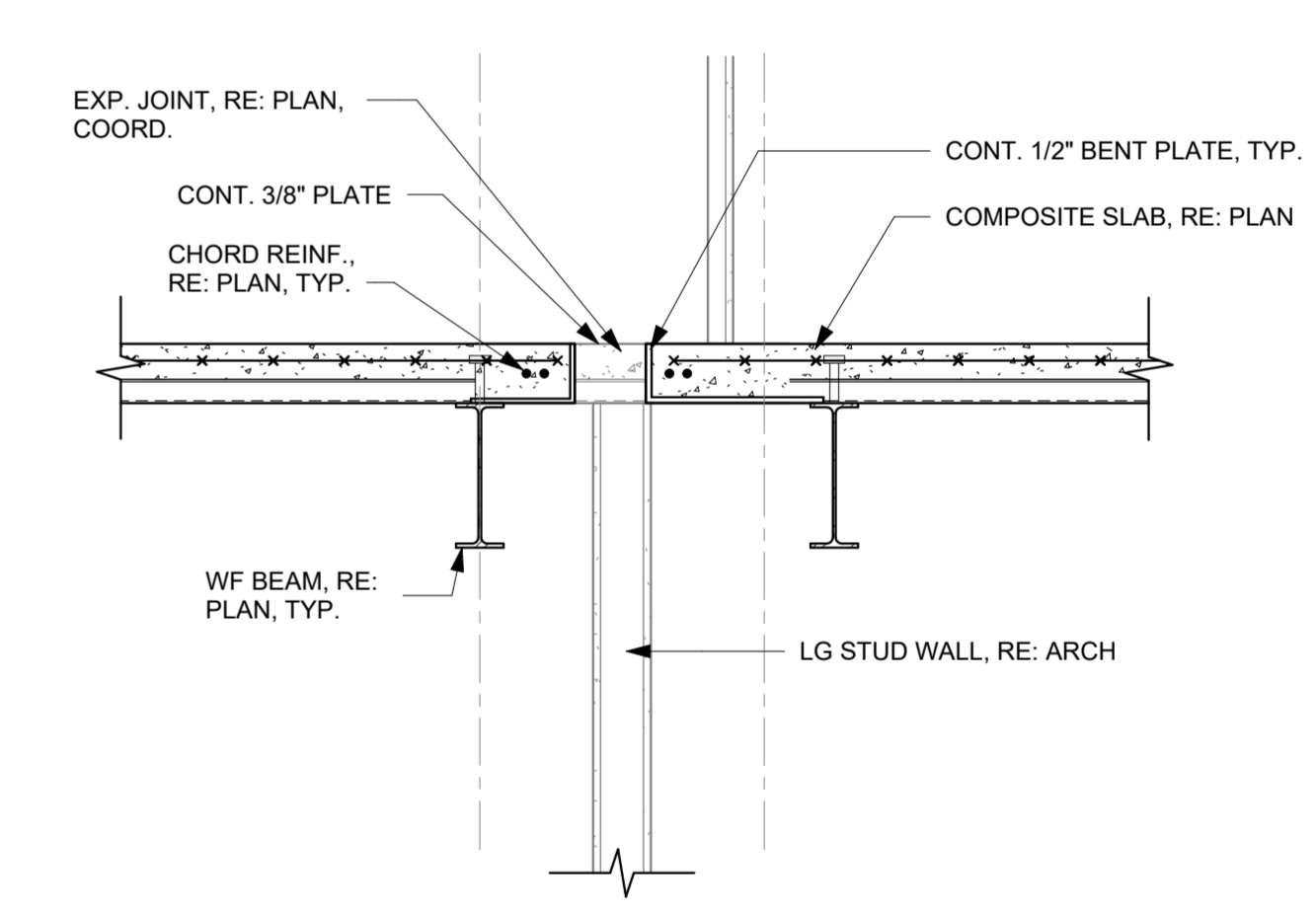
5 FFS - SECTION
3/4" = 1'-0"



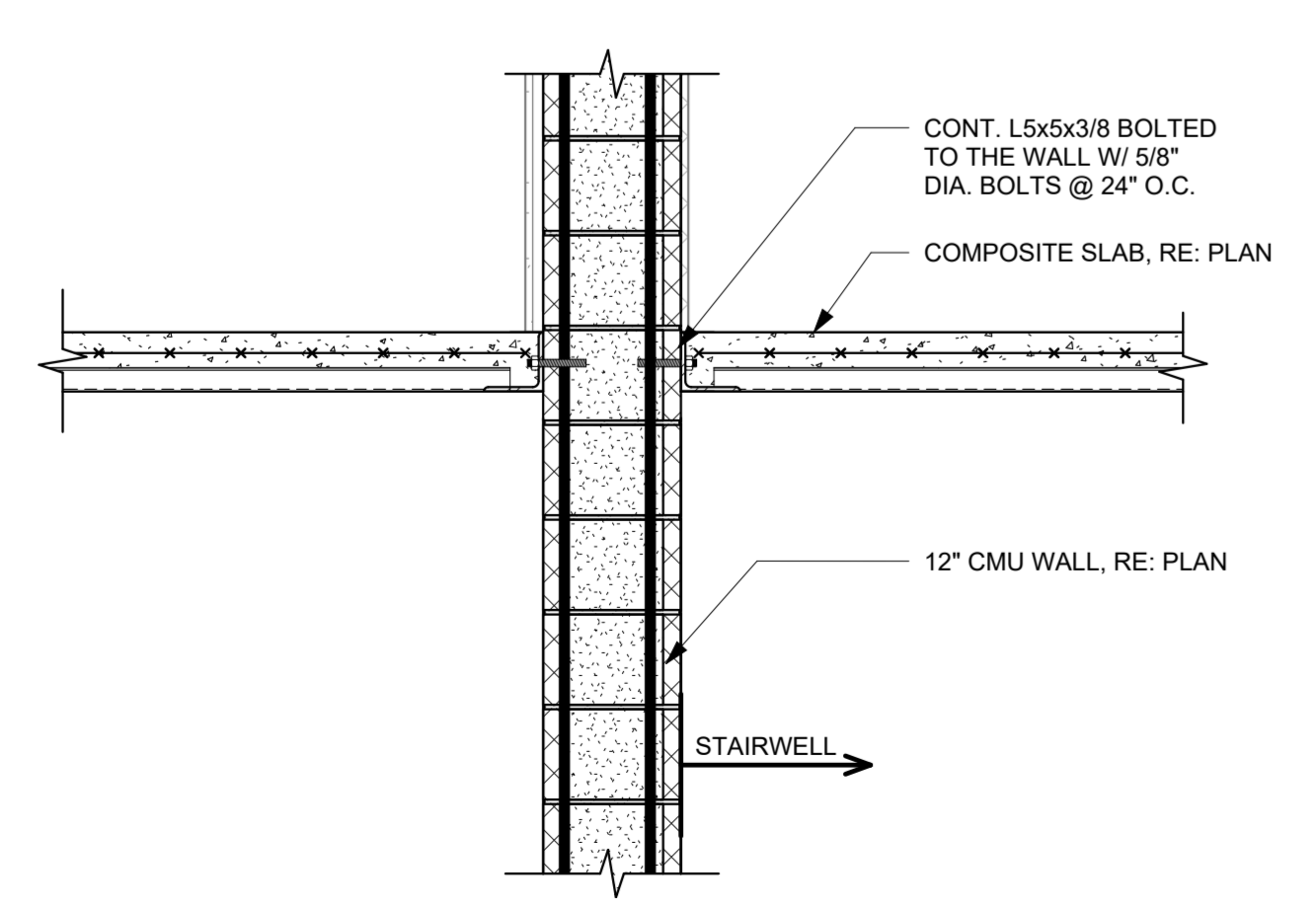
6 FFS - SECTION
3/4" = 1'-0"



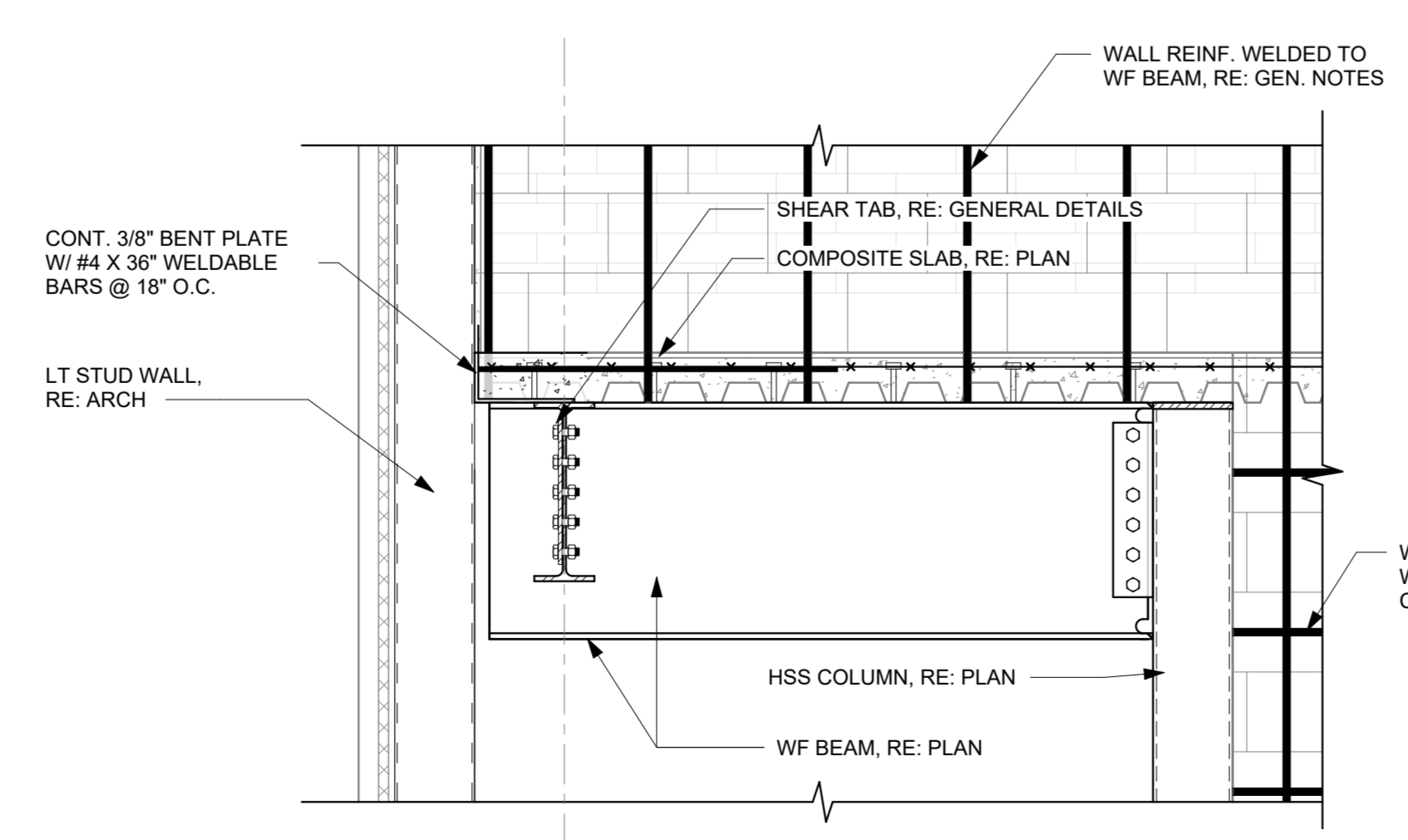
7 FFS - SECTION
3/4" = 1'-0"



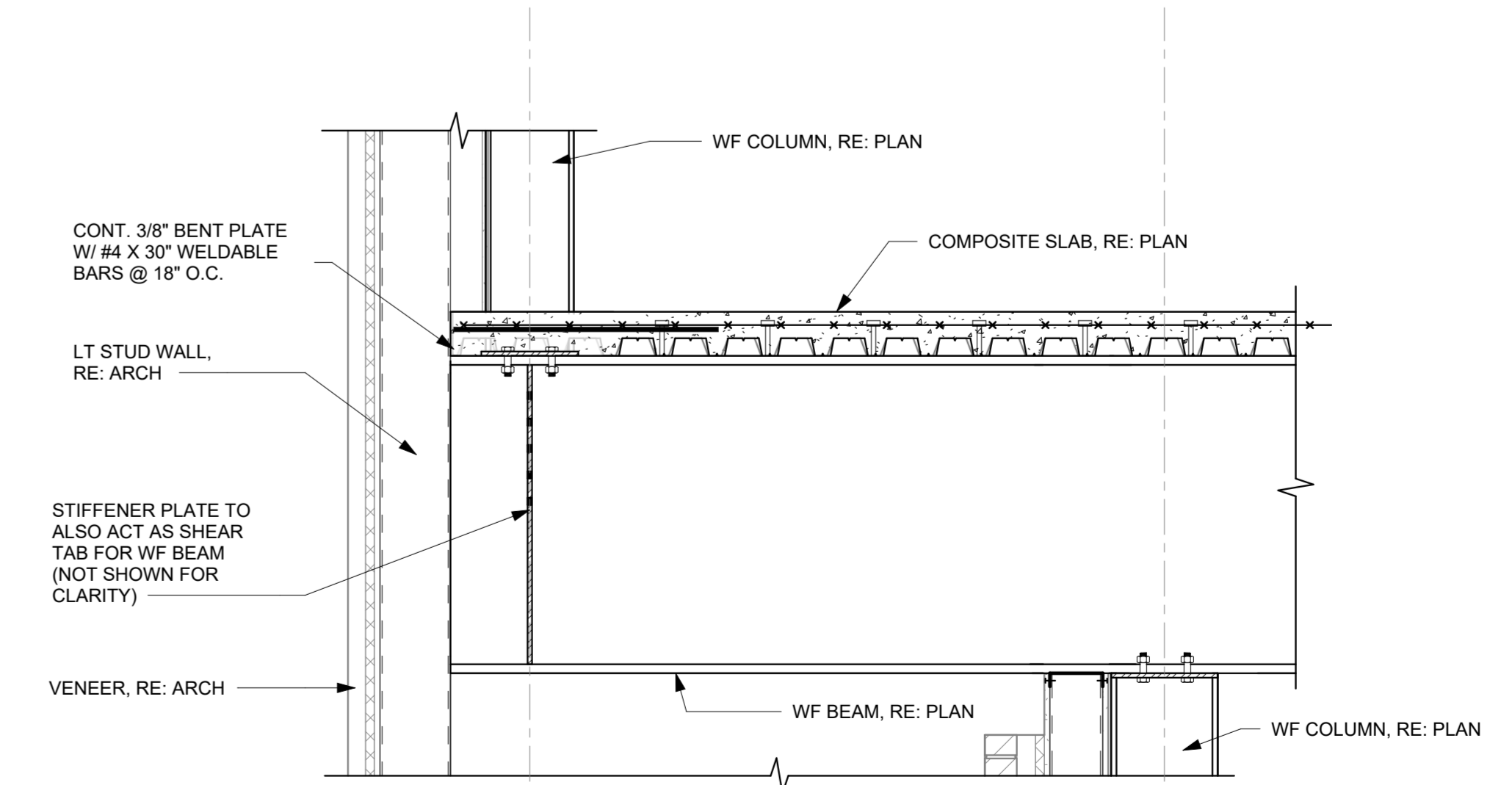
8 FFS - SECTION
3/4" = 1'-0"



9 FFS - SECTION
3/4" = 1'-0"

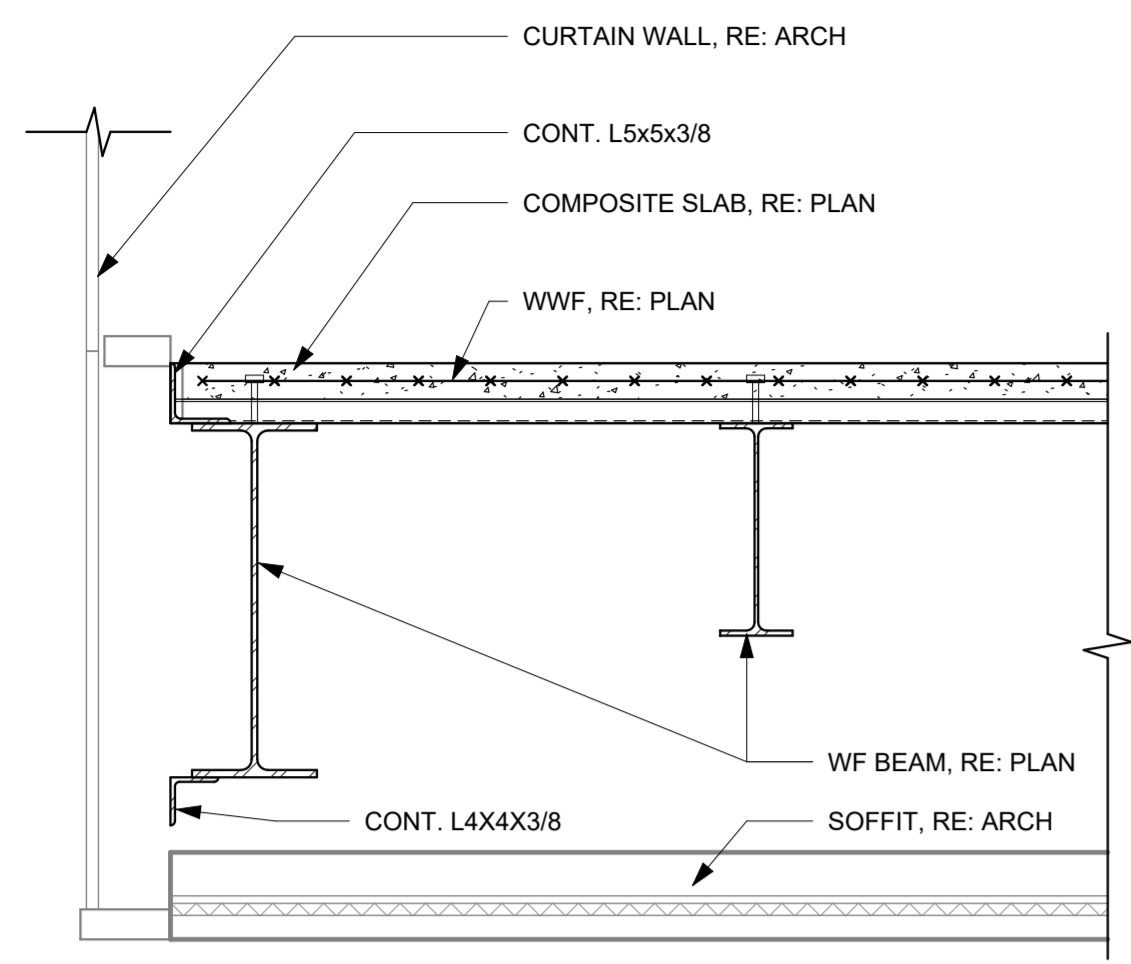


10 FFS - SECTION
3/4" = 1'-0"

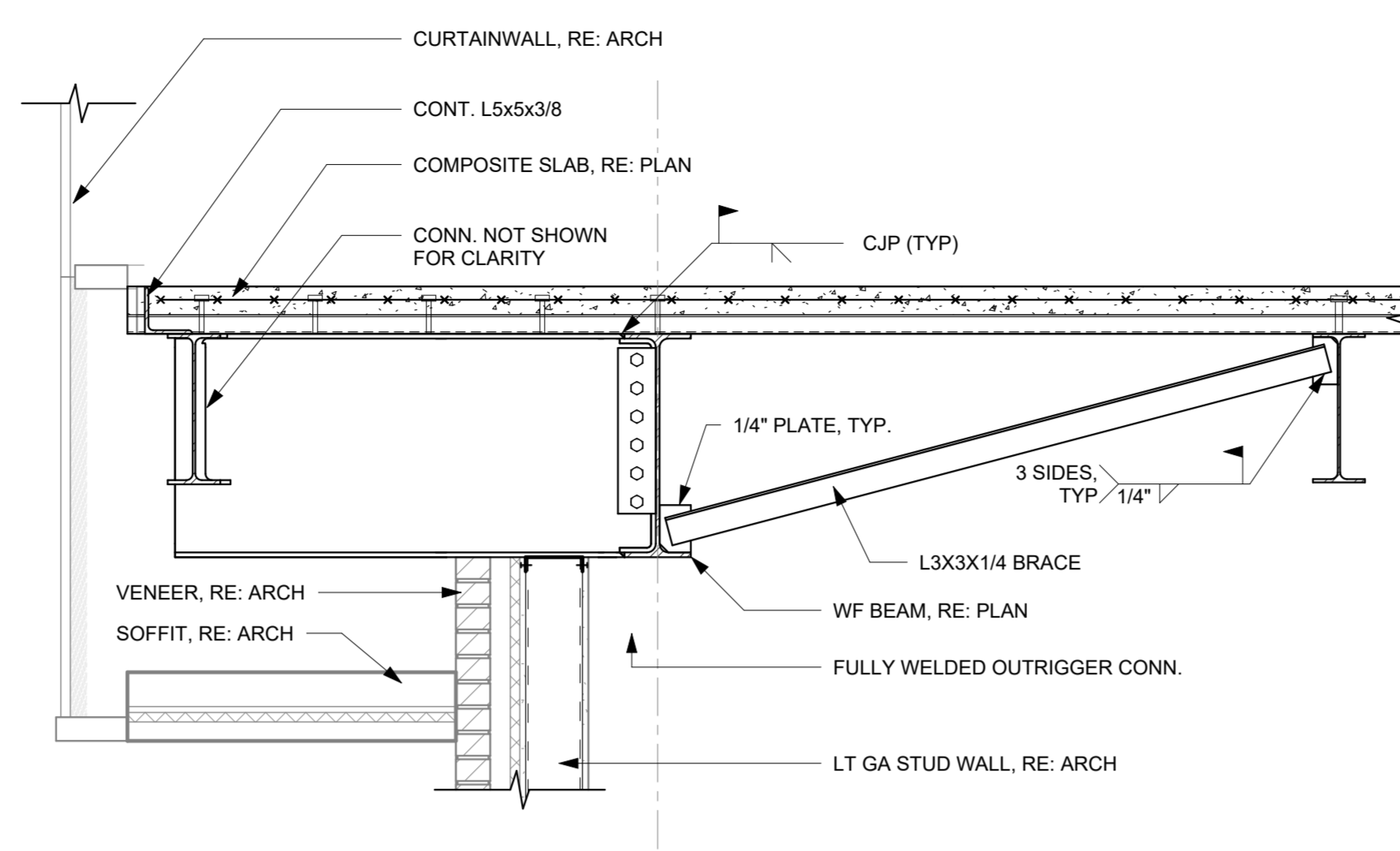


11 FFS - SECTION
3/4" = 1'-0"

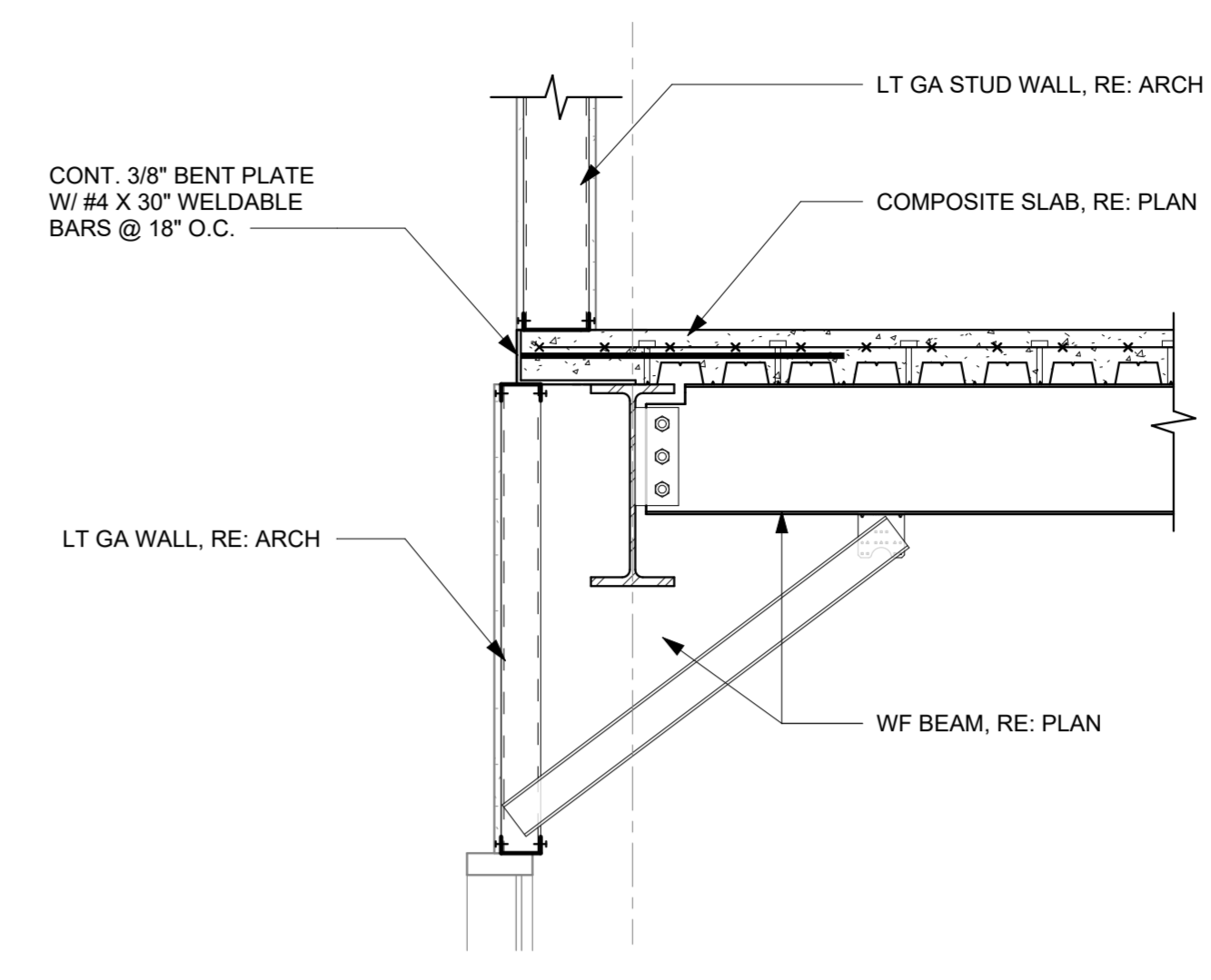




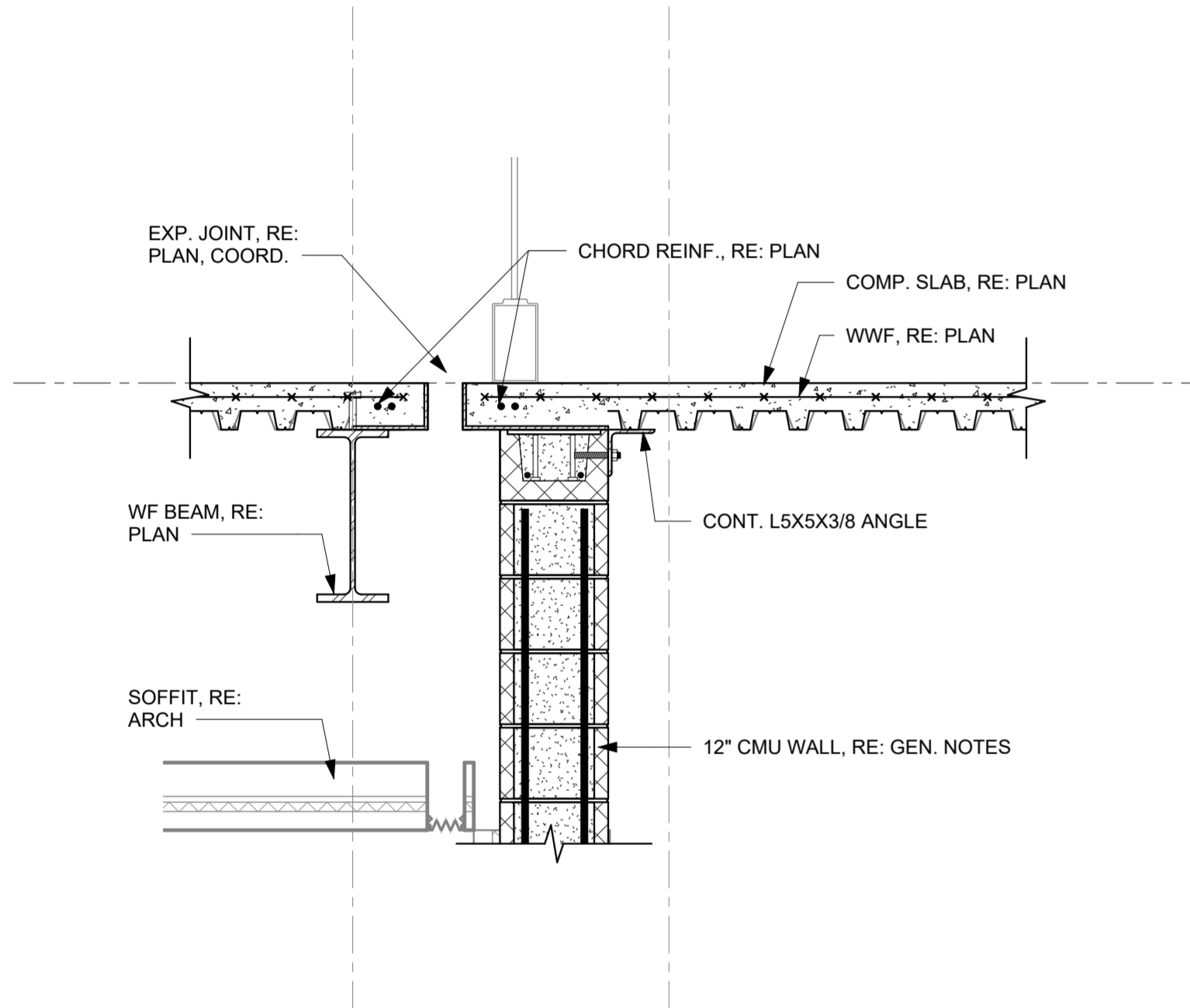
1 FFS - SECTION
3/4" = 1'-0"



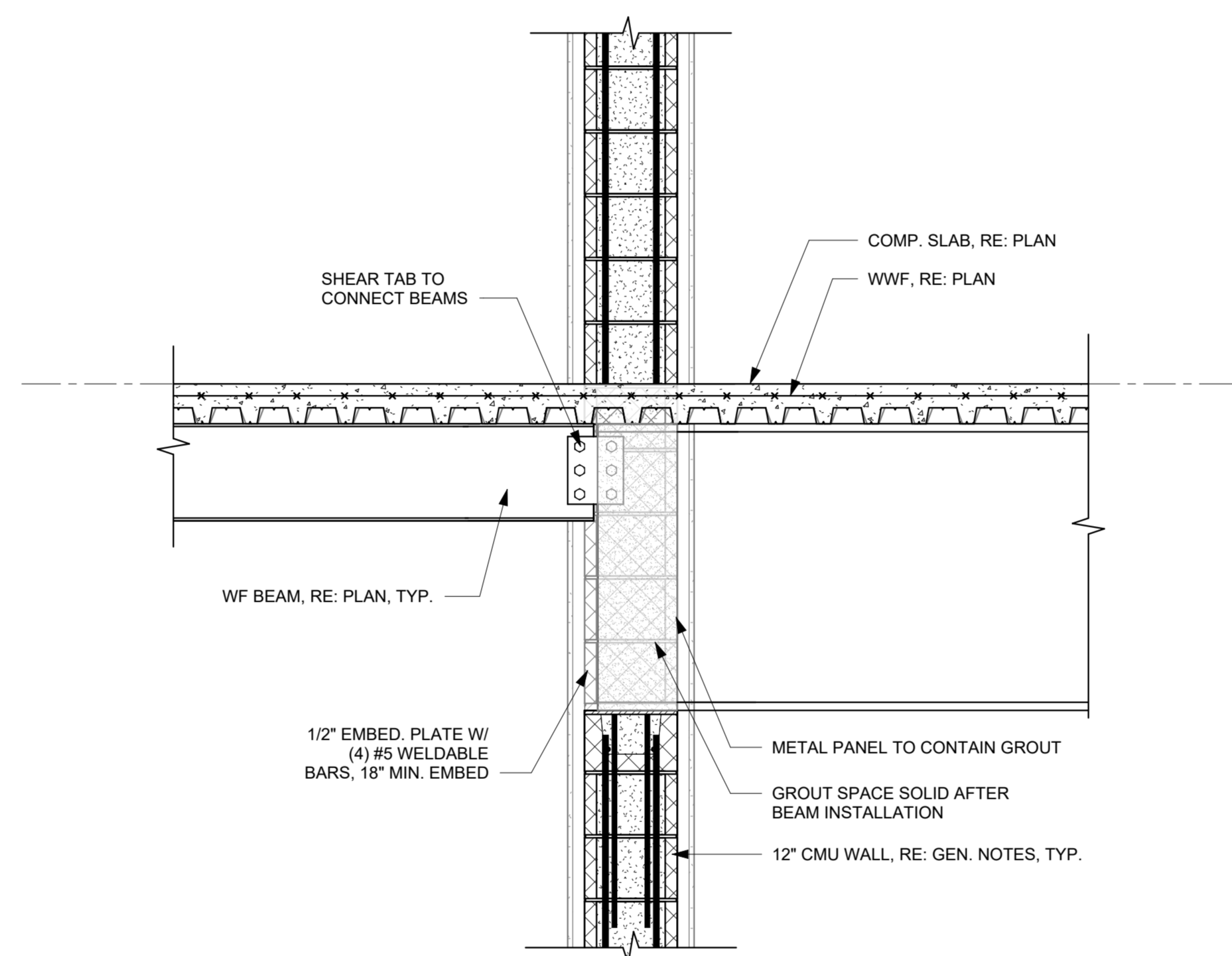
2 FFS - SECTION
3/4" = 1'-0"



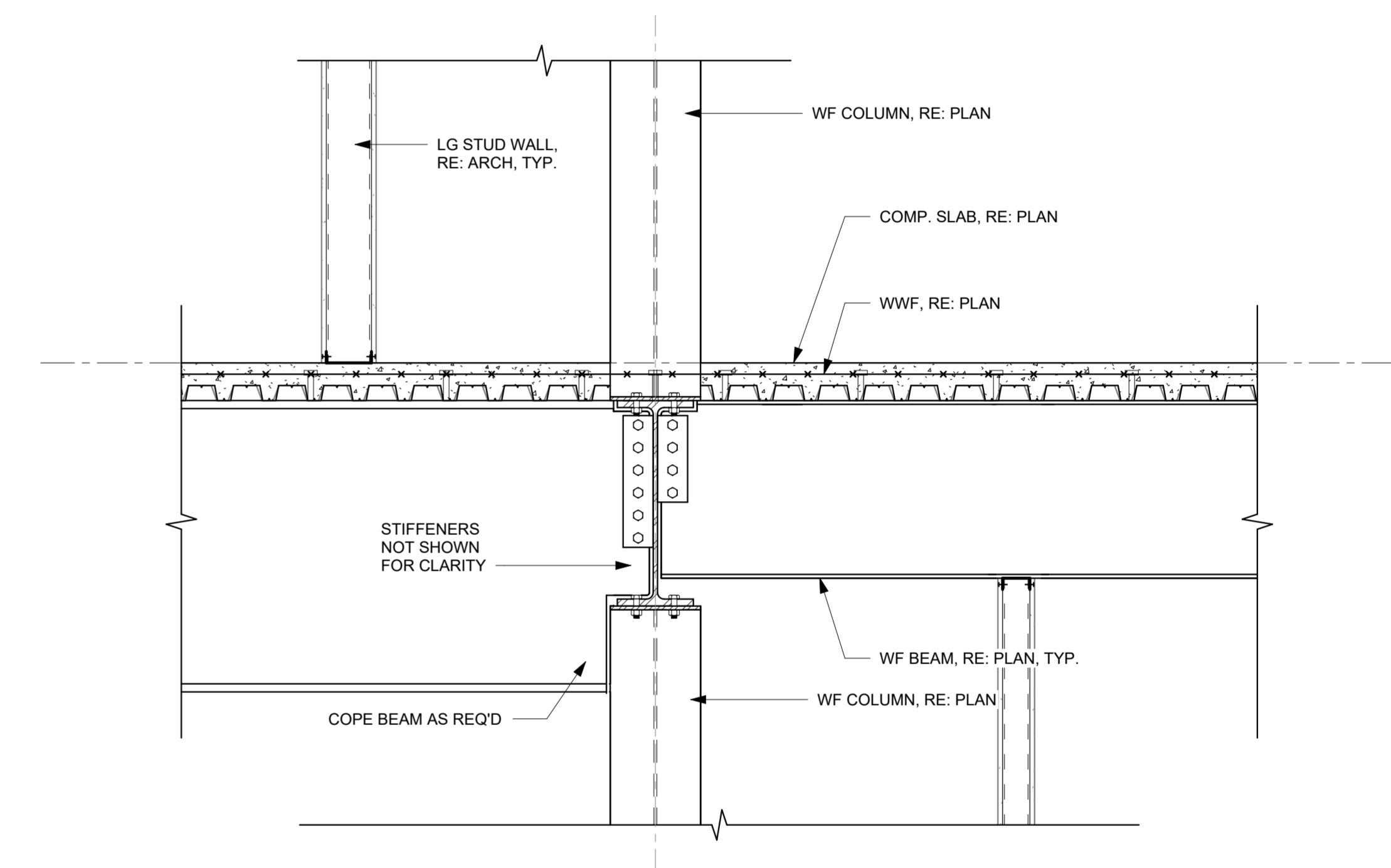
3 FFS - SECTION
3/4" = 1'-0"



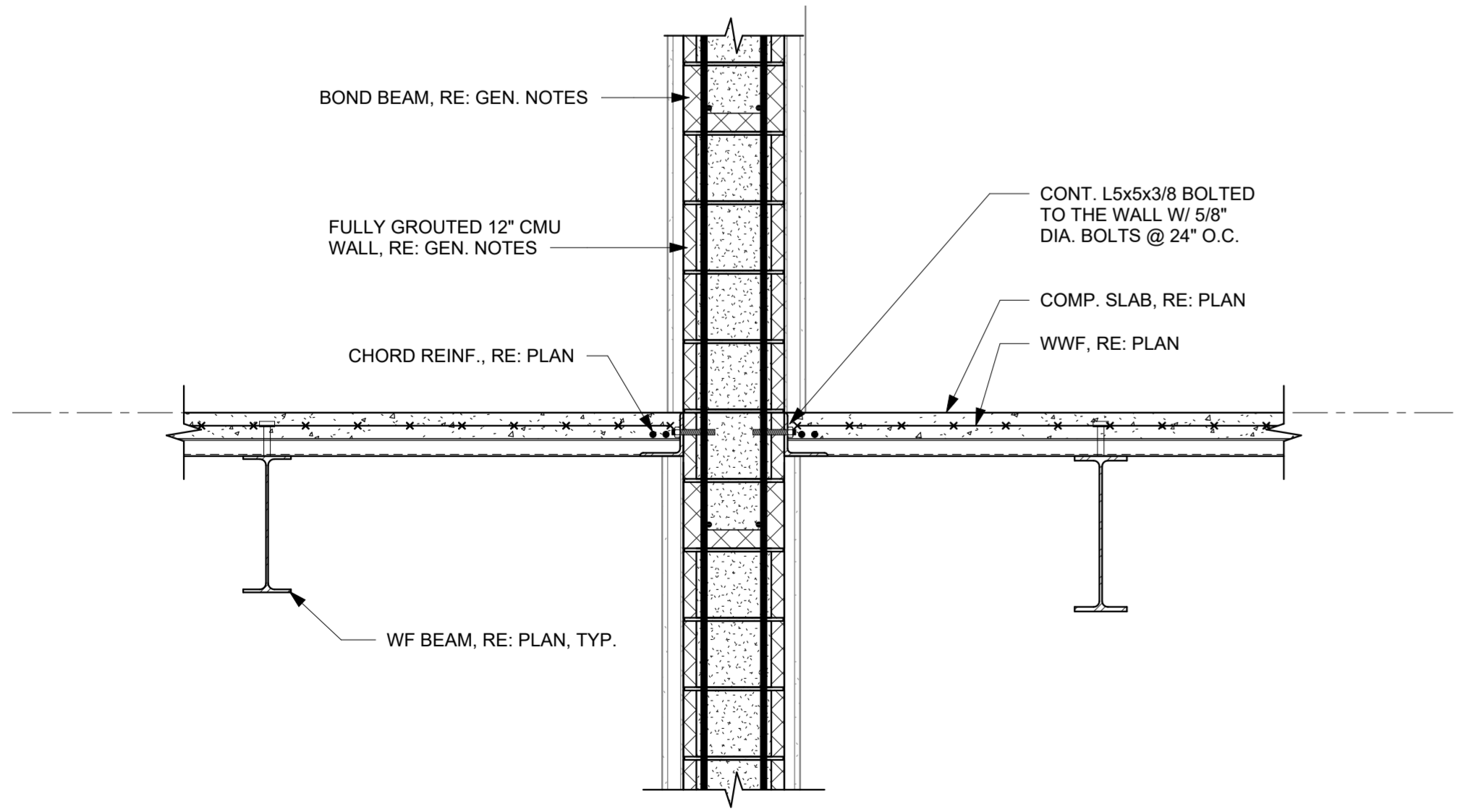
4 FFS - SECTION
3/4" = 1'-0"



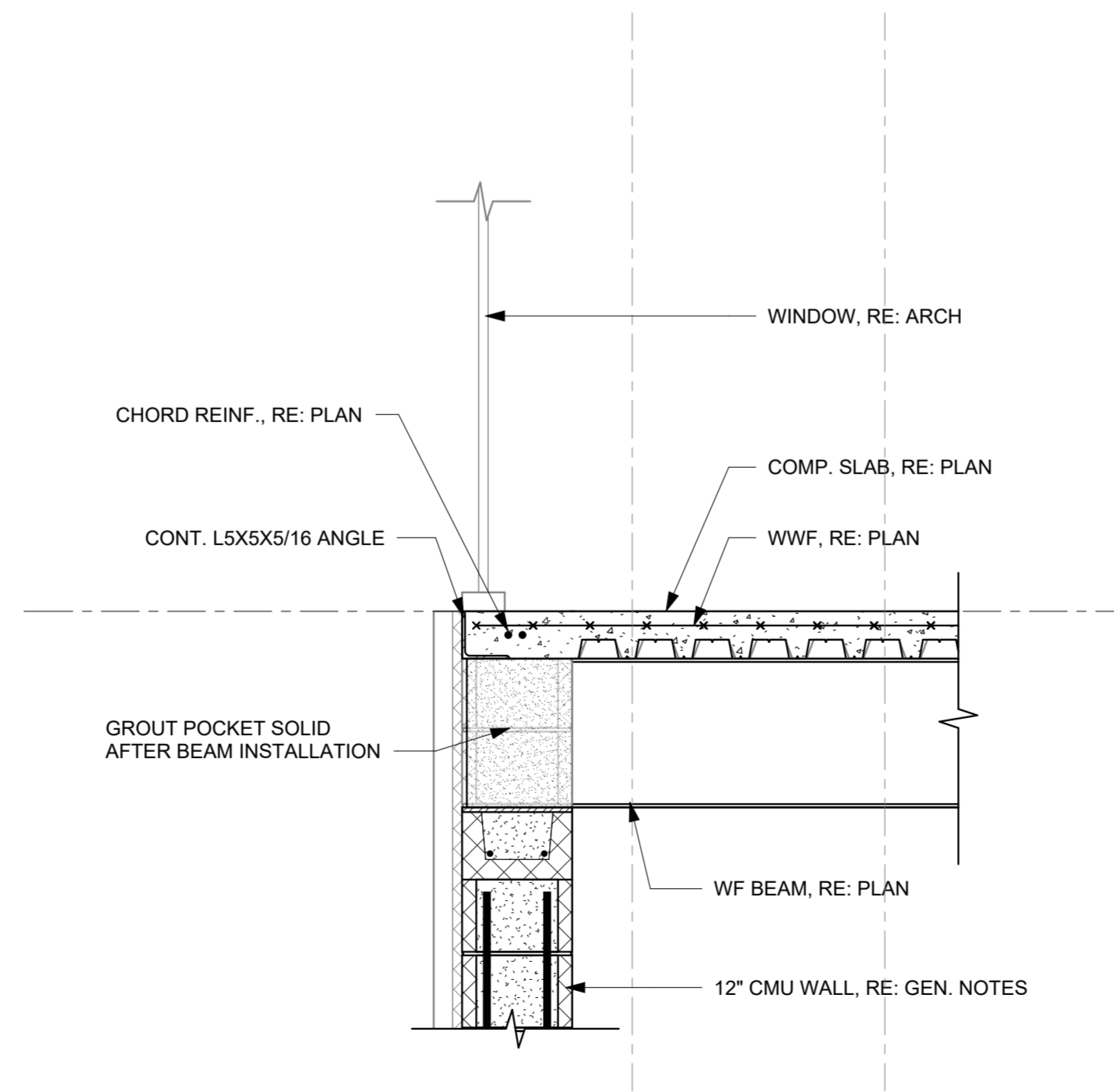
5 FFS - SECTION
3/4" = 1'-0"



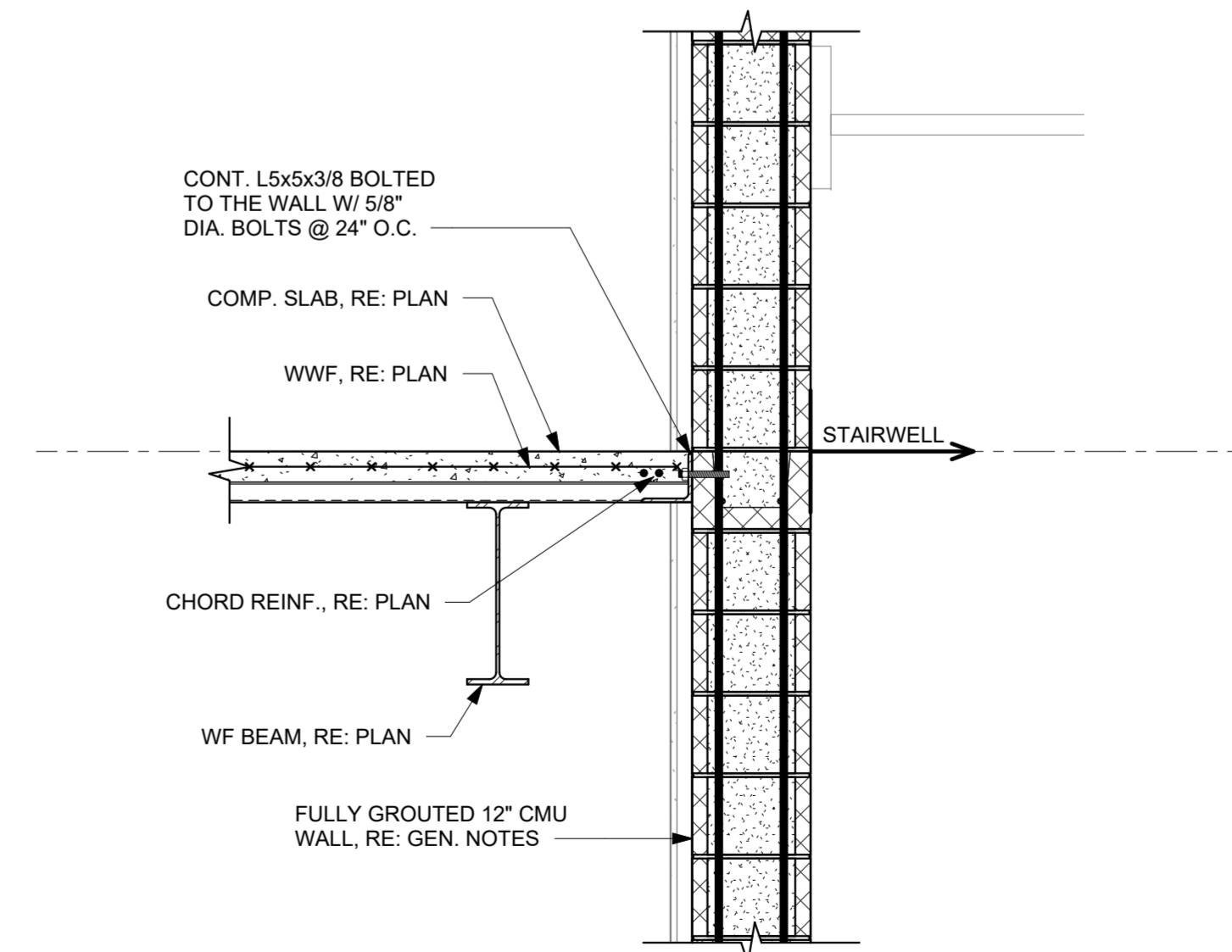
6 FFS - SECTION
3/4" = 1'-0"



7 FFS - SECTION
3/4" = 1'-0"



8 FFS - SECTION
3/4" = 1'-0"



9 FFS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

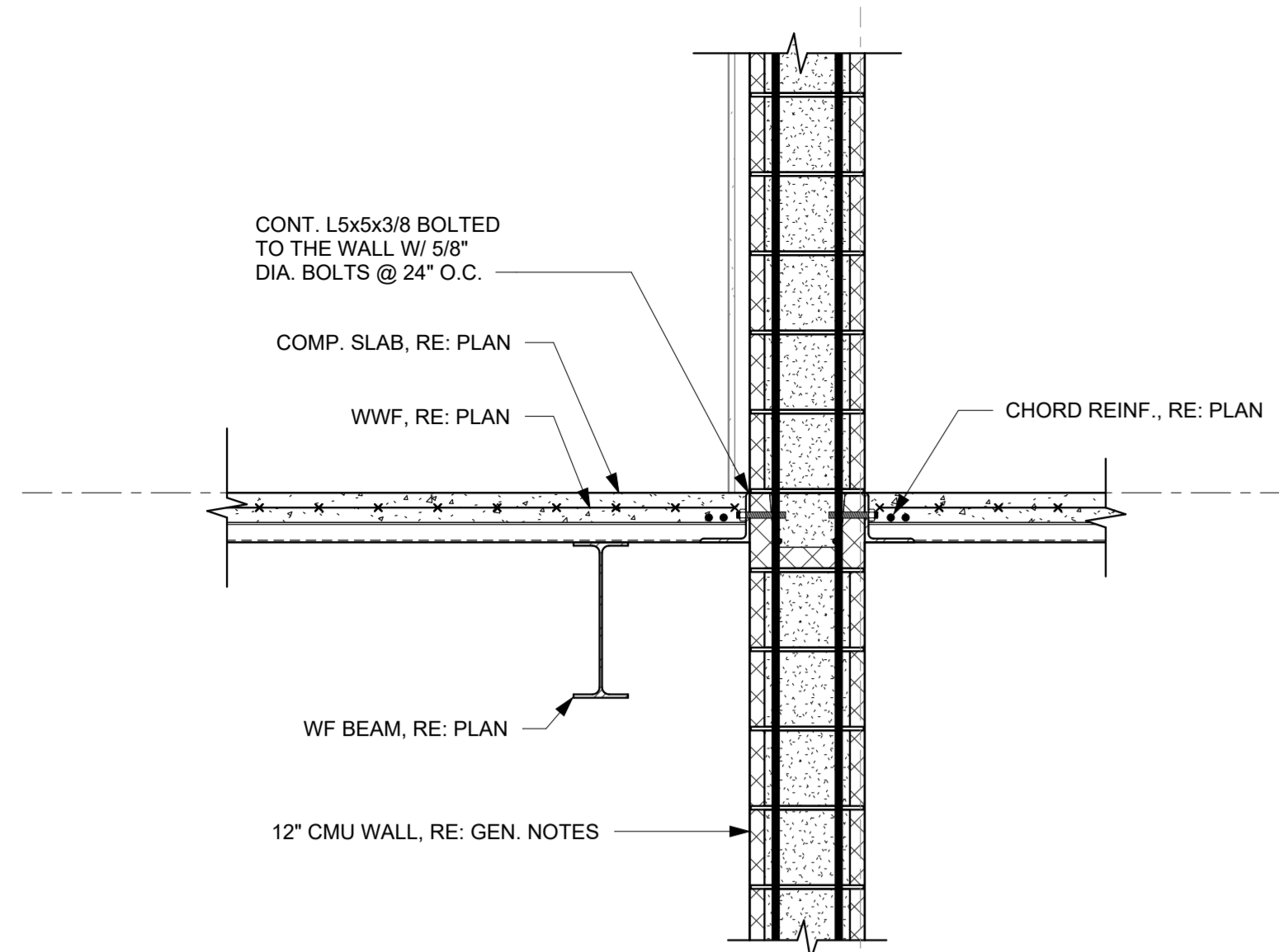
PROJECT NUMBER

-

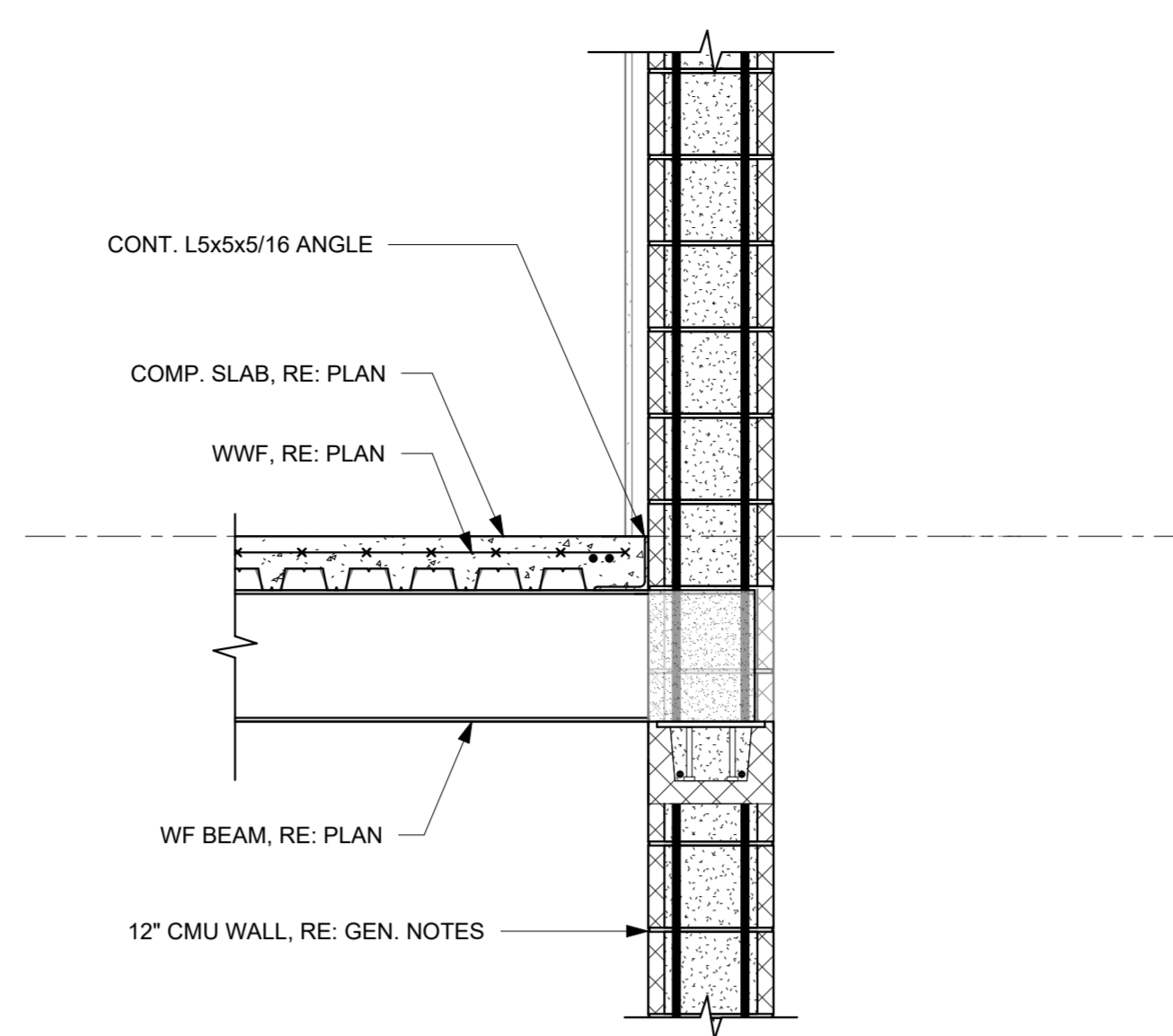
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

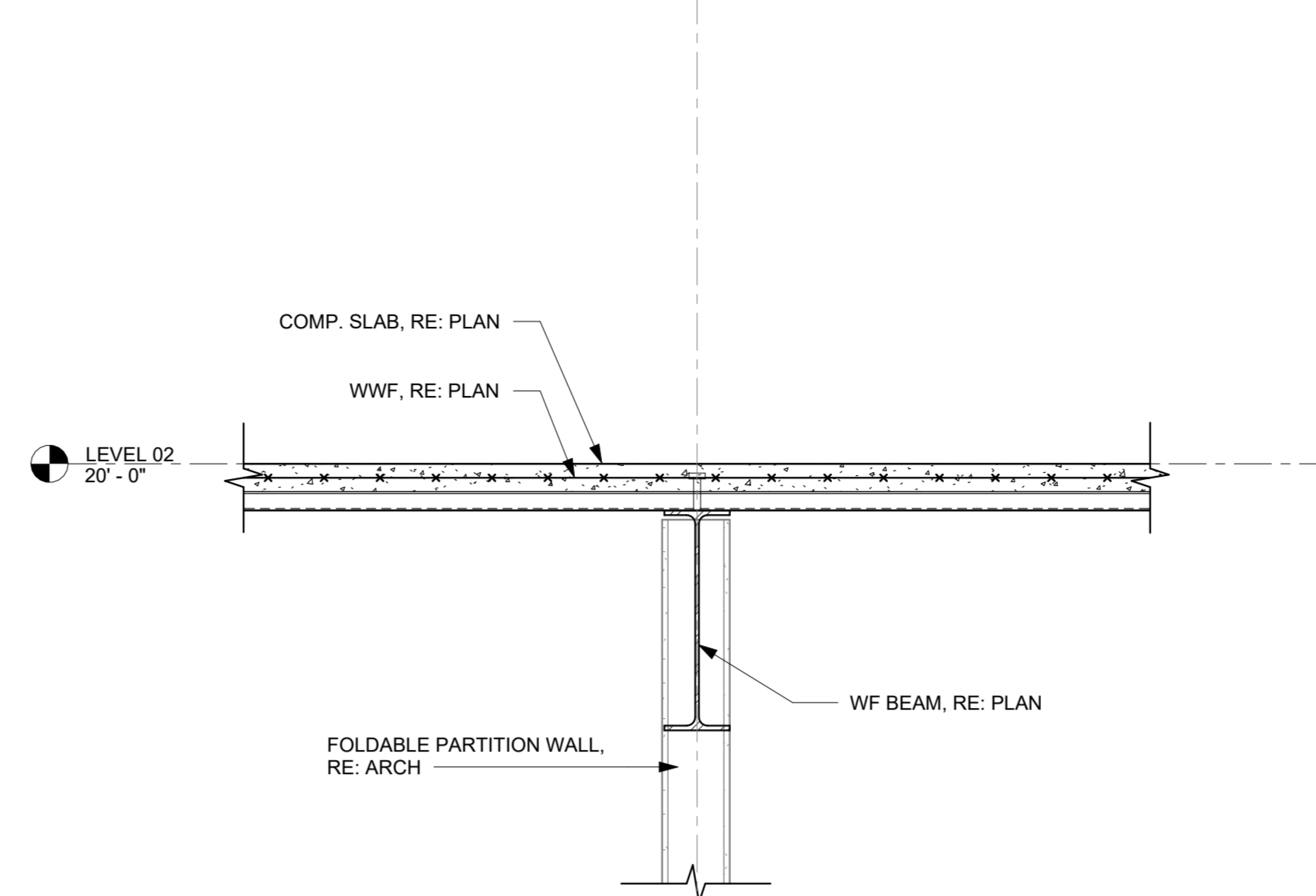
INFORMATION



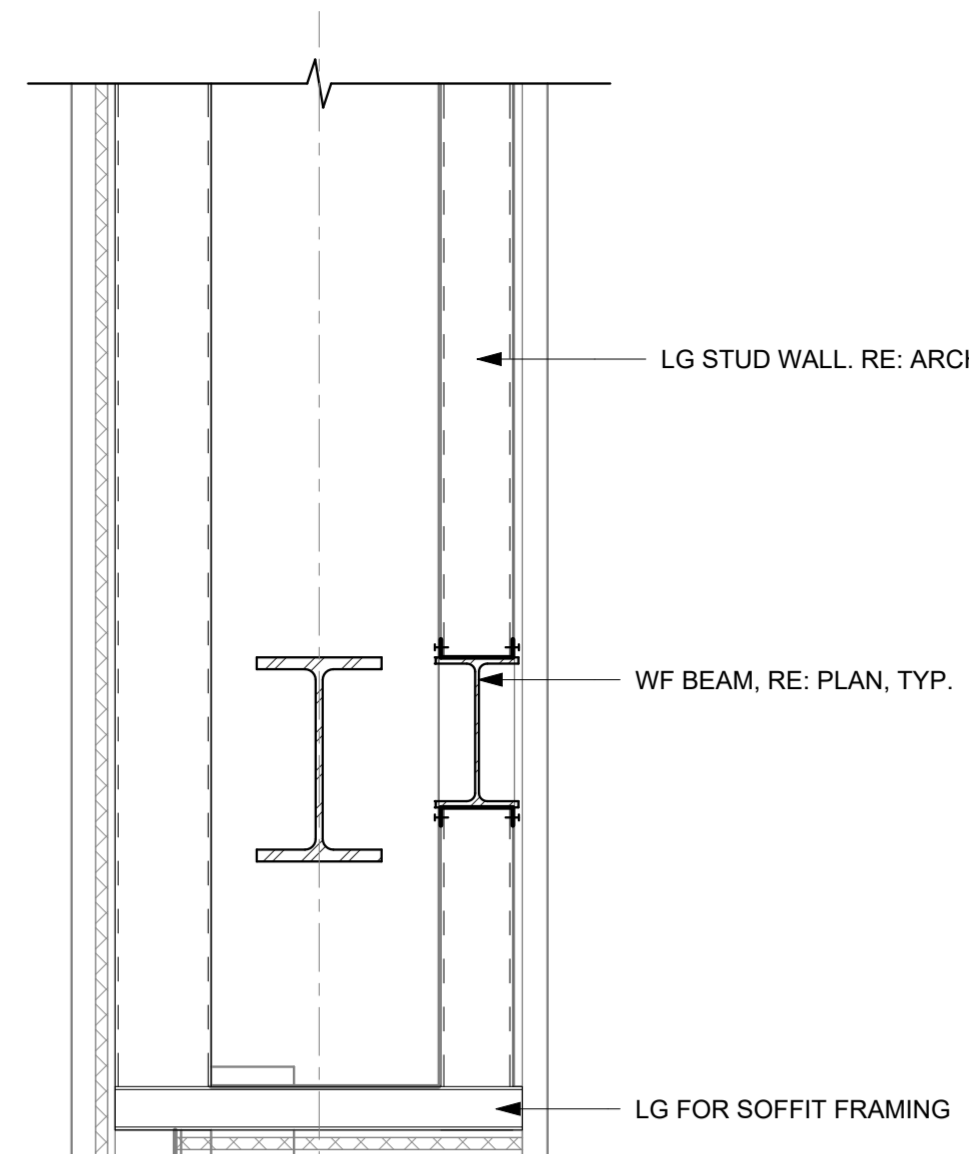
1 FFS - SECTION
3/4" = 1'-0"



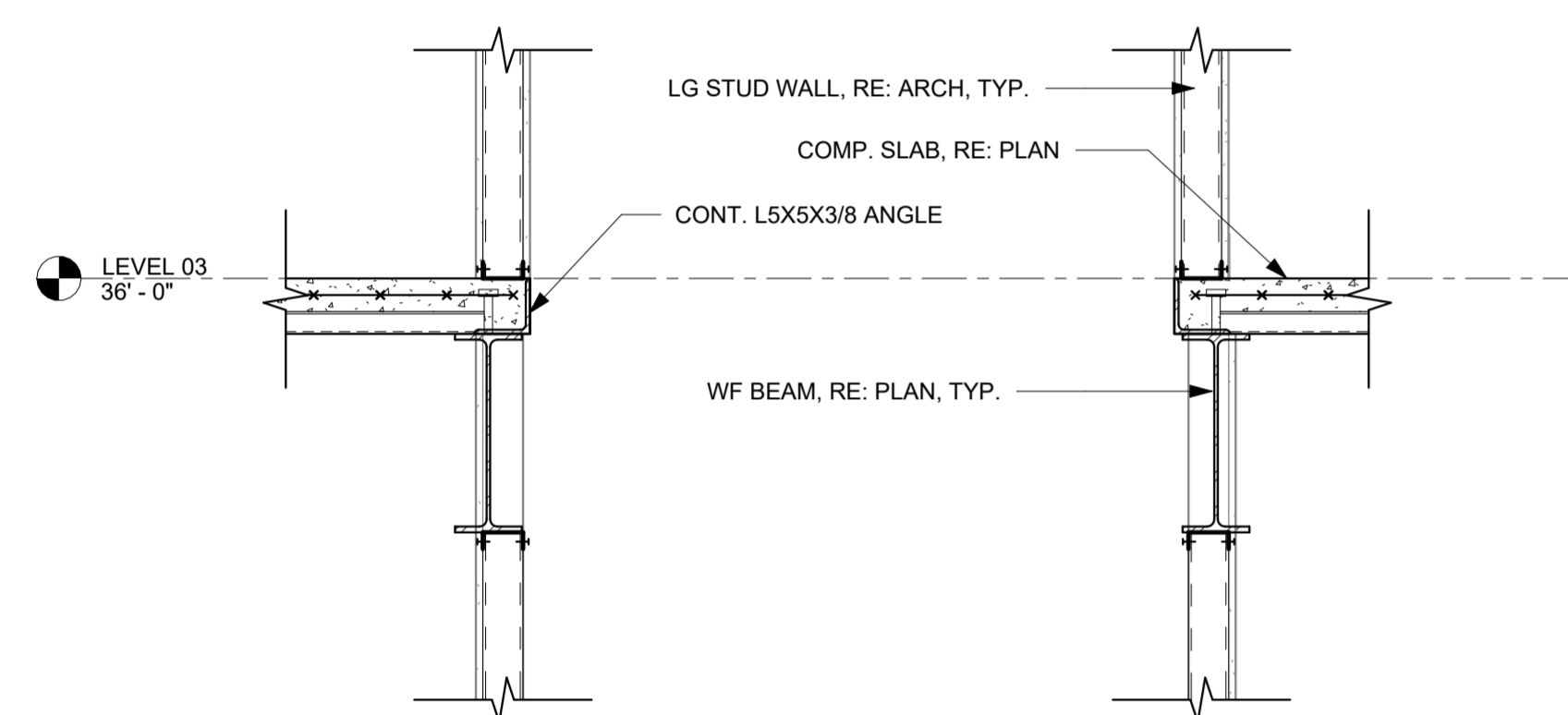
2 FFS - SECTION
3/4" = 1'-0"



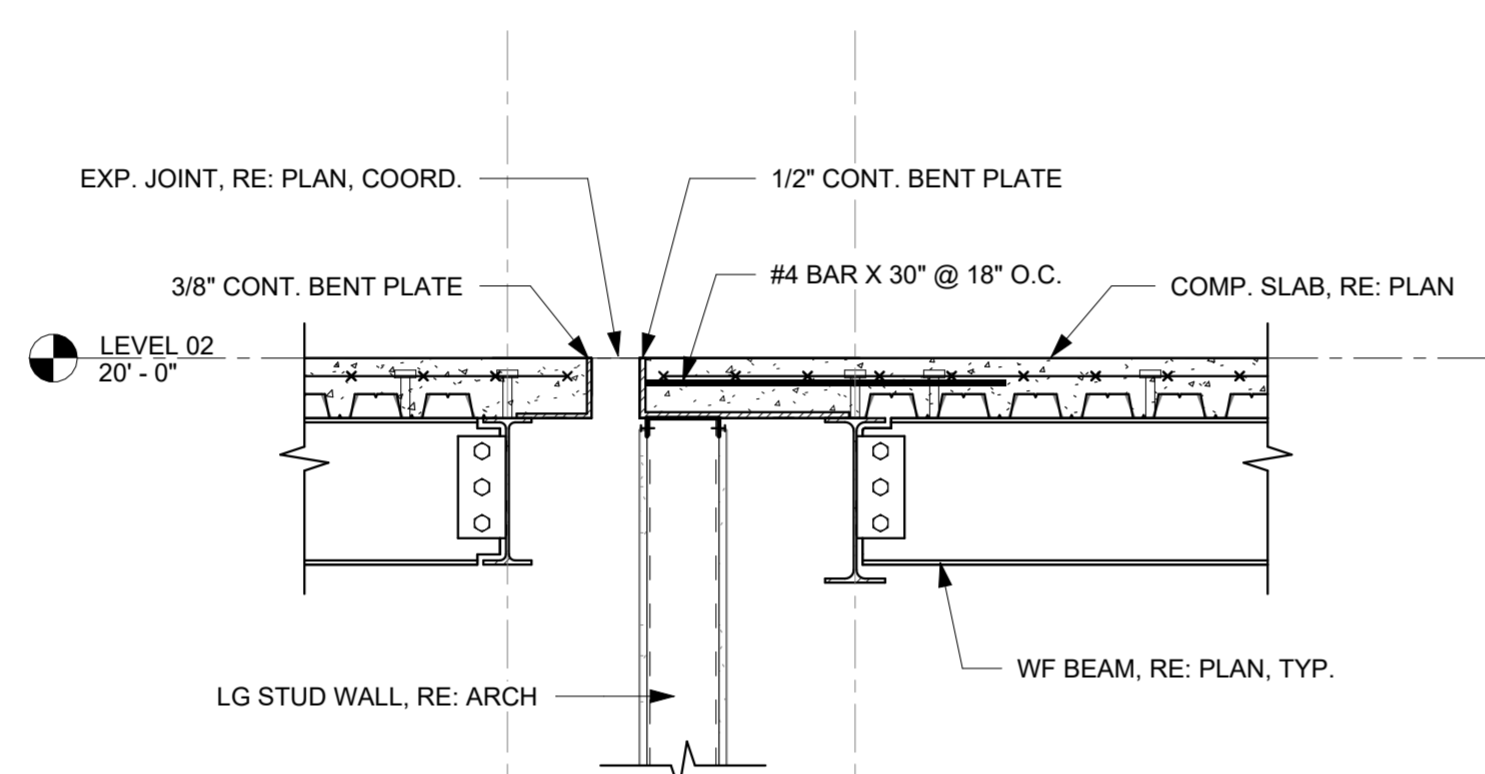
3 FFS - SECTION
3/4" = 1'-0"



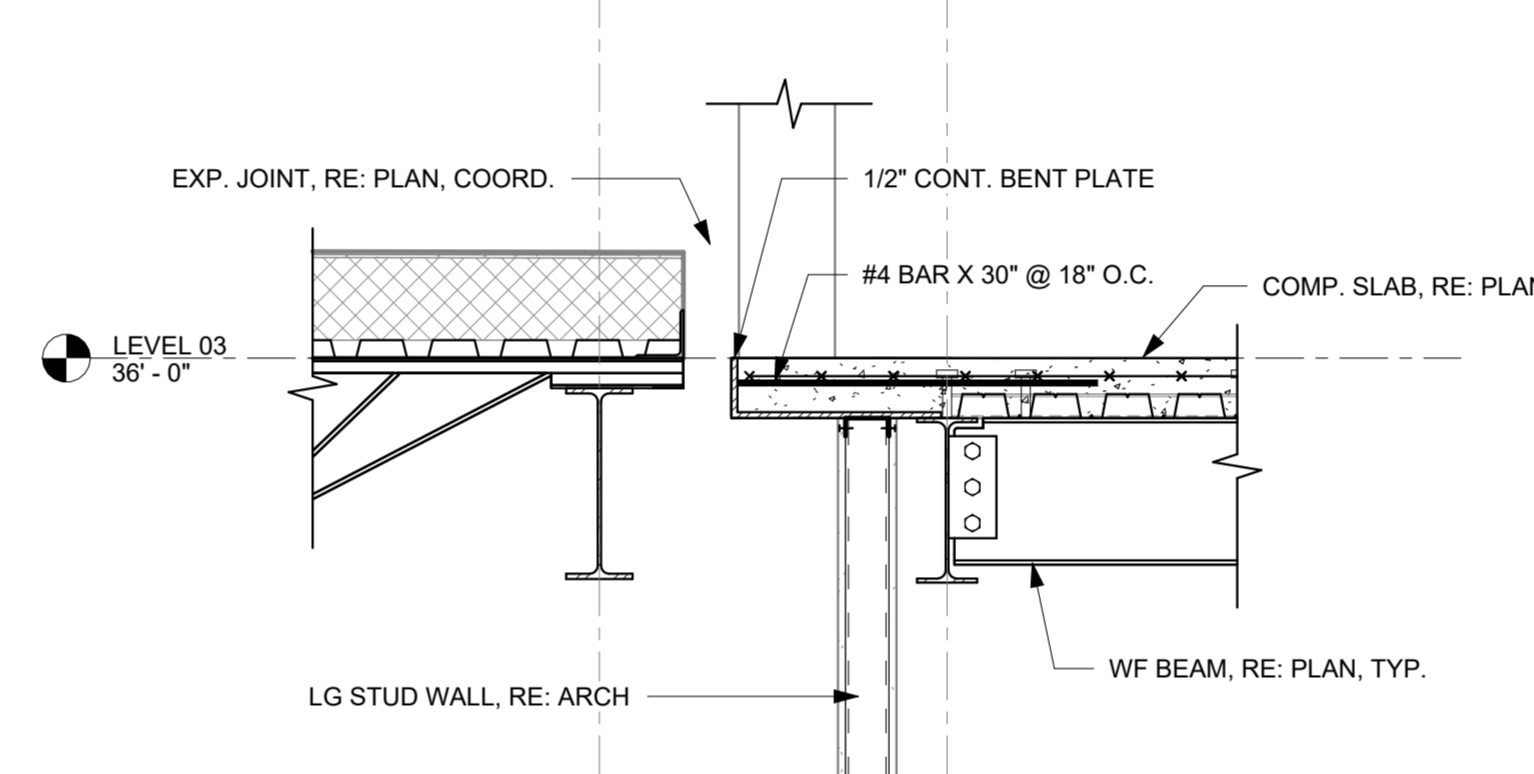
4 FFS - SECTION
3/4" = 1'-0"



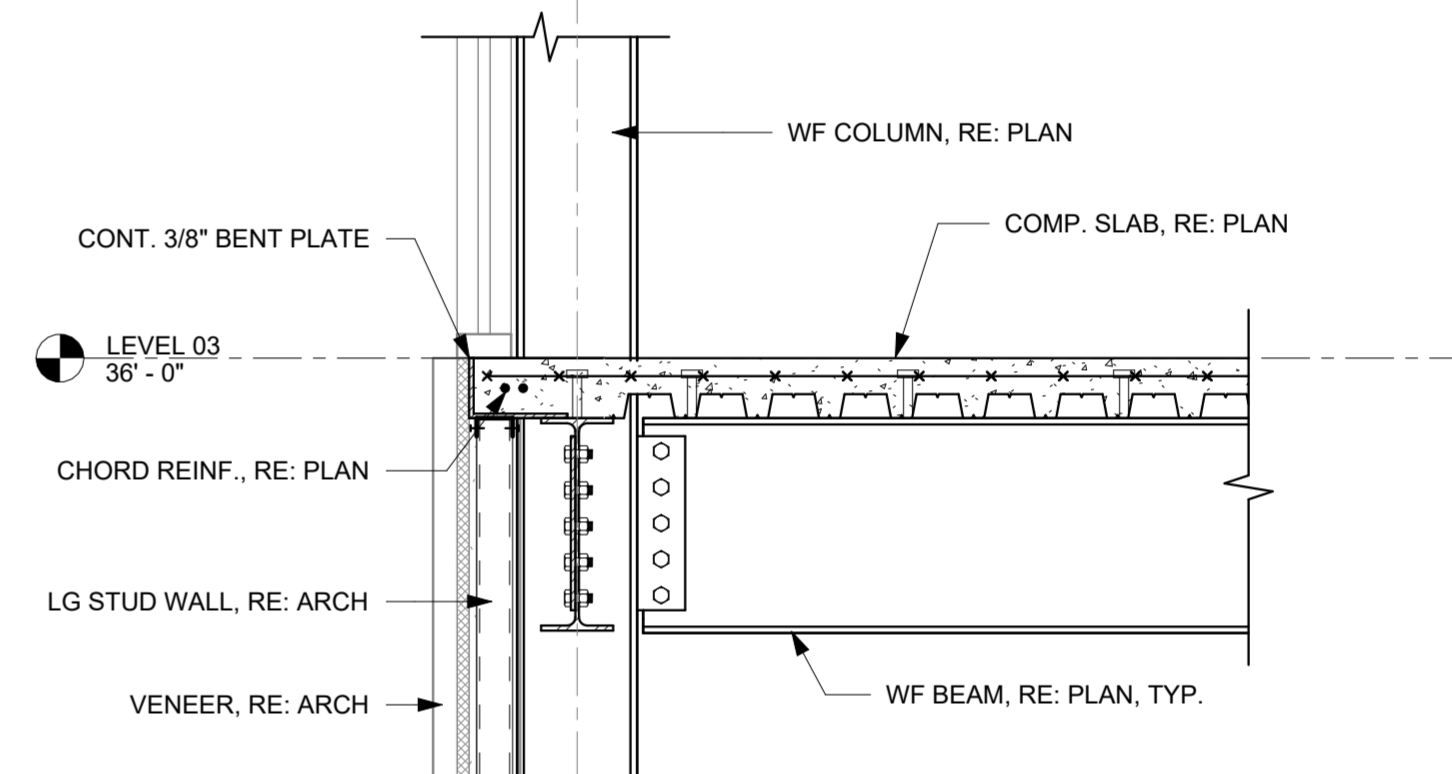
5 FFS - SECTION
3/4" = 1'-0"



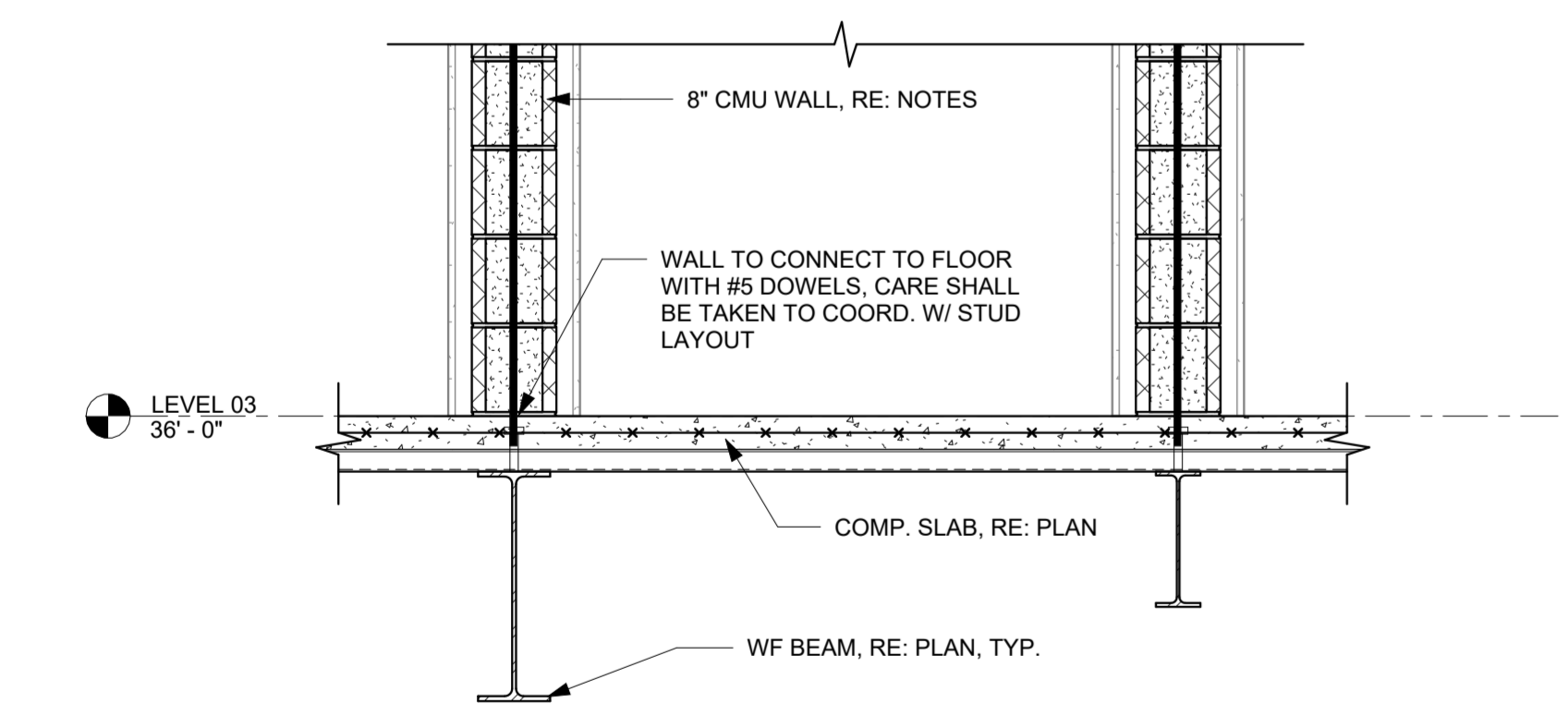
6 FFS - SECTION
3/4" = 1'-0"



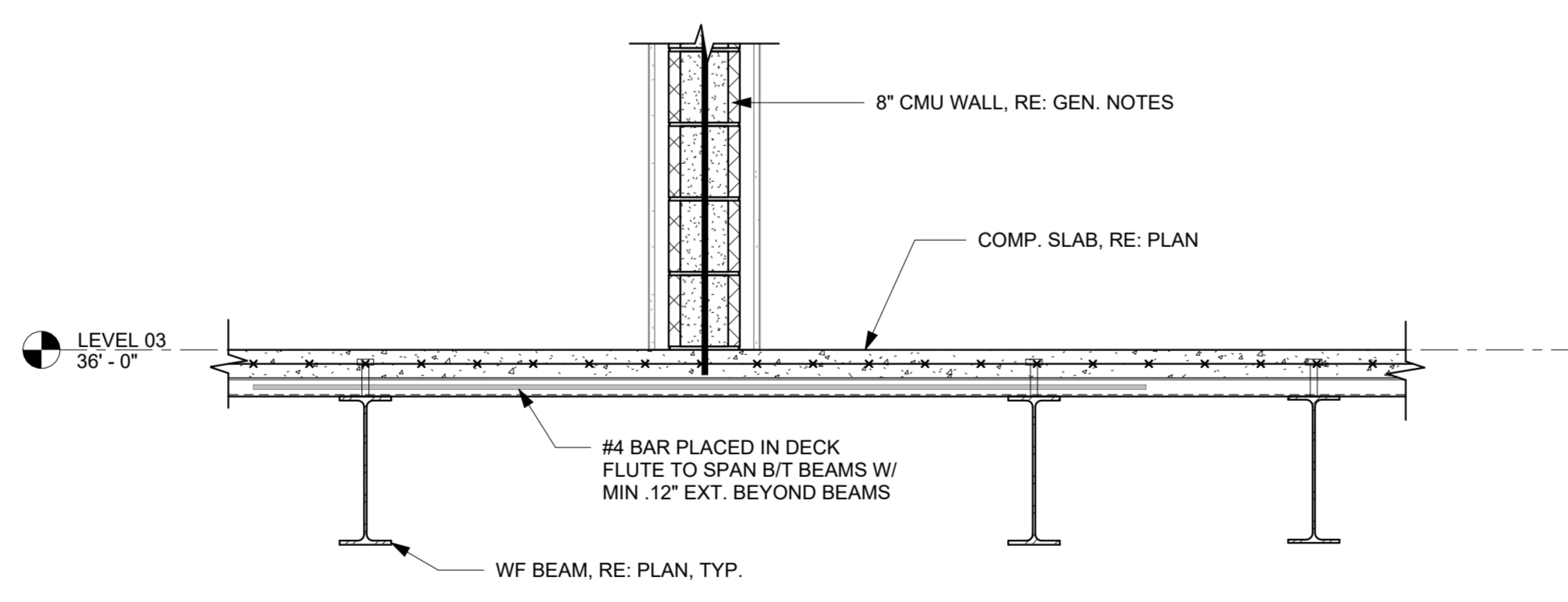
7 FFS - SECTION
3/4" = 1'-0"



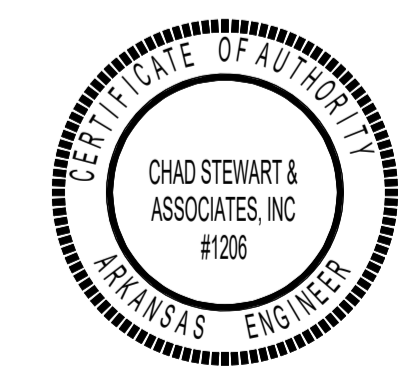
8 FFS - SECTION
3/4" = 1'-0"

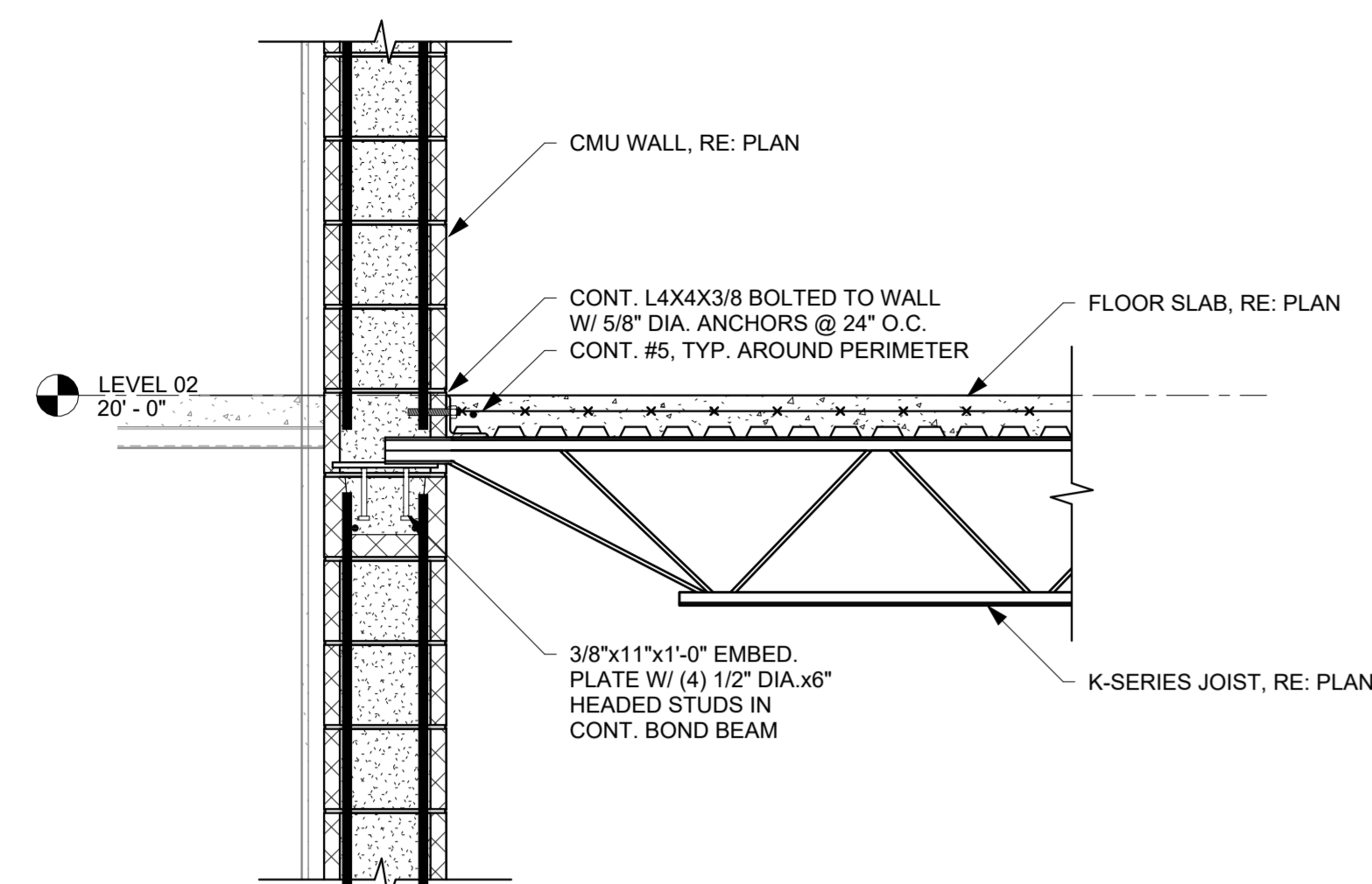


9 FFS - SECTION
3/4" = 1'-0"

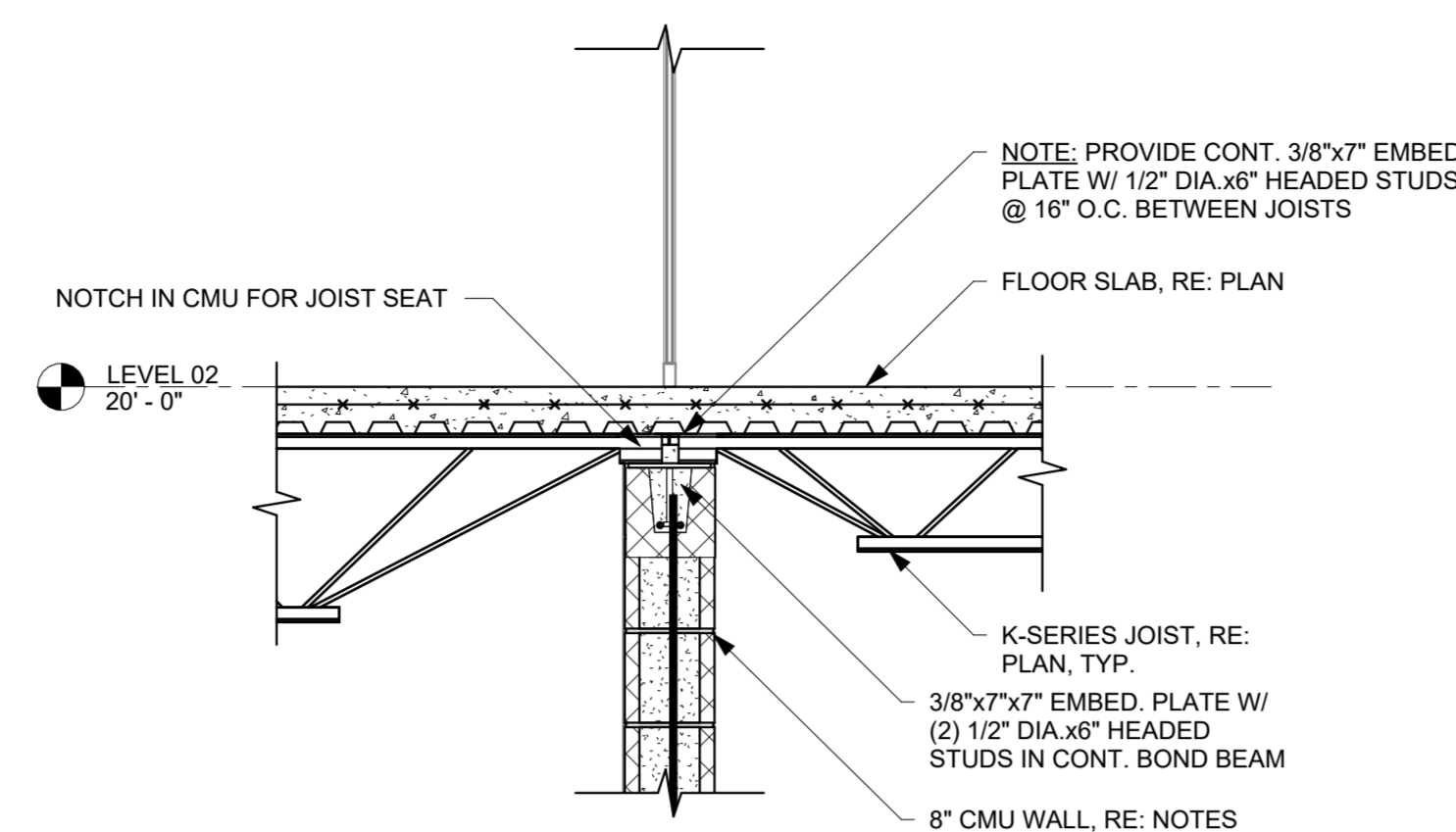


10 FFS - SECTION
3/4" = 1'-0"

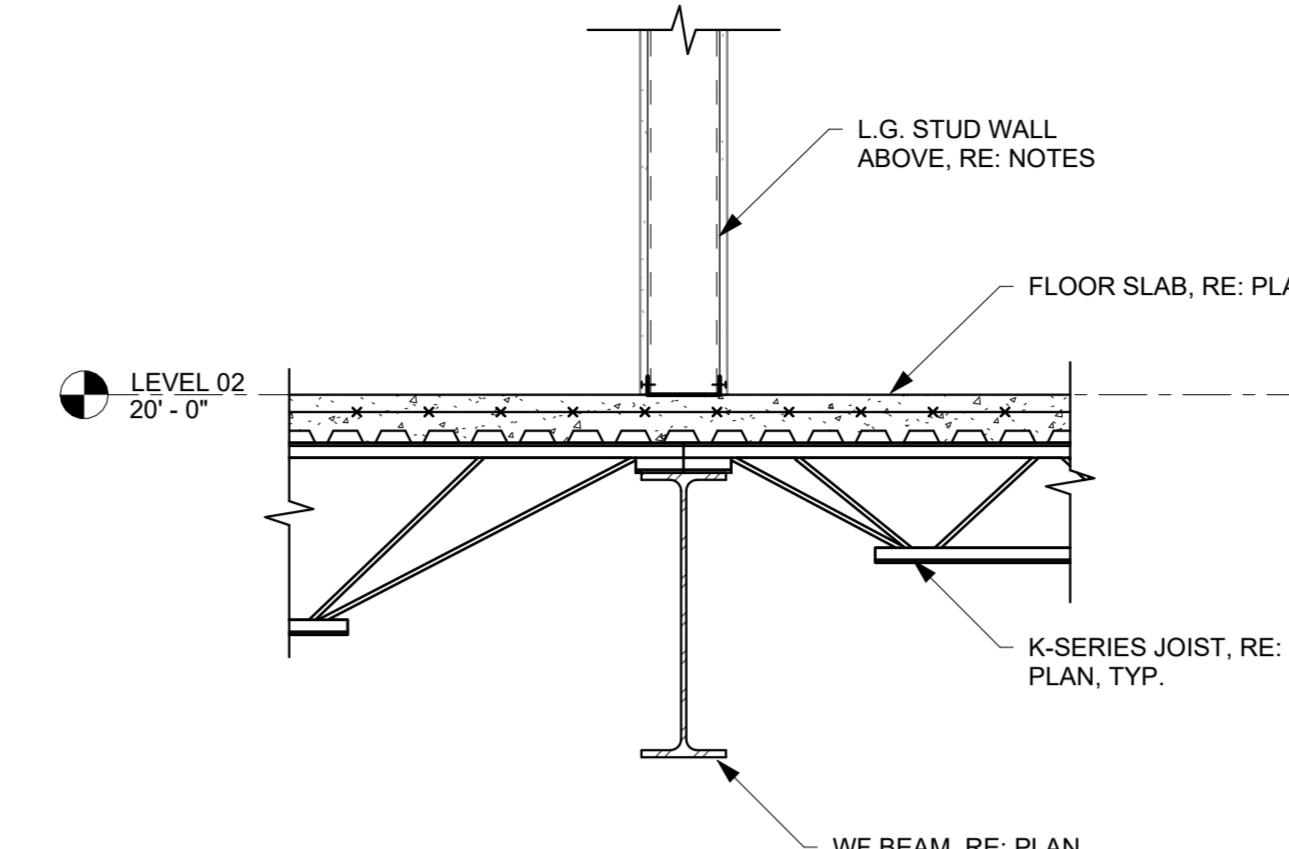




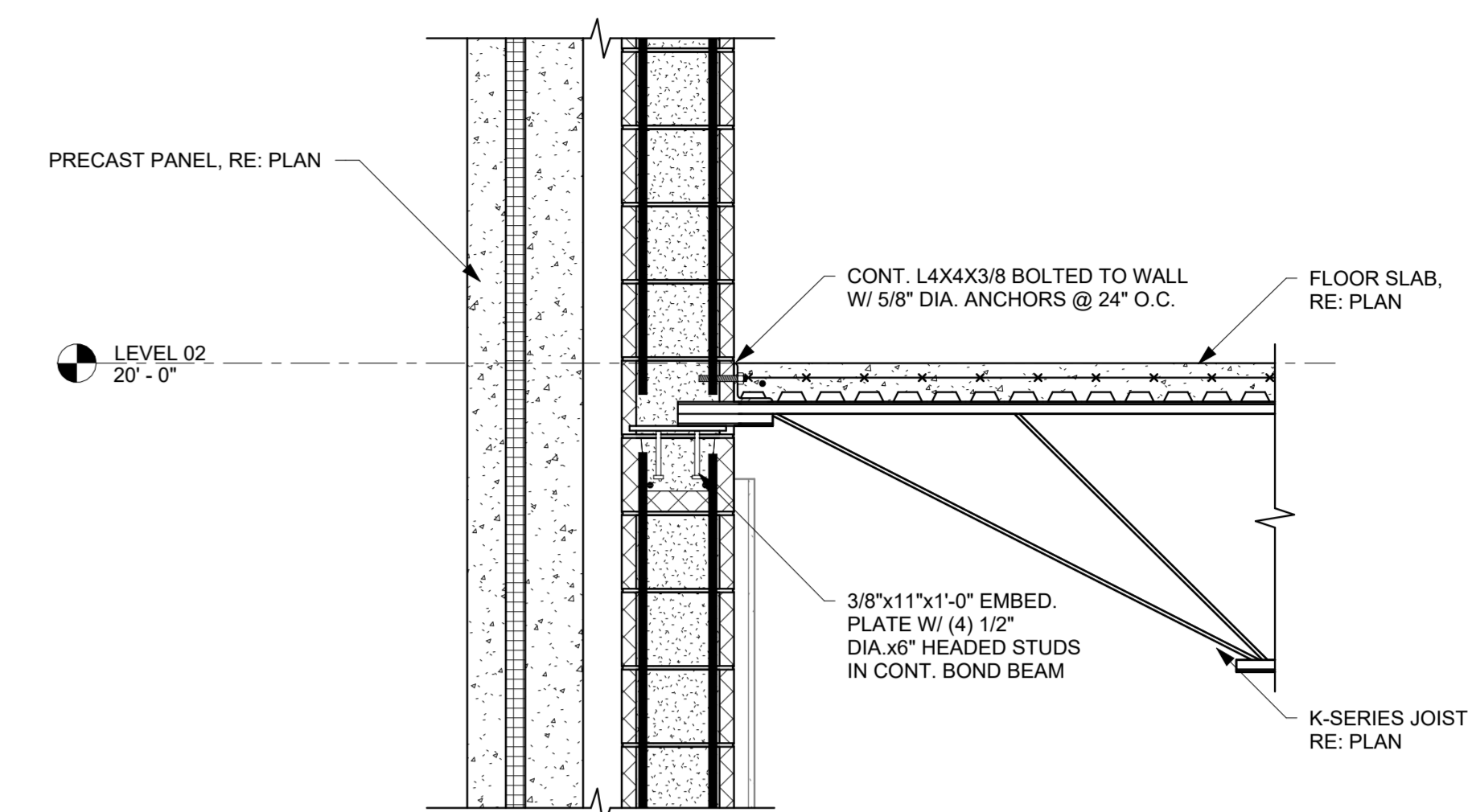
1 FFS - SECTION
3/4" = 1'-0"



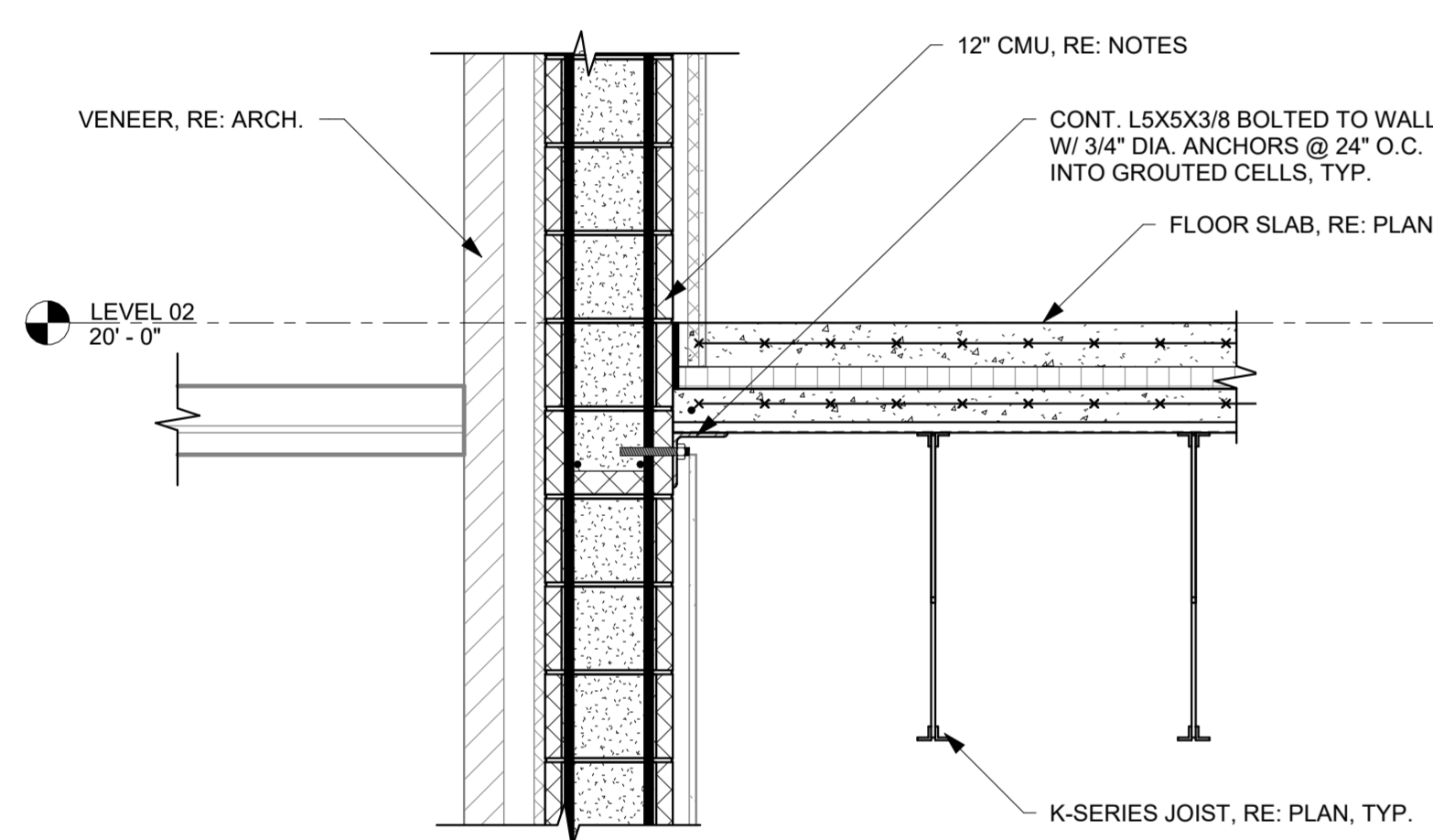
2 FFS - SECTION
3/4" = 1'-0"



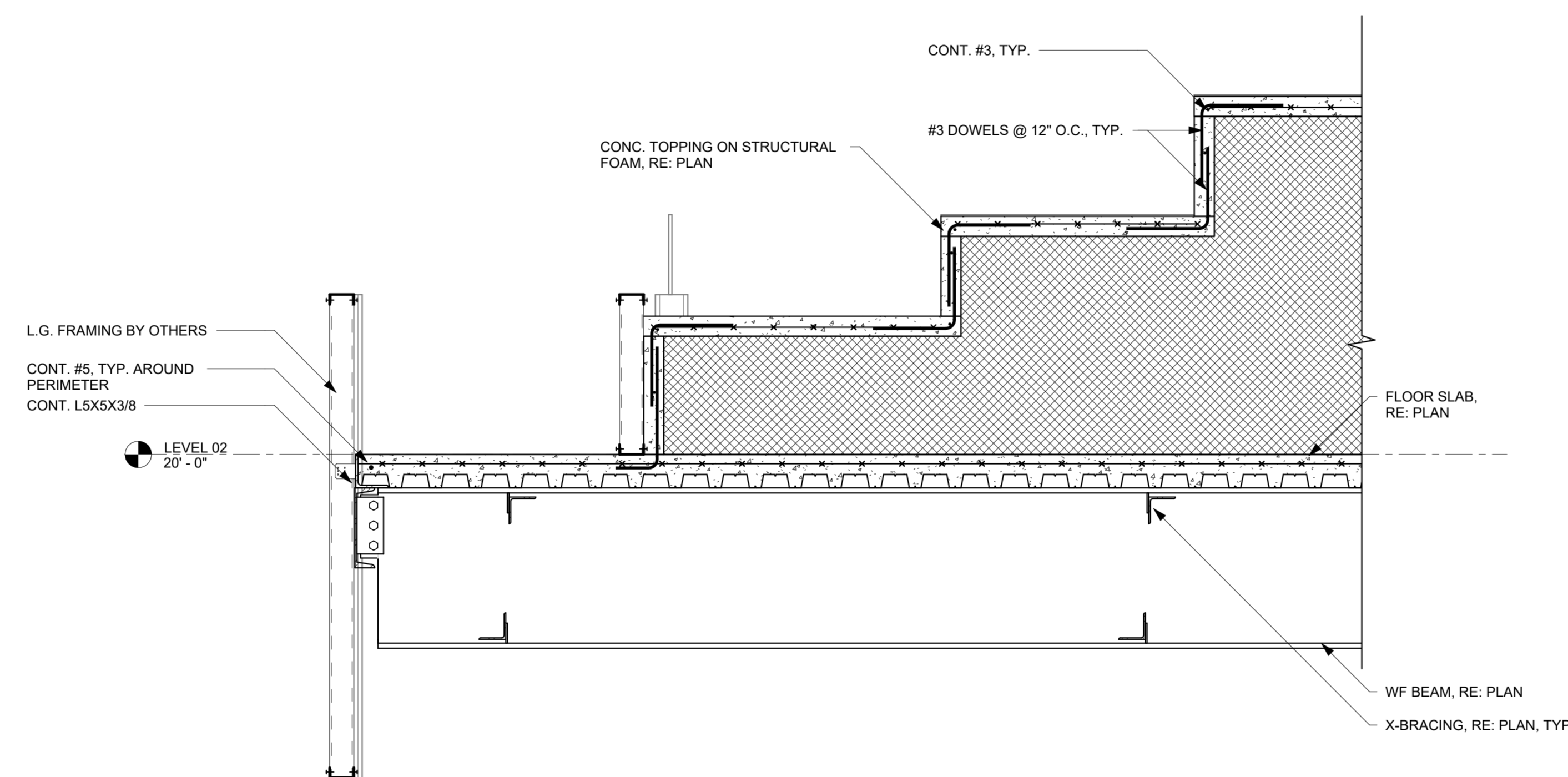
3 FFS - SECTION
3/4" = 1'-0"



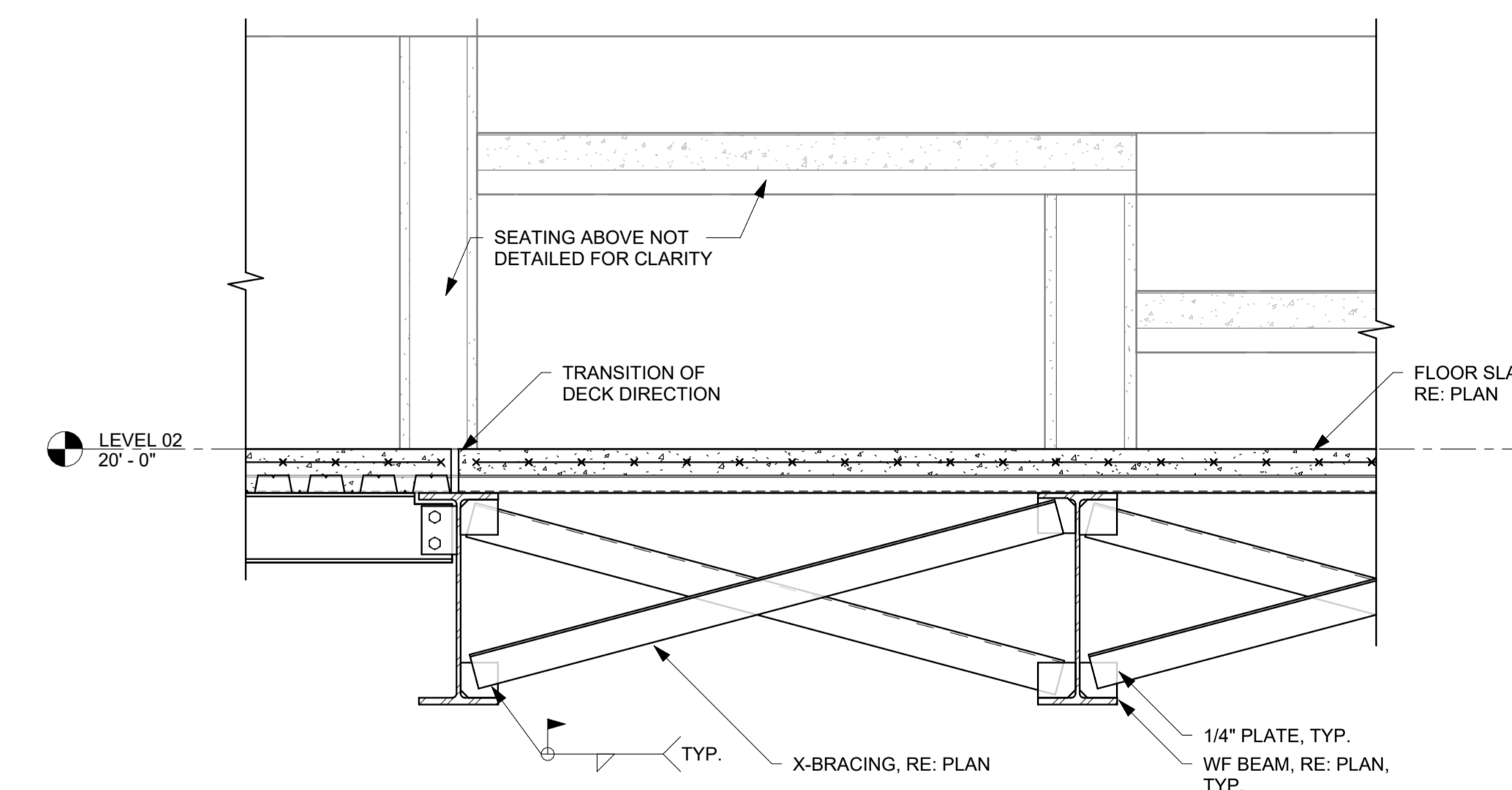
4 FFS - SECTION
3/4" = 1'-0"



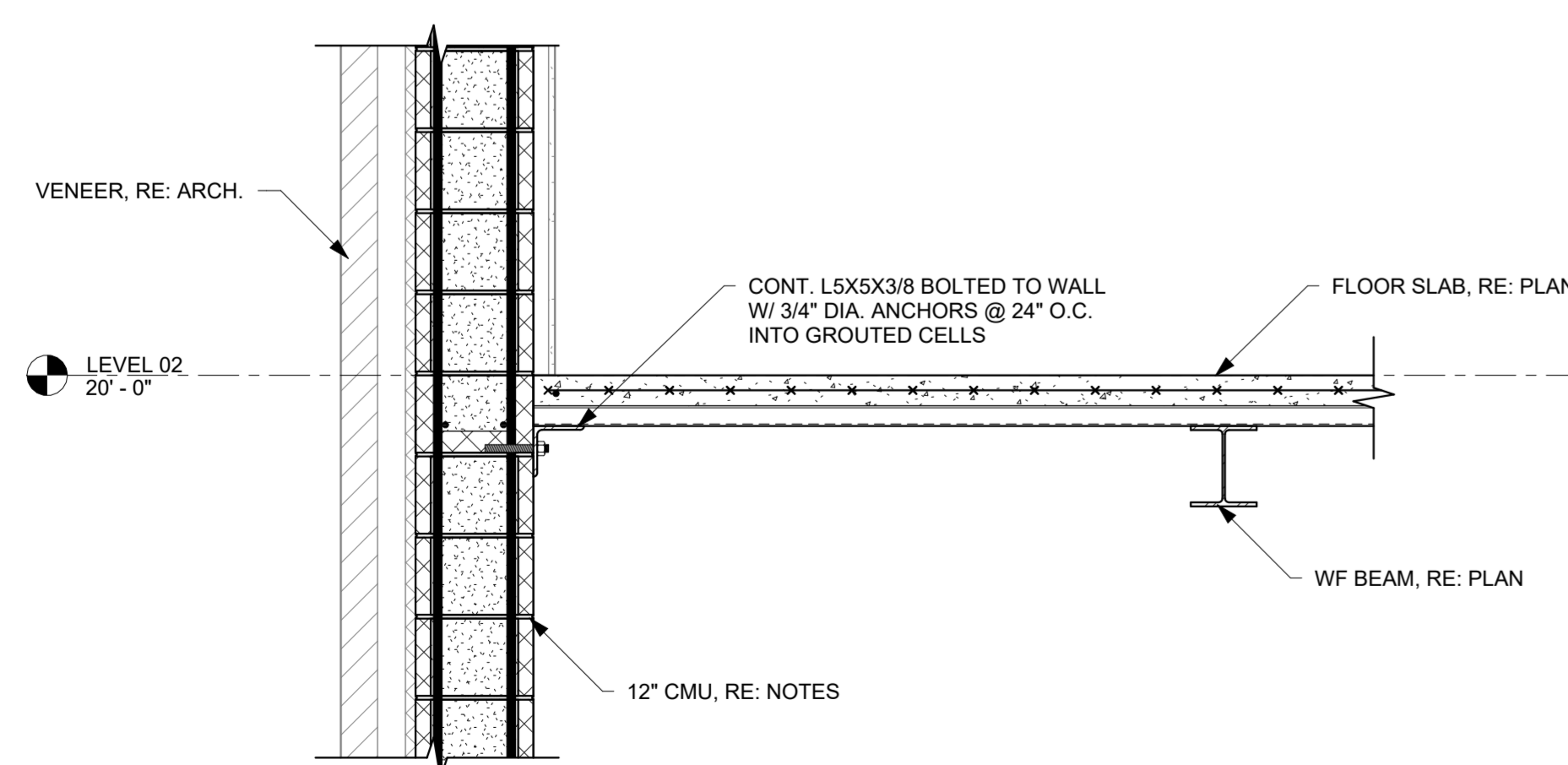
5 FFS - SECTION
3/4" = 1'-0"



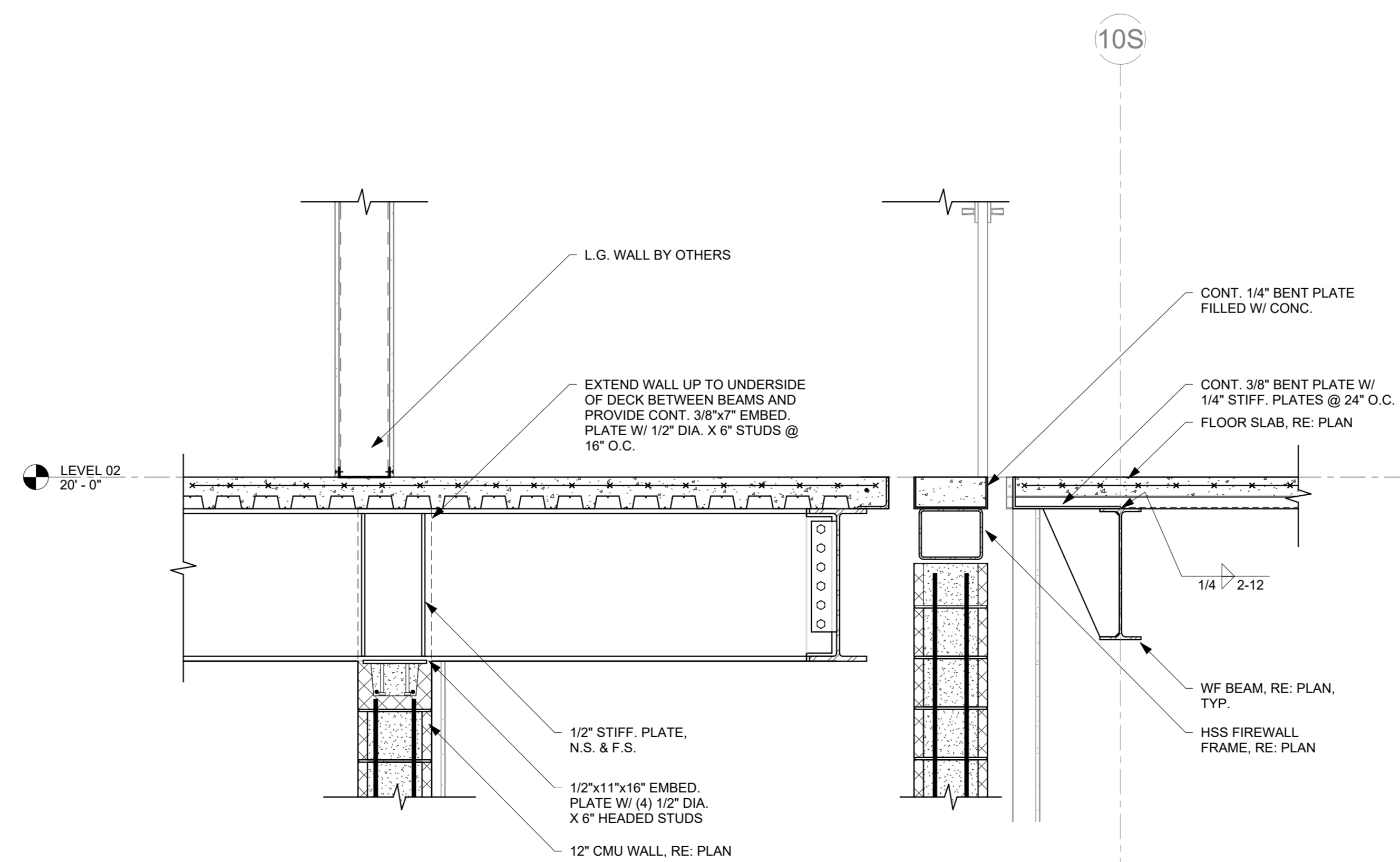
6 FFS - SECTION
3/4" = 1'-0"



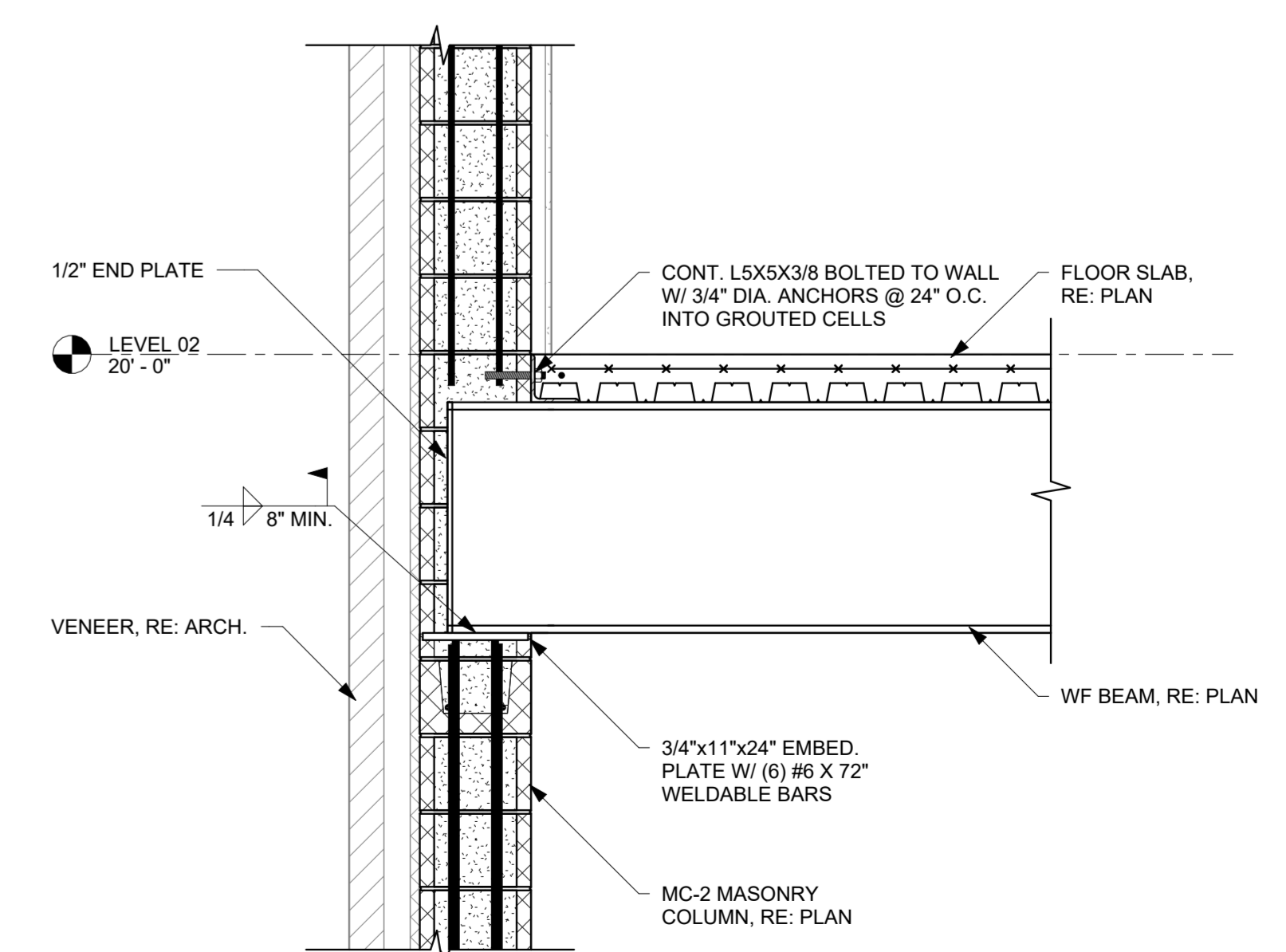
7 FFS - SECTION
3/4" = 1'-0"



8 FFS - SECTION
3/4" = 1'-0"



9 FFS - SECTION
3/4" = 1'-0"



10 FFS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

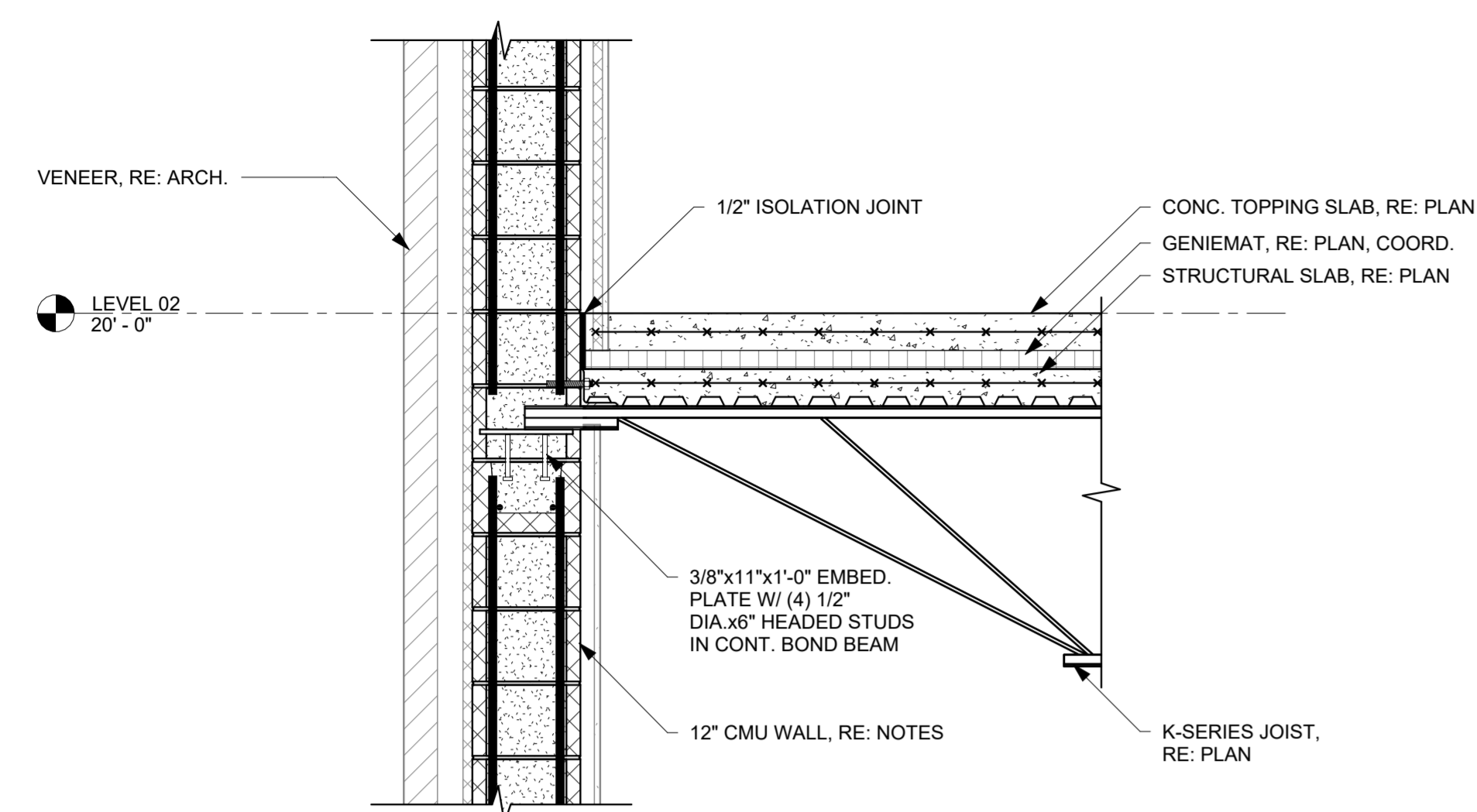
PROJECT NUMBER

-

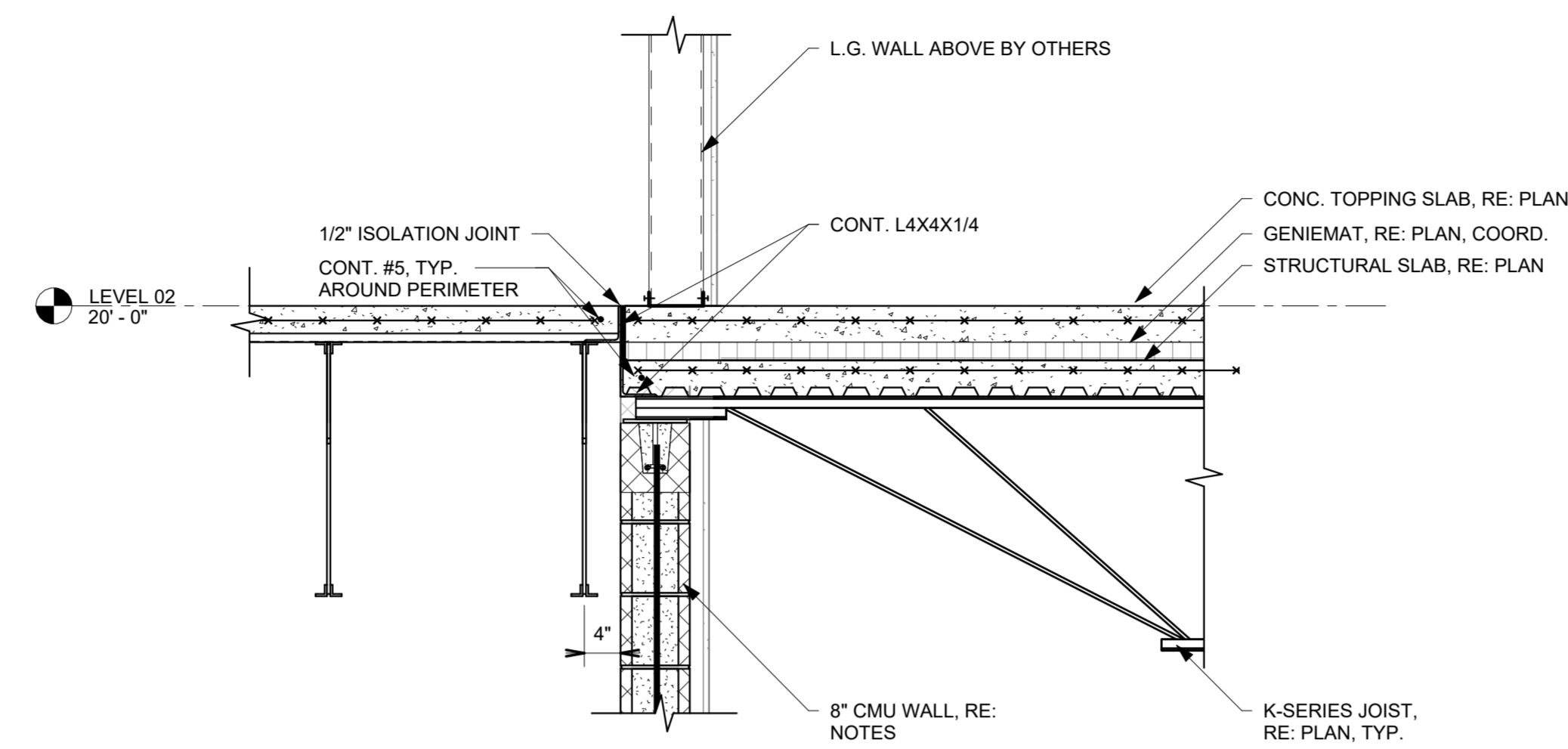
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

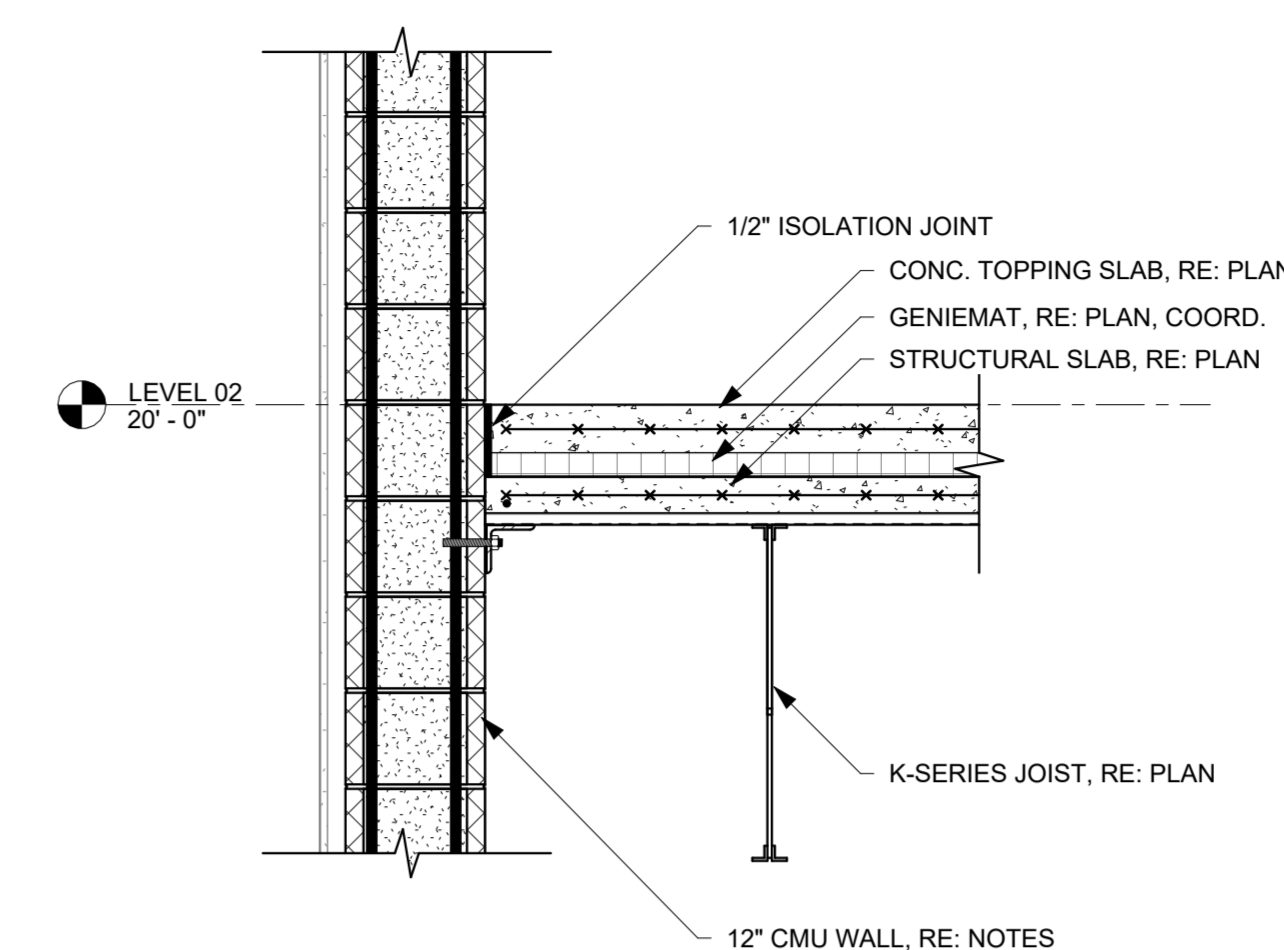
INFORMATION



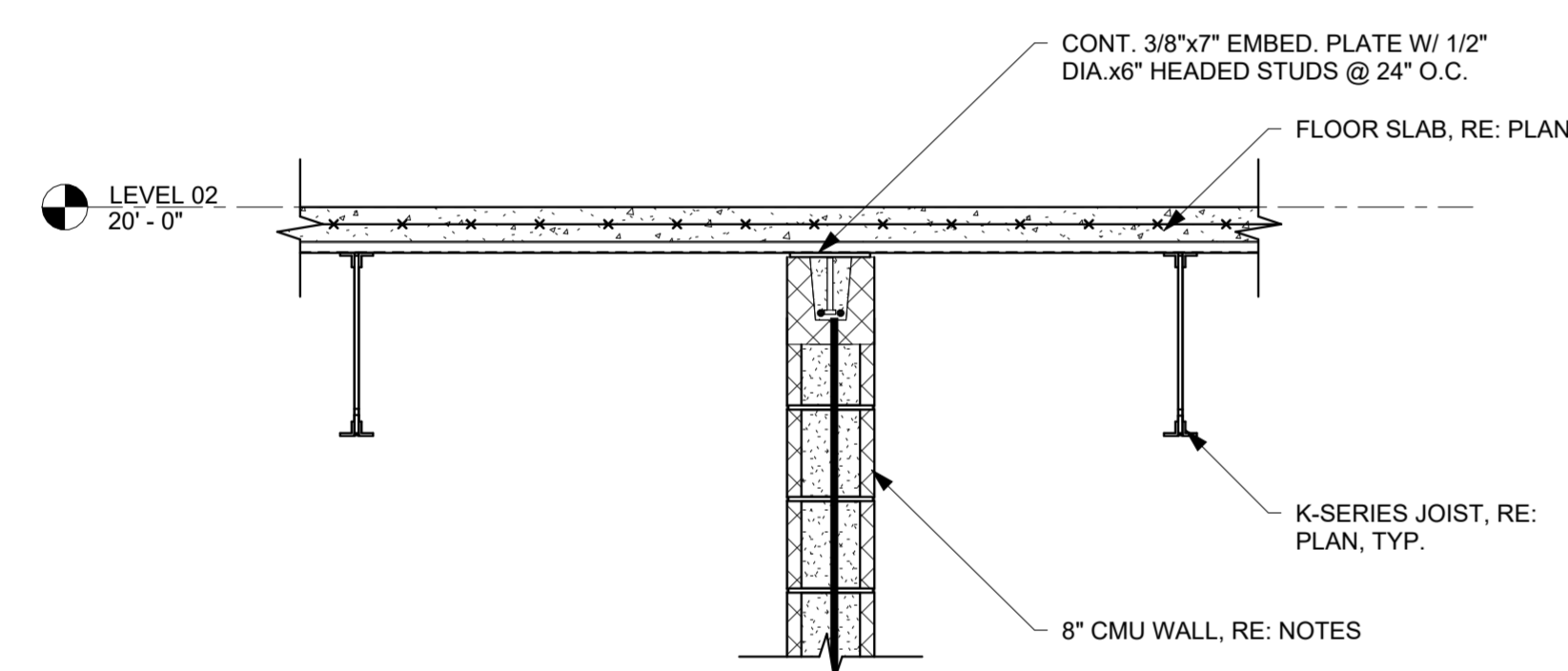
1 FFS - SECTION
 3/4" = 1'-0"



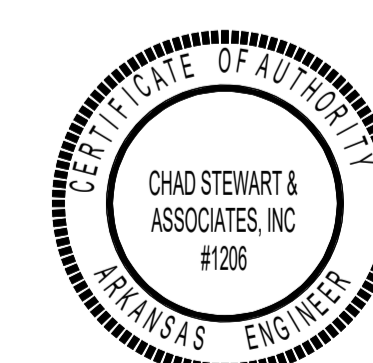
2 FFS - SECTION
 3/4" = 1'-0"



3 FFS - SECTION
 3/4" = 1'-0"



4 FFS - SECTION
 3/4" = 1'-0"

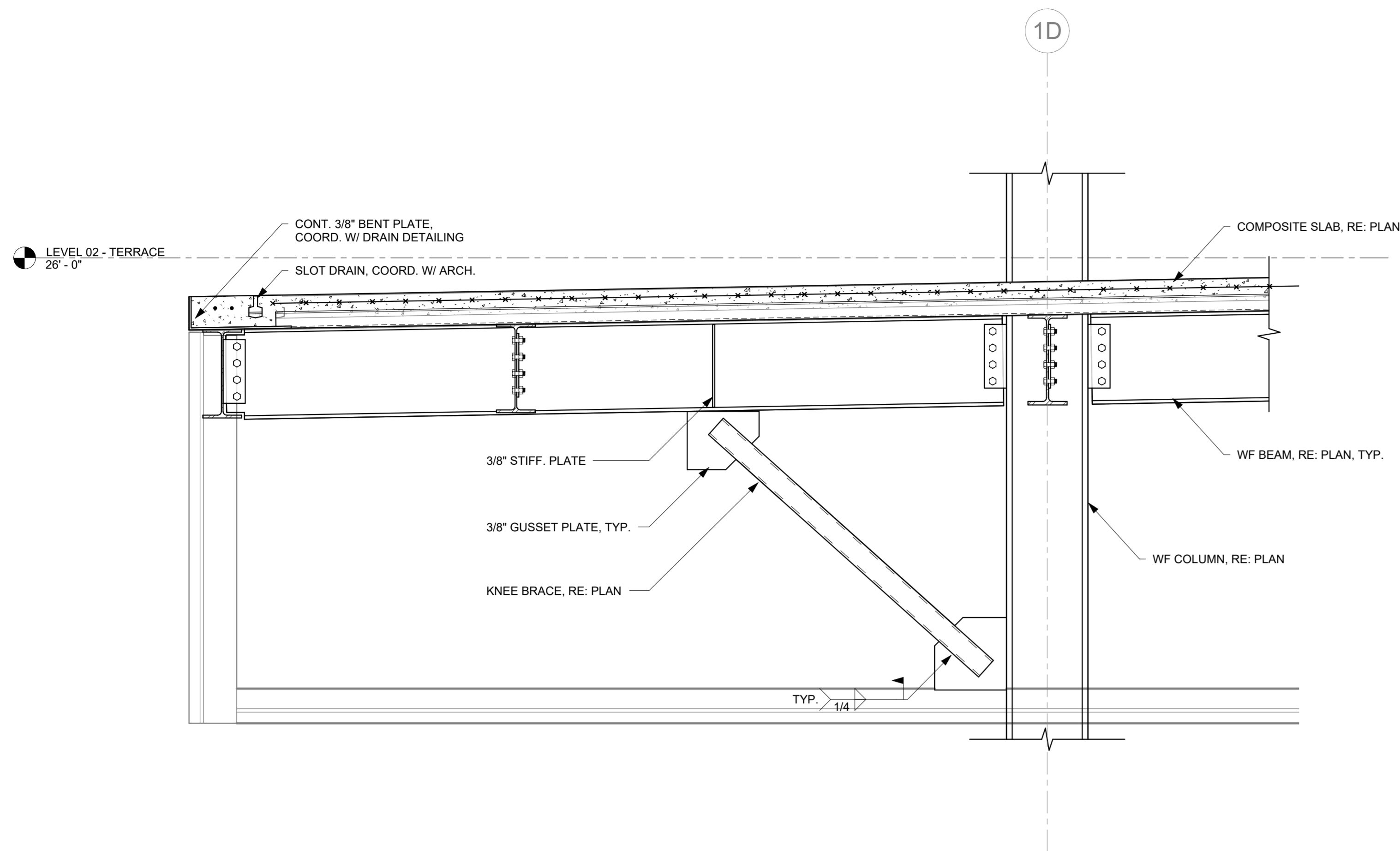
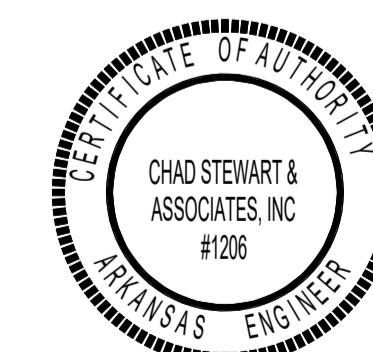


SHEET TITLE
 FLOOR FRAMING
 SECTIONS - AREA B

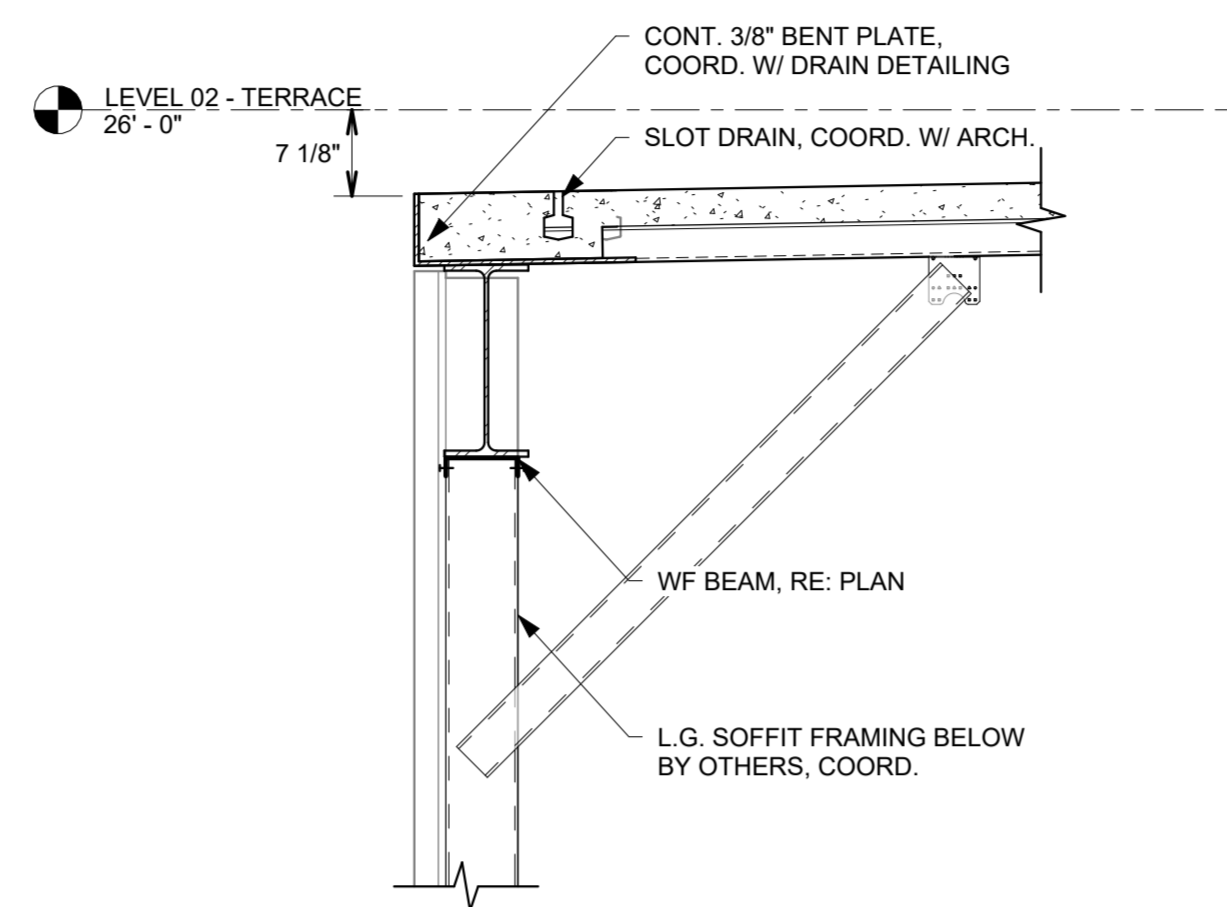
DATE
 17.10.24

SHEET NUMBER

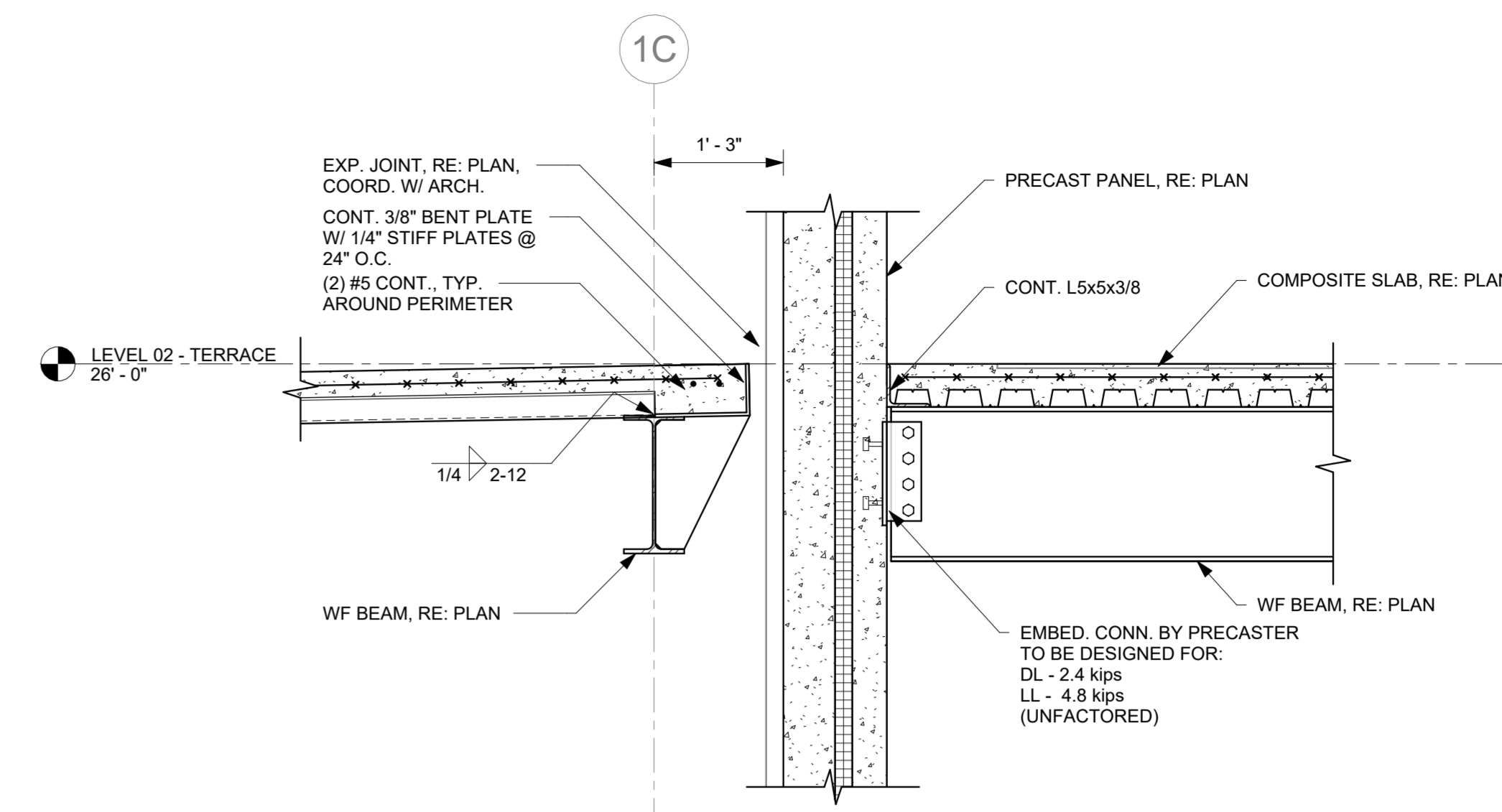
S303.2



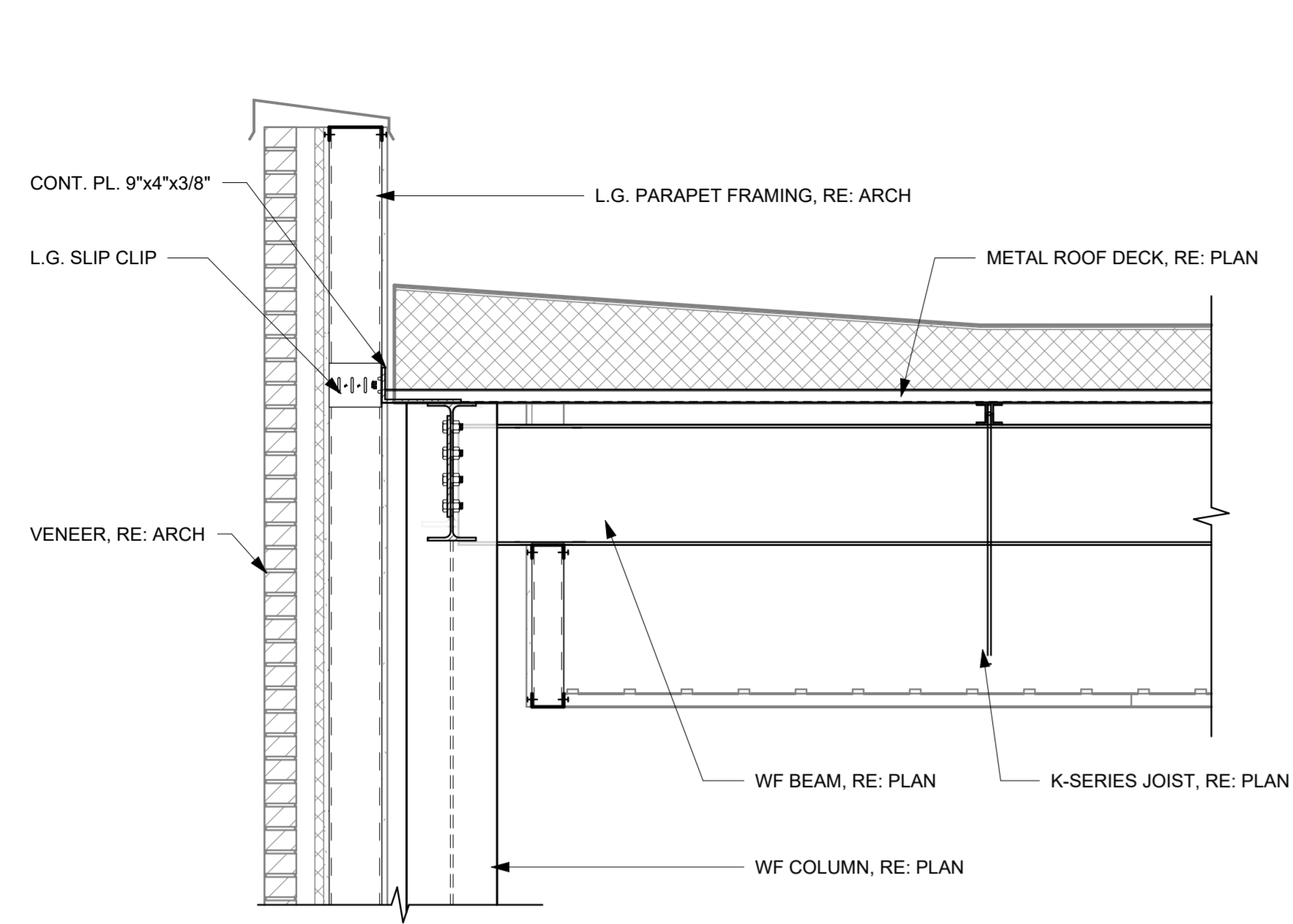
1 FFS - SECTION
 3/4" = 1'-0"



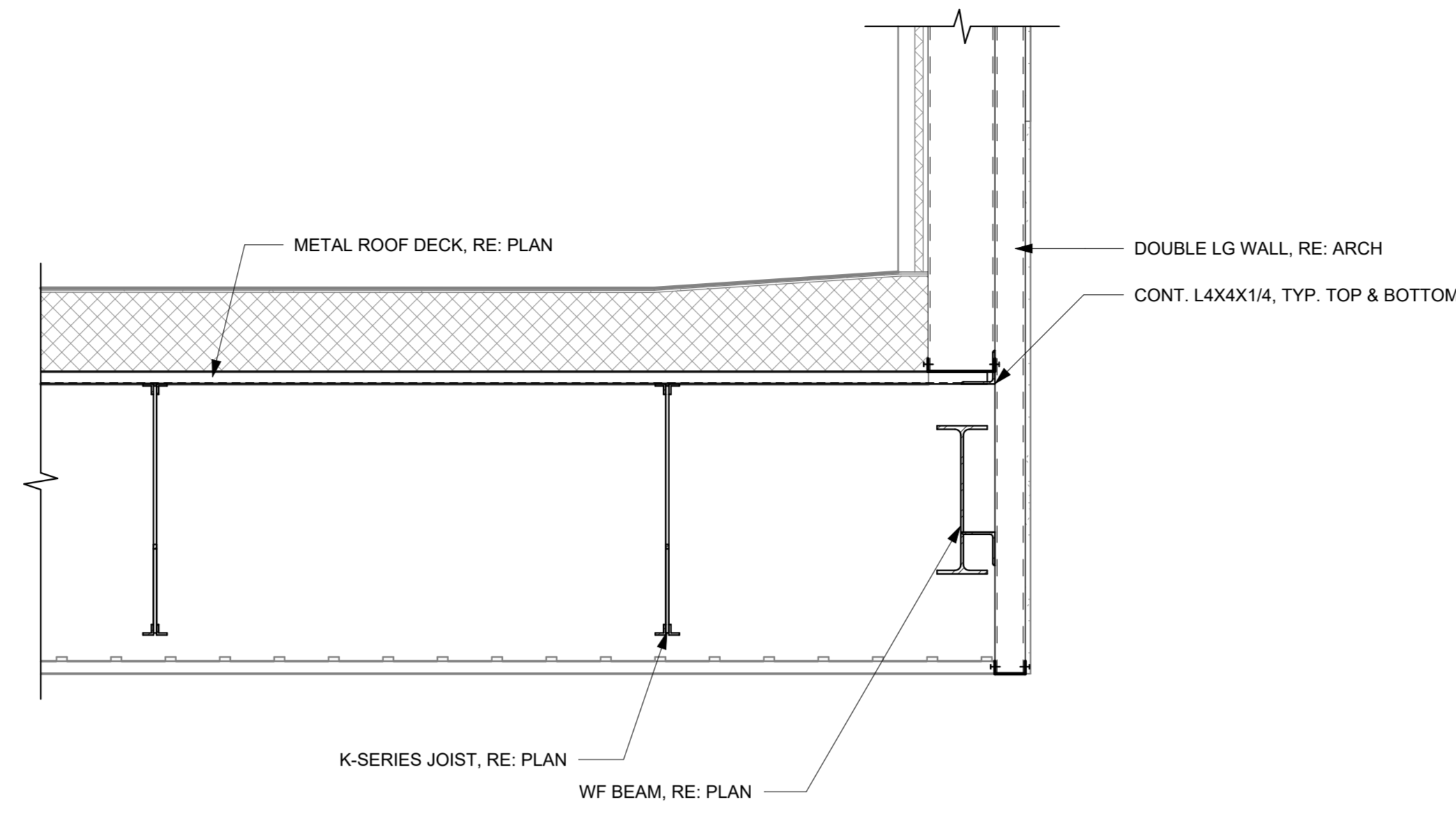
2 FFS - SECTION
 3/4" = 1'-0"



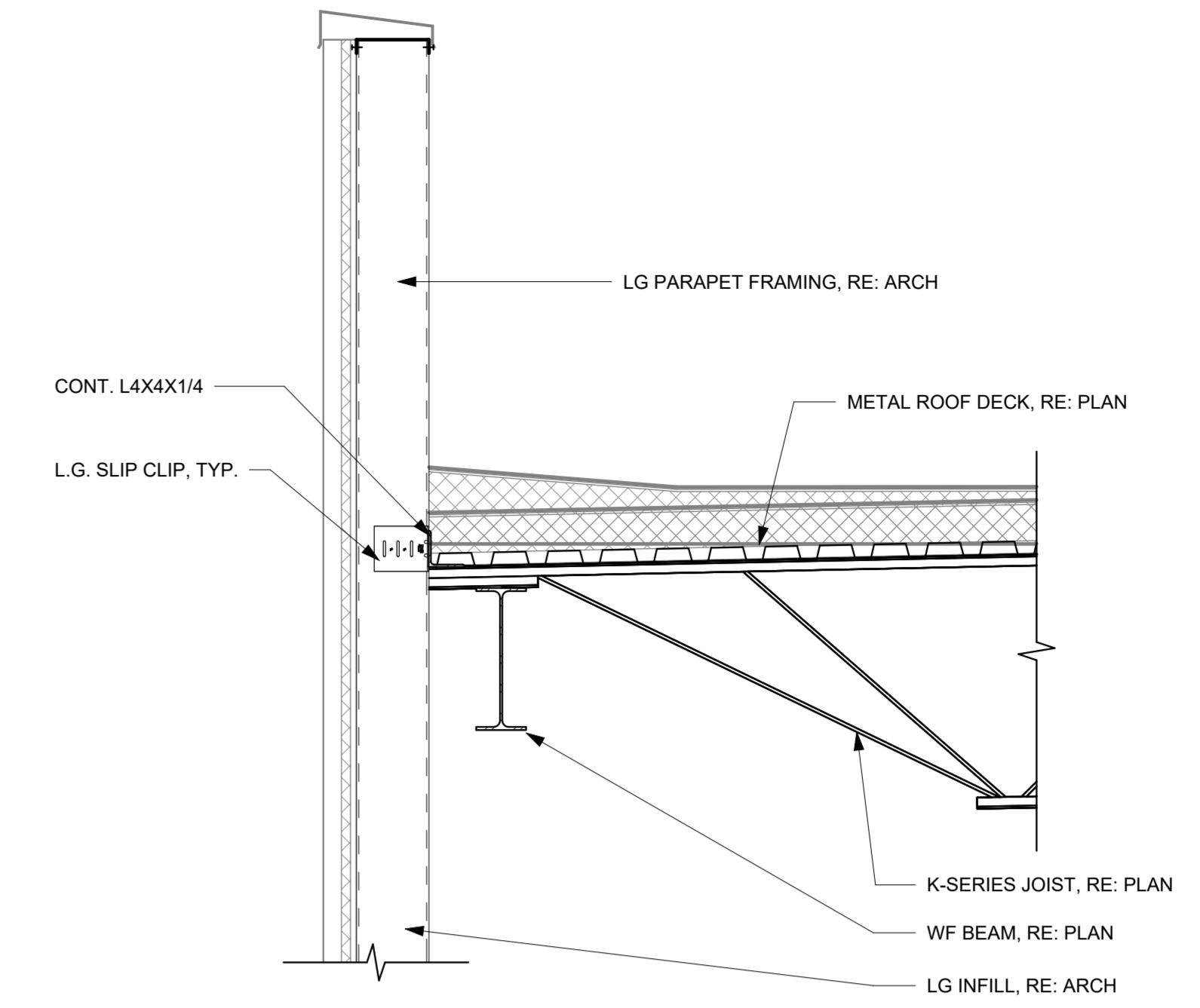
3 FFS - SECTION
 3/4" = 1'-0"



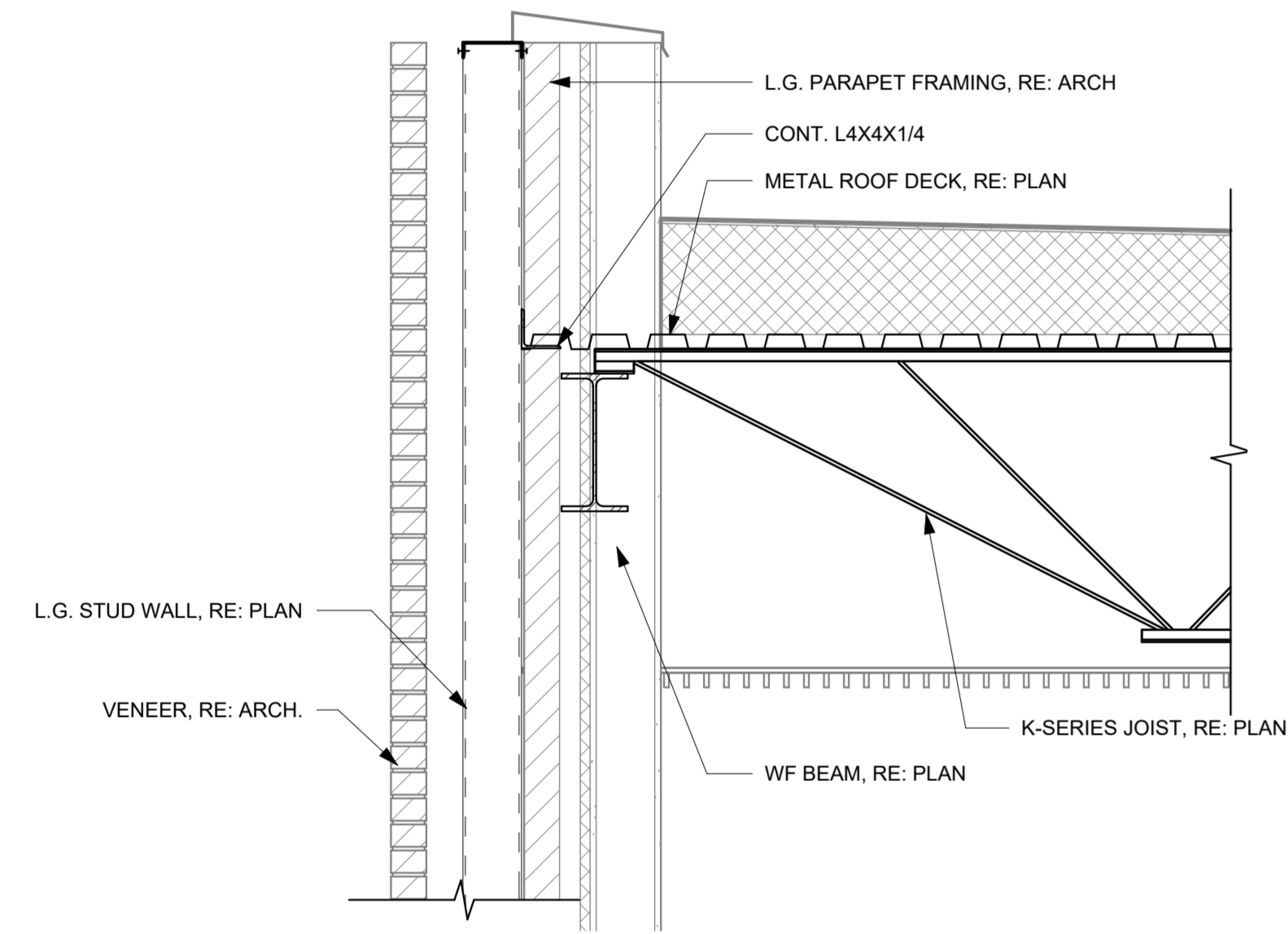
1 FRS - SECTION
3/4" = 1'-0"



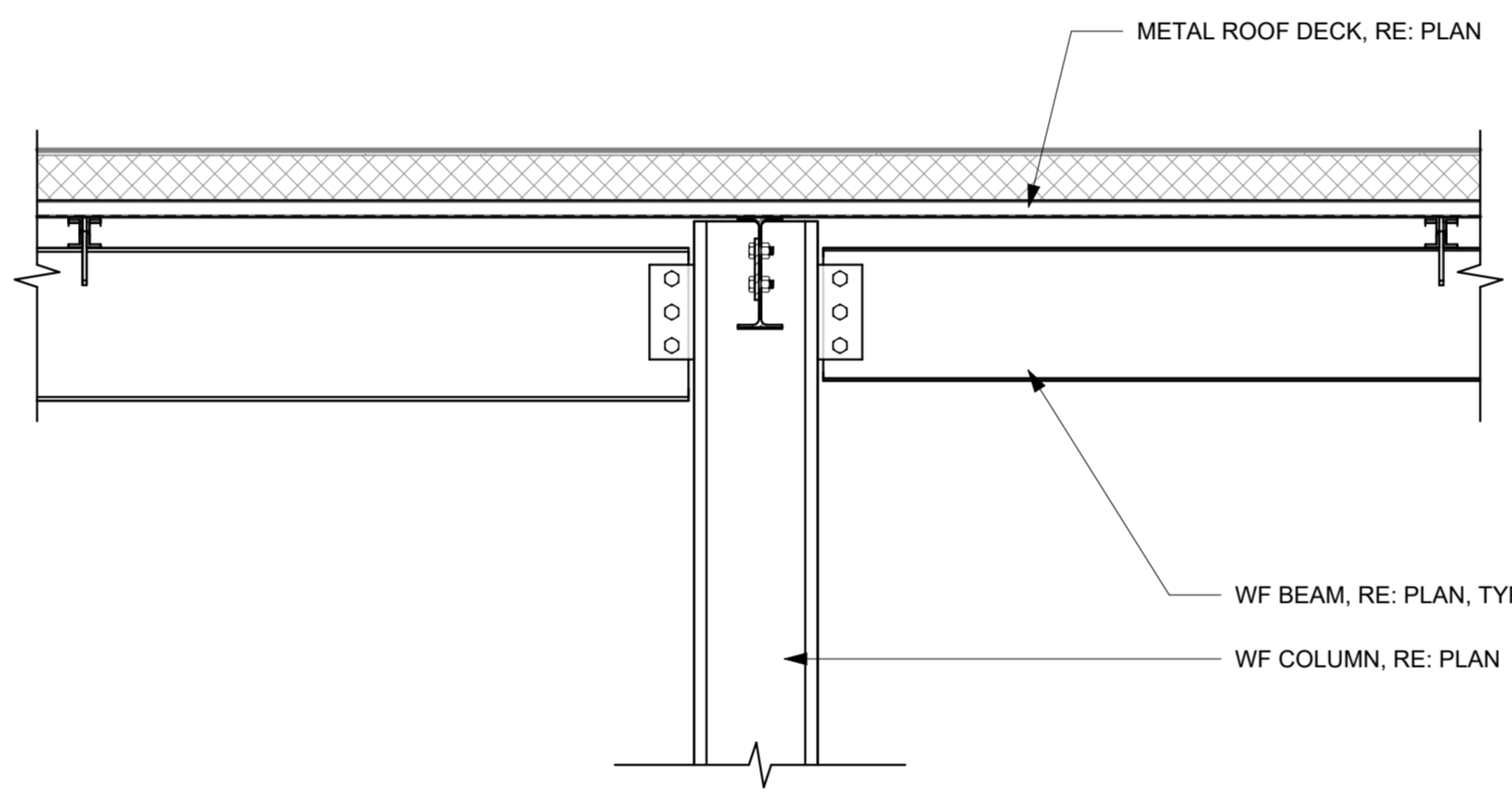
2 FRS - SECTION
3/4" = 1'-0"



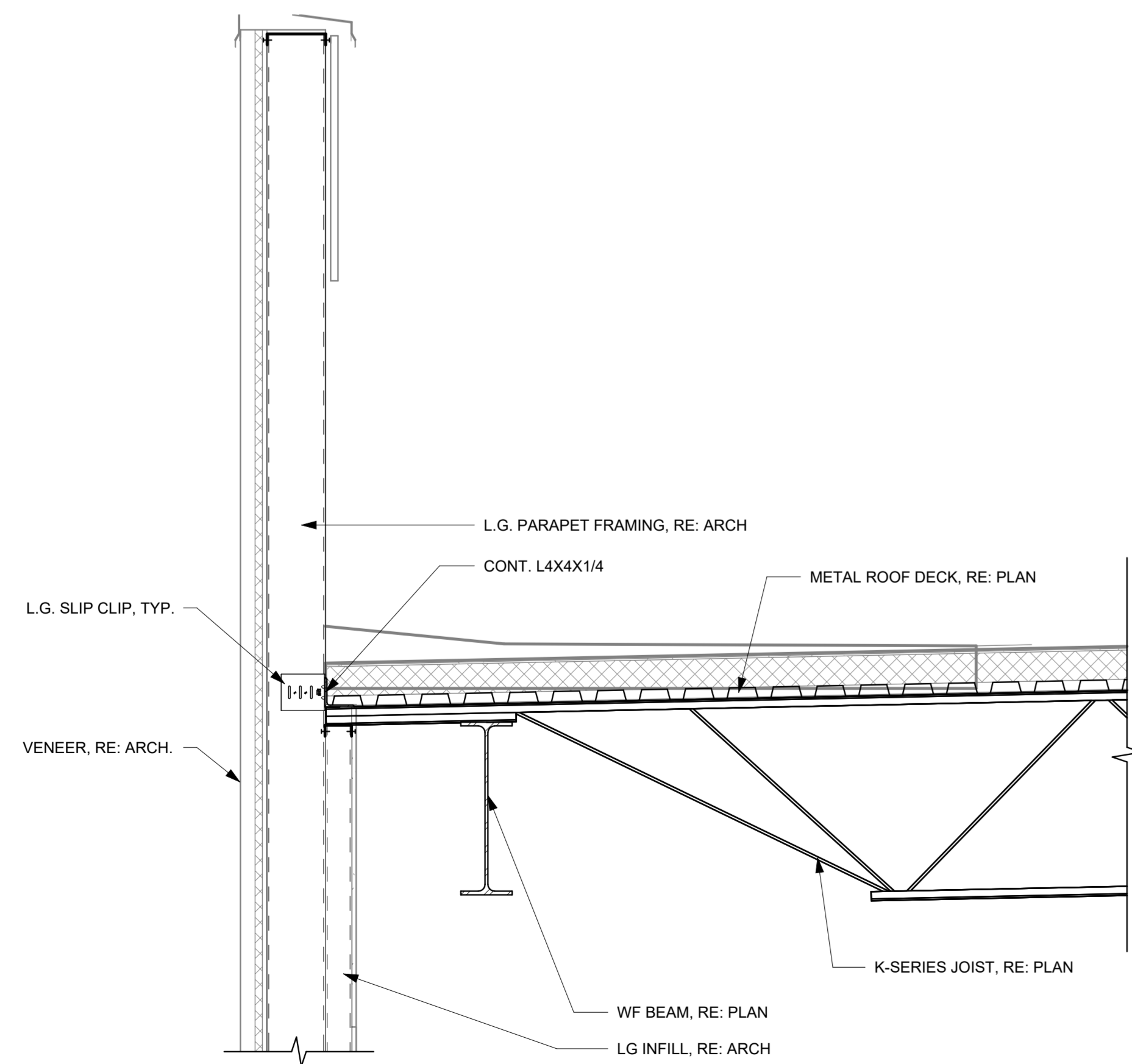
3 FRS - SECTION
3/4" = 1'-0"



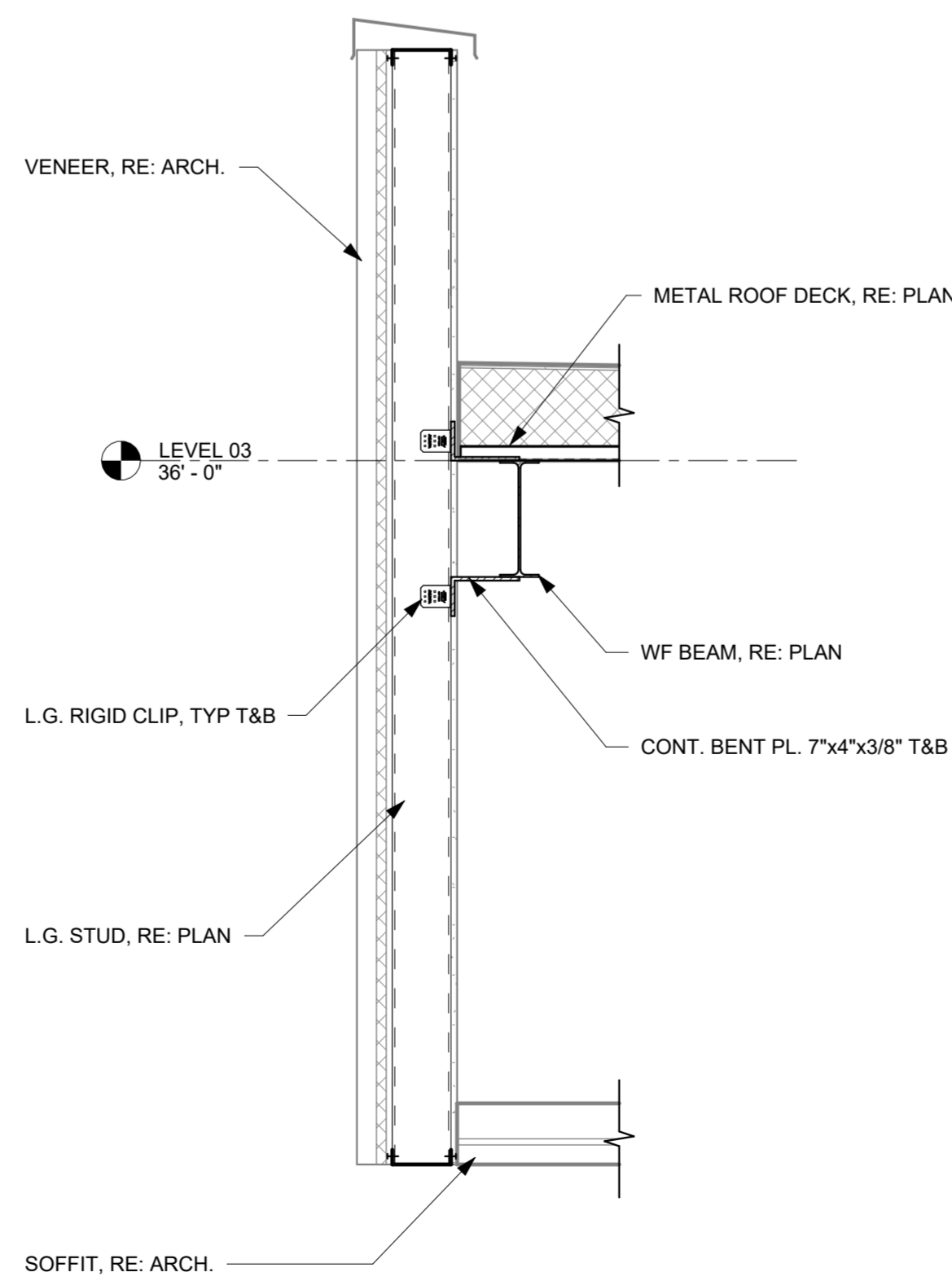
4 FRS - SECTION
3/4" = 1'-0"



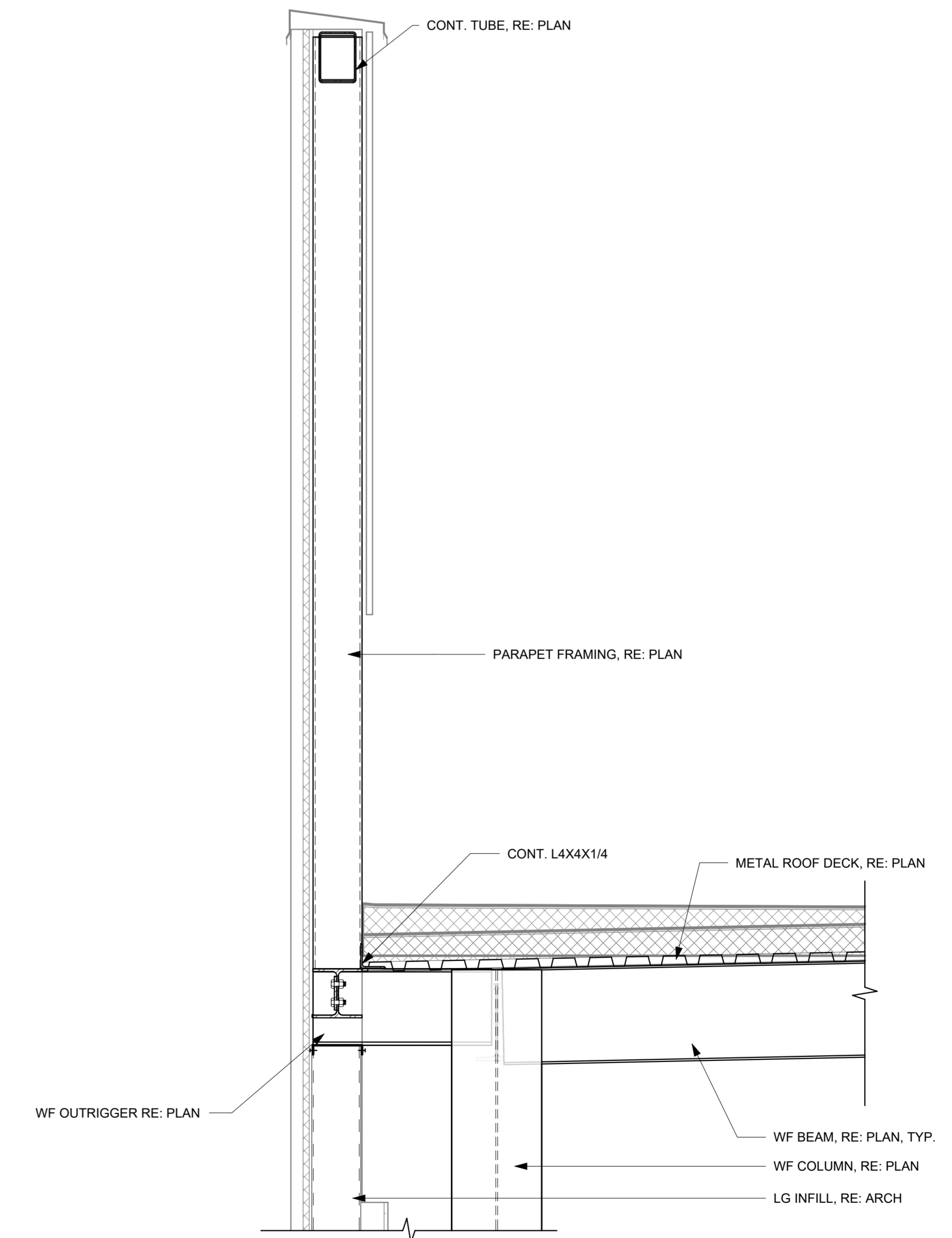
5 FRS - SECTION
3/4" = 1'-0"



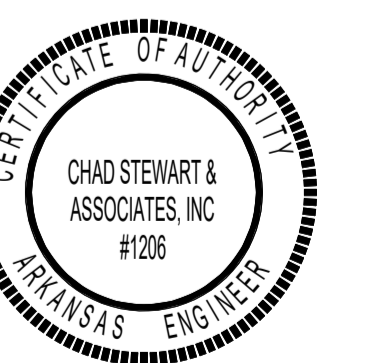
6 FRS - SECTION
3/4" = 1'-0"



7 FRS - SECTION
3/4" = 1'-0"



8 FRS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

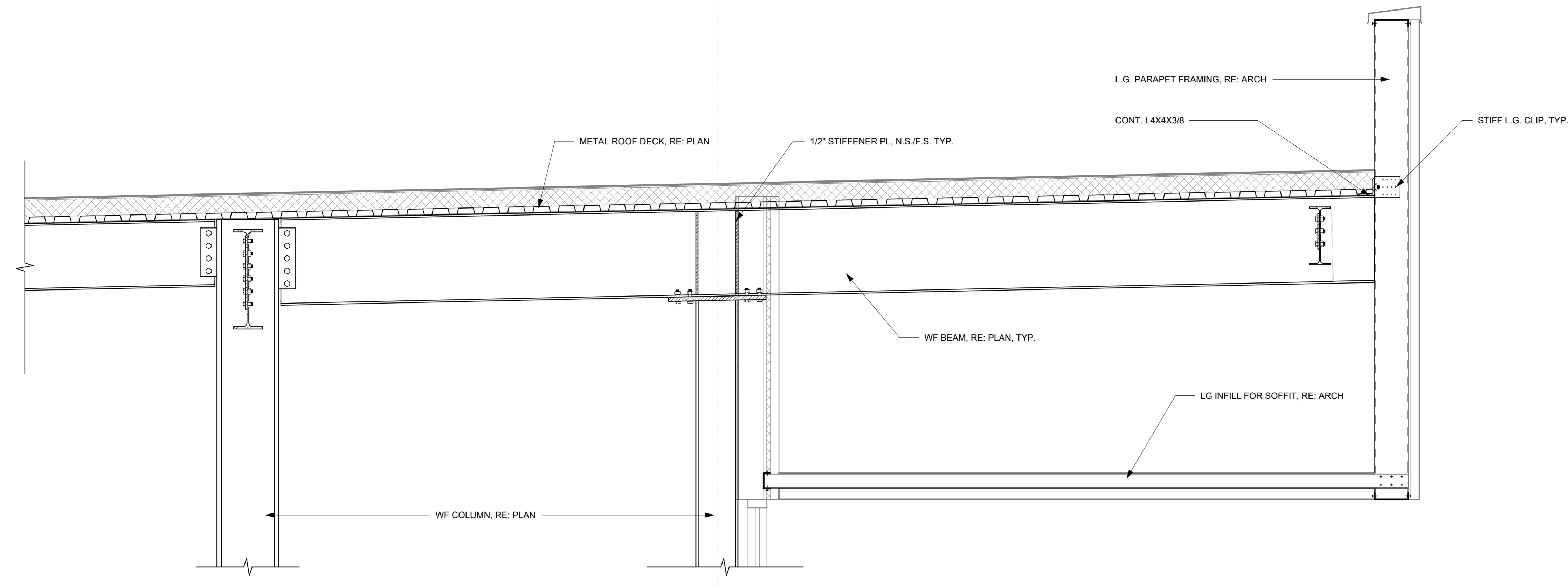
PROJECT NUMBER

-

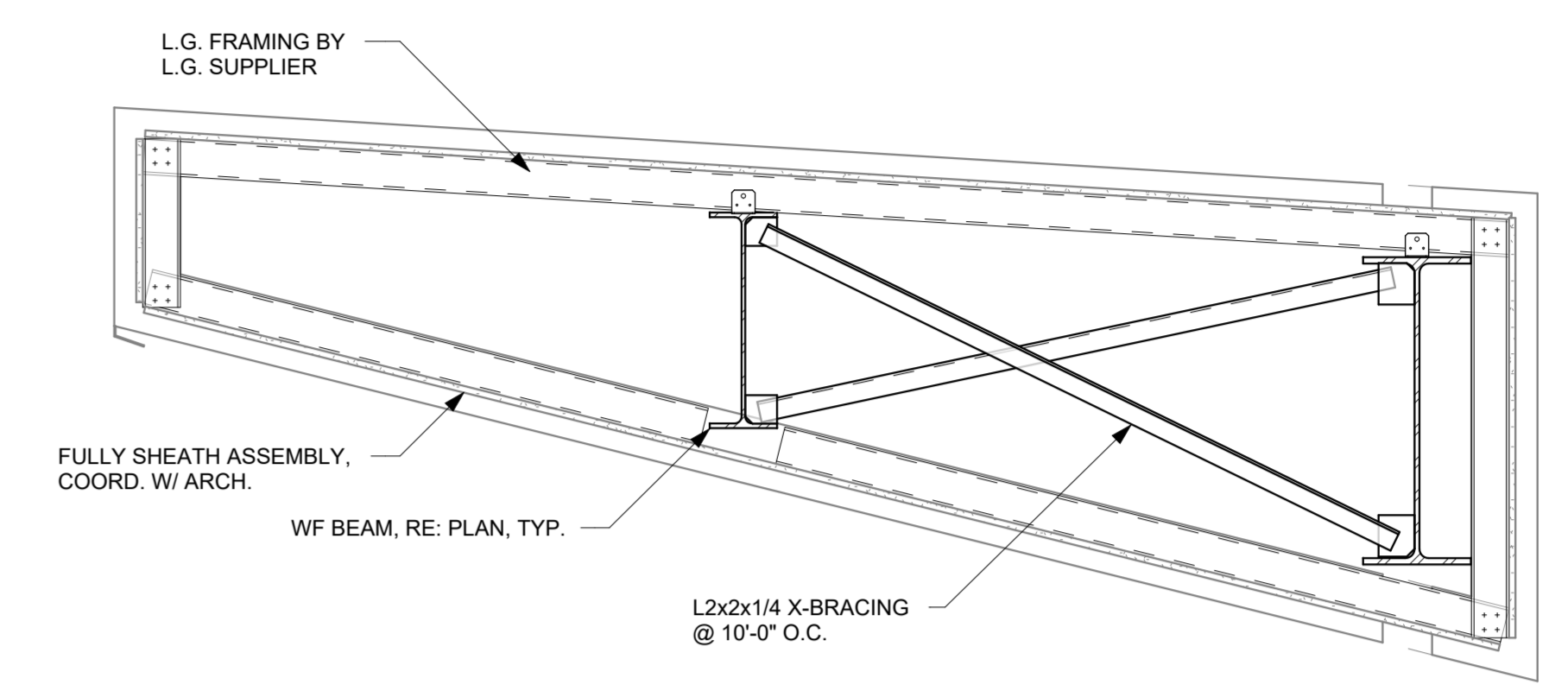
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

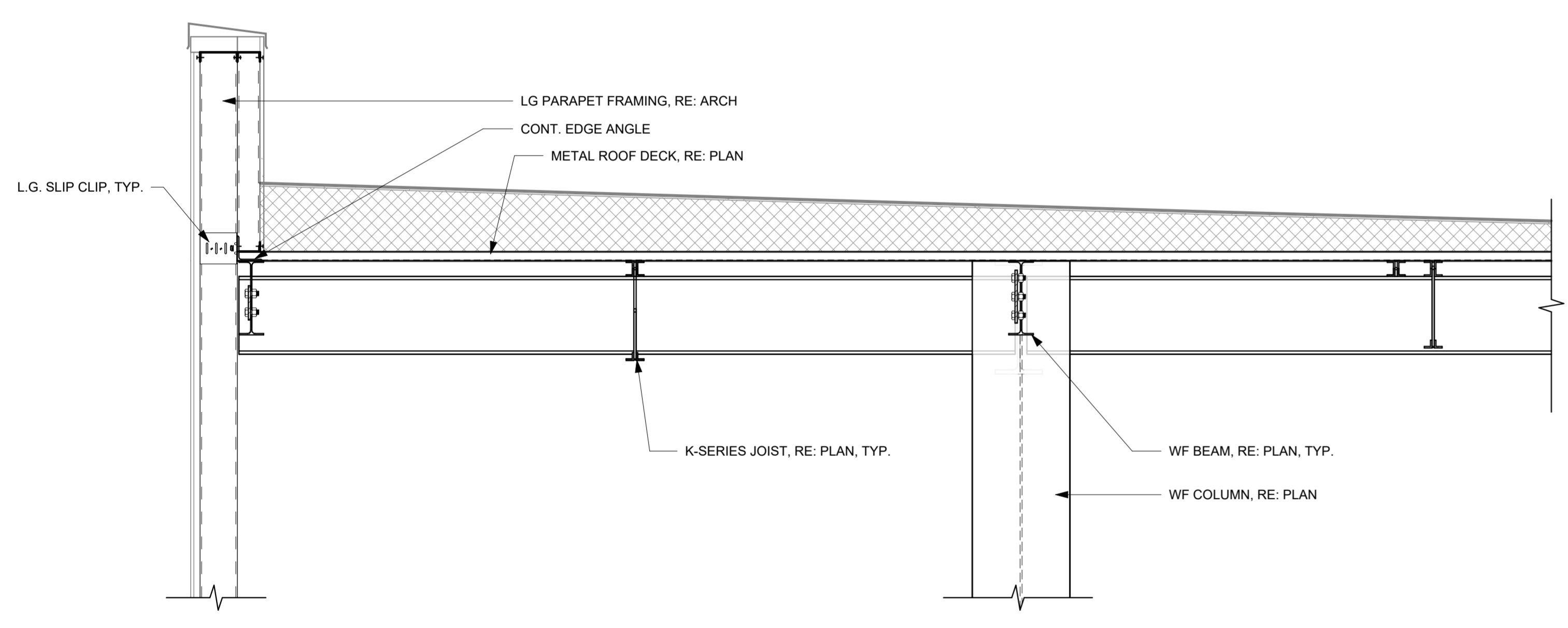
INFORMATION



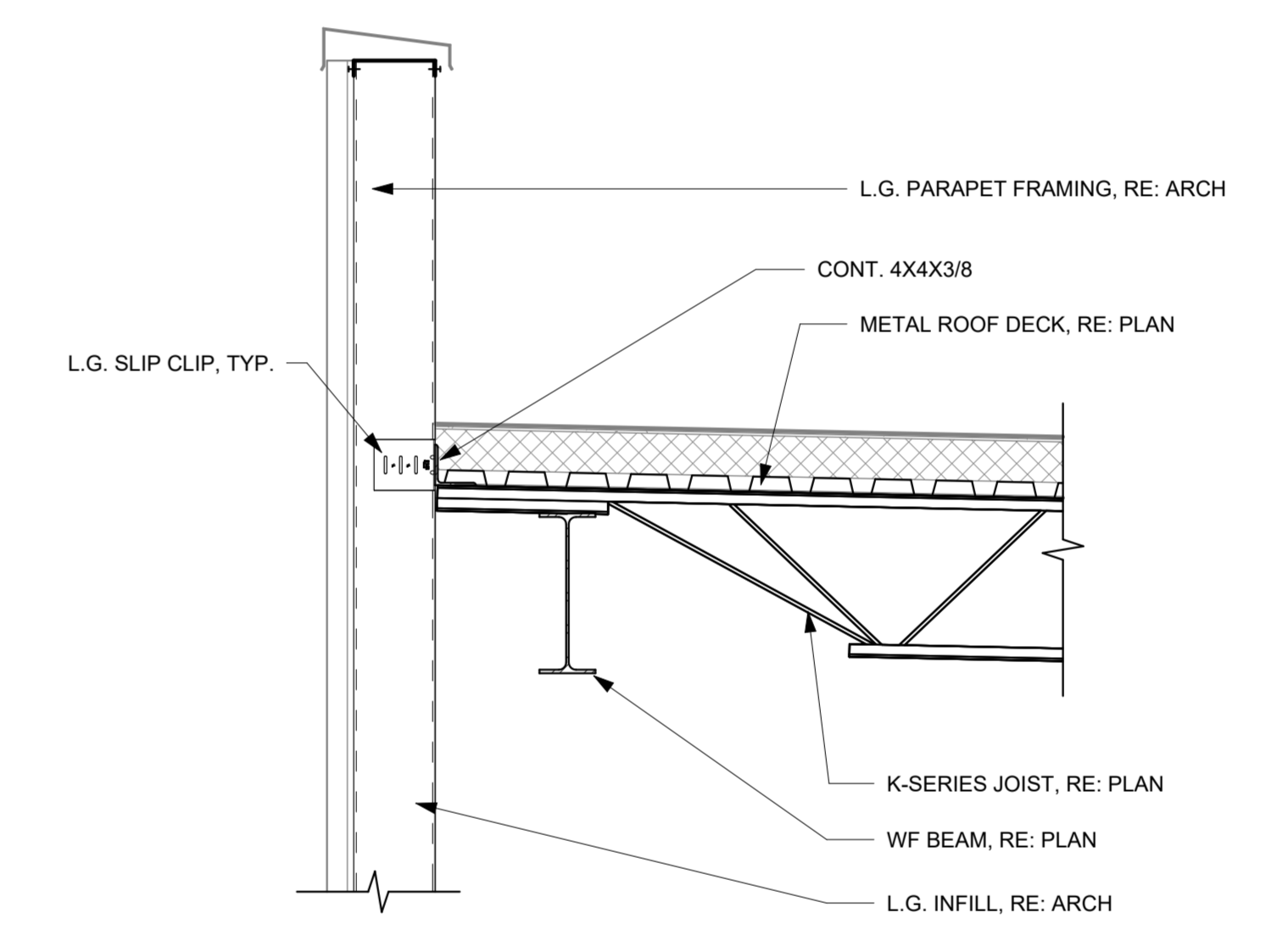
1 FRS - SECTION
3/4" = 1'-0"



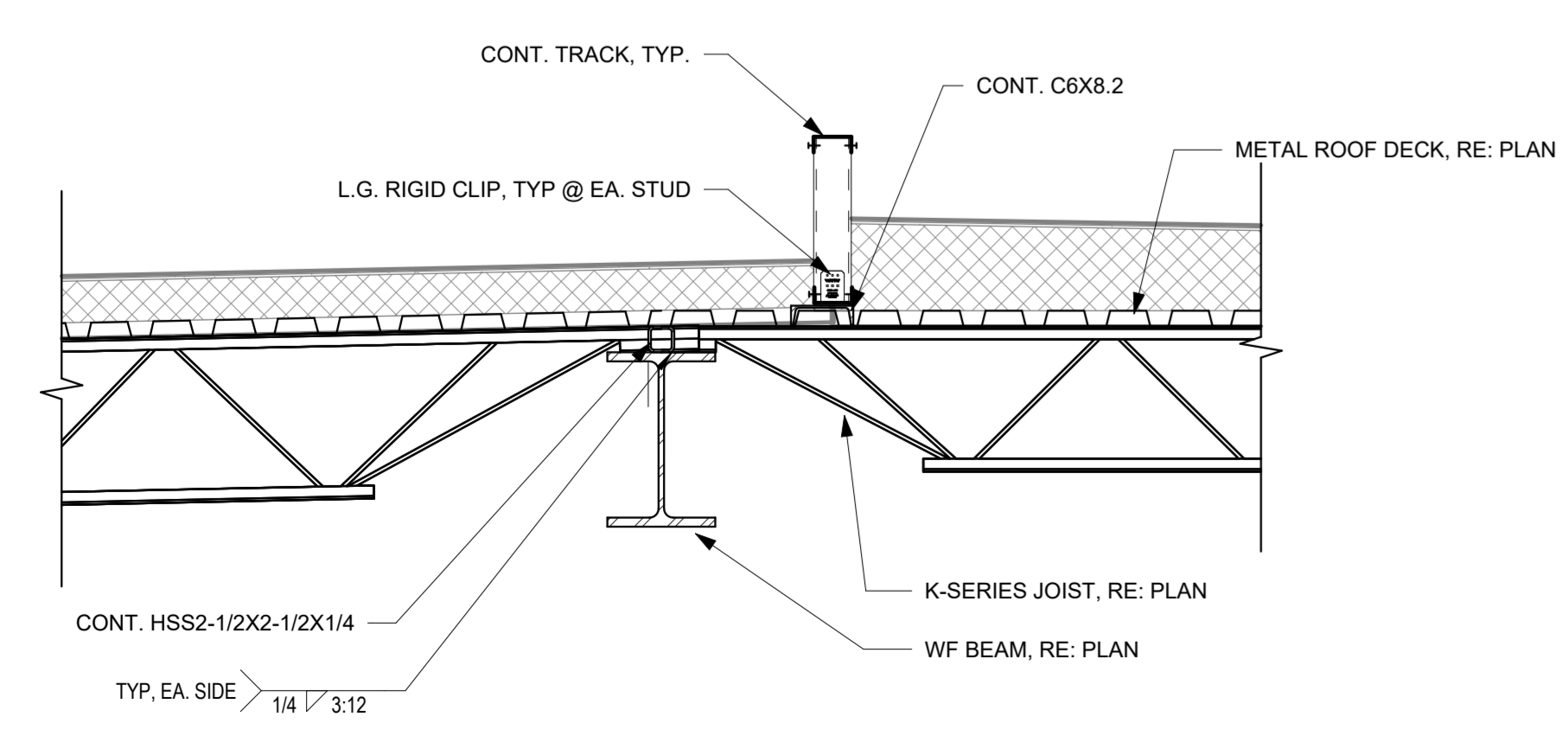
2 FRS - SECTION
3/4" = 1'-0"



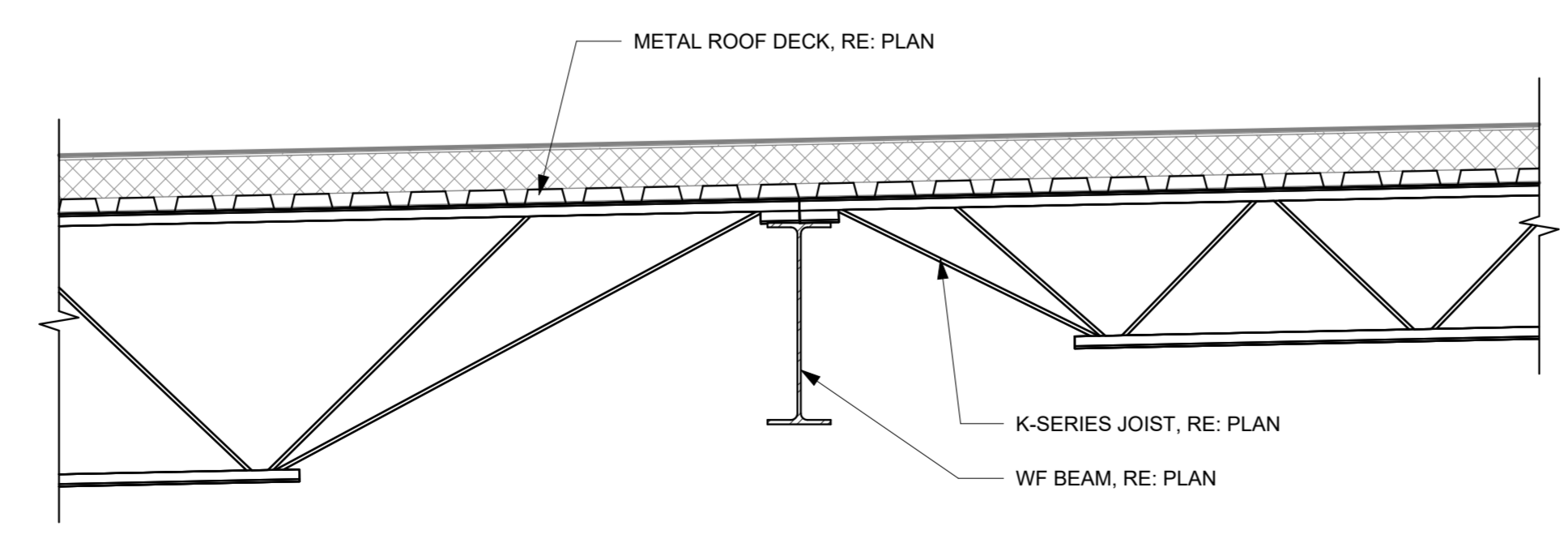
3 FRS - SECTION
3/4" = 1'-0"



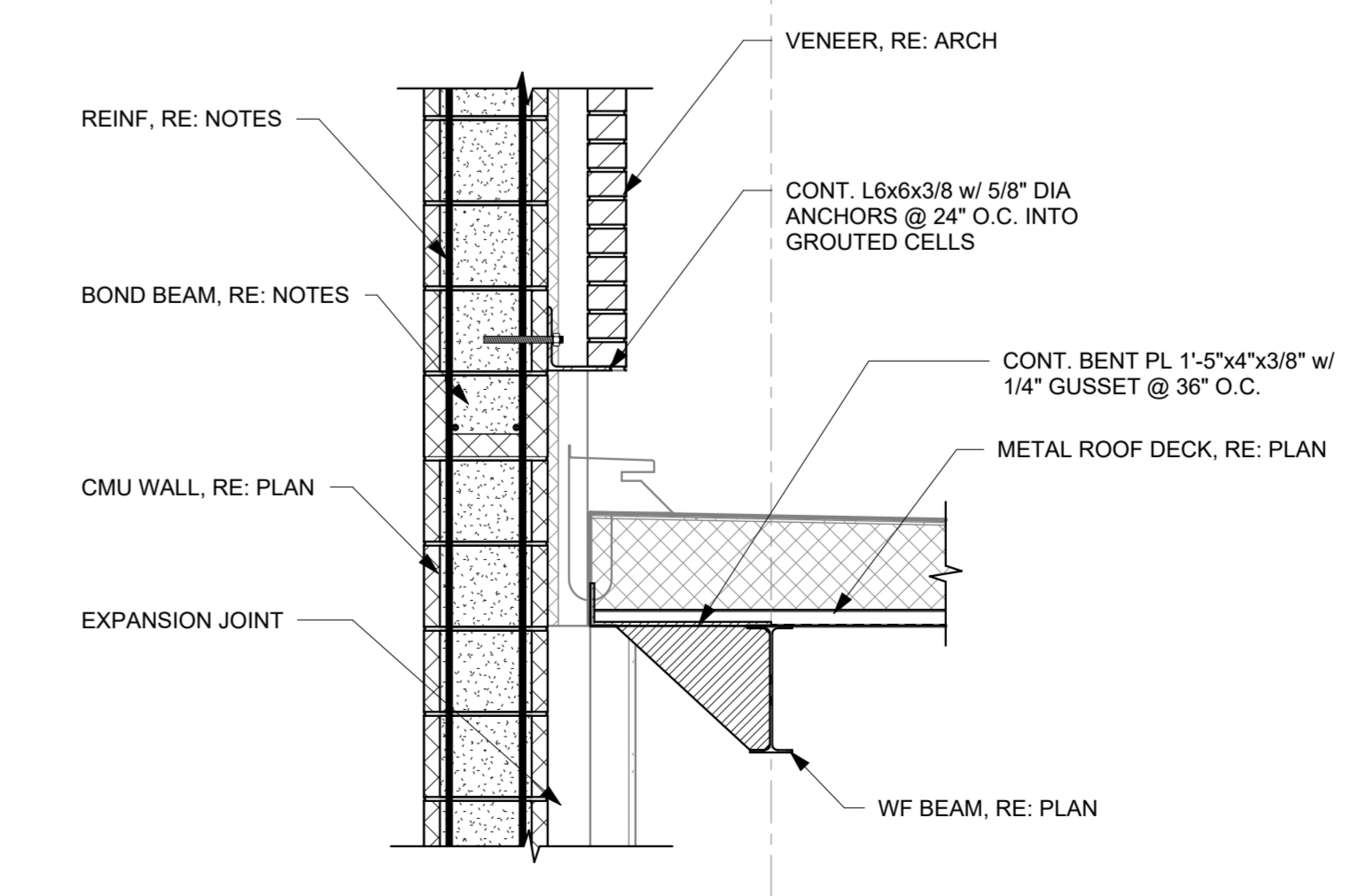
4 FRS - SECTION
3/4" = 1'-0"



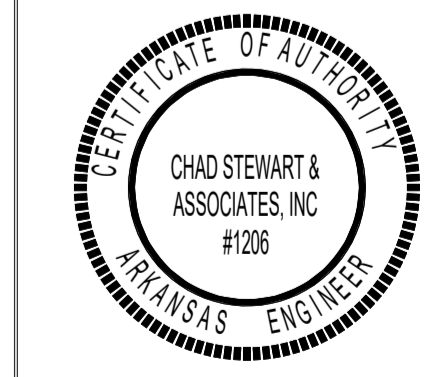
5 FRS - SECTION
3/4" = 1'-0"



6 FRS - SECTION
3/4" = 1'-0"



7 FRS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

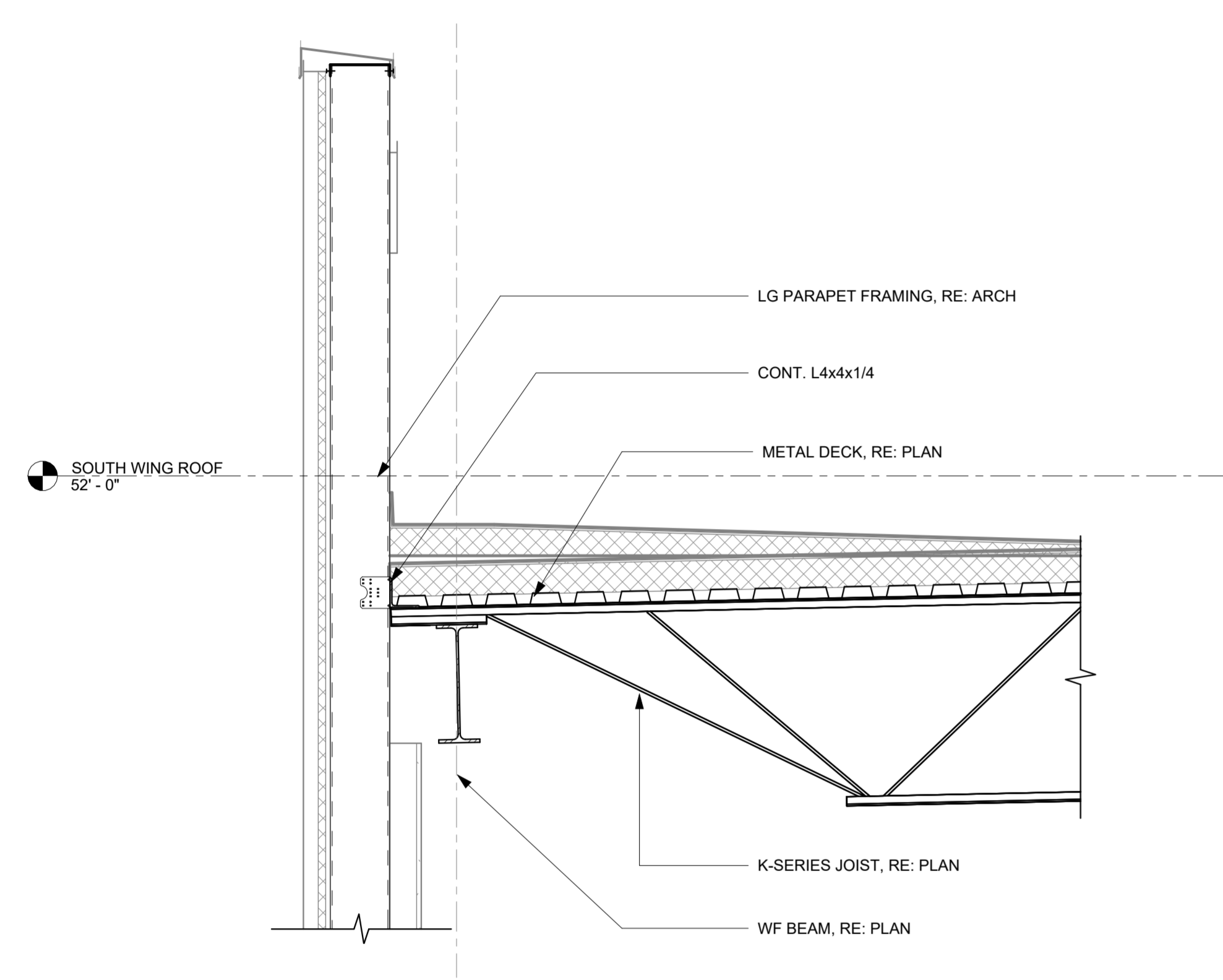
PROJECT NUMBER

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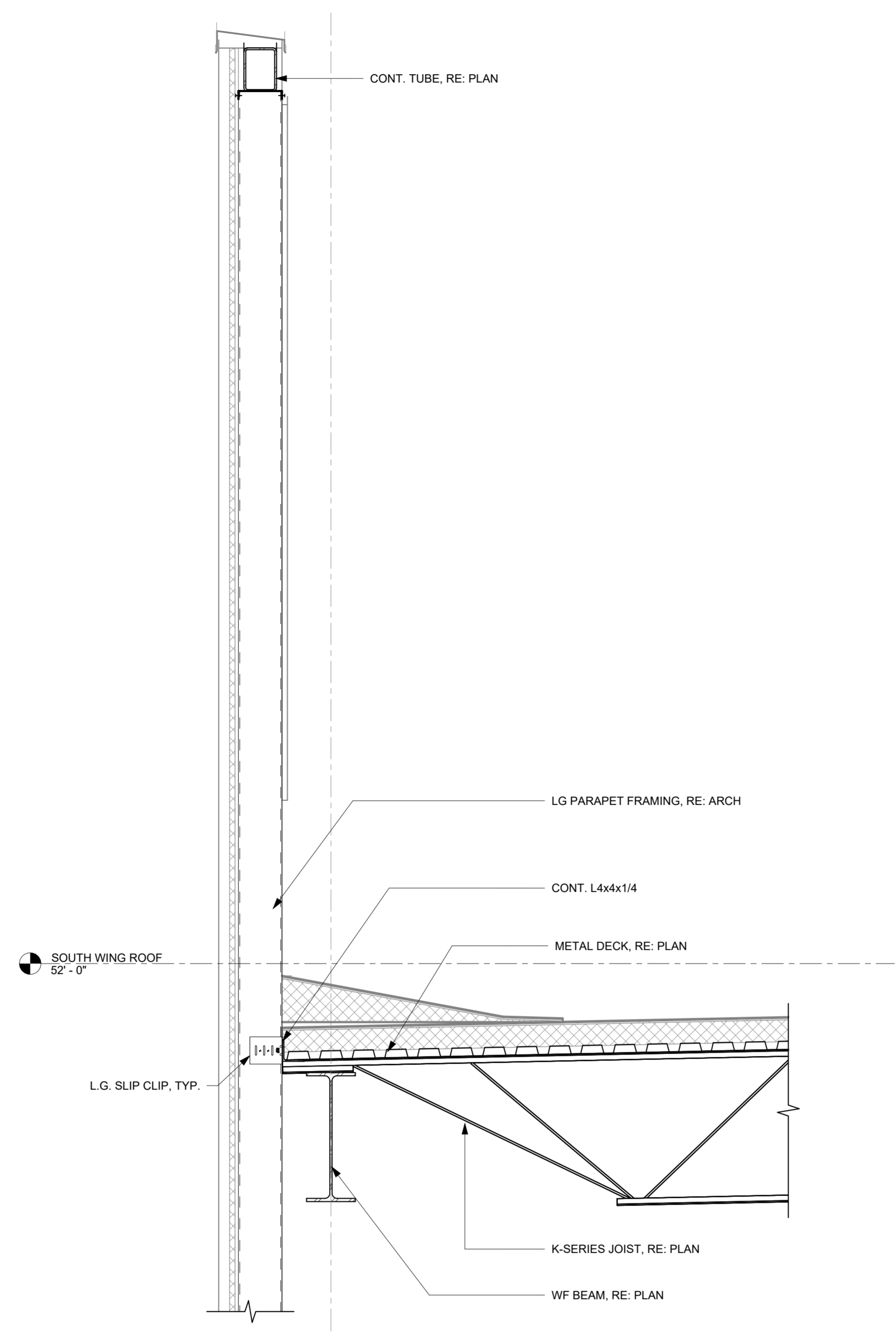
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

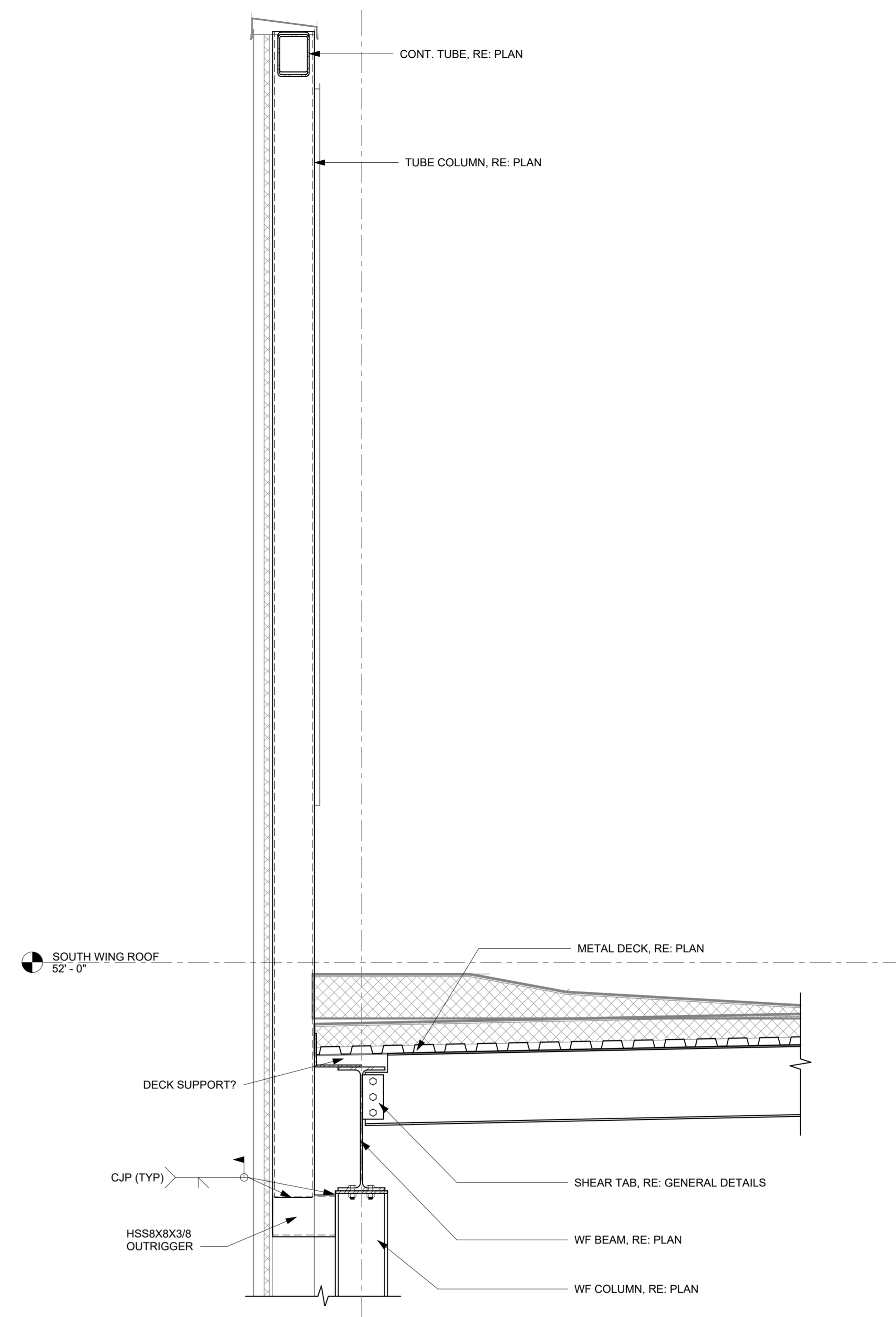
INFORMATION



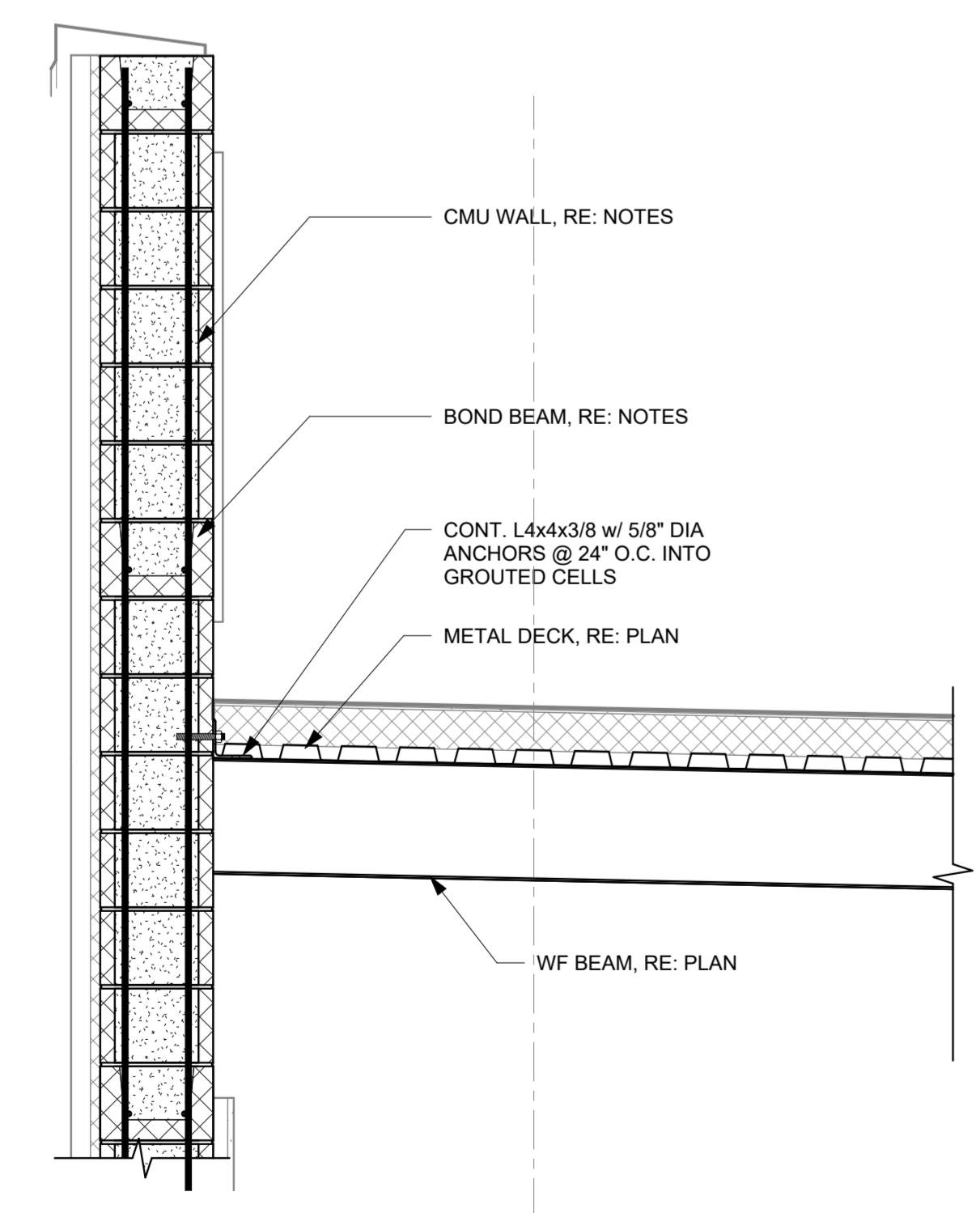
1 FRS - SECTION
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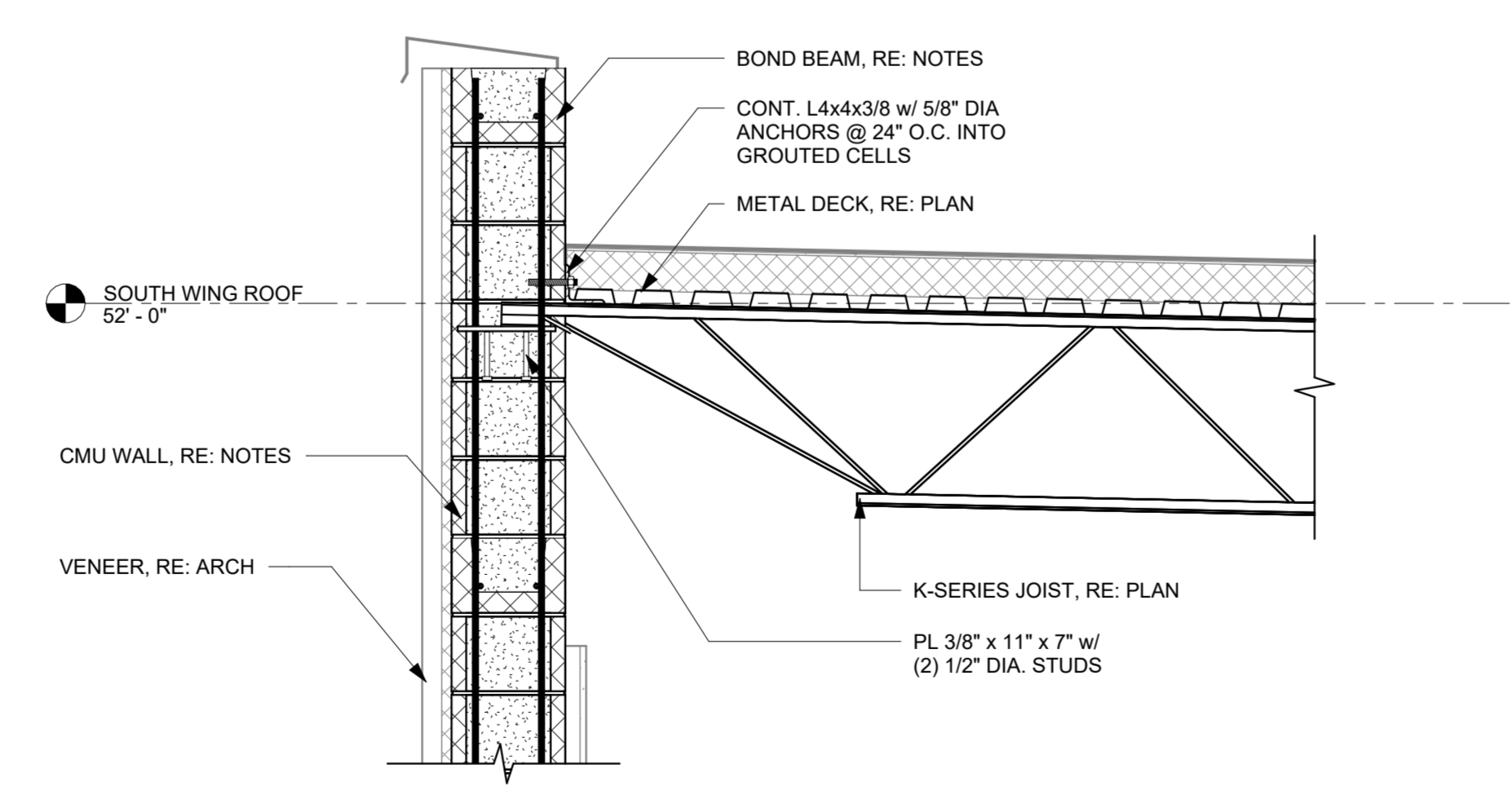
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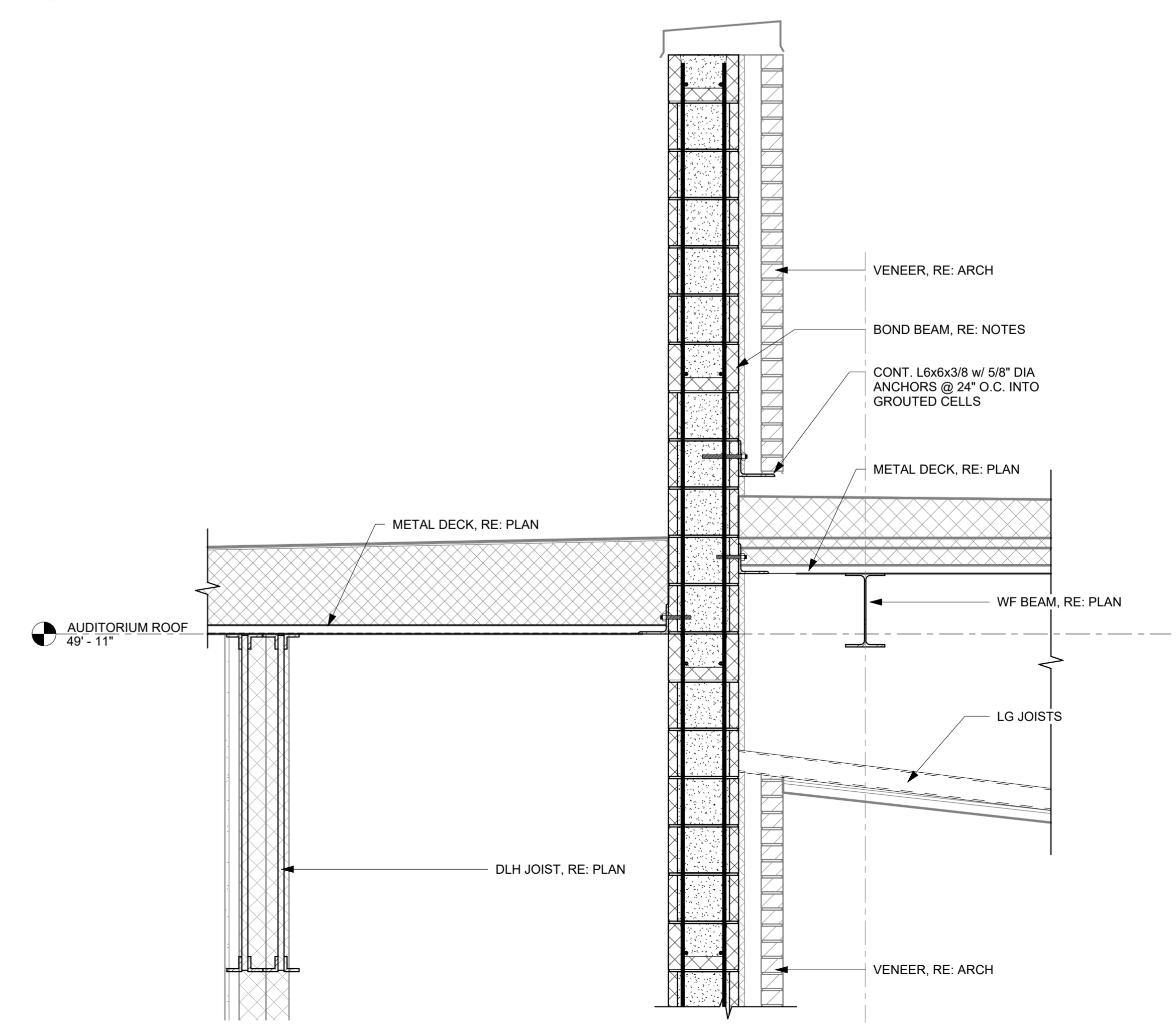
3 FRS - SECTION
3/4" = 1'-0"



4 FRS - SECTION
3/4" = 1'-0"



5 FRS - SECTION
3/4" = 1'-0"



6 FRS - SECTION
3/4" = 1'-0"



SHEET TITLE

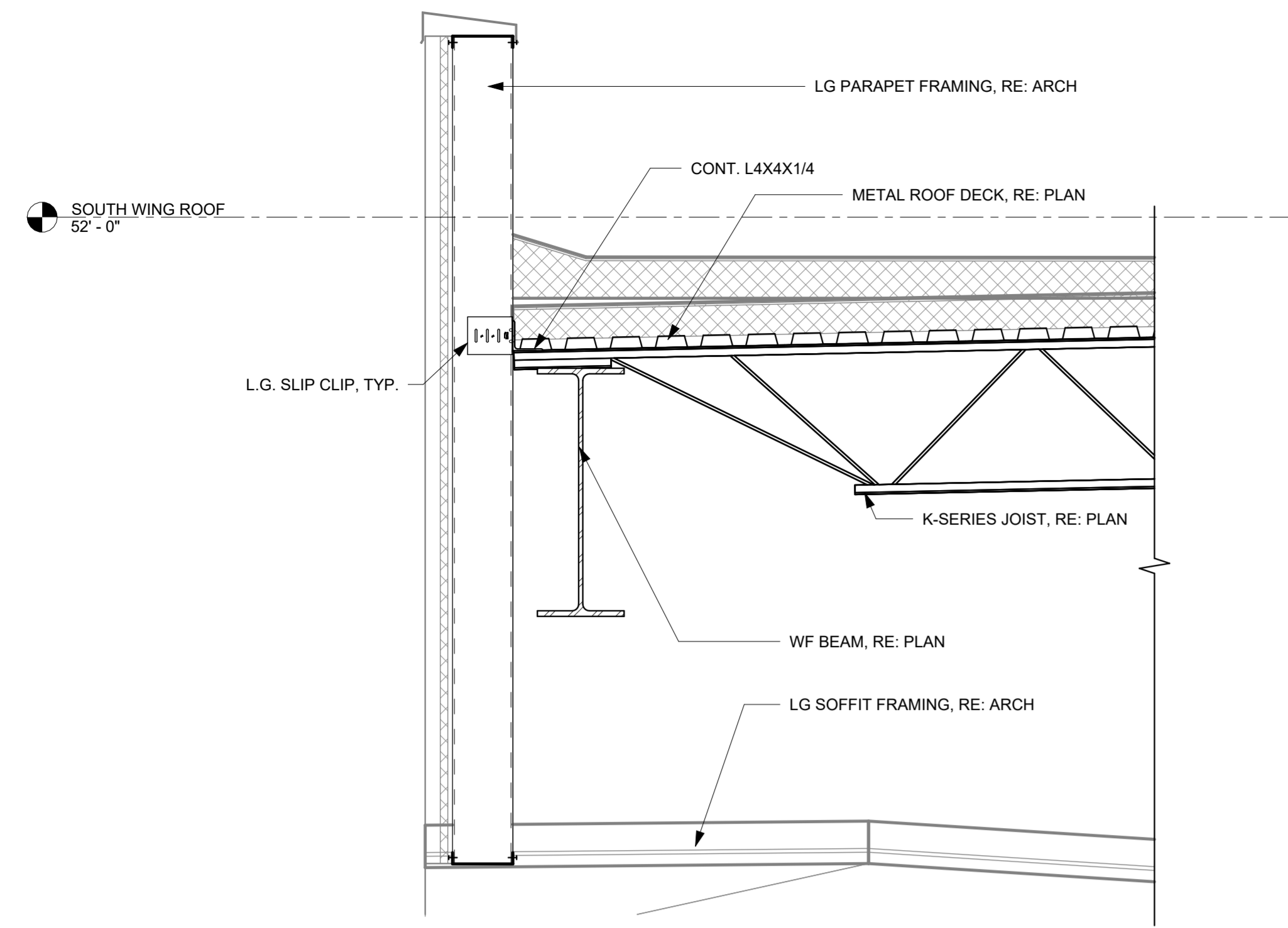
ROOF FRAMING SECTIONS - AREA A SOUTH

DATE

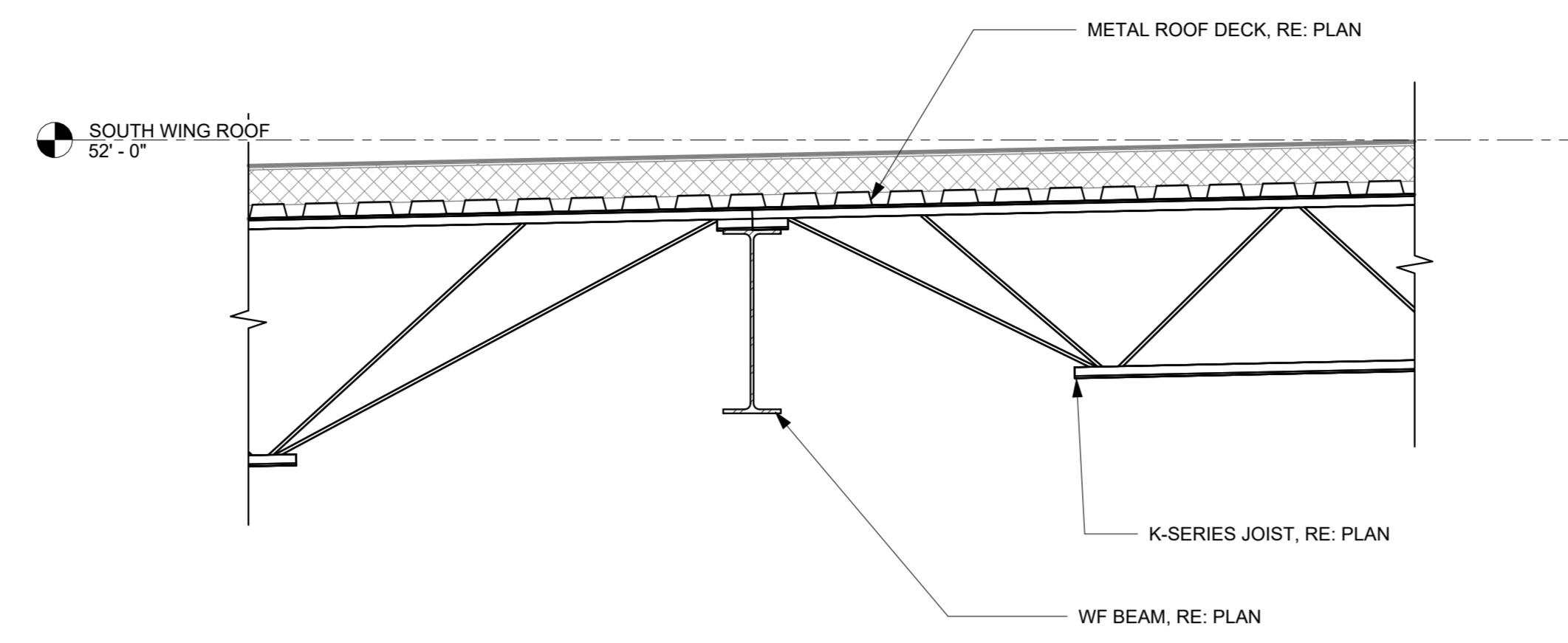
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SHEET NUMBER

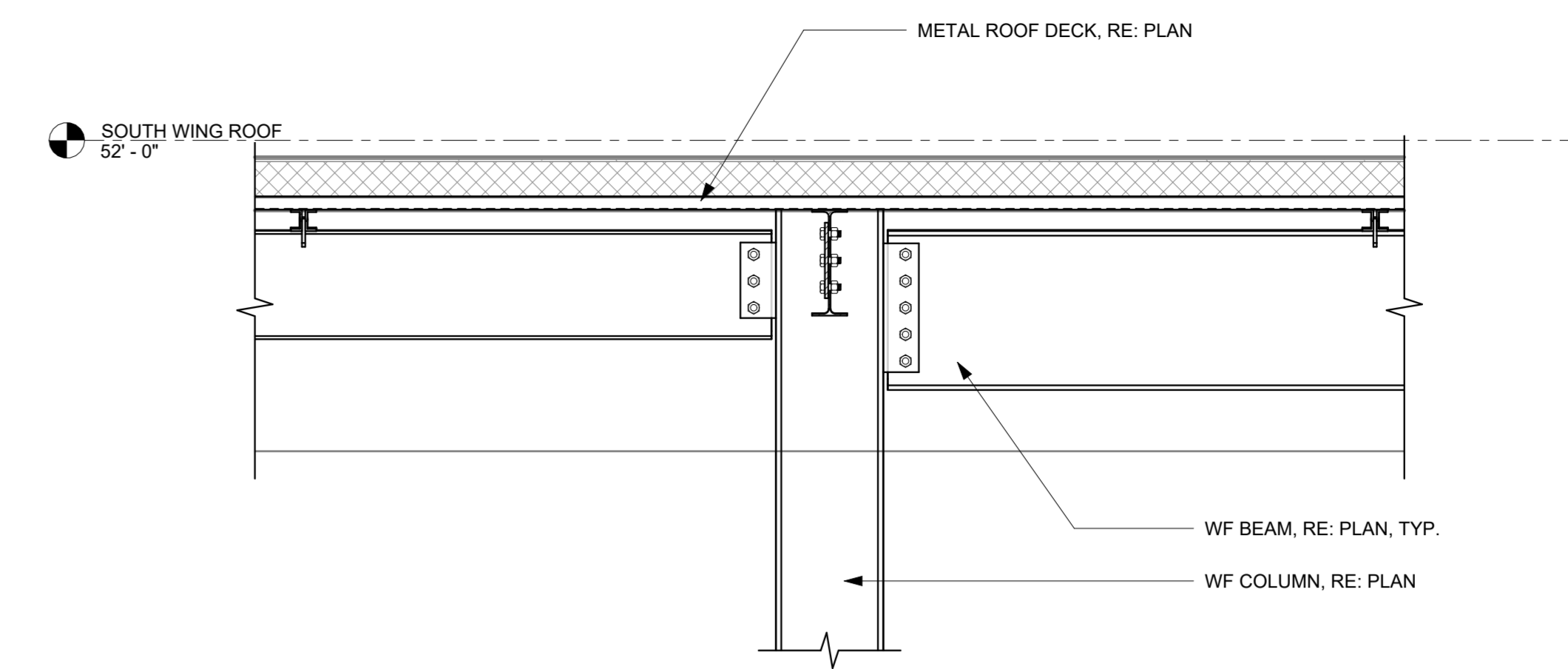
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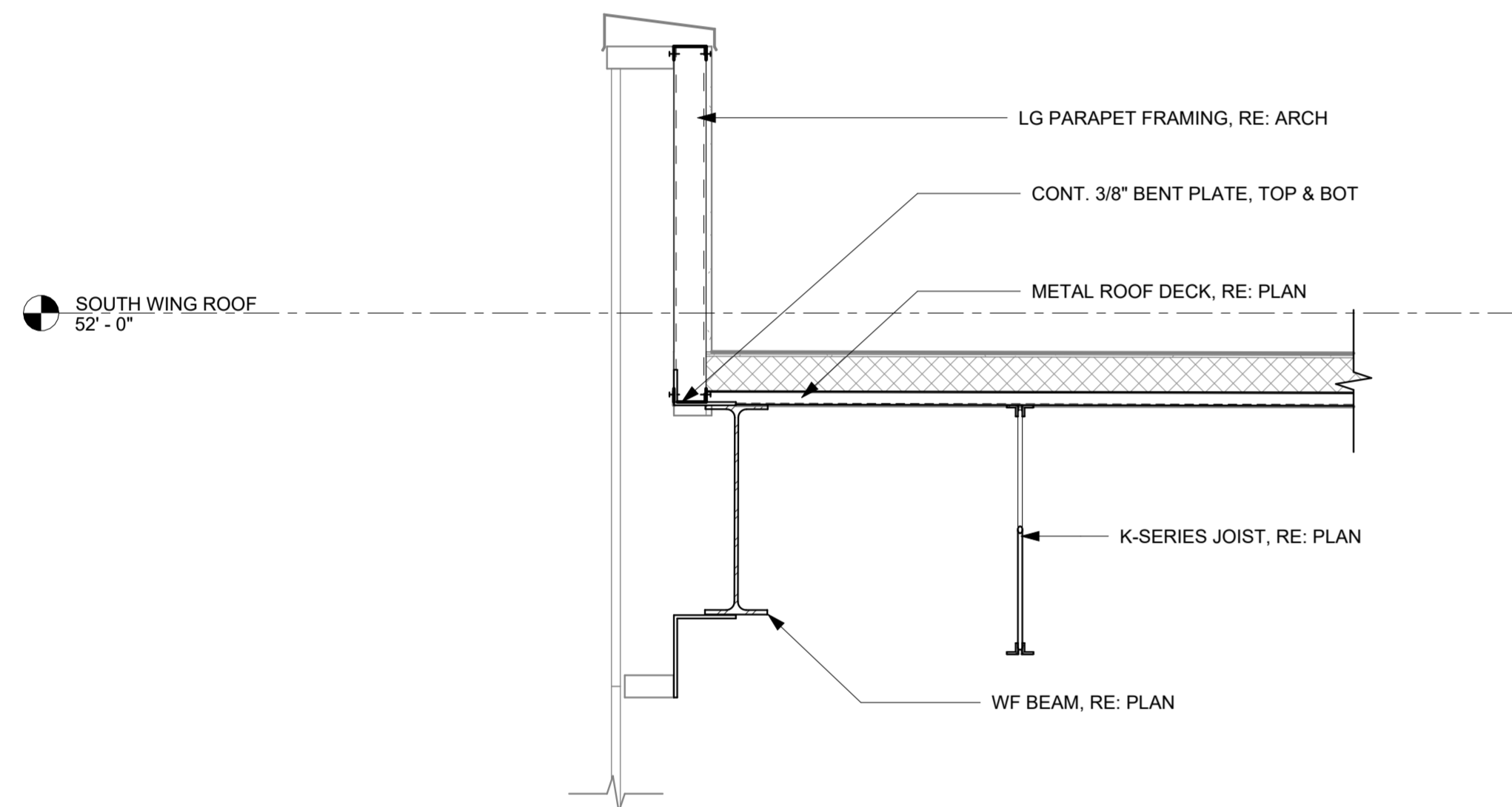
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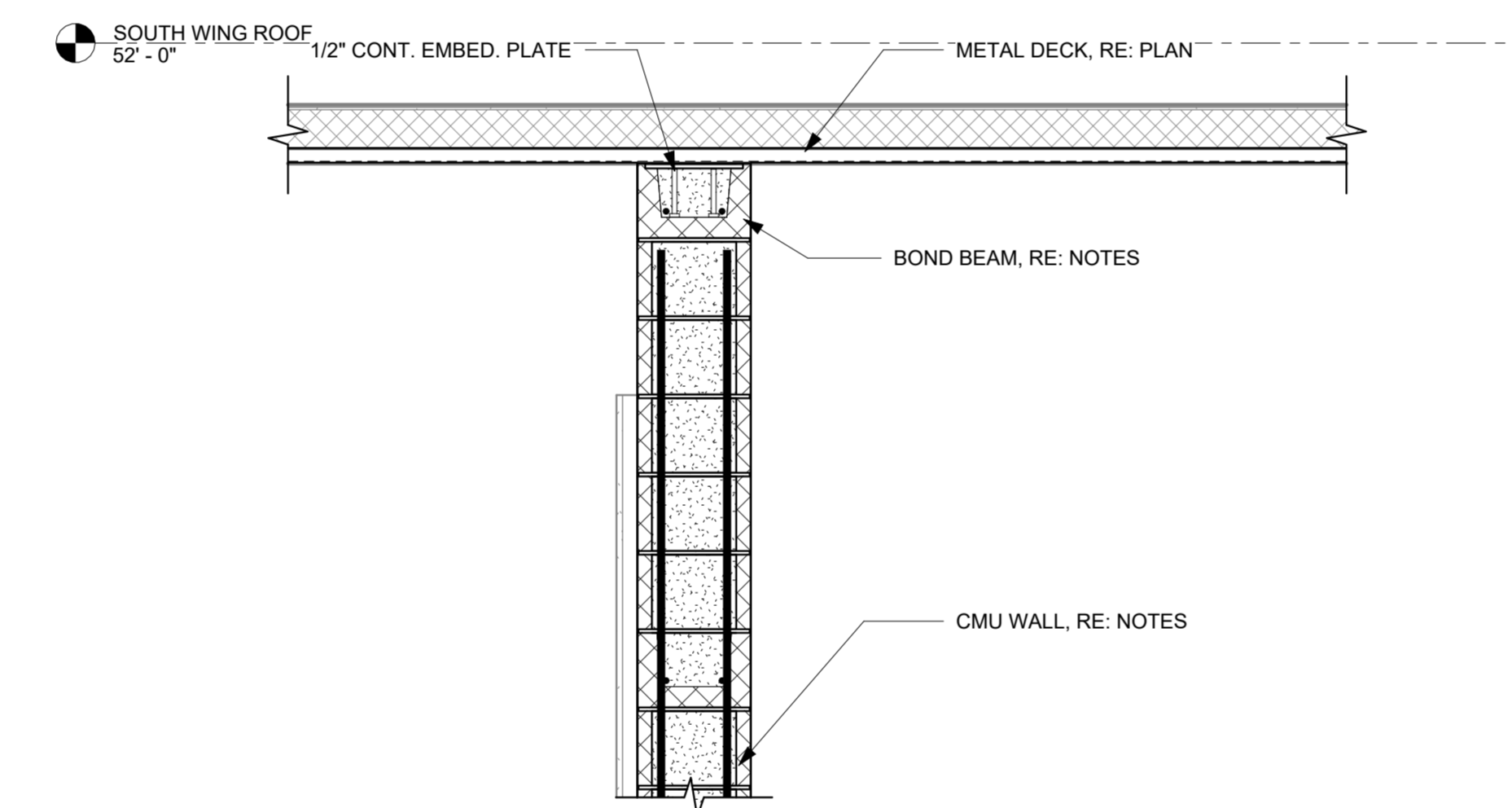
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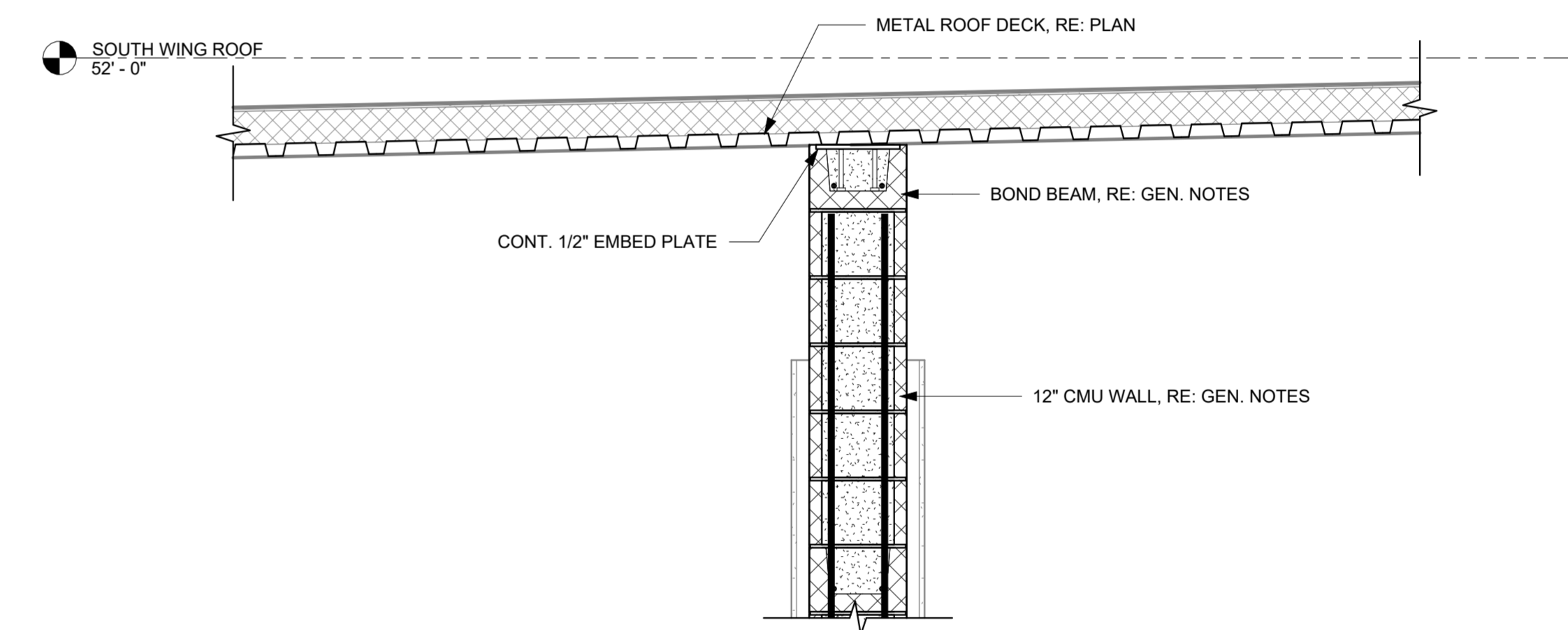
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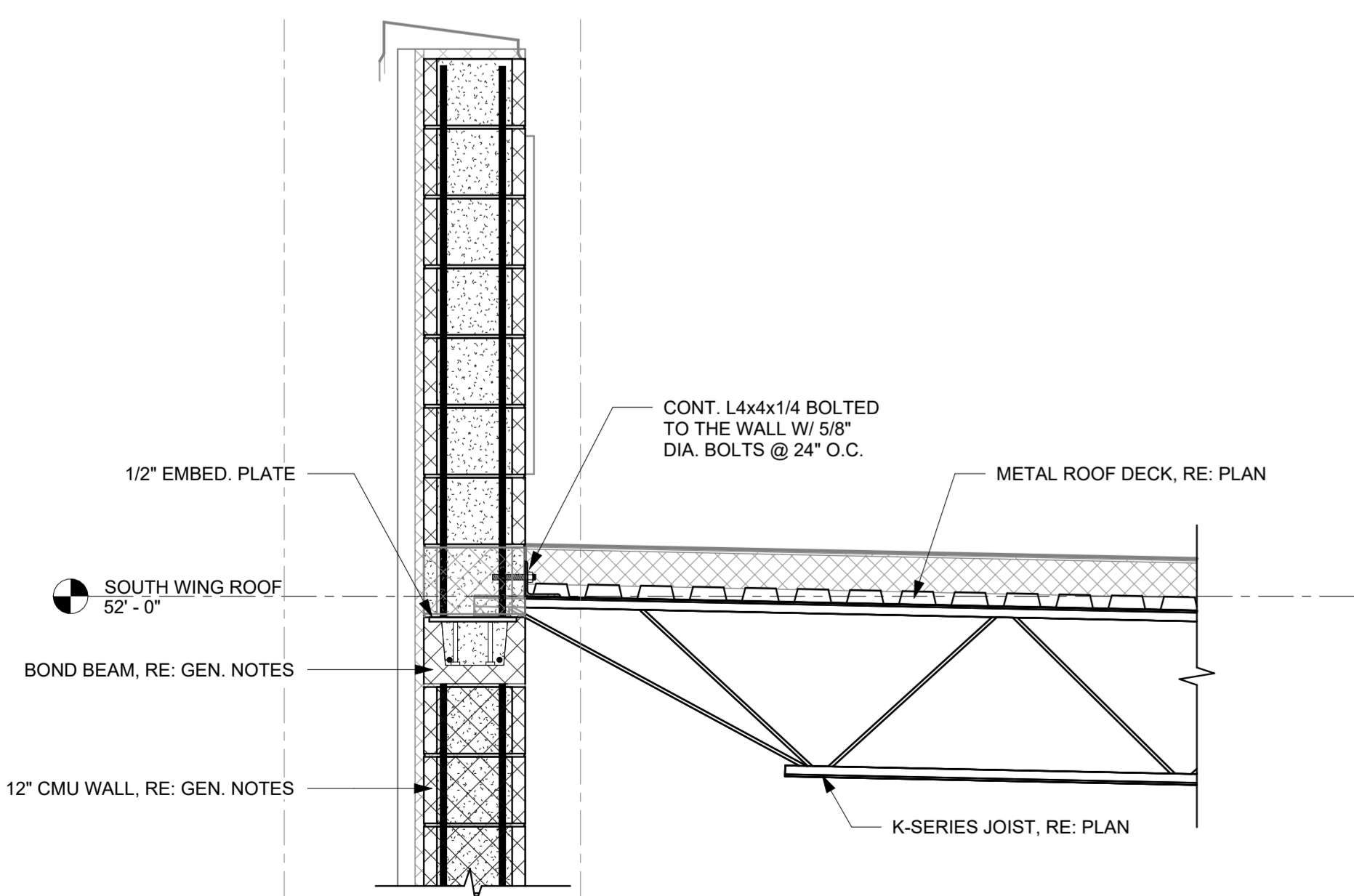
4 FRS - SECTION
3/4" = 1'-0"



5 FRS - SECTION
3/4" = 1'-0"

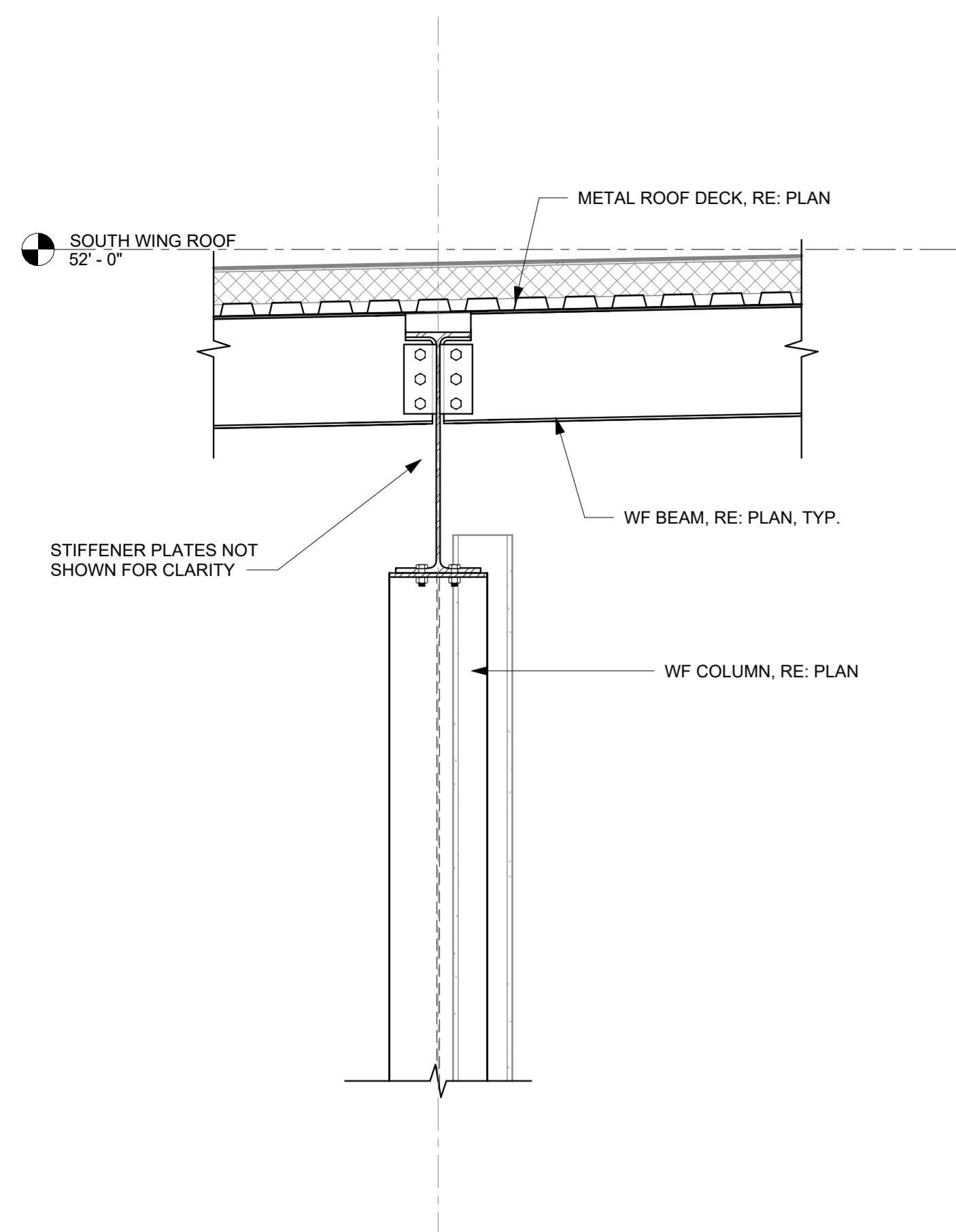


6 FRS - SECTION
3/4" = 1'-0"

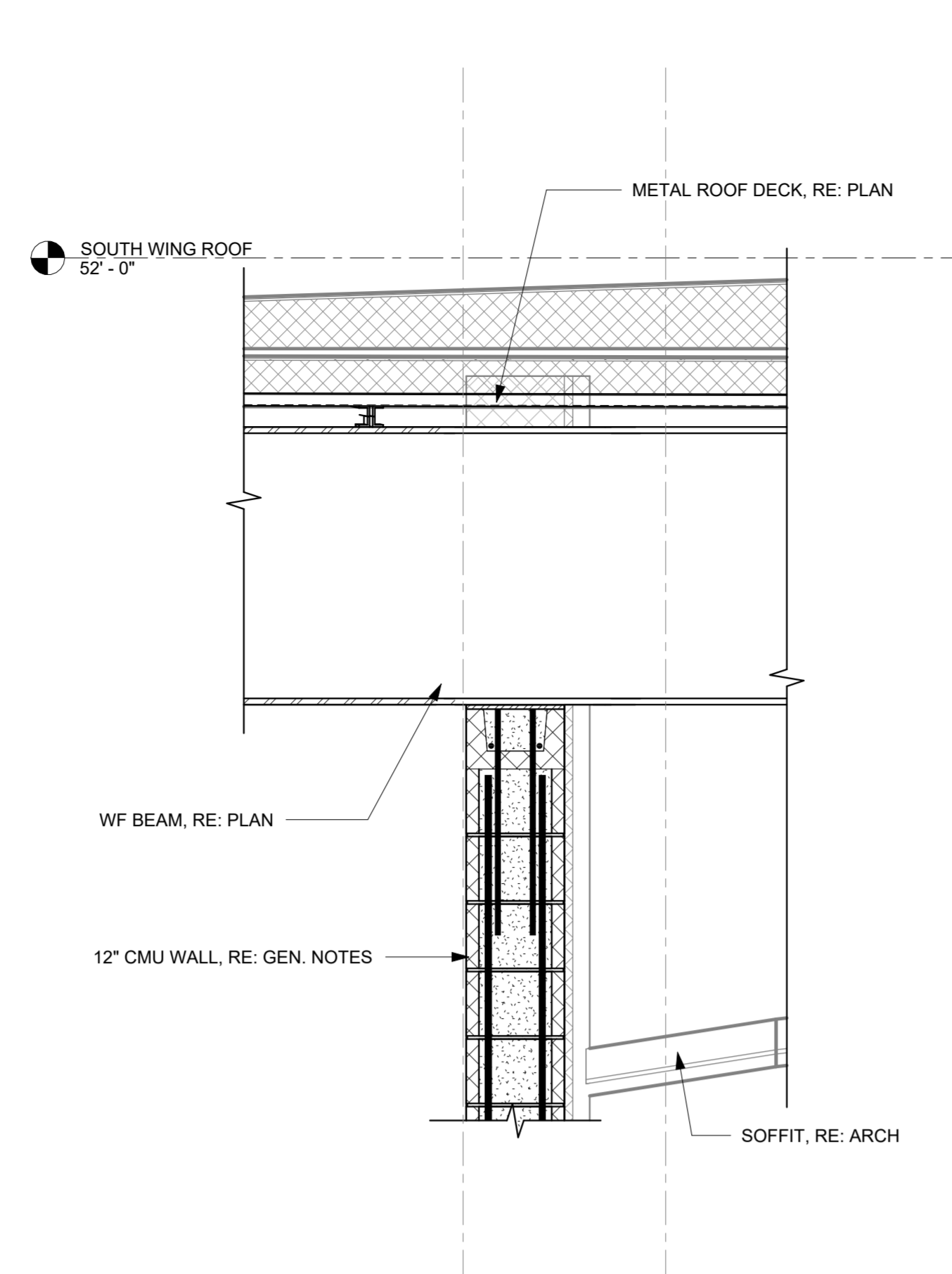


7 FRS - SECTION
3/4" = 1'-0"

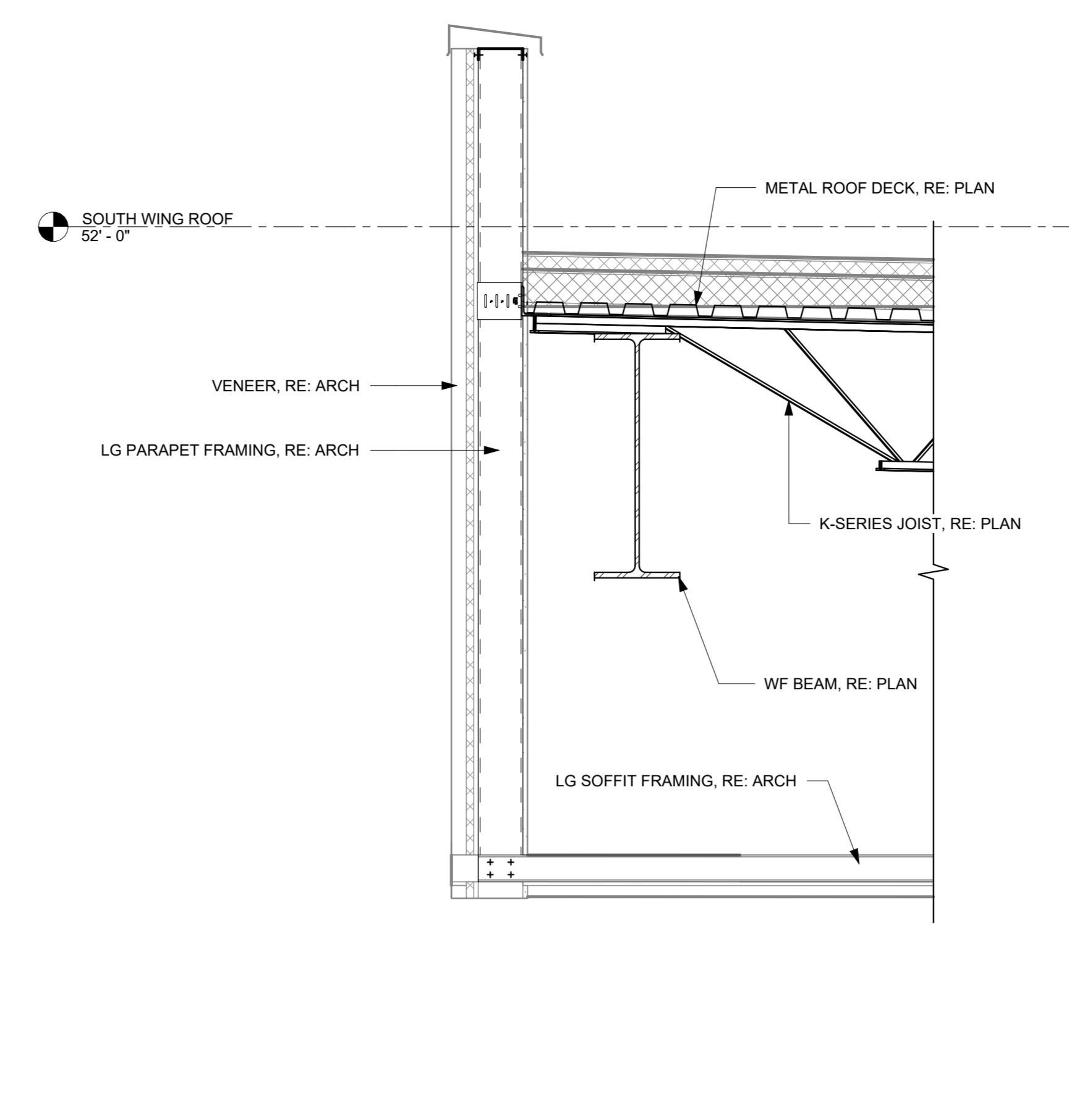




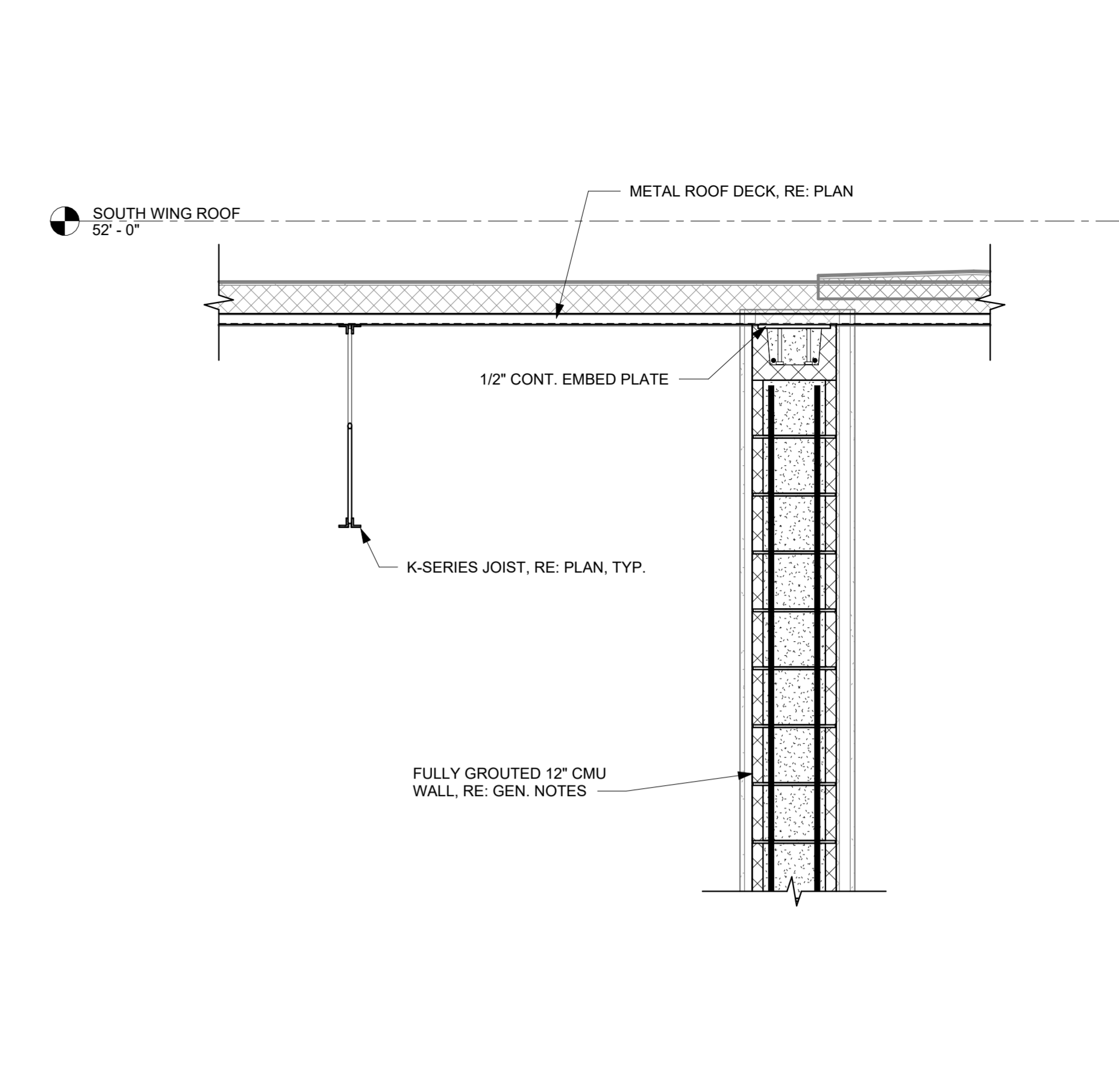
1 FRS - SECTION
3/4" = 1'-0"



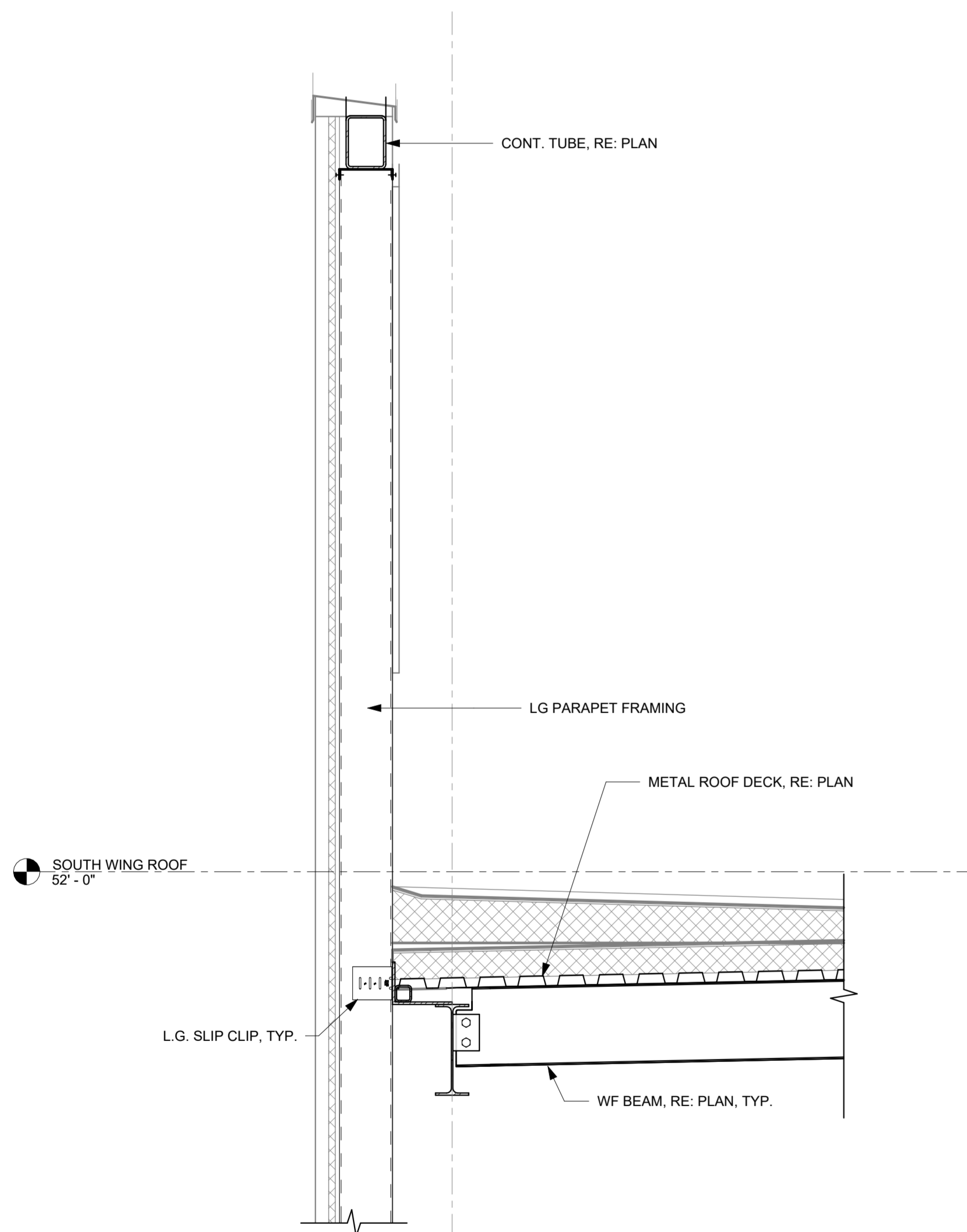
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3/4" = 1'-0"



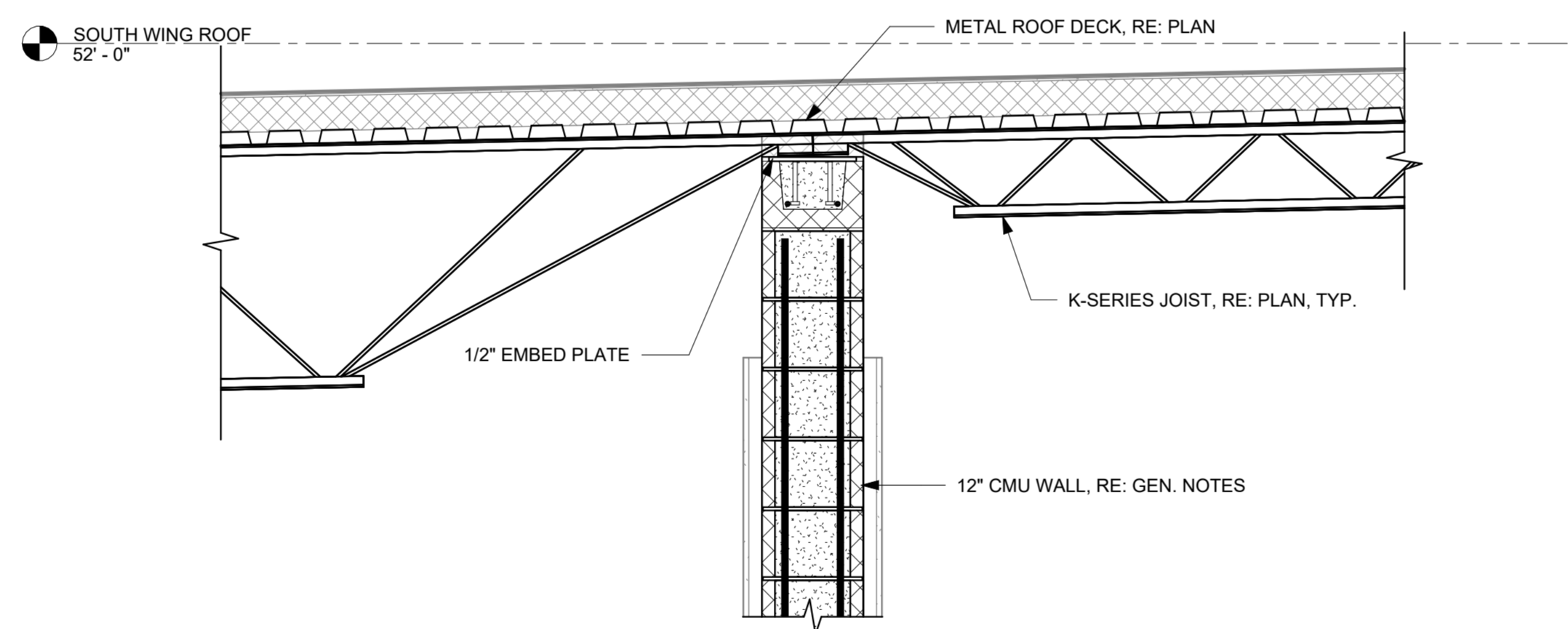
3 FRS - SECTION
3/4" = 1'-0"



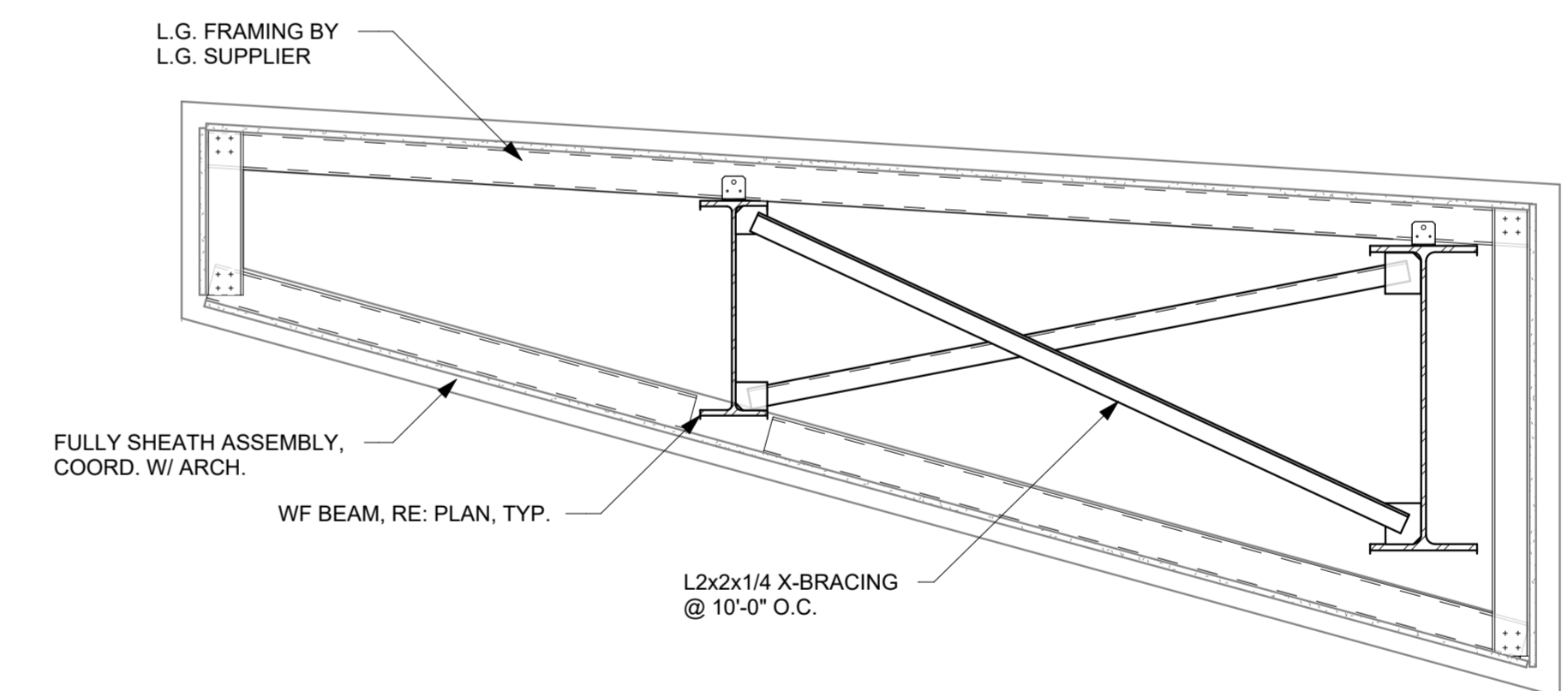
4 FRS - SECTION
3/4" = 1'-0"



5 FRS - SECTION
3/4" = 1'-0"



6 FRS - SECTION
3/4" = 1'-0"



7 FRS - SECTION
3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

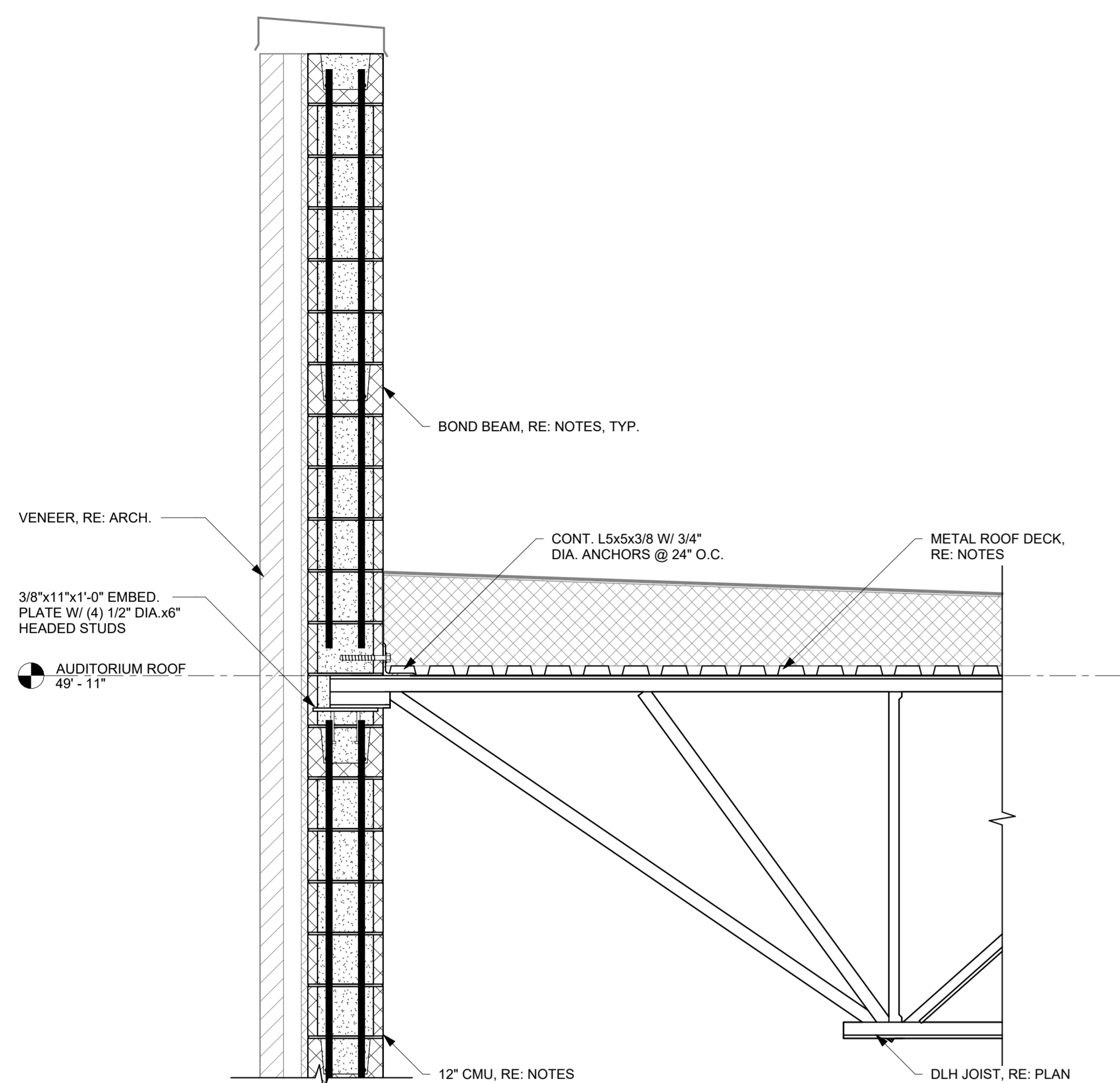
PROJECT NUMBER

-

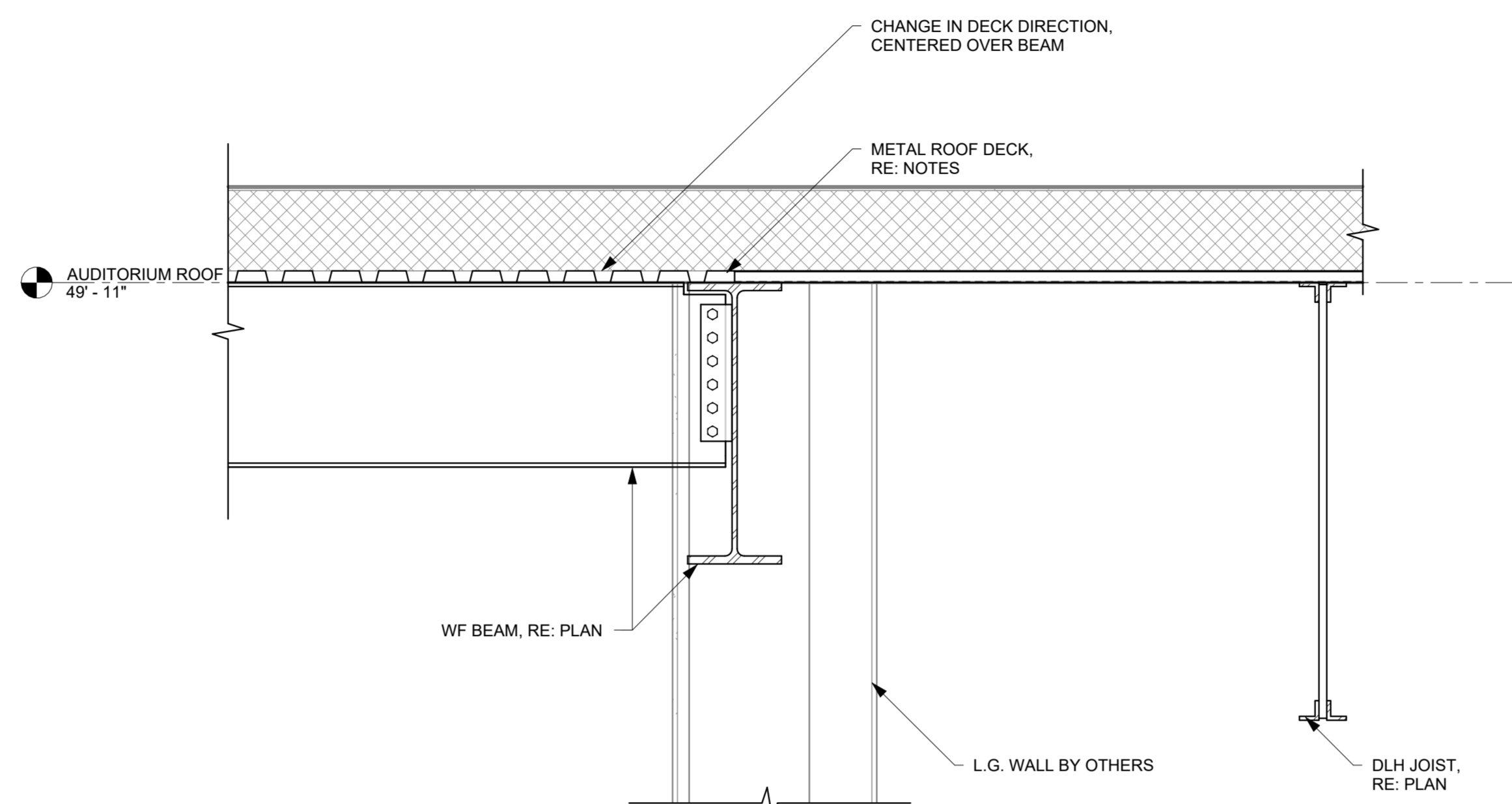
DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

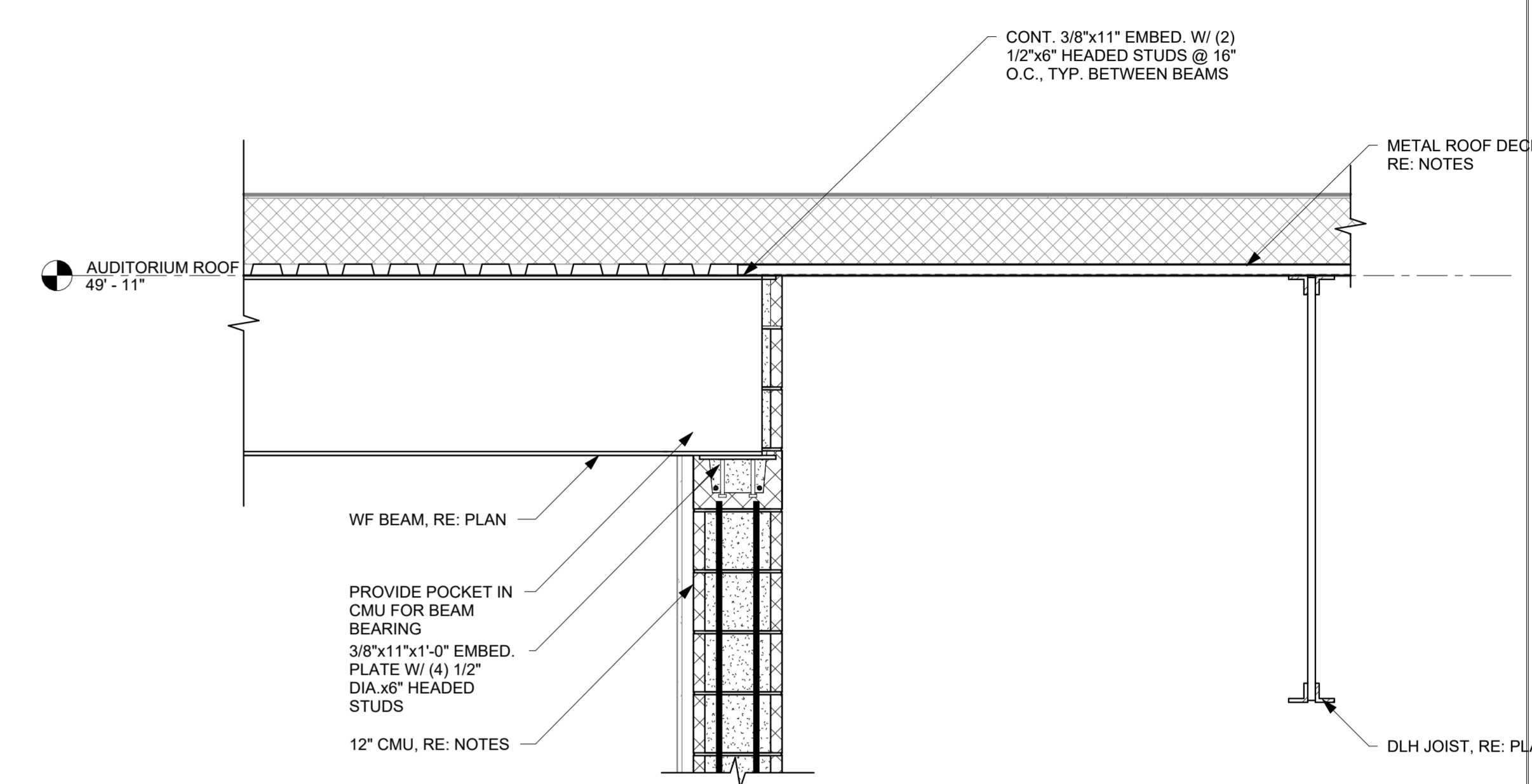
INFORMATION



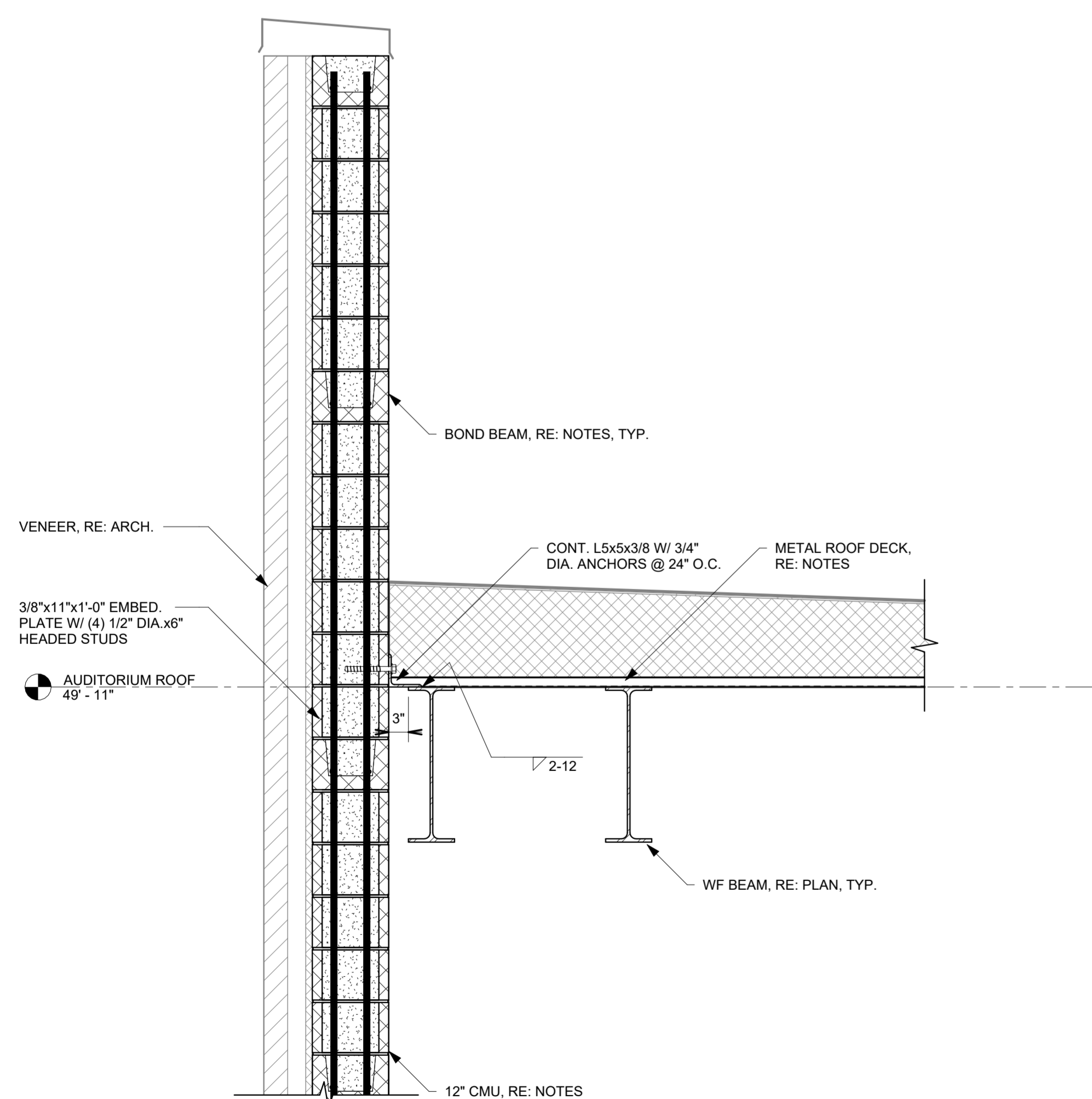
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3/4" = 1'-0"



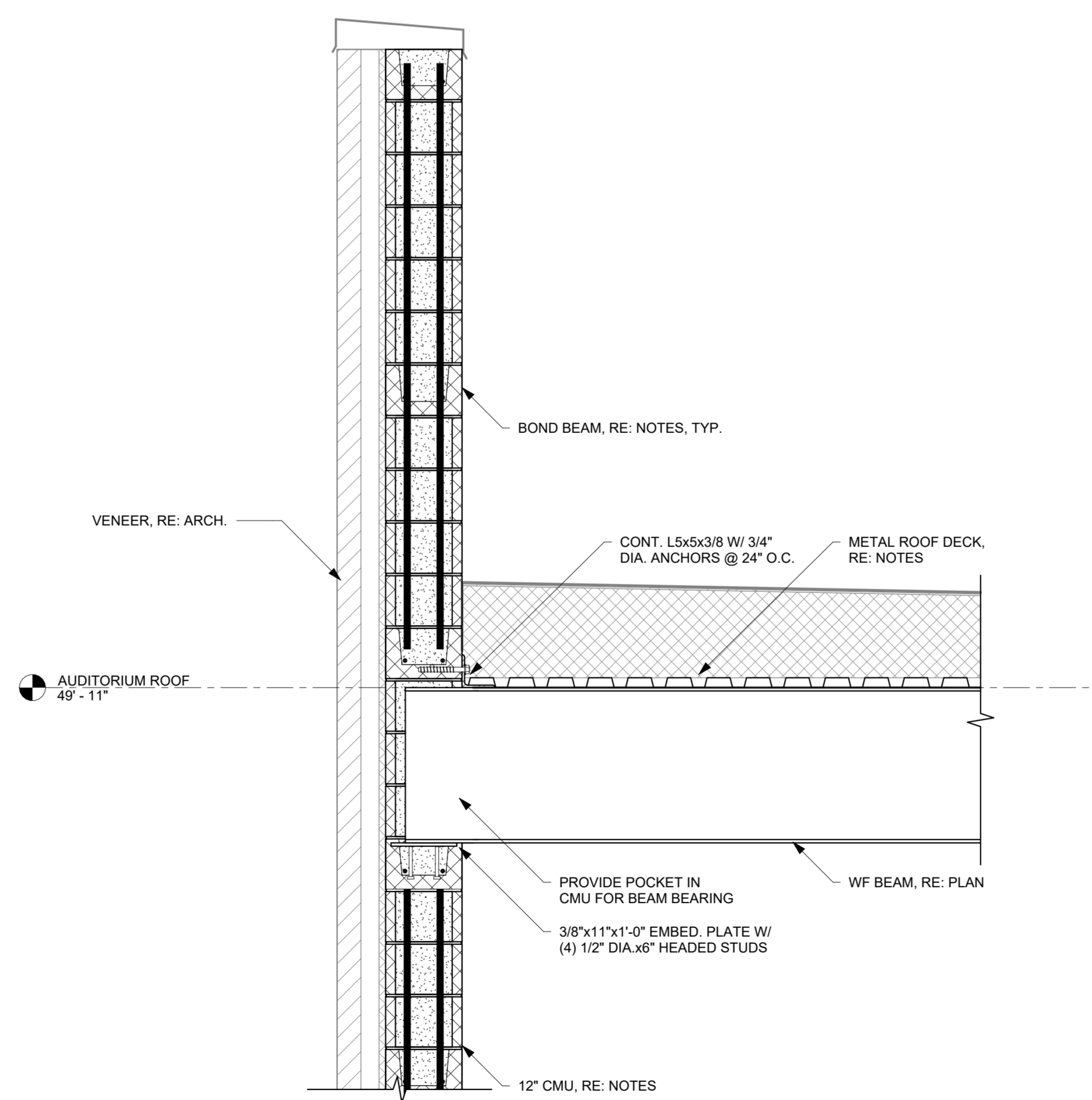
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3/4" = 1'-0"



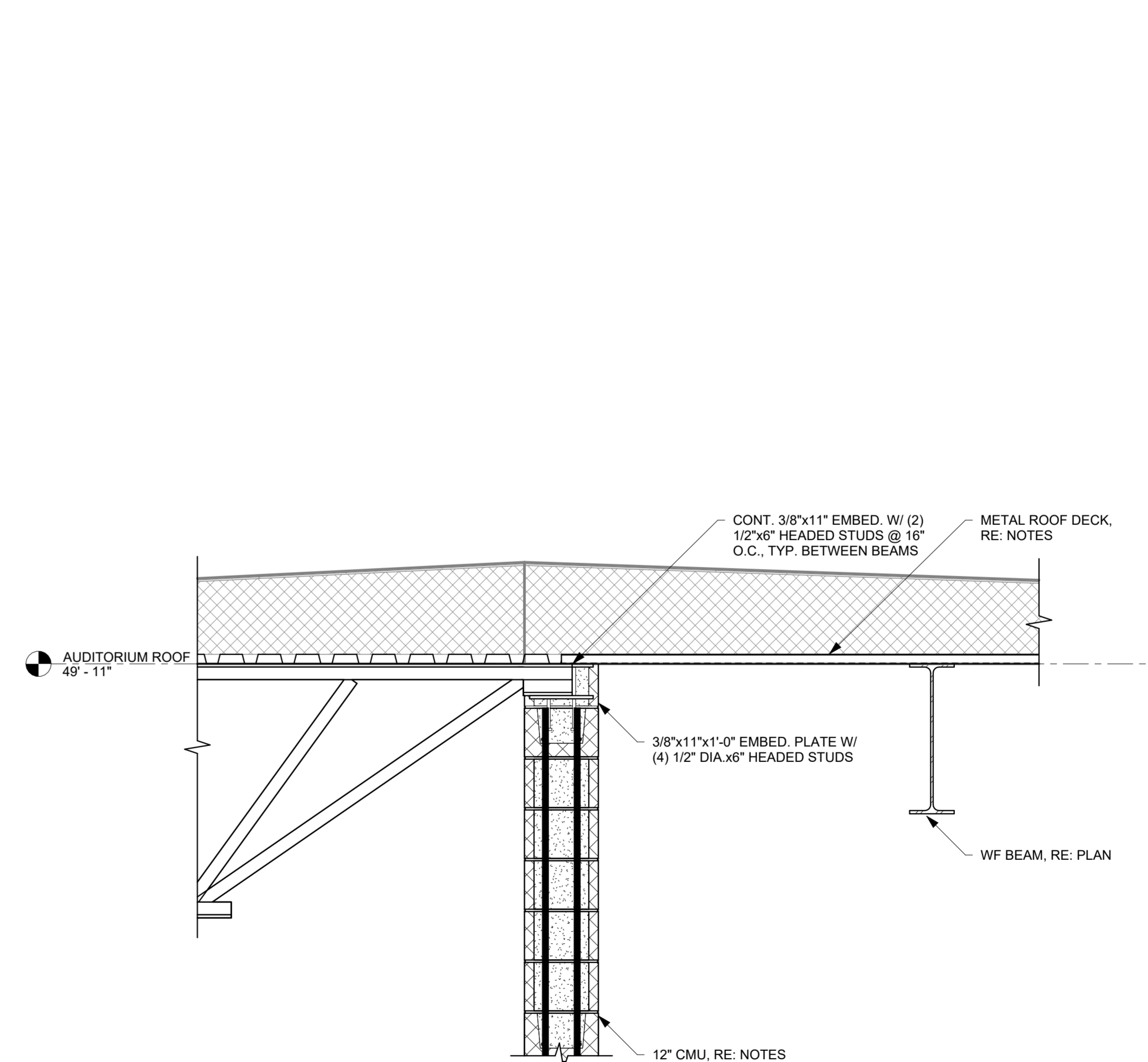
3 FRS - SECTION
3/4" = 1'-0"



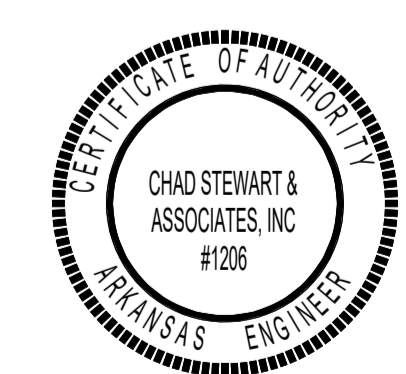
4 FRS - SECTION
3/4" = 1'-0"



5 FRS - SECTION
3/4" = 1'-0"



6 FRS - SECTION
3/4" = 1'-0"



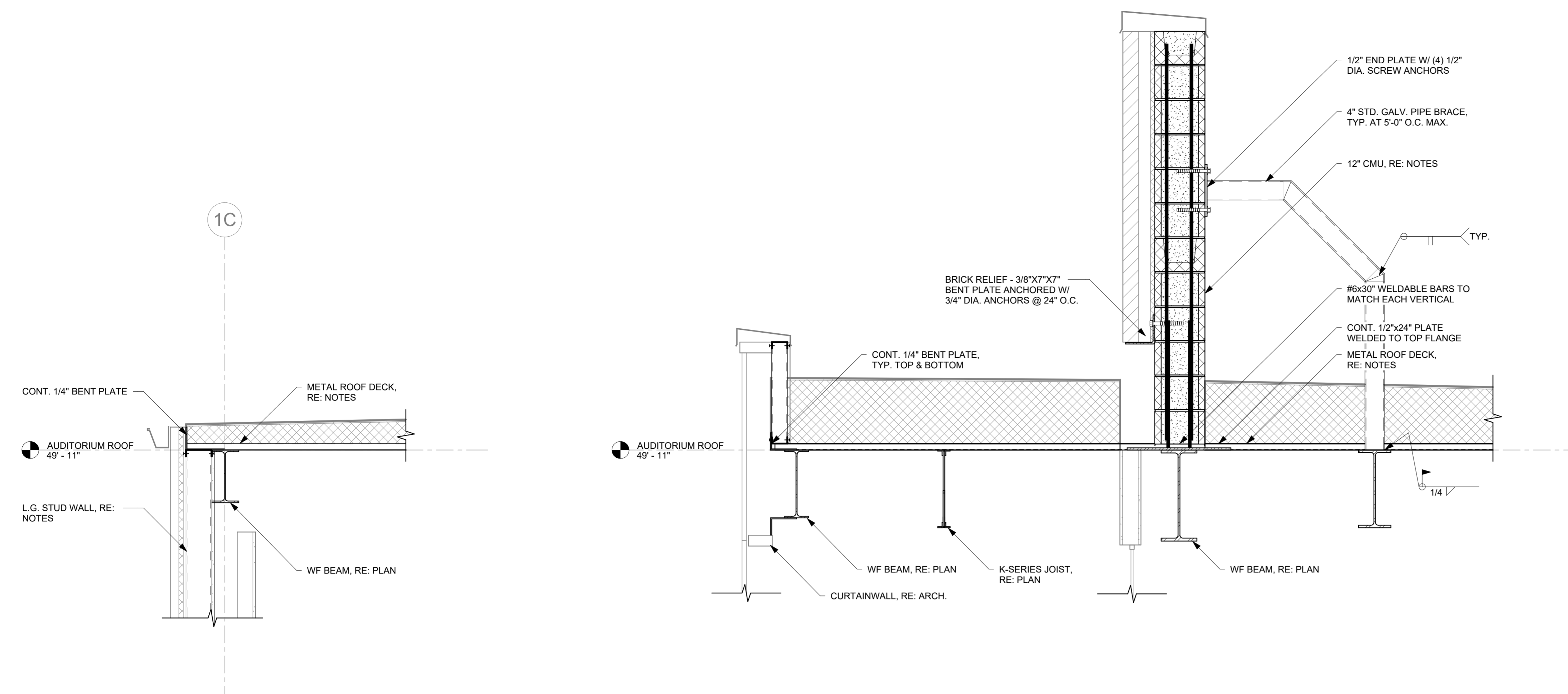
SHEET TITLE
ROOF FRAMING
SECTIONS - AREA B

DATE

17.10.24

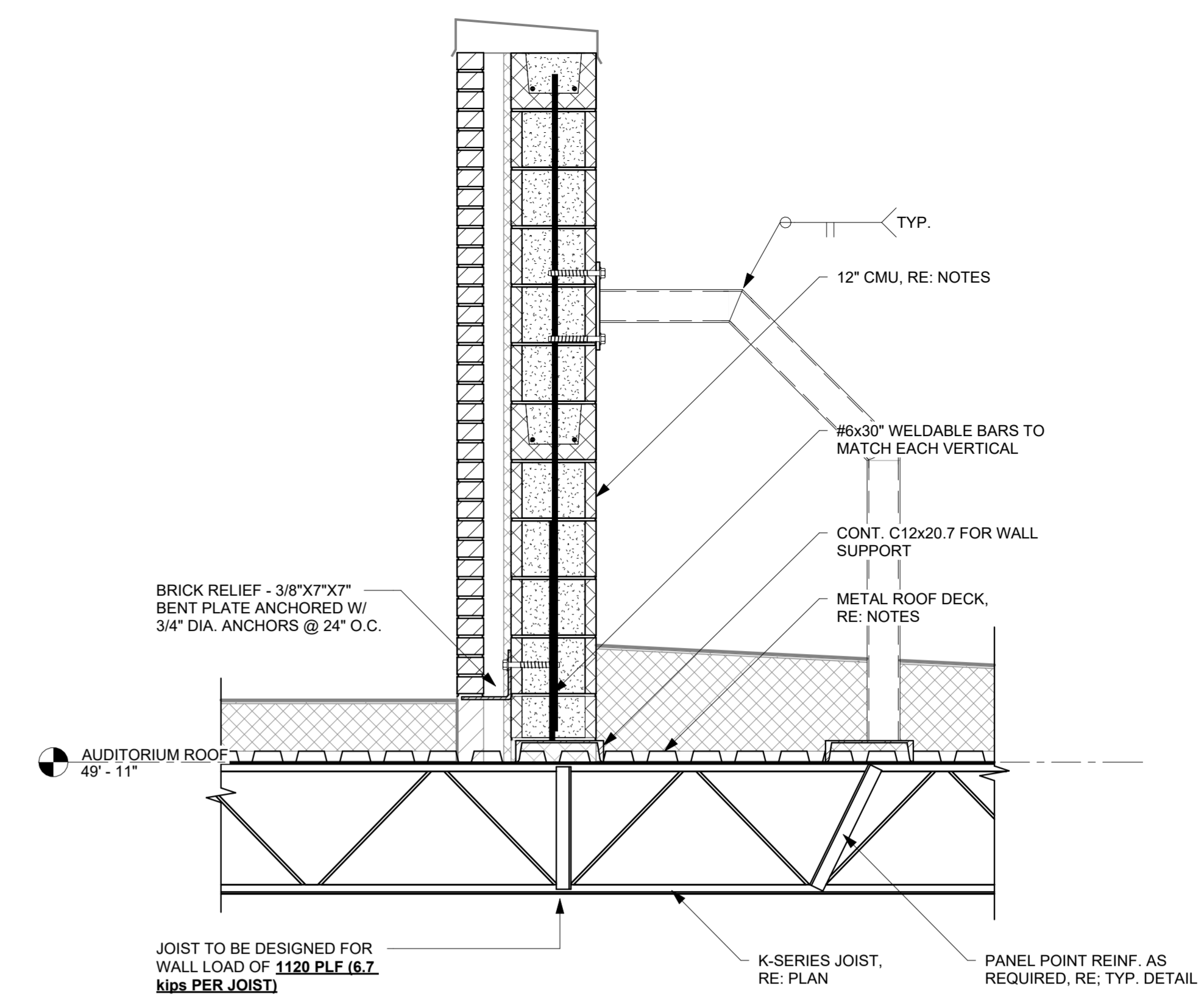
SHEET NUMBER

S403.1

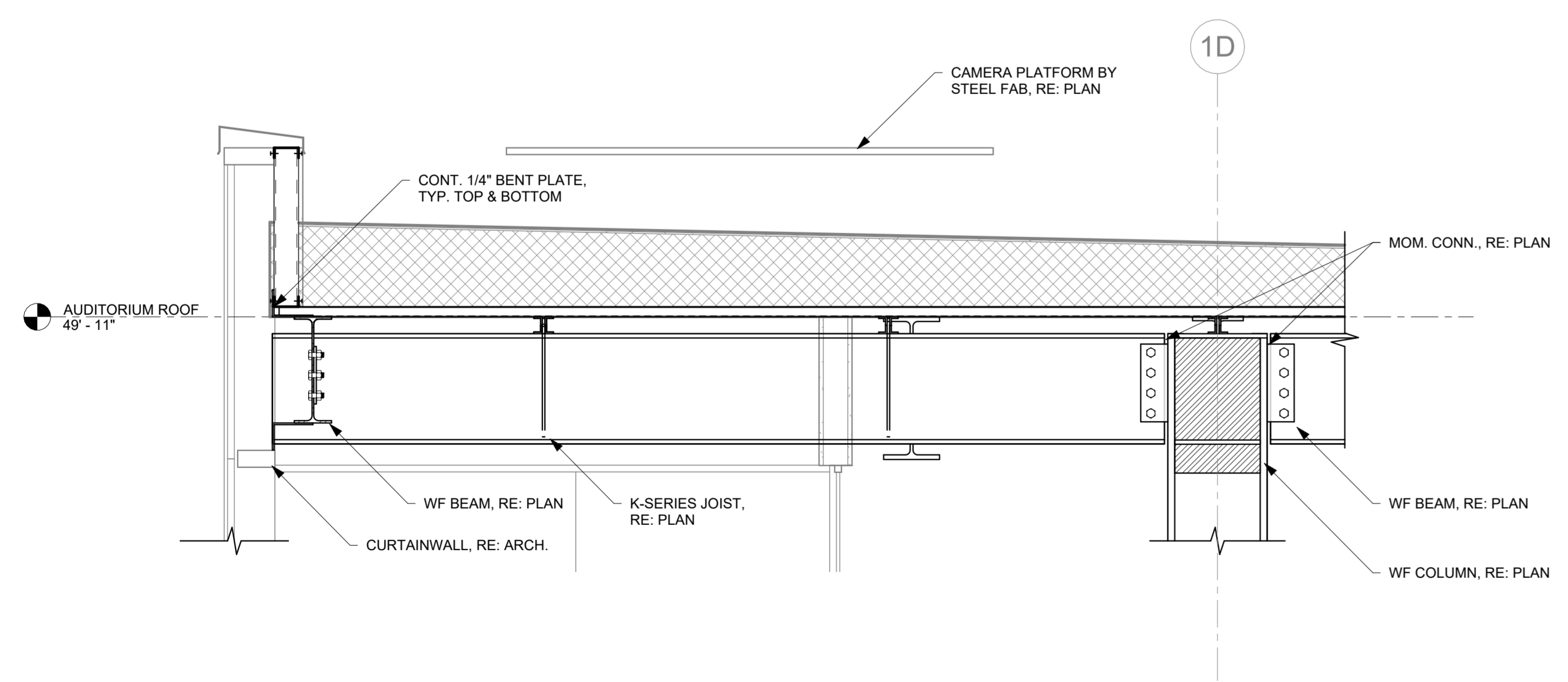


1 FRS - SECTION
 3/4" = 1'-0"

2 FRS - SECTION
 3/4" = 1'-0"



3 FRS - SECTION
 3/4" = 1'-0"



4 FRS - SECTION
 3/4" = 1'-0"





PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
 WYNNE AR 72396

PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION

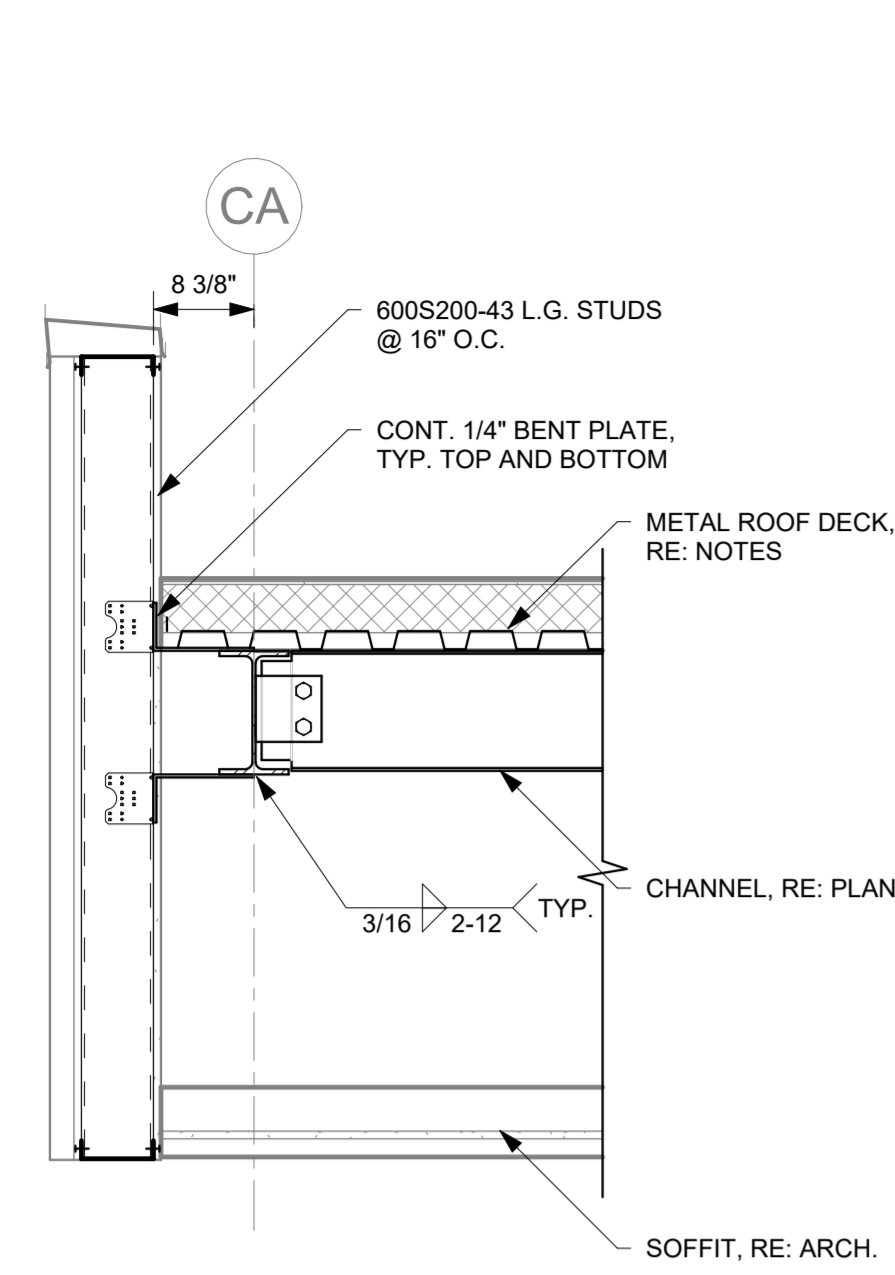


SHEET TITLE
 ROOF FRAMING
 SECTIONS - AREA C

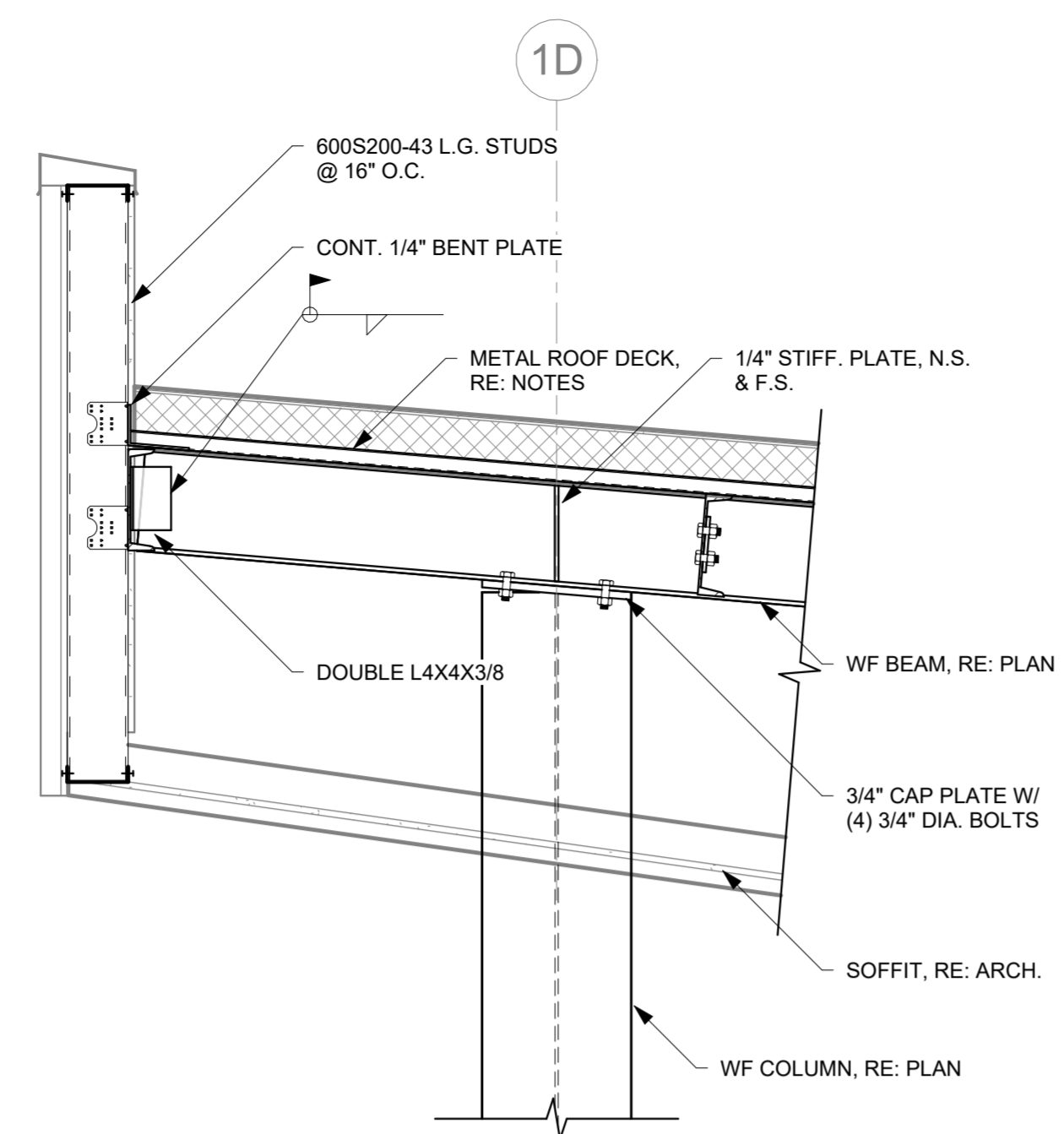
DATE
 17.10.24

SHEET NUMBER

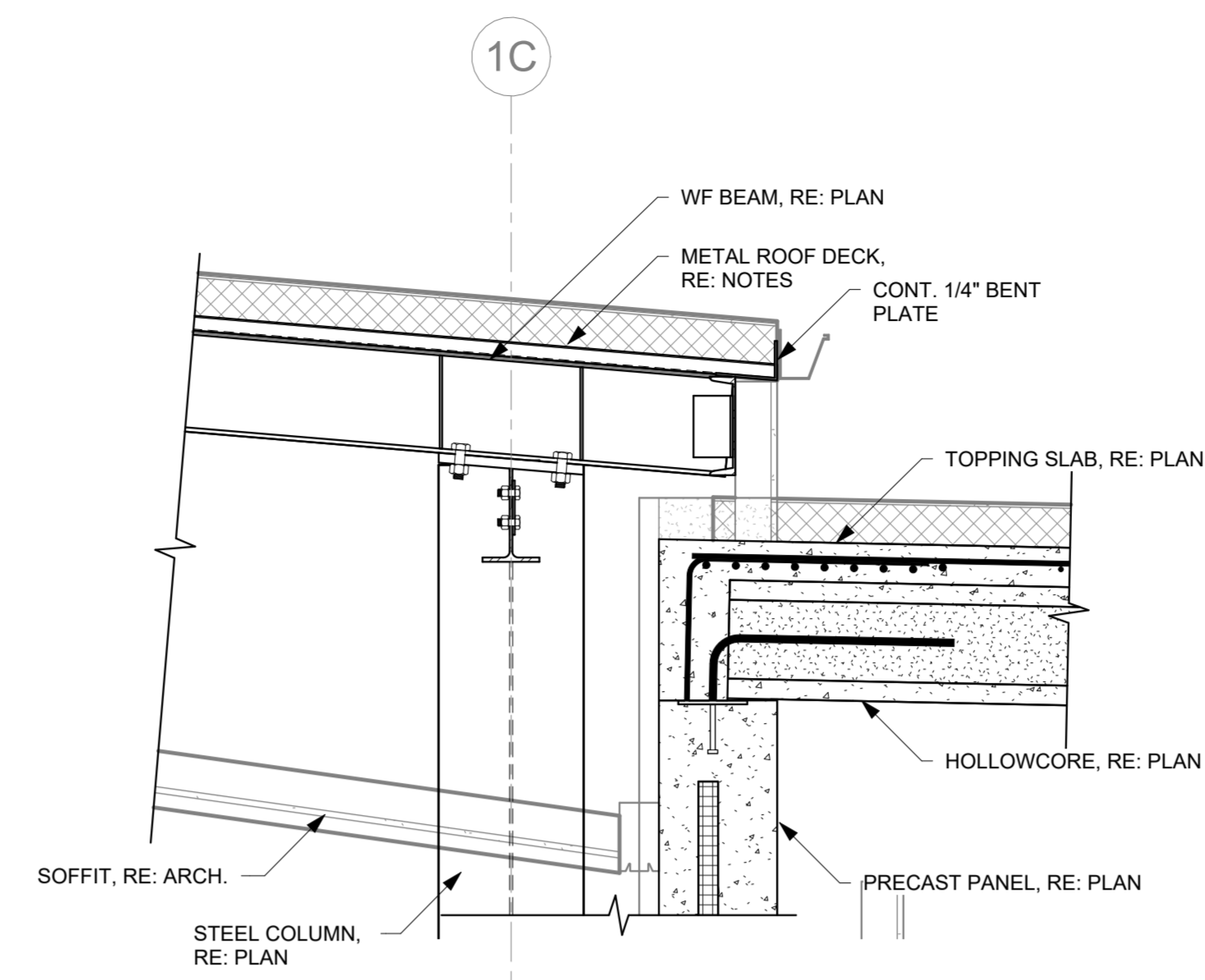
S404.1



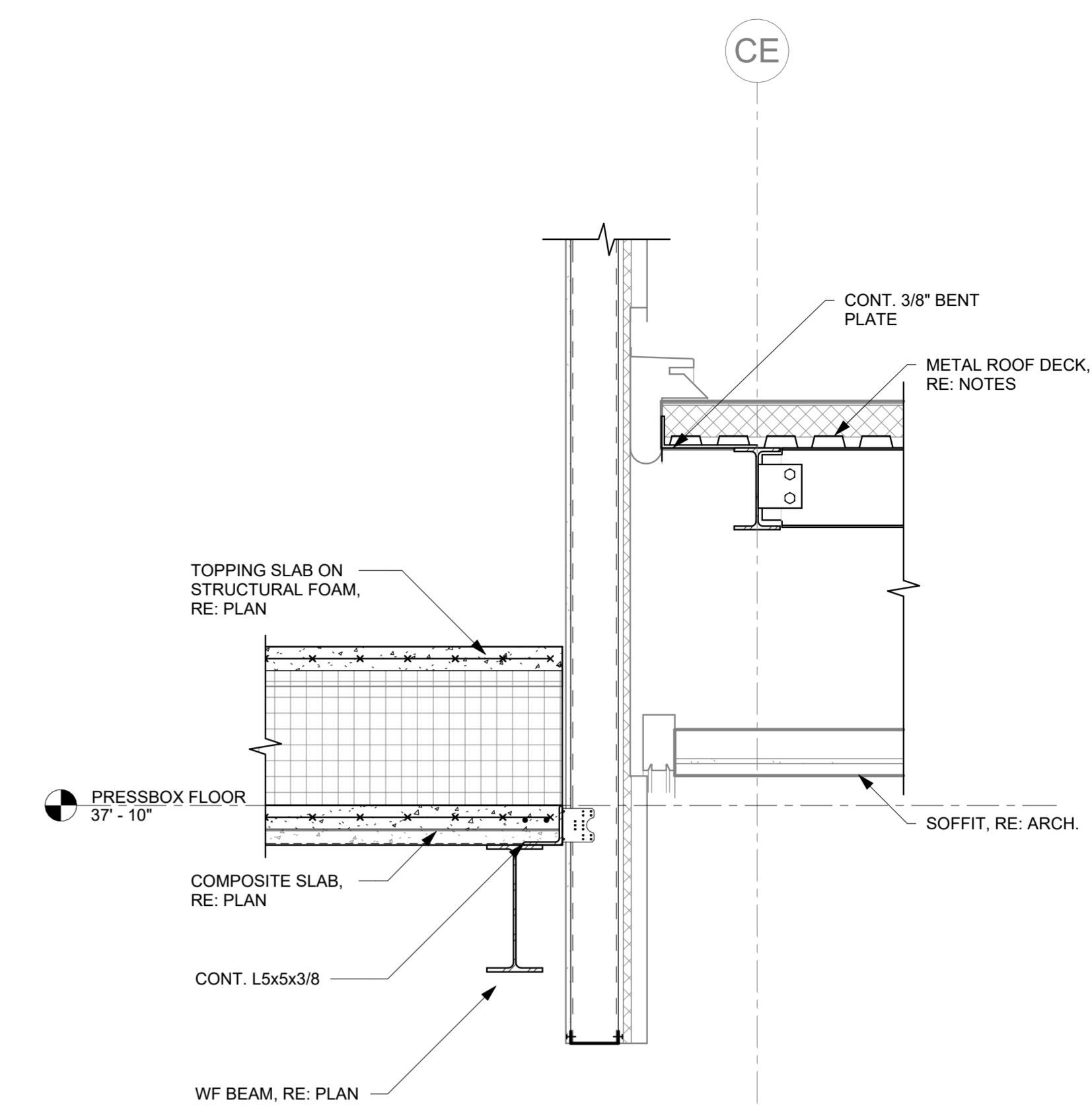
1 FRS - SECTION
 3/4" = 1'-0"



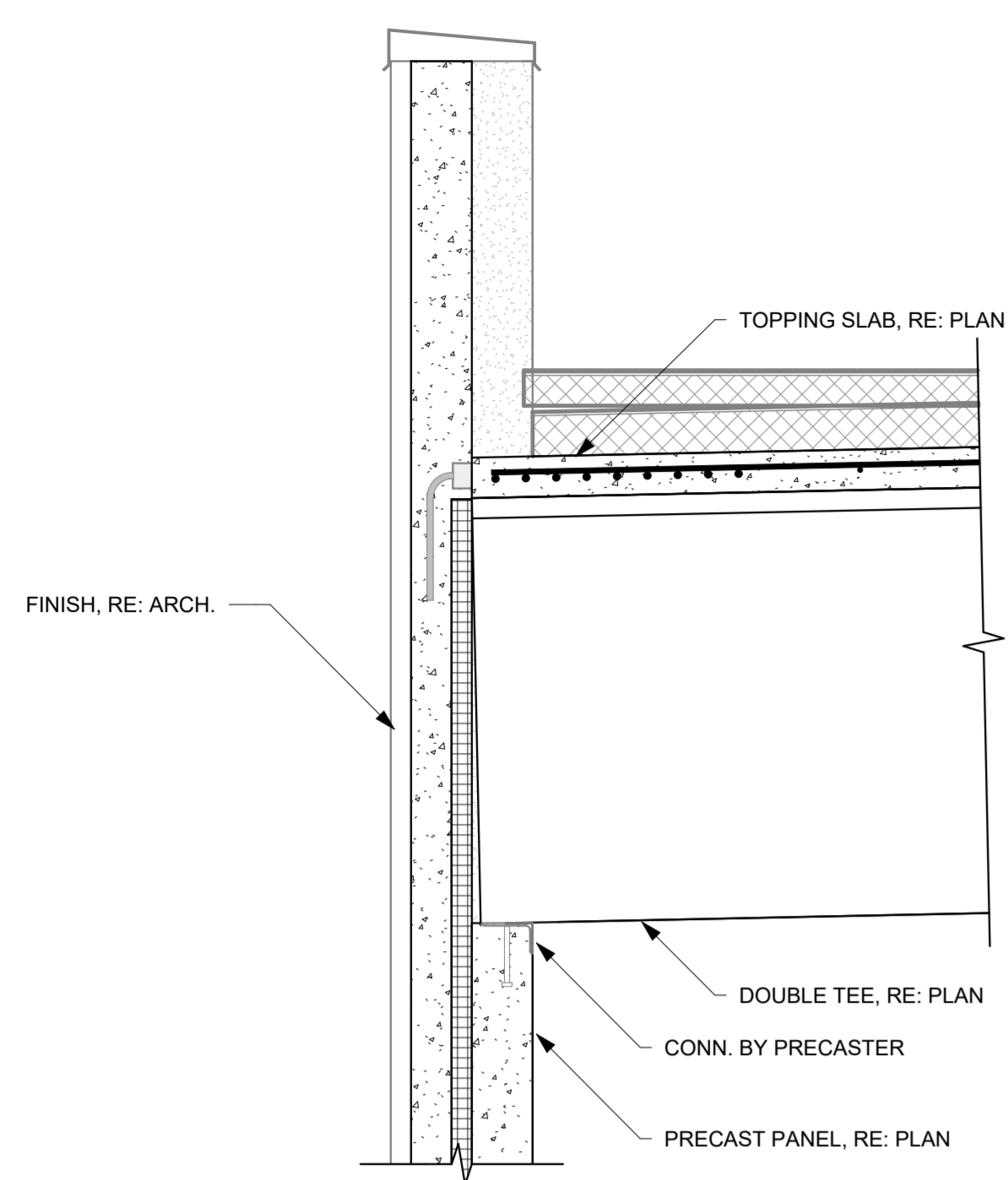
2 FRS - SECTION
 3/4" = 1'-0"



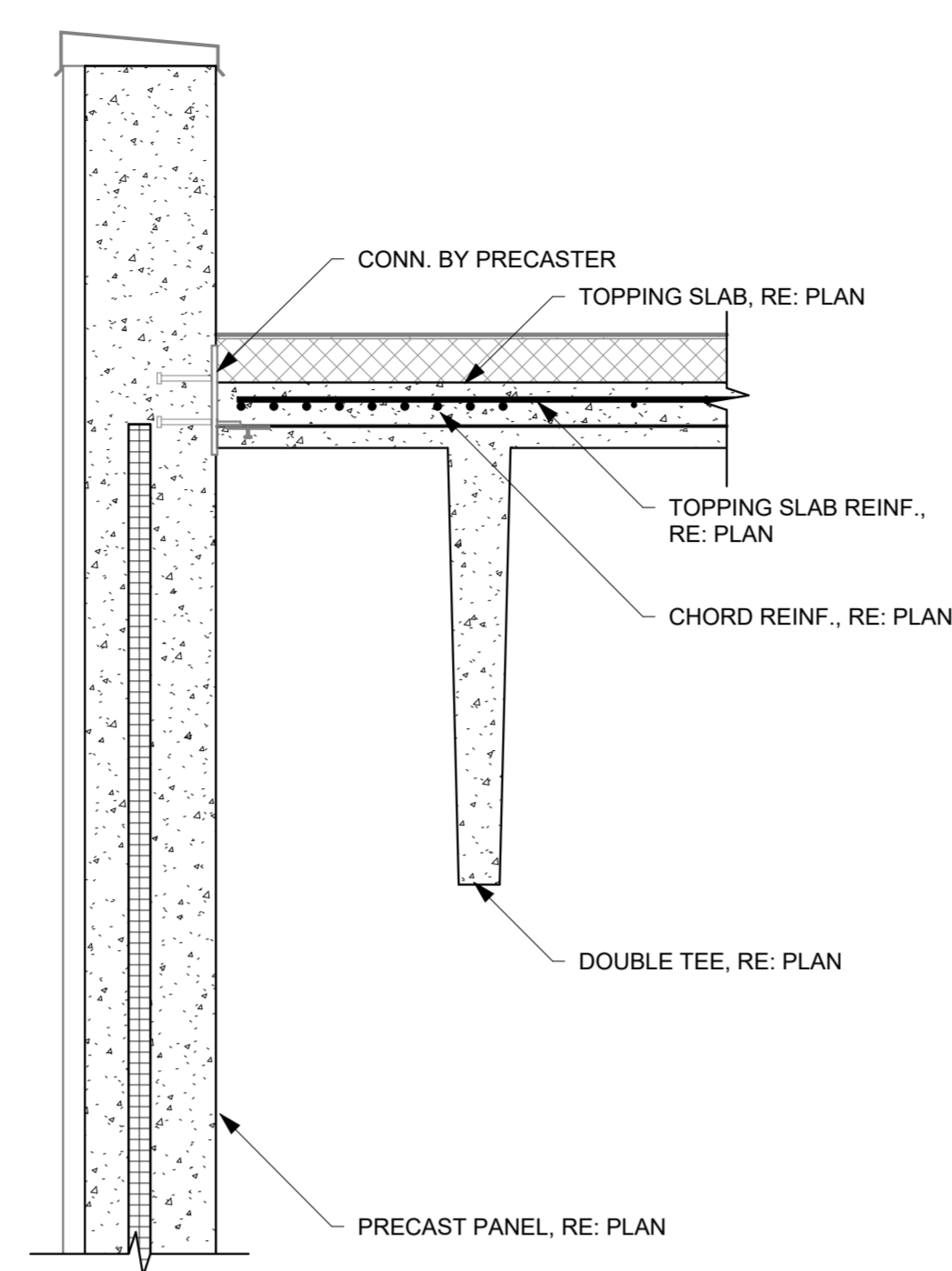
3 FRS - SECTION
 3/4" = 1'-0"



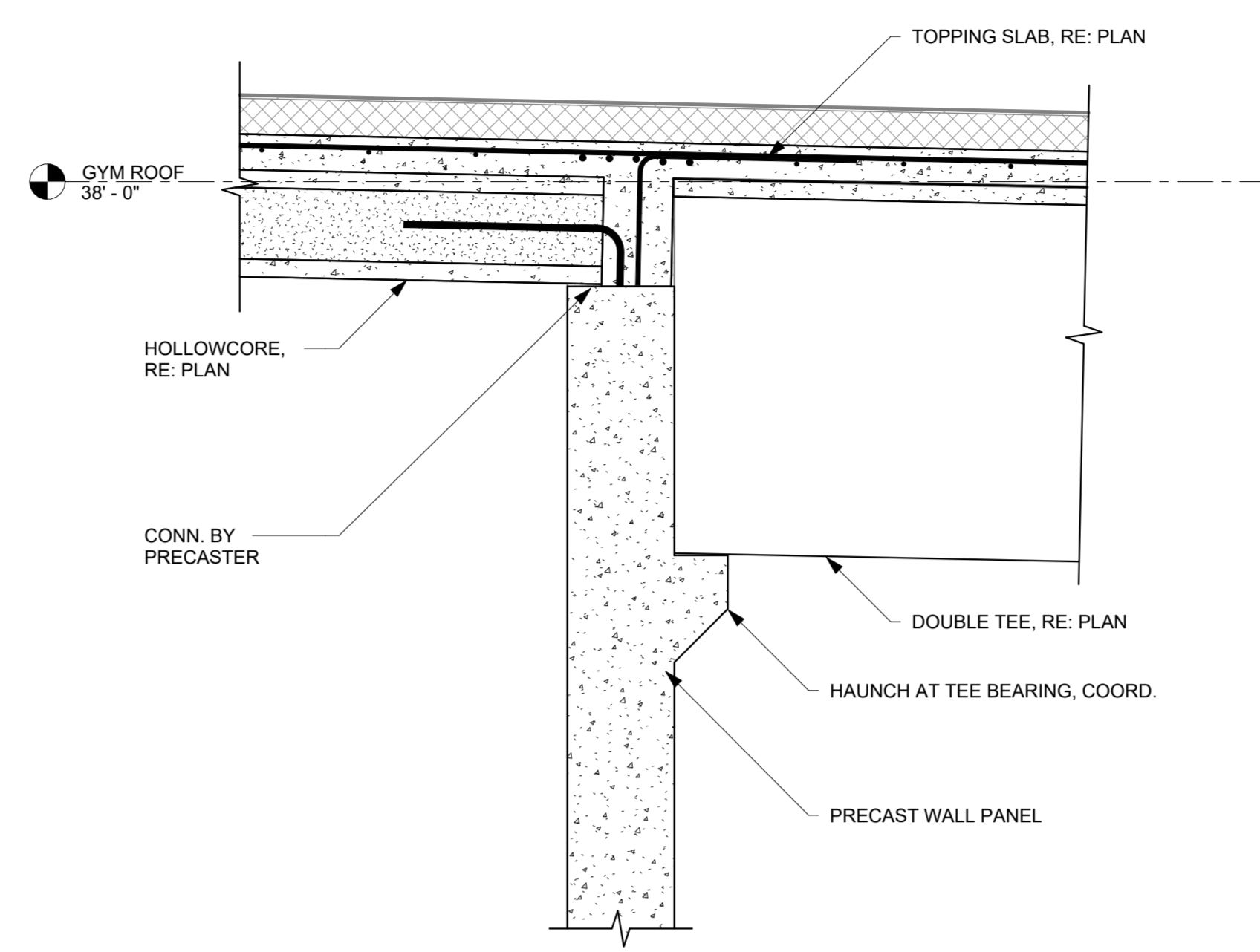
4 FRS - SECTION
 3/4" = 1'-0"



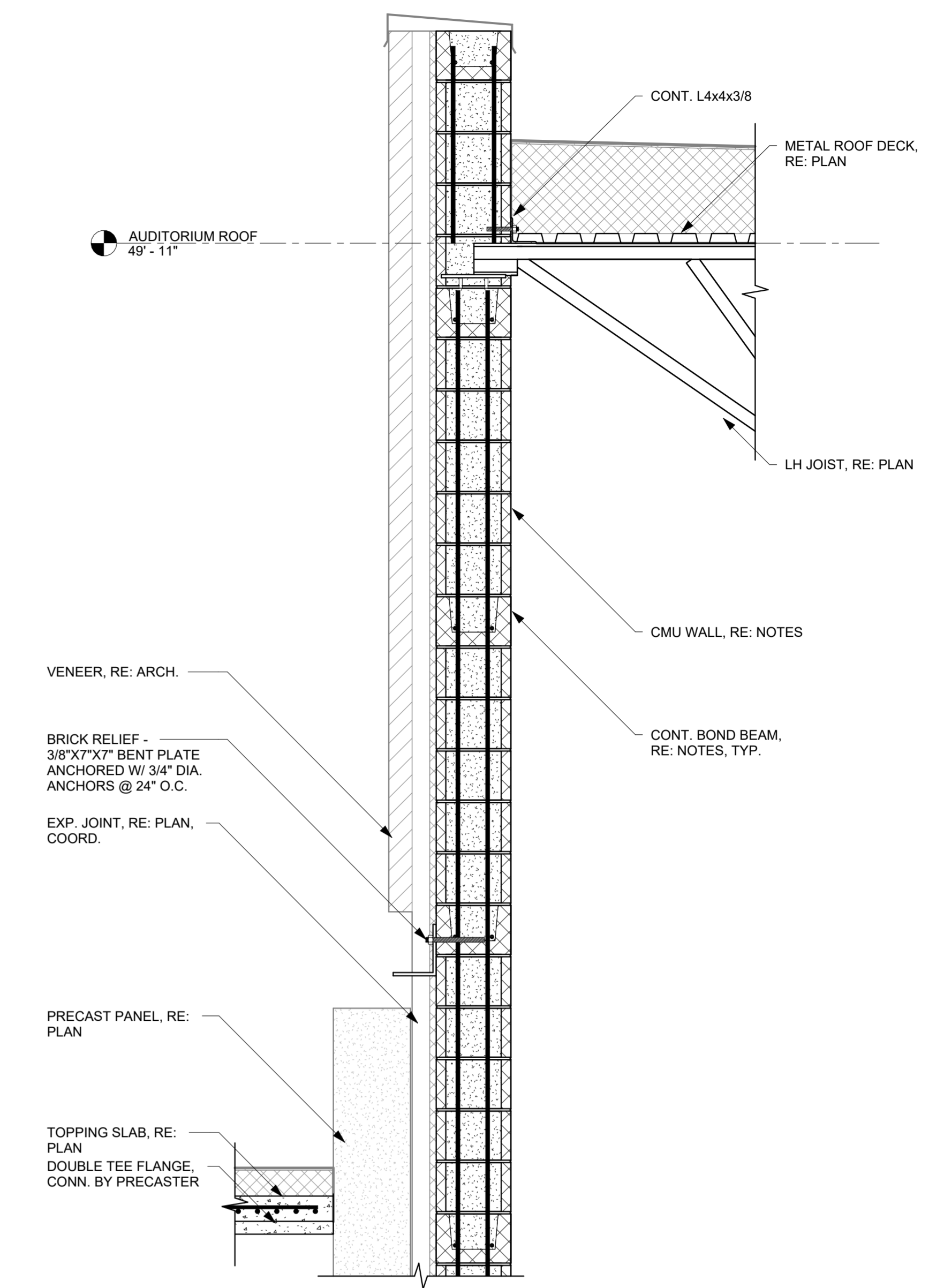
1 FRS - SECTION
 3/4" = 1'-0"



2 FRS - SECTION
 3/4" = 1'-0"



3 FRS - SECTION
 3/4" = 1'-0"



4 FRS - SECTION
 3/4" = 1'-0"

SPECIAL INSPECTIONS

- A. SCOPE: THE PROVISIONS OF THIS SHEET SHALL GOVERN THE QUALITY, WORKMANSHIP, & REQUIREMENTS FOR MATERIALS COVERED. MATERIALS OF CONSTRUCTION & TESTS SHALL CONFORM TO THE APPLICABLE STANDARDS OF THE 2006 IBC.
- B. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED UNDER SECTION 1704. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING INSPECTION.
- C. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN SECTION 109 OF THE BUILDING CODE, AND ALL QUALITY CONTROL, TESTING SPECIFIED IN THE RESPECTIVE SPECIFICATION SECTIONS IN THE PROJECT MANUAL.
- D. REPORTS:
- SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH APPROVED CONSTRUCTION DOCUMENTS.
 - DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.
 - REPORTS OF ALL INSPECTIONS, TEST PERFORMED, DISCREPANCY NOTICES AND CORRECTIVE ACTIONS SHALL BE SUBMITTED TO THE REGISTERED DESIGN PROFESSIONAL ON A WEEKLY BASIS. SUCH REPORTS SHALL ALSO BE SUBMITTED TO THE BUILDING OFFICIAL IF REQUESTED.
 - A FINAL REPORT OF INSPECTIONS DOCUMENTING ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF THE WORK.
- E. THE INSPECTION AND TESTING AGENT(S) SHALL BE ENGAGED BY THE OWNER'S REPRESENTATIVE OR THE SPECIAL INSPECTOR, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR(S) AND/OR TESTING AGENCIES SHALL BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND/OR THE DESIGN PROFESSIONAL.

| MINIMUM VERIFICATION | REQUIRED FOR QUALITY ASSURANCE ^(a) | | | REFERENCE FOR CRITERIA |
|---|---|---------|---------|------------------------|
| | LEVEL 1 | LEVEL 2 | LEVEL 3 | |
| PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS. | R | R | R | ART 1.5 |
| PRIOR TO CONSTRUCTION, VERIFICATION OF F_u AND $F_{t,c}$ EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE | NR | R | R | ART 1.4 B |
| DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE | NR | R | R | ART 1.5 & 1.6.3 |
| PRIOR TO CONSTRUCTION, VERIFICATION OF F_u AND $F_{t,c}$ FOR EVERY 5,000 SQ. FT. | NR | NR | R | ART 1.4 B |
| DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT | NR | NR | R | ART 1.4 B |

QUALITY ASSURANCE LEVEL FOR THIS PROJECT IS LEVEL 2
(a) R = REQUIRED, NR = NOT REQUIRED

| INSPECTION TASK | FREQUENCY ^(a) | | | REFERENCE FOR CRITERIA | |
|--|--------------------------|-------------------------------------|---------|---------------------------------|---|
| | LEVEL 1 | LEVEL 2 | LEVEL 3 | TMS 402 | TMS 602 |
| 1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: | | | | | |
| a. PROPORTIONS OF SITE PREPARED MORTAR | NR | P | P | - | ART 2.1, 2.6 A, & 2.6 C |
| b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES | NR | P | P | - | ART 2.4 B, & 2.4 H |
| c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES | NR | P | P | - | ART 3.4, & 3.6 A |
| d. PRESTRESSING TECHNIQUE | NR | P | P | - | ART 3.6 B |
| e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY | NR | C ^(b) / P ^(c) | C | - | ART 2.1 C.1 |
| f. SAMPLE PANEL CONSTRUCTION | NR | P | C | - | ART 1.6 D |
| 2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: | | | | | |
| a. GROUT SPACE | NR | P | C | - | ART 3.2 D & 3.2 F |
| b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES | NR | P | P | SEC. 10.8 & 10.9 | ART 2.4 & 3.6 |
| c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS | NR | P | C | SEC. 6.1, 6.3.1, 6.3.6, & 6.3.7 | ART 3.2 & 3.4 |
| d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS | NR | P | P | - | ART 2.6 B & 2.4 G.1b |
| 3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION: | | | | | |
| a. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS | NR | P | P | - | ART 1.5 |
| b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION | NR | P | P | - | ART 3.3 B |
| c. SIZE AND LOCATION OF STRUCTURAL MEMBERS | NR | P | P | - | ART 3.3 F |
| d. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION | NR | P | C | SEC 1.2.1(e), 6.2.1, & 6.3.1. | - |
| e. WELDING OF REINFORCEMENT | NR | C | C | SEC 6.1.6.1.2 | - |
| f. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F) | NR | P | P | - | ART 1.8 C & 1.8 D |
| g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE | NR | C | C | - | ART 3.6 B |
| h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE | NR | C | C | - | ART 3.5 & 3.6 C |
| i. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS | NR | C ^(b) / P ^(c) | C | - | ART 3.3 B.9 & 3.3 F.1.b |
| 4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS | NR | P | C | - | ART 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4 |

QUALITY ASSURANCE LEVEL FOR THIS PROJECT IS LEVEL 2
(a) FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, NR = NOT REQUIRED, P = PERIODIC, C = CONTINUOUS
(b) REQUIRED FOR THE FIRST 5000 SQUARE FEET OF AAC MASONRY
(c) REQUIRED AFTER 5000 SQUARE FEET OF AAC MASONRY

| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC | COMMENTS | IBC REFERENCE |
|--|------------|----------|----------|---------------|
| 1. ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS, AND INTERIOR AND EXTERIOR VENEER | - | X | a, b, c | 1705.13.5 |
| 2. ANCHORAGE OF ACCESS FLOORS | - | X | - | 1705.13.5.1 |

- TABLE NOTES**
- NOT REQUIRED FOR EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS AND INTERIOR AND EXTERIOR VENEER 30 FEET OR LESS IN HEIGHT ABOVE GRADE OR WALKING SURFACE.
 - NOT REQUIRED FOR EXTERIOR CLADDING, AND INTERIOR AND EXTERIOR VENEER WEIGHING 5 PSF OR LESS.
 - NOT REQUIRED FOR INTERIOR NONBEARING WALLS WEIGHING 15 PSF OR LESS.

NOTE: THIS TABLE ONLY REQUIRED FOR SDC D OR HIGHER, SEE 1705.13.5.

| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC | COMMENTS | IBC REFERENCE |
|---|------------|----------|----------|---------------|
| 1. EXAMINATION OF DESIGNATED SEISMIC SYSTEMS REQUIRING SEISMIC QUALIFICATION TO IBC 1705.12.3 | - | - | - | 1705.13.4. |
| 2. VERIFICATION THE LABEL, ANCHORAGE OR MOUNTING CONFORMS TO THE CERTIFICATE OF COMPLIANCE. | - | - | - | 1705.13.4 |

NOTE: THIS TABLE ONLY REQUIRED FOR SDC C OR HIGHER, SEE 1705.13.4. SEE SECTION 13.2.2 IN ASCE 7.

| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC | REFERENCED STANDARD | IBC REFERENCE |
|--|------------|----------|---|----------------|
| 1. INSPECTION OF REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT. | - | X | ACI 318: Ch. 20, 25.2, 25.3, 26.6.1 - 26.6.3 | - |
| 2. REINFORCING BAR WELDING: | | | | |
| a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706. | - | X | AWS D1.4 | - |
| b. INSPECT SINGLE PASS FILLET WELDS, MAXIMUM OF 5/16", AND | - | X | ACI 318: 26.6.4 | - |
| c. INSPECT ALL OTHER WELDS. | X | - | - | - |
| 3. INSPECTION OF ANCHORS CAST IN CONCRETE. | - | X | ACI 318: 17.8.2 | - |
| 4. INSPECTION OF ANCHORS POST INSTALLED IN HARDENED CONCRETE MEMBERS: | | | | |
| a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS | X | - | ACI 318: 17.8.2.4 | - |
| b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4a. | - | X | ACI 318: 17.8.2 | - |
| 5. VERIFY USE OF REQUIRED DESIGN MIX. | - | X | ACI 318: Ch. 19, 26.4.3, 26.4.4 | 1904.1, 1904.2 |
| 6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. | X | - | ASTM C31 ASTM C172 ACI 318: 26.5, 26.12 | - |
| 7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES. | X | - | ACI 318: 26.5 | - |
| 8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES. | - | X | ACI 318: 26.5.3-26.5.5 | 1910.9 |
| 9. INSPECTION OF PRESTRESSED CONCRETE FOR: | | | | |
| a. APPLICATION OF PRESTRESSING FORCES, AND | X | - | ACI 318: 26.10 | - |
| b. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM. | X | - | - | - |
| 10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS. | - | X | ACI 318: 26.9 | - |
| 11. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E, OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: | | | | |
| a. INSTALLATION OF EMBEDDED PARTS | X | - | - | - |
| b. COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS. | X | - | ACI 550.5 | - |
| c. COMPLETION OF CONNECTIONS IN THE FIELD. | X | - | - | - |
| 12. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5. | - | X | ACI 318: 26.13.1.3 | - |
| 13. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS. | - | X | ACI 318: 26.11.2 | - |
| 14. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED. | - | X | ACI 318: 26.11.1.2(b) | - |

- TABLE NOTES:**
- WHERE APPLICABLE FOR SEISMIC RESISTANCE, SEE SECTION 1705.13
 - SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH ACI 17.8.2 OR OTHER QUALIFICATION PROCEDURES WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED. SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

| VERIFICATION AND INSPECTION TASK | CONTINUOUS DURING TASK LISTED | PERIODICALLY DURING TASK LISTED |
|--|-------------------------------|---------------------------------|
| 1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. | - | X |
| 2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL. | - | X |
| 3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS. | - | X |
| 4. DURING FILL PLACEMENT VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL. | X | - |
| 5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY. | - | X |

| VERIFICATION AND INSPECTION TASK | CONTINUOUS DURING TASK LISTED | PERIODICALLY DURING TASK LISTED |
|---|---|---------------------------------|
| 1. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT. | X | - |
| 2. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES. | X | - |
| 3. FOR CONCRETE ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3. | IN ACCORDANCE WITH STATEMENT OF SPECIAL INSPECTIONS | |

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PROJECT NAME

WSD - NEW SENIOR HIGH SCHOOL

LOCATION

800 E JACKSON AVE
WYNNE AR 72396

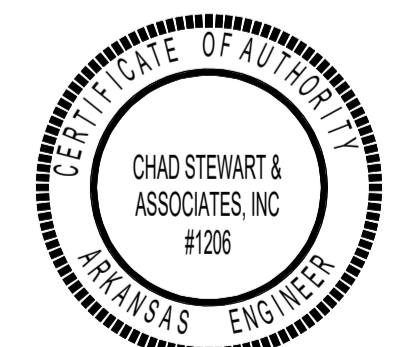
PROJECT NUMBER

-

DEVELOPER/OWNER

WYNNE SCHOOL DISTRICT

INFORMATION



SHEET TITLE

SPECIAL INSPECTIONS

DATE

17.10.24

SHEET NUMBER

S501

| AISC 360, SECTION N5 REQUIRED QUALITY ASSURANCE OF STRUCTURAL STEEL | | |
|---|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| 1. REVIEW MATERIAL TEST REPORTS AND CERTIFICATIONS LISTED IN AISC360, SECTION N3.2, FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. | X | - |
| 2. PRIOR TO CONCRETE PLACEMENT, INSPECTOR SHALL BE ON THE PREMISES DURING PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS THAT SUPPORT STRUCTURAL STEEL. VERIFY COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS: a. DIAMETER. b. GRADE. c. TYPE. d. LENGTH. e. EMBEDMENT DEPTH. | - | X |
| 3. INSPECT THE ERECTED STEEL TO VERIFY COMPLIANCE WITH DETAILS ON THE CONSTRUCTION DOCUMENTS: a. BRACES. b. STIFFENERS. c. MEMBER LOCATIONS. d. PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. | - | X |
| 4. VERIFY PLACEMENT AND INSTALLATION OF STEEL DECK. | X | - |
| 5. DOCUMENT THE ACCEPTANCE OR REJECTION OF STEEL ELEMENTS AND DECKING. | X | - |
| 6. STRUCTURAL STEEL WELDING AND HIGH-STRENGTH BOLTING (SEE ACCOMPANYING TABLES.) | | |

| AISC 360, SECTION N5 REQUIRED QUALITY ASSURANCE OF HIGH-STRENGTH BOLTING | | |
|---|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1) | | |
| 1. VERIFY MANUFACTURER'S CERTIFICATIONS FOR FASTENER MATERIALS. | X | - |
| 2. VERIFY FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS. | - | X |
| 3. VERIFY CORRECT FASTENERS USED FOR THE JOINT (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE). | - | X |
| 4. VERIFY CORRECT BOLTING PROCEDURE USED FOR THE JOINT. | - | X |
| 5. VERIFY CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS. | - | X |
| 6. OBSERVE AND DOCUMENT PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL FOR FASTENER ASSEMBLIES AND METHODS USED. (a) | - | X |
| 7. VERIFY PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS. | - | X |
| INSPECTION TASKS DURING BOLTING (TABLE N5.6-2) | | |
| 1. VERIFY FASTENER ASSEMBLIES ARE PLACED IN ALL HOLES, AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED. | (a) | X |
| 2. VERIFY JOINTS BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION. | (a) | X |
| 3. VERIFY FASTENER COMPONENTS NOT TURNED BY THE WRENCH ARE PREVENTED FROM ROTATING. (a) | (a) | X |
| 4. VERIFY FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES. (a) | (a) | X |
| INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3) | | |
| 1. DOCUMENT THE ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS. | X | - |

TABLE NOTES:
a. TASK NOT REQUIRED FOR SNUG-TIGHTENED JOINTS.
b. INSPECTION SHALL BE CONTINUOUS FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN INSTALLED BY THE CALIBRATED WRENCH METHOD OR TURN-OF-NUT METHOD WITHOUT MATCH MARKING.

| AISC 360, SECTION N5 REQUIRED QUALITY ASSURANCE OF STRUCTURAL STEEL WELDING | | |
|--|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1) | | |
| 1. WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS. | X | - |
| 2. VERIFY WELDING PROCEDURE SPECIFICATIONS (WPS) AND CONSUMABLE CERTIFICATES. | X | - |
| 3. VERIFY MATERIALS (TYPE / GRADE). | - | X |
| 4. VERIFY WELDER IDENTIFICATION SYSTEM. a. THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE. | - | X |
| 5. VERIFY FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY): a. JOINT PREPARATION. b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOF FACE, BEVEL). c. CLEANLINESS (CONDITION OF STEEL SURFACES). d. TACKING (TACK WELD QUALITY AND LOCATION). e. BACKING TYPE AND FIT (IF APPLICABLE). | - | X |
| 6. VERIFY FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y-, AND K- JOINTS WITHOUT BACKING: a. JOINT PREPARATION. b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOF FACE, BEVEL). c. CLEANLINESS (CONDITION OF STEEL SURFACES). d. TACKING (TACK WELD QUALITY AND LOCATION). | - | X |
| 7. VERIFY CONFIGURATION AND FINISH OF ACCESS HOLES. | - | X |
| 8. VERIFY FIT-UP OF FILLET WELDS: a. DIMENSIONS (ALIGNMENT, GAPS AT ROOT). b. CLEANLINESS (CONDITION OF STEEL SURFACES). c. TACKING (TACK WELD QUALITY AND LOCATION). | - | X |

| INSPECTION TASKS DURING WELDING (TABLE N5.4-2) | | |
|--|---|---|
| 1. VERIFY CONTROL AND HANDLING OF WELDING CONSUMABLES. a. PACKAGING. b. EXPOSURE CONTROL. | - | X |
| 2. VERIFY NO WELDING OVER CRACKED TACK WELDS. | - | X |
| 3. VERIFY ENVIRONMENTAL CONDITIONS. a. WIND SPEED WITHIN LIMITS. b. PRECIPITATION AND TEMPERATURE. | - | X |
| 4. VERIFY WPS FOLLOWED: a. SETTINGS ON WELDING EQUIPMENT. b. TRAVEL SPEED. c. SELECTED WELDING MATERIALS. d. SHIELDING GAS TYPE / FLOW RATE. e. PREHEAT APPLIED. f. INTERPASS TEMPERATURE MAINTAINED (MIN / MAX). g. PROPER POSITION (F, V, H, OH). | - | X |
| 5. VERIFY WELDING TECHNIQUES: a. INTERPASS AND FINAL CLEANING. b. EACH PASS WITHIN PROFILE LIMITATIONS. c. EACH PASS MEETS QUALITY REQUIREMENTS. | - | X |
| 6. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS. | X | - |

| INSPECTION TASKS AFTER WELDING (TABLE N5.4-3) | | |
|---|---|---|
| 1. VERIFY WELDS CLEANED. | - | X |
| 2. VERIFY SIZE, LENGTH, AND LOCATION OF WELDS. | X | - |
| 3. VERIFY WELDS MEET VISUAL ACCEPTANCE CRITERIA: a. CRACK PROHIBITION. b. WELD / BASE-METAL FUSION. c. CRATER CROSS SECTION. d. WELD PROFILES. e. WELD SIZE. f. UNDERCUT. g. POROSITY. | X | - |
| 4. VERIFY ARC STRIKES. | X | - |
| 5. VERIFY K-AREA: a. WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES OF THE WELD. | X | - |
| 6. WELD ACCESS HOLES IN HEAVY SHAPES AND BUILT UP HEAVY SHAPES: a. AFTER ROLLED HEAVY SHAPES (SEE SECTION A3.1g) AND ROLLED HEAVY SHAPES (SEE SECTION A3.1g) ARE WELDED, VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS. | X | - |
| 7. VERIFY BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED). | X | - |
| 8. VERIFY REPAIR ACTIVITIES. | X | - |
| 9. DOCUMENT THE ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER. | X | - |
| 10. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR. | - | X |

| NON-DESTRUCTIVE TESTING (NDT) OF WELDS (SECTION N5.5) | | |
|---|---|---|
| 1. CJP WELDS (RISK CATEGORY II): a. PERFORM ULTRASONIC TESTING ON 10% OF CJP GROOVE WELDS IN BUTT, T-, AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16" THICK OR GREATER. IF TESTS SHOW UNACCEPTABLE DEFECTS, INCREASE TESTING RATE PER AISC 360 PART N5.6f. | - | X |
| 2. CJP WELDS (RISK CATEGORY III OR IV): a. PERFORM UT ON CJP GROOVE WELDS IN BUTT, T-, AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16" THICK OR GREATER. | X | - |
| 3. WELDED JOINTS SUBJECT TO FATIGUE: a. PERFORM RADIOGRAPHIC TESTING OR ULTRASONIC TESTING AT JOINTS IDENTIFIED AS SUBJECT TO FATIGUE. | X | - |

NOTE: 1) FOR PROJECTS WITH NON-SEISMIC STEEL ONLY, IDENTIFY SPECIFIC JOINTS ON DRAWINGS OR REMOVE THE NDT REQUIREMENT 2) WHERE APPROVED BY THE EOR AND THE AHJ, NDT FREQUENCY FOR RISK CATEGORY III OR IV PROJECTS CONTAINING MORE THAN 40 WELDS TO BE TESTED MAY BE REDUCED TO 25% FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR PROVIDED THAT THE REJECTION RATE IS DEMONSTRATED TO BE 5% OR LESS BASED ON A MINIMUM SAMPLE OF 40 WELDS PERFORMED ON THE PROJECT. SEE AISC 360-16 SECTION N5.5e.
3) FOR RISK CATEGORY II OR HIGHER STRUCTURES WITH NDT WELD FREQUENCY LESS THAN 100% IN EFFECT, A REJECTION RATE OF MORE THAN 5% BASED ON A SAMPLE SIZE OF 20 CONSECUTIVE COMPLETED WELDS FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR WILL REQUIRE 100% TESTING FREQUENCY FOR THAT WELDER OR WELDING OPERATOR UNTIL A MINIMUM SAMPLE OF 40 CONSECUTIVE WELDS PERFORMED ON THE PROJECT IS DEMONSTRATED TO HAVE A 5% OR LESS REJECTION RATE. SEE AISC 360-16 SECTION N5.5f

| AISC 341, CHAPTER J ADDITIONAL QUALITY ASSURANCE OF STRUCTURAL STEEL (IN ADDITION TO AISC 360 SPECIAL INSPECTION TABLE TASKS) | | |
|---|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| 1. REVIEW PROCEDURES AND CERTIFICATIONS LISTED IN AISC 341, SECTION J2. | X | - |
| OTHER INSPECTION TASKS (TABLE J8-1) | | |
| 1. INSPECT AND DOCUMENT REDUCED BEAM SECTIONS (RBS): a. CONTOUR AND FINISH. b. DIMENSIONAL TOLERANCES. | X | - |
| 2. AFTER COMPLETION OF OTHER TRADES, VERIFY AND DOCUMENT THAT PROTECTED ZONES HAVE NO HOLES OR UNAPPROVED ATTACHMENTS. | X | - |

NOTE: THIS TABLE ONLY REQUIRED FOR SDC C OR HIGHER.
SEE 1705.11.1, SPECIAL INSPECTIONS NOT REQUIRED FOR SDC C STRUCTURES NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE WITH R OF 3 OR LESS, EXCLUDING CANTILEVER COLUMN SYSTEMS.

| AISC 341, SECTION J6 ADDITIONAL QUALITY ASSURANCE OF STRUCTURAL STEEL WELDING FOR THE SEISMIC FORCE RESISTING SYSTEM (SFRS) (IN ADDITION TO AISC 360 SPECIAL INSPECTION TABLE TASKS) | | |
|---|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| INSPECTION TASKS PRIOR TO WELDING (TABLE J6-1), ADDITIONAL TASKS TO SPECIFICATION TABLE N5.4-1 | | |
| NO ADDITIONAL TASKS | | |
| INSPECTION TASKS DURING WELDING (TABLE J6-2), ADDITIONAL TASKS TO SPECIFICATION TABLE N5.4-2 | | |
| 4. VERIFY WPS FOLLOWED: h. INTERMIX OF FILLER METALS AVOIDED UNLESS APPROVED | - | X |
| INSPECTION TASKS AFTER WELDING (TABLE J6-3), ADDITIONAL TASKS TO SPECIFICATION TABLE N5.4-3 | | |
| 7. VERIFY BACKING REMOVED, WELD TABS REMOVED AND FINISHED, AND FILLET WELDS ADDED (IF REQUIRED). | X | - |
| 11. VERIFY PLACEMENT OF REINFORCING OR CONTOURING FILLET WELDS (IF REQUIRED). | X | - |
| 12. DOCUMENT TASKS IN ACCORDANCE WITH AISC 341, SECTION J5.3. | X | - |
| NON-DESTRUCTIVE TESTING OF WELDS (SECTION J6.2) | | |
| 1. WELDING IN K-AREA: a. NO MORE THAN 48 HOURS AFTER WELDING PERFORM MT ON WELD, INCLUDING BASE METAL WITHIN 3 INCHES. | X | - |
| 2. CJP WELDS: a. PERFORM UT ON CJP GROOVE WELDS IN MATERIALS 5/16" THICK OR GREATER. EXCEPTION: FOR ORDINARY MOMENT FRAMES, ONLY REQUIRED FOR DEMAND CRITICAL WELDS. | X | - |
| 3. CJP WELDS (BEAM-TO-COLUMN): a. PERFORM MT ON 25% OF ALL BEAM-TO-COLUMN CJP GROOVE WELDS. EXCEPTION: FOR ORDINARY MOMENT FRAMES, ONLY REQUIRED FOR DEMAND CRITICAL WELDS. | - | X |
| 4. BASE METAL FOR LAMELLAR TEARING: a. PERFORM UT TESTING PER SECTION J6.2C WHERE ≥ 3/4" MATERIAL IS CJP WELDED TO FACE OF ≥ 1-1/2" MATERIAL. | X | - |
| 5. BEAM COPES AND ACCESS HOLES: a. PERFORM MT OR PT ON THERMALLY CUT ACCESS HOLES IN HEAVY SHAPES. 1. FLANGE > 1-1/2" (ROLLED SHAPES). 2. WEB > 1-1/2" (BUILT-UP SHAPES). | X | - |
| 6. REDUCED BEAM SECTION (RBS): a. PERFORM MT ON CUT SURFACES REPAIRED BY WELDING AND WHERE SHARP NOTCHES REMOVED BY GRINDING. | X | - |
| 7. WELD TAB REMOVAL SITES: a. PERFORM MT ON BEAM-TO-COLUMN JOINTS AT THE ENDS OF WELDS WHERE WELD TABS HAVE BEEN REMOVED. EXCEPTION: NOT REQUIRED AT CONTINUITY PLATE WELD TAB REMOVAL SITES. | X | - |

NOTE: THIS TABLE ONLY REQUIRED FOR SDC C OR HIGHER.
SEE 1705.11.1, SPECIAL INSPECTIONS NOT REQUIRED FOR SDC C STRUCTURES NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE WITH R OF 3 OR LESS, EXCLUDING CANTILEVER COLUMN SYSTEMS.

| IBC TABLE 1705.2.3 REQUIRED SPECIAL INSPECTION OF OPEN WEB STEEL JOISTS AND JOIST GIRDERS | | |
|--|------------|----------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| 1. INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS: | | |
| a. END CONNECTIONS - WELDING OR BOLTED. | - | X |
| b. BRIDGING - HORIZONTAL OR DIAGONAL. | - | - |
| 1) STANDARD BRIDGING. | - | X |
| 2) BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1 | - | X |

TABLE NOTES:
a. SEE SJI SPECIFICATIONS LISTED IN SECTION 2207.1 OF IBC 2021

