

STRUCTURAL GENERAL NOTES (cont'd)

I. STRUCTURAL STEEL:

- THE DESIGN OF STRUCTURAL STEEL IS BASED ON AISC 360-16, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
- ALL STEEL MEMBERS SHALL CONFORM TO:

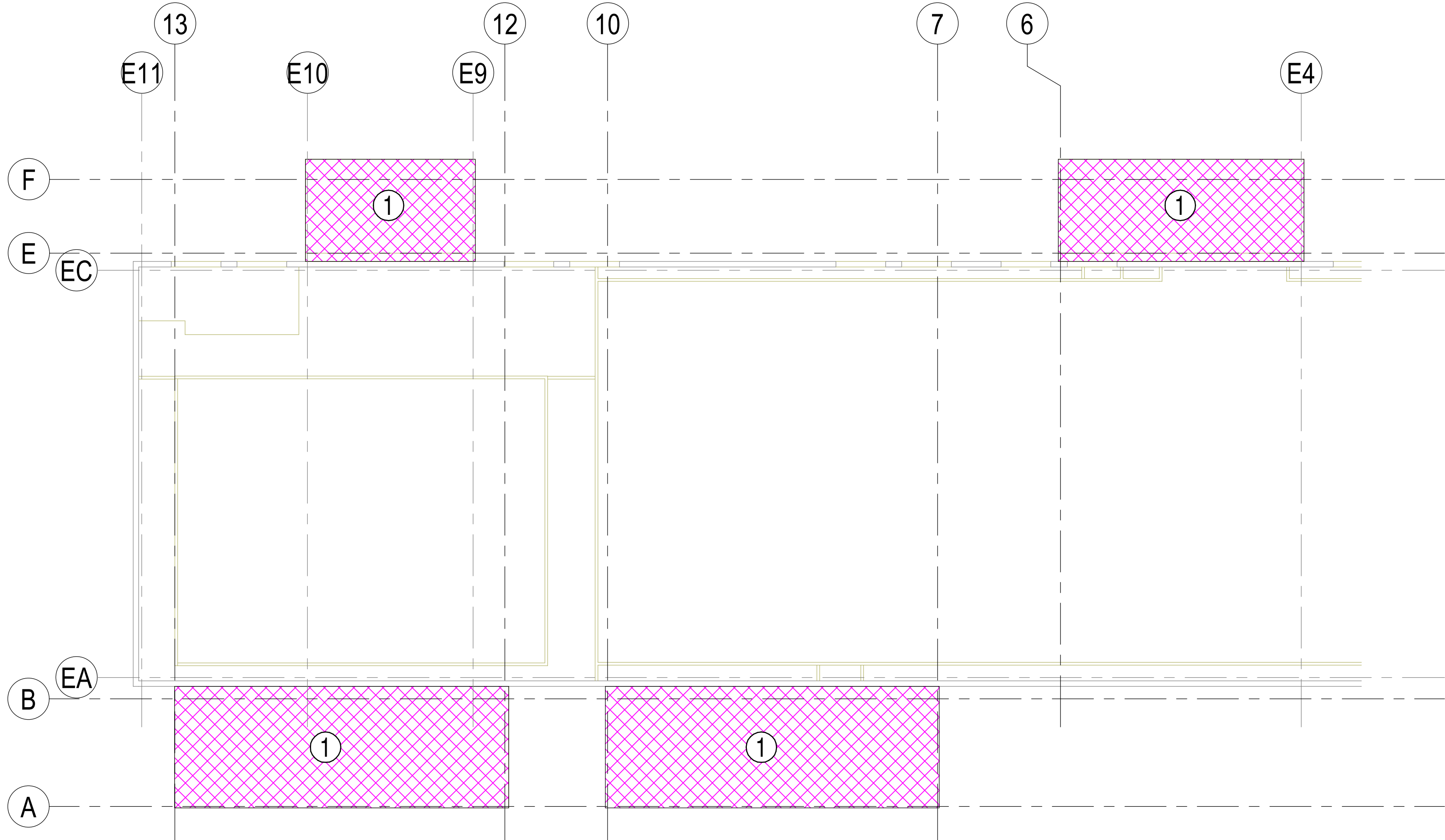
SECTION	ASTM STANDARD	YIELD STRENGTH
WIDE FLANGE AND CHANNELS	A992	50 KSI
ANGLES, PLATES, AND BARS	A572	50 KSI
RECTANGULAR AND SQUARE HSS	A500 GRADE C OR A1085	50 KSI
- ALL BOLTED CONNECTIONS FOR STRUCTURAL STEEL TO STEEL SHALL BE ASTM F3125, GRADE F1852 "TWIST-OFF" STYLE TENSION CONTROL BOLT ASSEMBLIES (SHOP AND FIELD), UNLESS NOTED OTHERWISE. "H.S. BOLTS" DESIGNATES F1852 BOLT ASSEMBLIES.
- ALL WELDING ELECTRODES FOR STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO AWS A5.1 GRADE E-70 BARE ELECTRODES.
- COLUMN ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36. ANCHOR RODS SHALL HAVE A PLATE WASHER PER AISC TABLE 14-1 AND ONE HEAVY HEX NUT AT THE TOP AND ONE HEAVY HEX NUT AT THE BOTTOM TACK WELDED TO THE ROD, UNLESS NOTED OTHERWISE.
- PLACE AND SECURE ANCHOR RODS IN FOOTING EXCAVATION PRIOR TO POURING CONCRETE FOR FOOTING. DO NOT PLACE ANCHOR RODS IN WET CONCRETE.
- PROVIDE LEVELING NUTS OR SHIM PACKS AS REQUIRED TO LEVEL COLUMN BASE PLATES. IF SHIM PACKS ARE USED, ENCASE SHIM PACKS WITH 1" MIN COVER OF NON-SHRINK GROUT WHEN PLACING GROUT UNDER BASE PLATE.
- CONNECTION DETAILING:
 - CONNECTIONS SHALL BE DETAILED AS INDICATED IN THE DRAWINGS, UNO.
 - THE FABRICATOR'S STEEL DETAILER SHALL SELECT AND COMPLETE TYPICAL CONNECTIONS BASED ON THE PLANS AND THE FOLLOWING:
 - TYPICAL STEEL GIRDER TO COLUMN CONNECTIONS: DETAIL 3/S-401.
 - TYPICAL STEEL BEAM-TO-BEAM CONNECTIONS: DETAIL 2/S-401.
 - TYPICAL CONNECTIONS SHALL USE, AS A MINIMUM, THE NUMBER OF BOLTS INDICATED IN THE TYPICAL DETAILS.
 - IF BEAM END REACTIONS ARE LARGER THAN THE CAPACITY INDICATED IN THE SCHEDULES THE ARCHITECT/ENGINEER SHALL BE NOTIFIED FOR GUIDANCE.
 - ANY NON-TYPICAL CONNECTIONS THAT ARE NOT DETAILED IN THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR FOR THE LOADS INDICATED IN THE DRAWINGS. THE DESIGN SHALL BE DONE BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF ARKANSAS. THE CONNECTION DETAILS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR APPROVAL.
- ALL STEEL FABRICATION AND ERECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE.
- THE STRUCTURAL STEEL FRAMING FABRICATOR SHALL BE AN AISC CERTIFIED BUILDING FABRICATOR (BU).
- ALL STEEL DETAILS SHALL BE IN ACCORDANCE WITH THE REQUIREMENT OF THE LATEST AISC SPECIFICATIONS WITH LATEST REVISIONS.
- SUPPLY STRUCTURAL STEEL FRAMING CONNECTIONS THAT COMPLY WITH OSHA STANDARDS. IF MEETING THESE STANDARDS CONFLICTS WITH ANYTHING SHOWN IN THESE DRAWINGS THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING ADVISING OF ANY REQUIRED REVISIONS AND ACQUIRE THE ENGINEER'S APPROVAL BEFORE PROCEEDING WITH THE WORK.
- THE MINIMUM PLATE THICKNESS SHALL BE 1/4", THE MINIMUM WELD SHALL HAVE A 1/4" THICK THROAT, THE MINIMUM BOLT DIAMETER SHALL BE 3/4", AND THE MINIMUM CONNECTION SHALL BE TWO BOLTS, U.N.O.
- ALL COLUMN BEARING PLATES SHALL BE SIZED AS SHOWN ON DETAIL 4/S-301 AND SHALL HAVE ROLLED OR GAS CUT EDGES. MINIMUM EDGE DISTANCE TO CENTER OF BOLT HOLE, SPACING OF HOLES, AND SIZES OF HOLES SHALL BE AS PER AISC MANUAL UNLESS NOTED OTHERWISE.
- PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES OF THE WEB OF BEAMS AT POINTS OF CONCENTRATED LOAD. MINIMUM STIFFENER PLATE THICKNESS TO BE 1/2" OR FLANGE THICKNESS OF COLUMNS ABOVE OR BELOW, WHICHEVER IS GREATER, U.N.O.
- PROVIDE 1/2" MINIMUM THICKNESS STIFFENERS ON ALL BEAMS RUNNING OVER TOPS OF COLUMNS. MINIMUM SIZE OF WELD TO BE 1/4" FILLET WELD, U.N.O.
- THE STEEL FABRICATOR MAY SPLICE THE COLUMNS EITHER AT EACH FLOOR OR AT ANY FLOOR AS LONG AS THE COLUMN SIZE REMAINS AS SHOWN ON THE LOWER LEVEL.
- ALL STEEL NOT REQUIRED TO BE SHOP PAINTED (SEE SPECIFICATIONS) SHALL BE CLEANED OF OIL, GREASE, DIRT, RUST, LOOSE MILL SCALE, ETC. AND ALL OTHER FOREIGN MATERIALS.
- GALVANIZING OF ALL STEEL MEMBERS SHALL CONFORM TO ASTM A123. ALL GALVANIZED STEEL REQUIRED TO BE PAINTED SHALL BE CLEANED AND PREPPED ACCORDING TO ASTM D6386.
- ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH AWS SPECIFICATIONS LATEST EDITIONS. WELDING SHALL BE INSPECTED AND TESTED AS NOTED IN THE SPECIFICATIONS. WELDING INSPECTION SHALL BE PERFORMED BY AN AWS CERTIFIED WELDING INSPECTOR.
- FINAL BOLTING OR WELDING SHALL NOT BE PERFORMED UNTIL THE STRUCTURE HAS BEEN PROPERLY ALIGNED.

J. COLD-FORMED STEEL FRAMING:

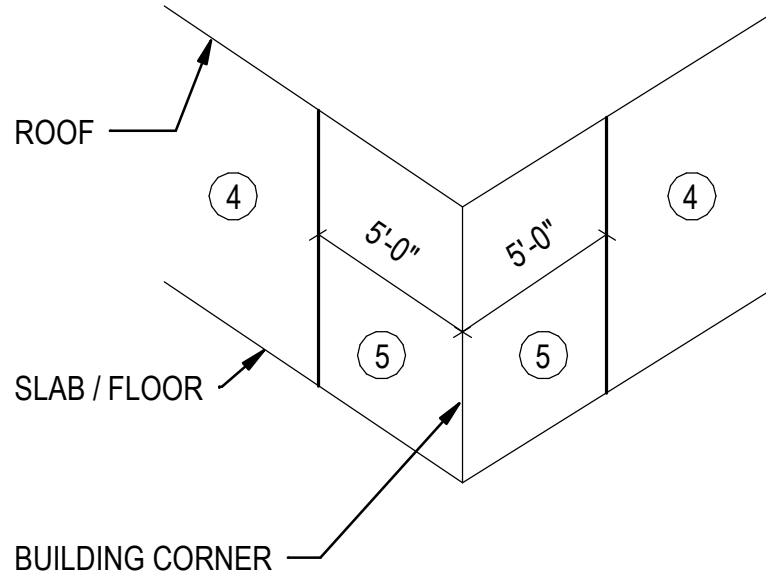
- THE DESIGN OF THE COLD FORMED STEEL FRAMING IS BASED ON AISI S100-16 w/S2-20, NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
- ALL MATERIAL SHALL BE COLOR CODED TO INDICATE THE GAUGE OF THE MATERIAL.

K. POST-INSTALLED ANCHORS IN CONCRETE:

- POST-INSTALLED ANCHORS (MECHANICAL OR ADHESIVE) SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR DOWELS. POST-INSTALLED ANCHORS SHALL BE BUILDING CODE COMPLIANT, INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND INSPECTED PER THE APPLICABLE ICC-ES OR IAPMO UES EVALUATION REPORT. SEE SPECIFICATIONS SECTION 033000 FOR ADDITIONAL INFORMATION.



1 COMPONENTS AND CLADDING WIND PRESSURES ROOF PLAN
NOT TO SCALE



ZONE	TRIBUTARY AREA (SQ. FT.)		
	10	50	100
2	-27/+25 PSF	-25/+23 PSF	-24/+22 PSF
3	-34/+25 PSF	-29/+23 PSF	-26/+22 PSF

NOTES:

- ULTIMATE WIND SPEED: 104 MPH
- NOMINAL WIND SPEED: 81 MPH
- WIND PRESSURES ARE BASED ON ASCE 7-16 STRENGTH DESIGN (ULTIMATE).
- POSITIVE / NEGATIVE VALUES INDICATE FORCES ARE ACTING TOWARDS / AWAY FROM ELEMENT, RESPECTIVELY.
- COMPONENTS SUBJECTED TO PARAPET WIND FORCE ON BOTH SIDES (e.g. WALL PANELS) SHALL BE DESIGNED FOR CUMULATIVE FORCES.
- SERVICE LEVEL LOADS MAY BE CALCULATED BY MULTIPLYING THE NUMBERS ABOVE BY 0.6.

GROSS WIND UPLIFT
(STRENGTH DESIGN)

ZONE	TRIBUTARY AREA (SQ. FT.)		
	10	50	100
1	-95/+16 PSF	-65/+16 PSF	-52/+16 PSF

COMPONENTS AND CLADDING WALL WIND PRESSURES

COMPONENTS AND CLADDING ROOF WIND PRESSURES

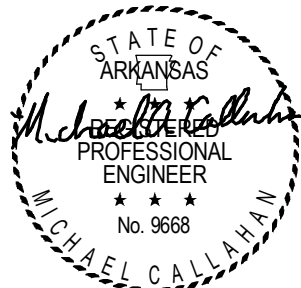
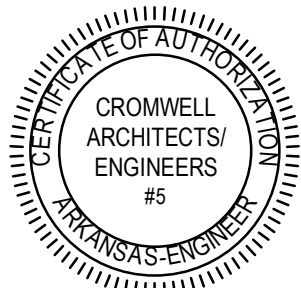
Project
AEROJET BUILDING 2SH8
COLD BOX
CONVERSION
EAST CAMDEN,
ARKANSAS

Design Phase

CONSTRUCTION
DOCUMENTS

Revisions		
No.	Date	Description

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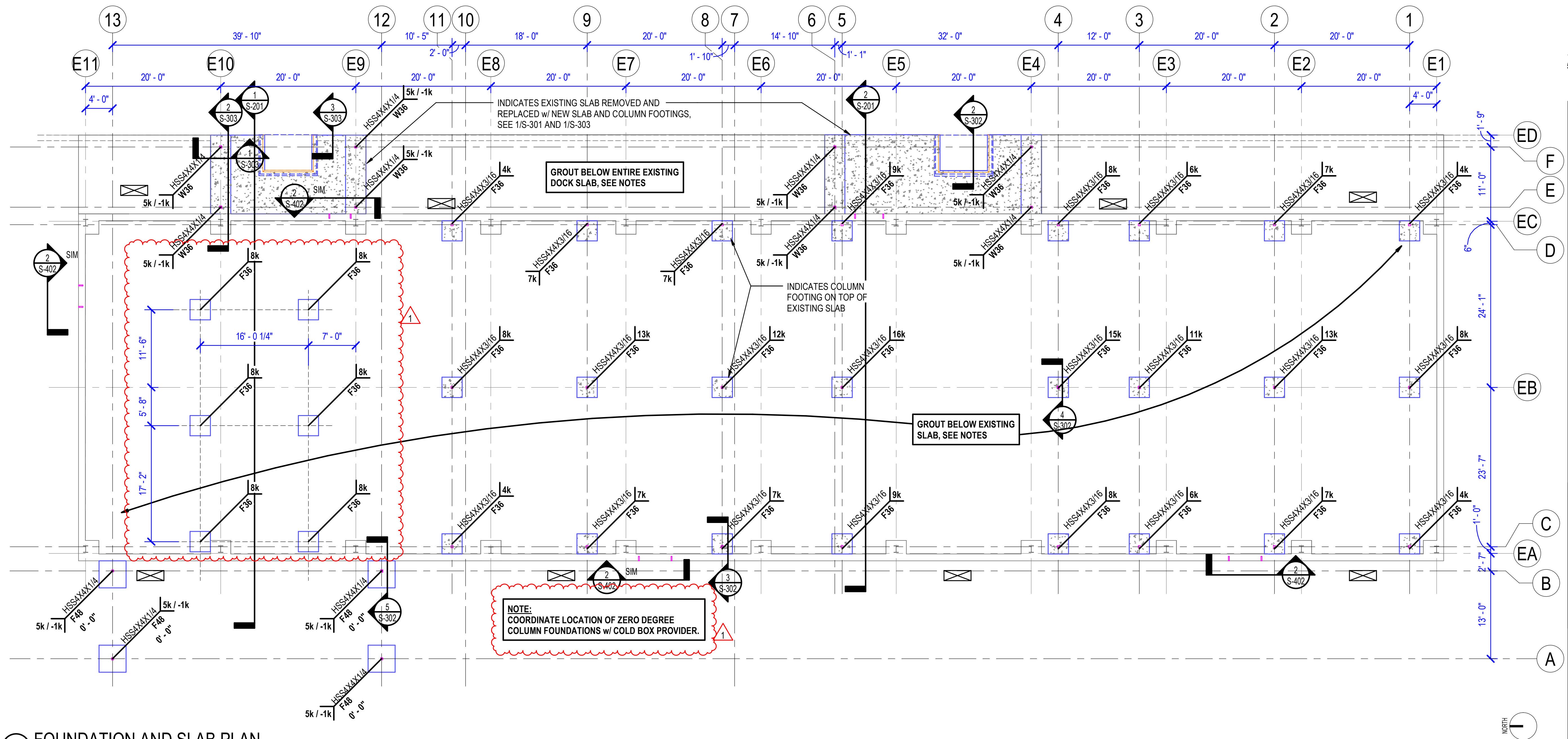
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Sheet Title

STRUCT. GEN. NOTES
CONT'D AND
COMPONENTS AND
CLADDING WIND
PRESSURES

Sheet Number

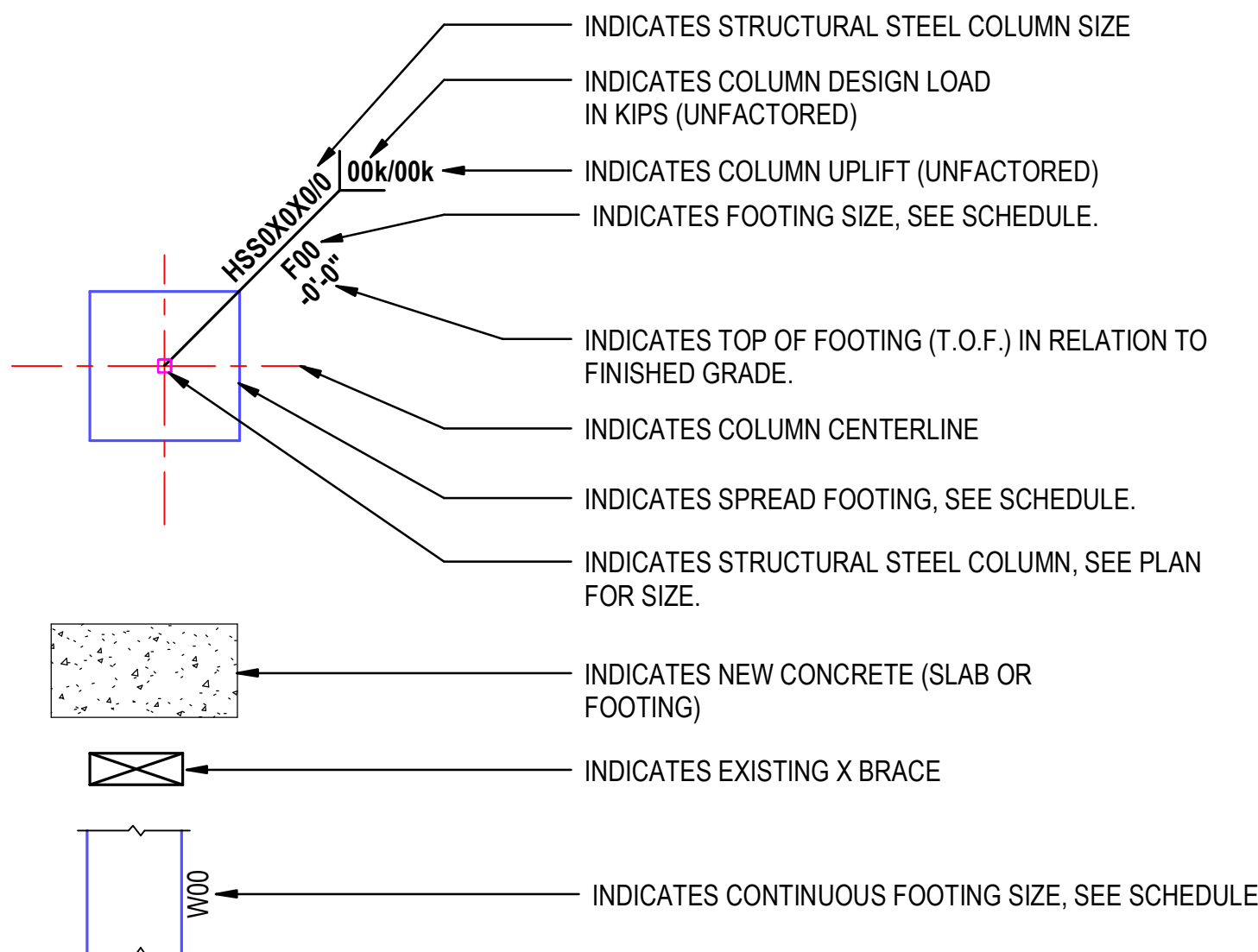
S-002



1 FOUNDATION AND SLAB PLAN
1/8" = 1'-0"

FINISHED FLOOR ELEVATION TO MATCH EXISTING BUILDING FLOOR ELEVATION

FOUNDATION AND SLAB LEGEND



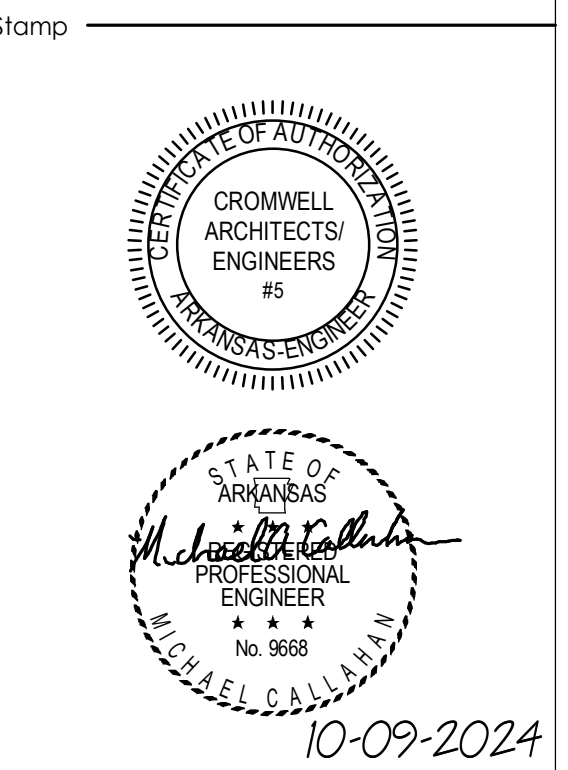
FOUNDATION SCHEDULE				
MARK #	L	W	T	REINFORCING
F36	3' - 0"	3' - 0"	1' - 0"	#5 AT 9" O.C. BOT EA WAY
F48	4' - 0"	4' - 0"	1' - 6"	#5 AT 9" O.C.TOP AND BOT EA WAY
W36	CONT.	3' - 0"	1' - 6"	SEE DETAIL 1/S-303

- GROUT NOTES:**
1. INSTALL HIGH-MOBILITY POLYURETHANE GROUT BELOW SLAB IN EXISTING BUILDING AND BELOW EXTERIOR DOCK SLAB USING A REGULAR PATTERN OF INJECTION HOLES.
 2. INSTALL GROUT BELOW DOCK SLAB PRIOR TO DEMOLITION OF SLAB WHERE SHOWN ON PLANS.
 3. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 38 PSI AND PLACED IN HARD CONTACT WITH SUBGRADE.
 4. APPLY GROUT AT LOW PRESSURE AND TERMINATE WHEN GROUT IS NOTED IN ADJACENT INJECTION POINTS OR WHEN MOVEMENT OF FLOOR SLAB OR OTHER FIXED FEATURES IS OBSERVED.
 5. ALL OPENINGS AND JOINTS SHALL BE THOROUGHLY SEALED PRIOR TO GROUTING.
 6. GROUT TAKE SHALL BE MEASURED DURING GROUTING.
 7. GROUT MIX DESIGN AND APPLICATION METHOD SHALL BE DEVELOPED BY A COMPETENT AND EXPERIENCED GROUTING CONTRACTOR.
 8. SEE SPECIFICATION SECTION 03 64 00 AND THE SLAB SURVEY REPORT BY GRUBBS, HOSKYN, BARTON & WYATT, LLC, dba UES FOR ADDITIONAL INFORMATION.

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1	03-03-2025	RB 001



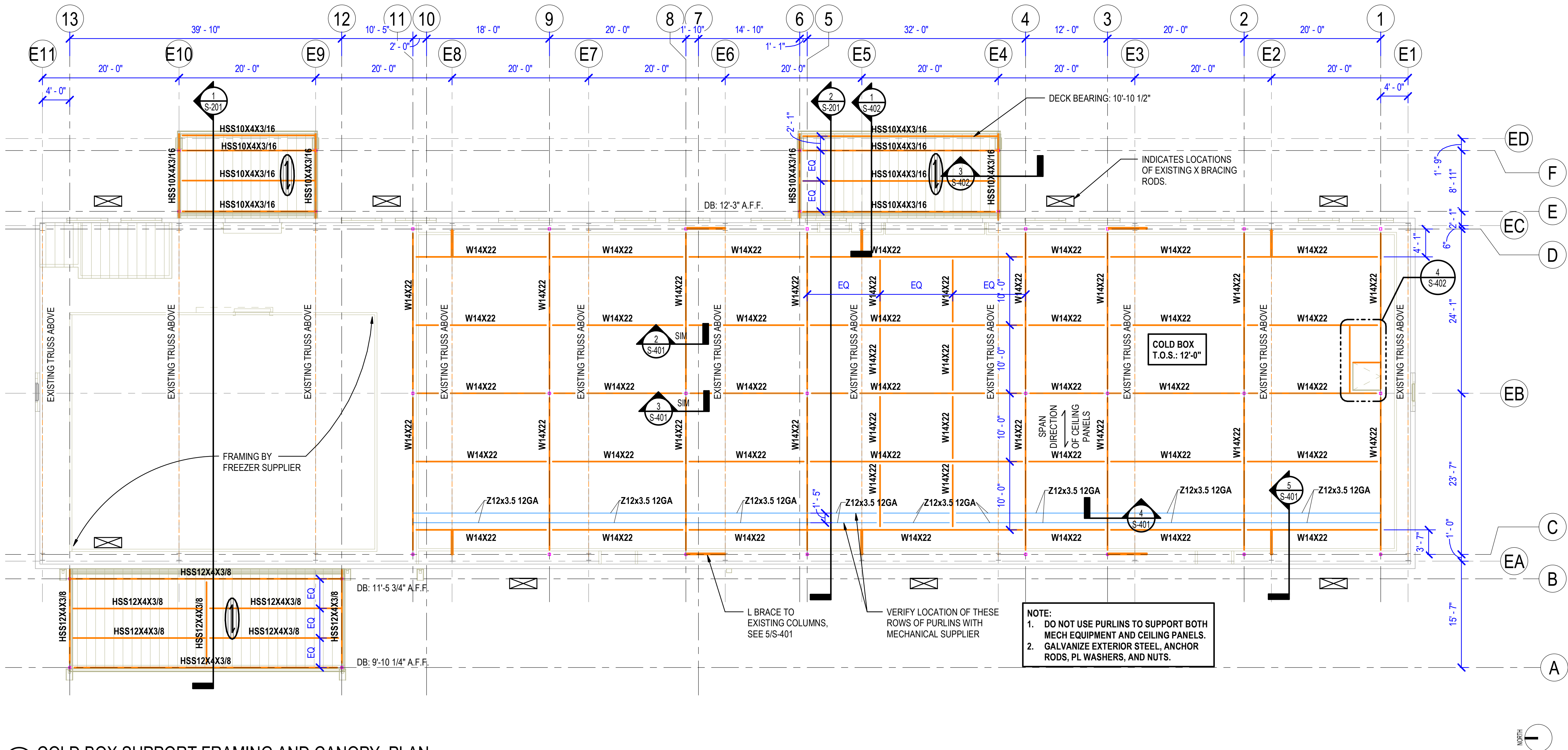
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FOUNDATION AND SLAB PLAN

Sheet Number S-101



1 COLD BOX SUPPORT FRAMING AND CANOPY PLAN
1/8" = 1'-0"

FRAMING LEGEND

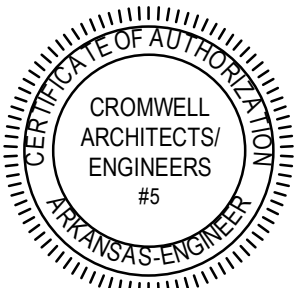
- INDICATES WIDE FLANGE BEAM SIZE OR HSS BEAM SIZE
- W00X00 or HSS00x00x0/0
- INDICATES CFS PURLIN SIZE AND SHAPE
- Z00X0.0 00GA
- INDICATES SPAN DIRECTION OF METAL DECK, SEE 1/S-401
- INDICATES EXPECTED SPAN DIRECTION OF METAL PANELS BY OTHERS

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COLD BOX FRAMING PLAN

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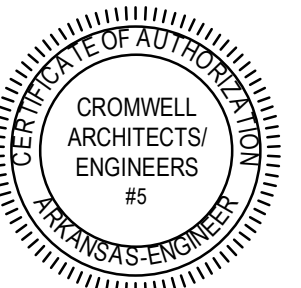
S-102

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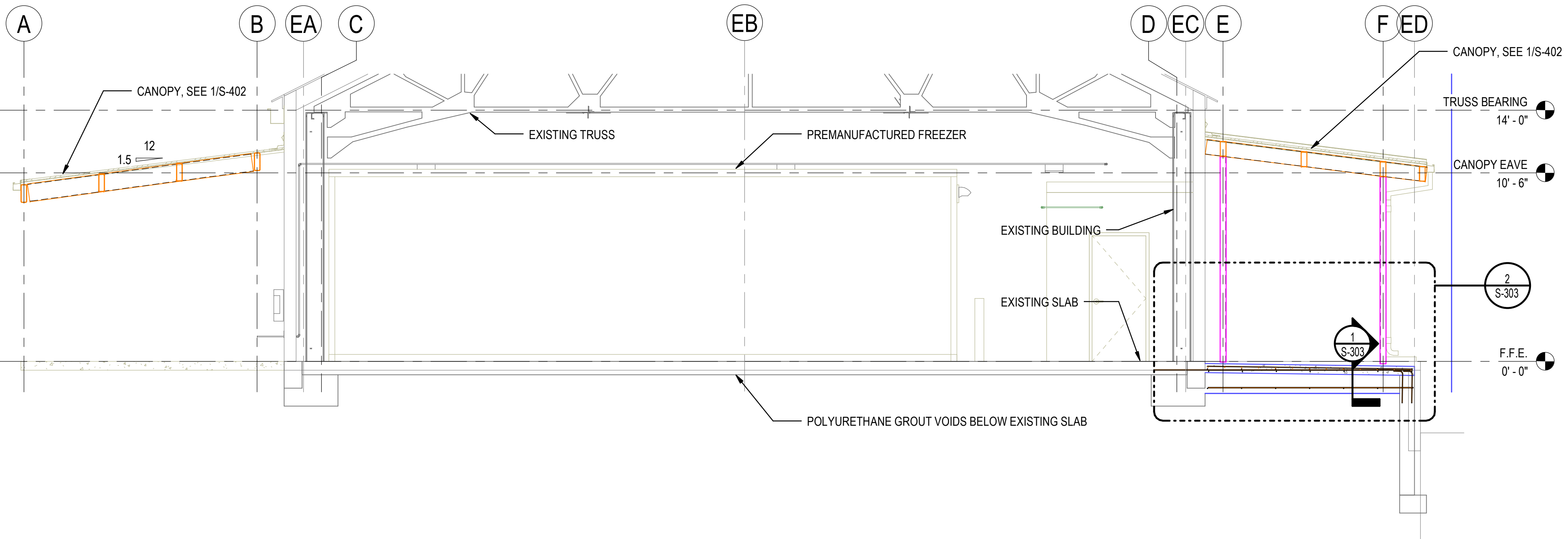
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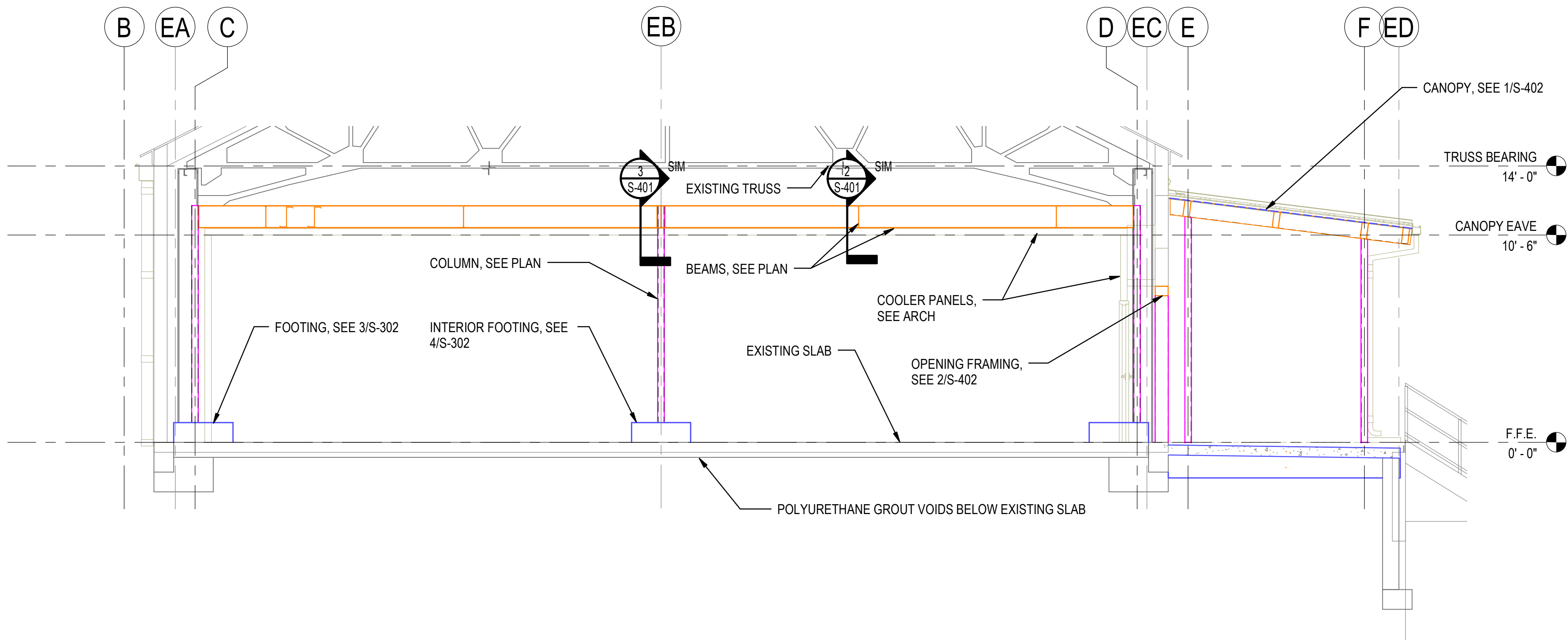
BUILDING SECTIONS

Sheet Number

S-201



1 SECTION AT FREEZER AREA
1/4" = 1'-0"



2 SECTION AT COOLER AREA
1/4" = 1'-0"

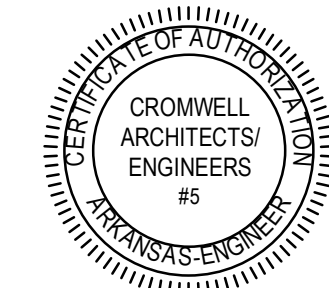
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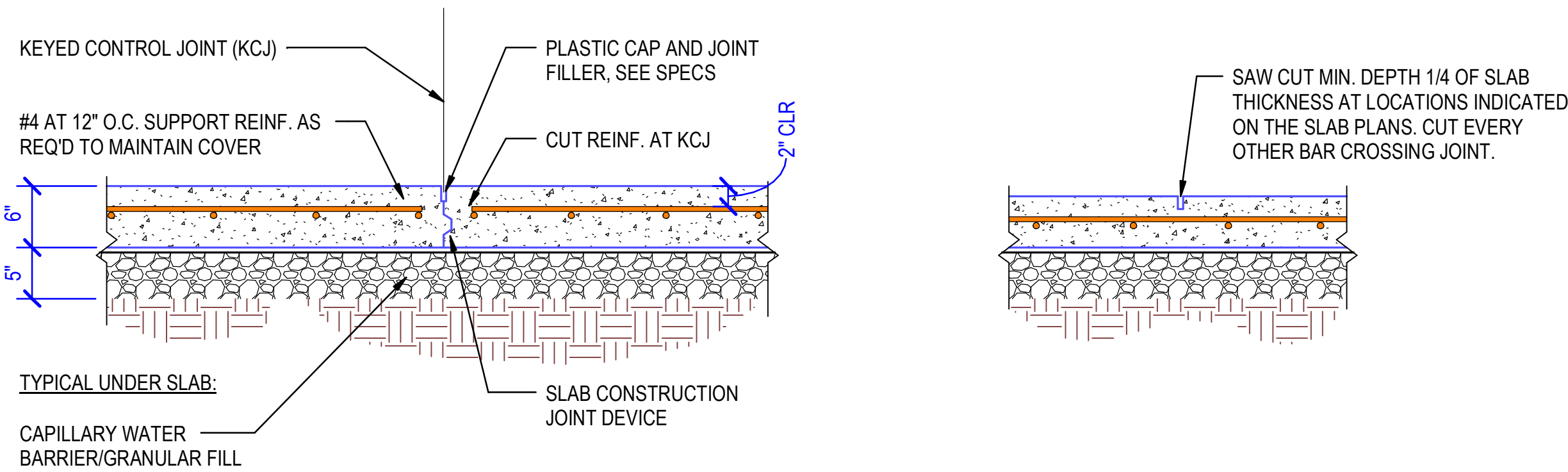
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Sheet Title

FOUNDATION AND
SLAB DETAILS

Sheet Number

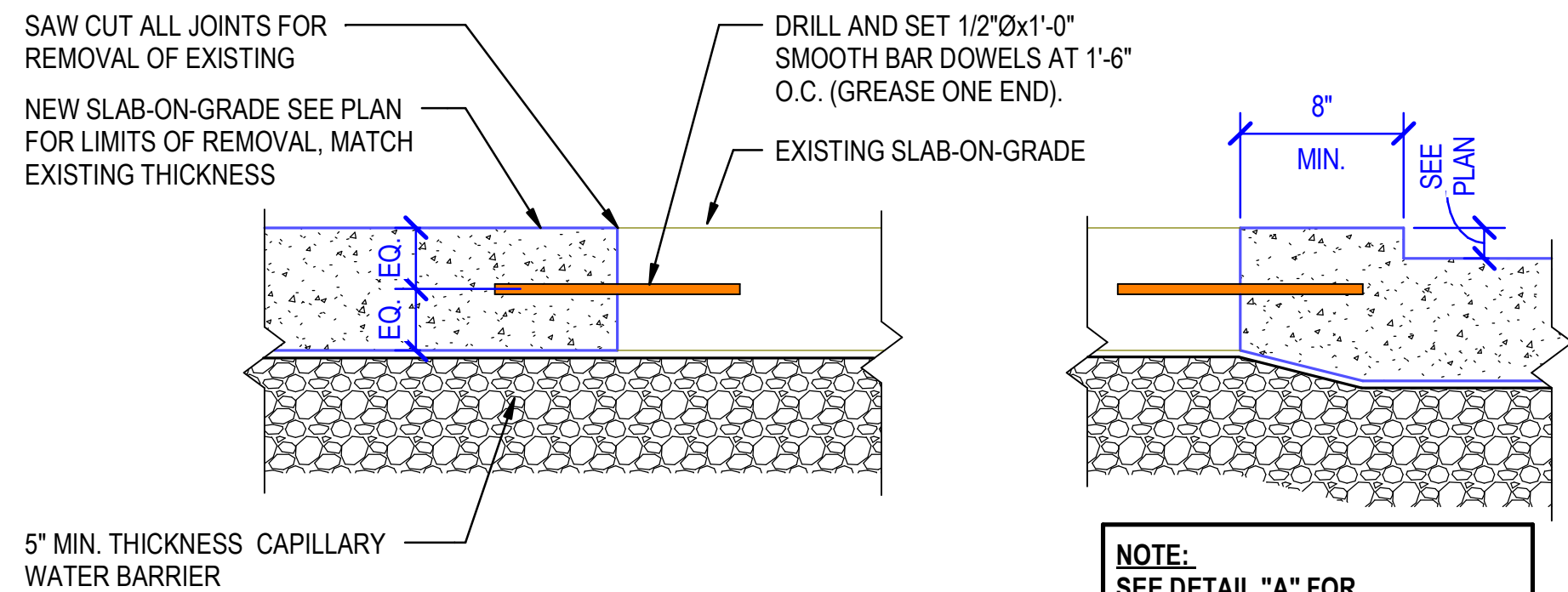
S-301



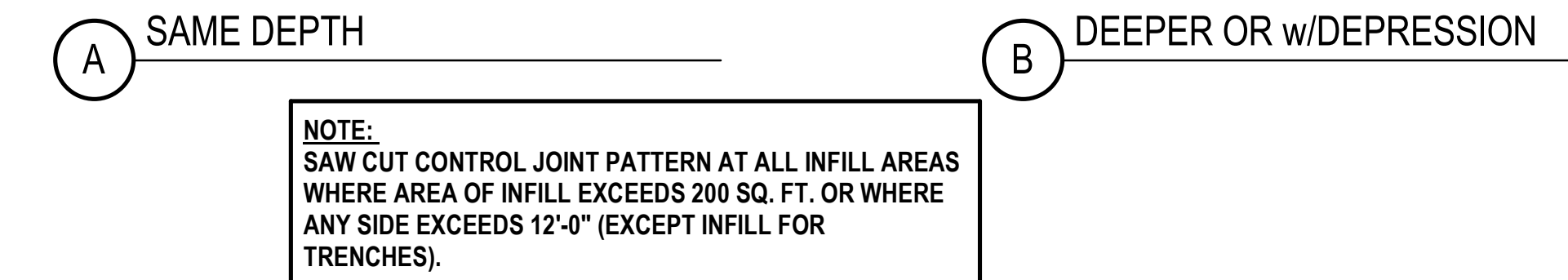
A KCJ (KEYED CONSTRUCTION JOINT)

B SAWN JOINT (SJ)

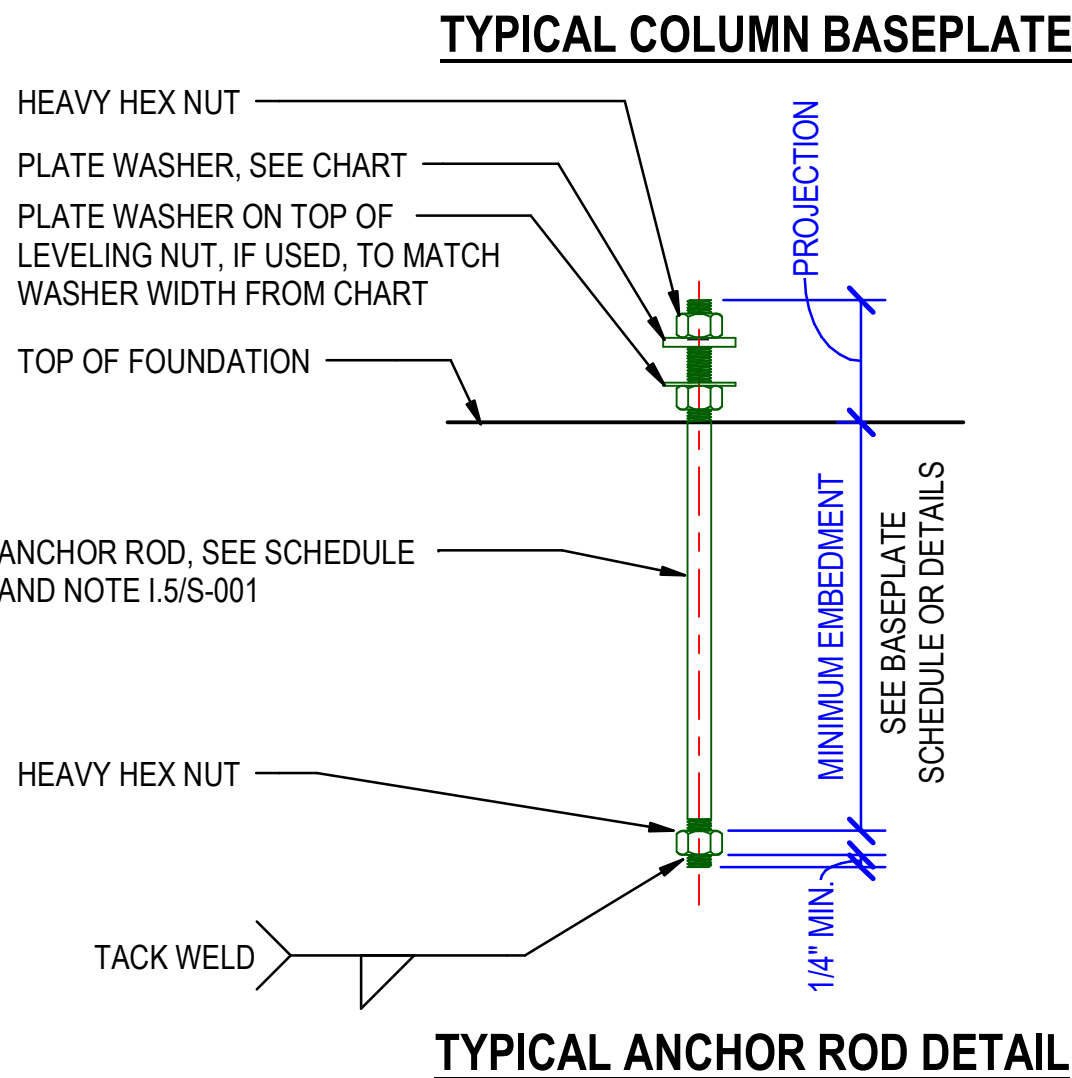
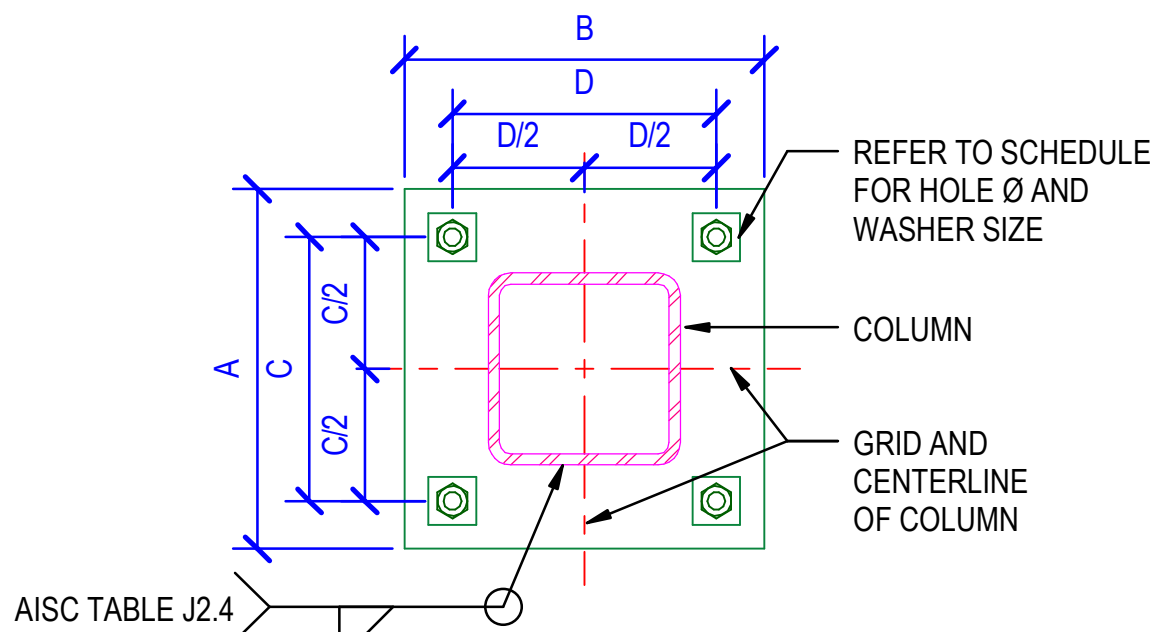
1 TYPICAL SLAB ON GRADE REINFORCING AND JOINT DETAILS
NOT TO SCALE



2 TYPICAL SLAB OPENINGS AND REENTRANT CORNERS
NOT TO SCALE



3 TYPICAL SLAB INFILL DETAIL
NOT TO SCALE

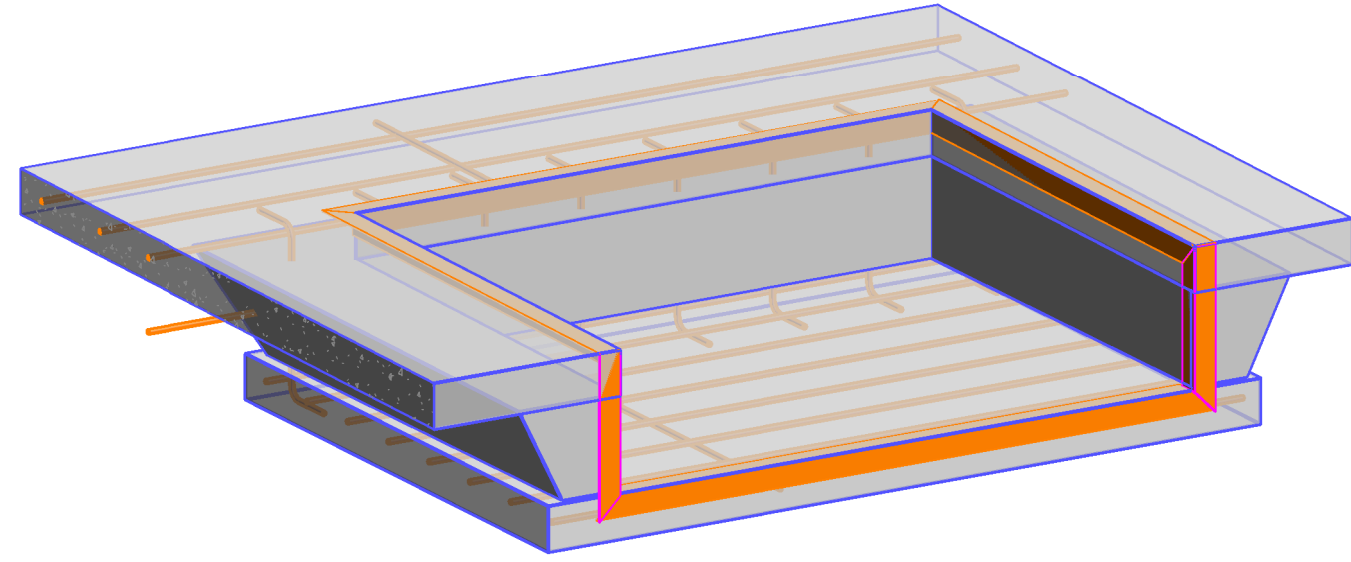


4 BASEPLATE SCHEDULE AND DETAILS
NOT TO SCALE

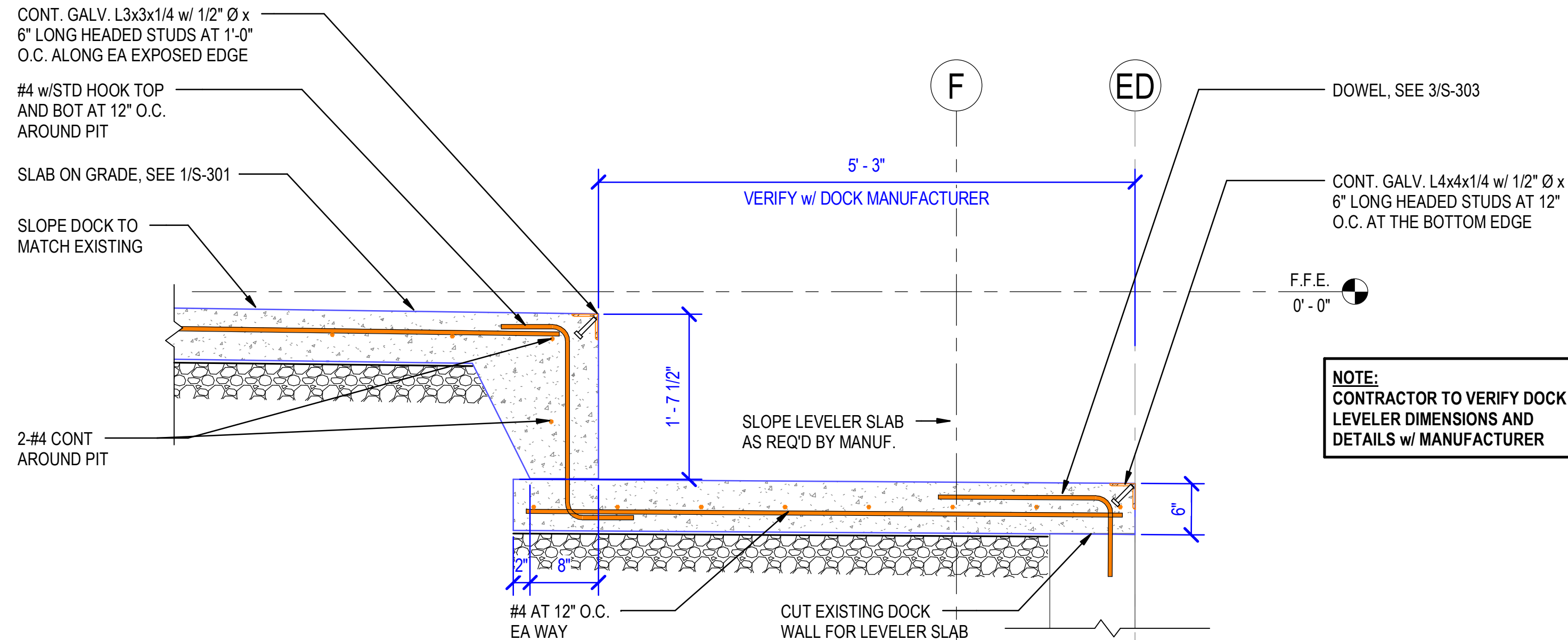
TYPICAL BASE PLATE SCHEDULE							
COL SIZE	PLATE THICKNESS	A	B	C	D	ANCHOR ROD Ø	MIN. EMBEDMENT
HSS4X4	1/2"	1'-0"	1'-0"	8"	8"	3/4"	8"

ANCHOR ROD WASHERS AND HOLES SIZE			
ANCHOR ROD Ø	MAX HOLE Ø	MIN. WASHER SIZE	MIN. WASHER THICKNESS
3/4	1 5/16"	2"	1/4"

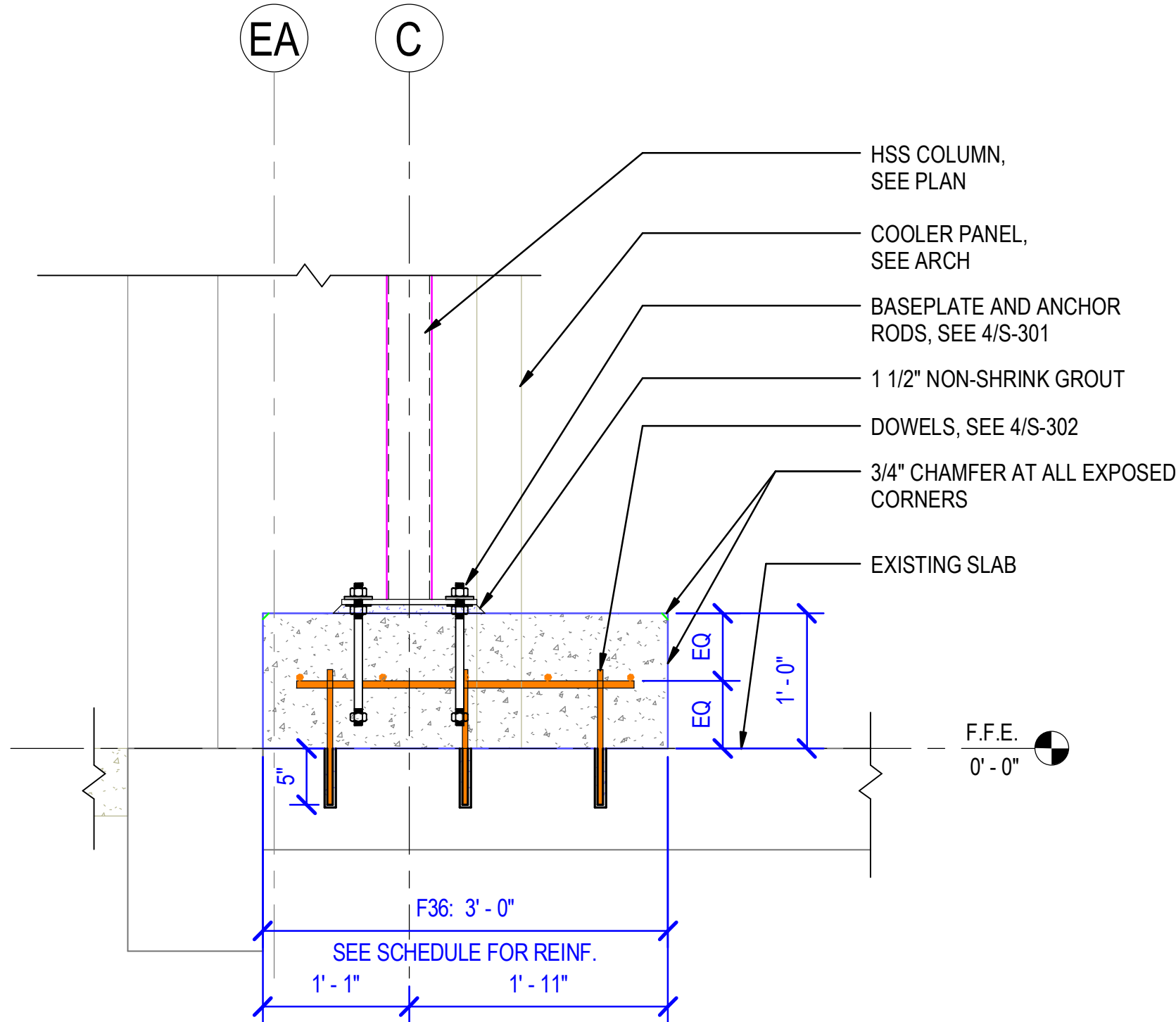
- NOTES:
- THIS CHART IS TAKEN FROM AISC TABLE 14-1. LATEST VERSION OF AISC TABLE 14-1 TAKES PRECEDENCE.
 - CIRCULAR OR SQUARE WASHERS ARE ACCEPTABLE.
 - ADEQUATE CLEARANCE MUST BE PROVIDED FOR THE WASHER SIZE SELECTED



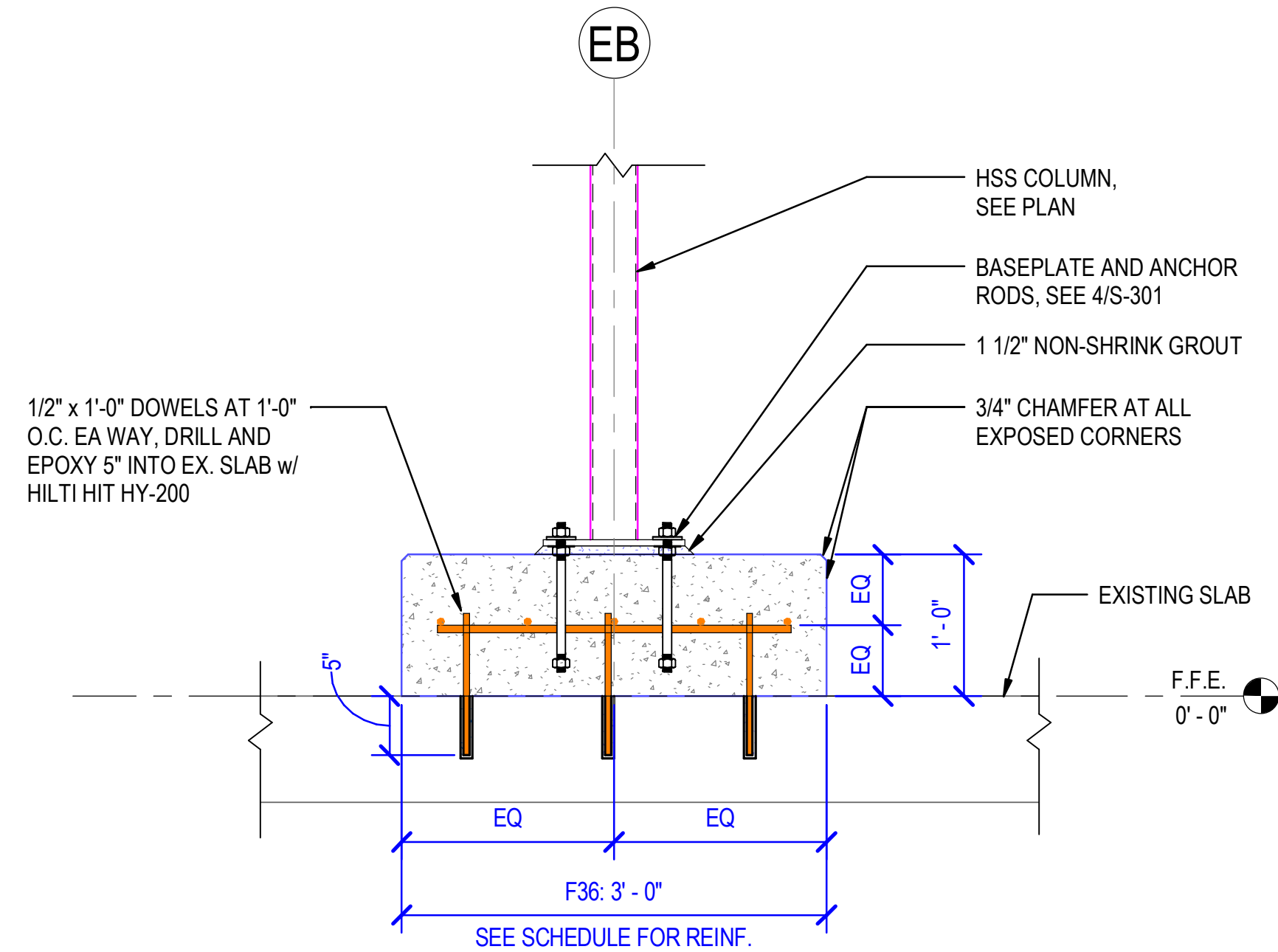
1 DOCK LEVELER PIT AT DOCK



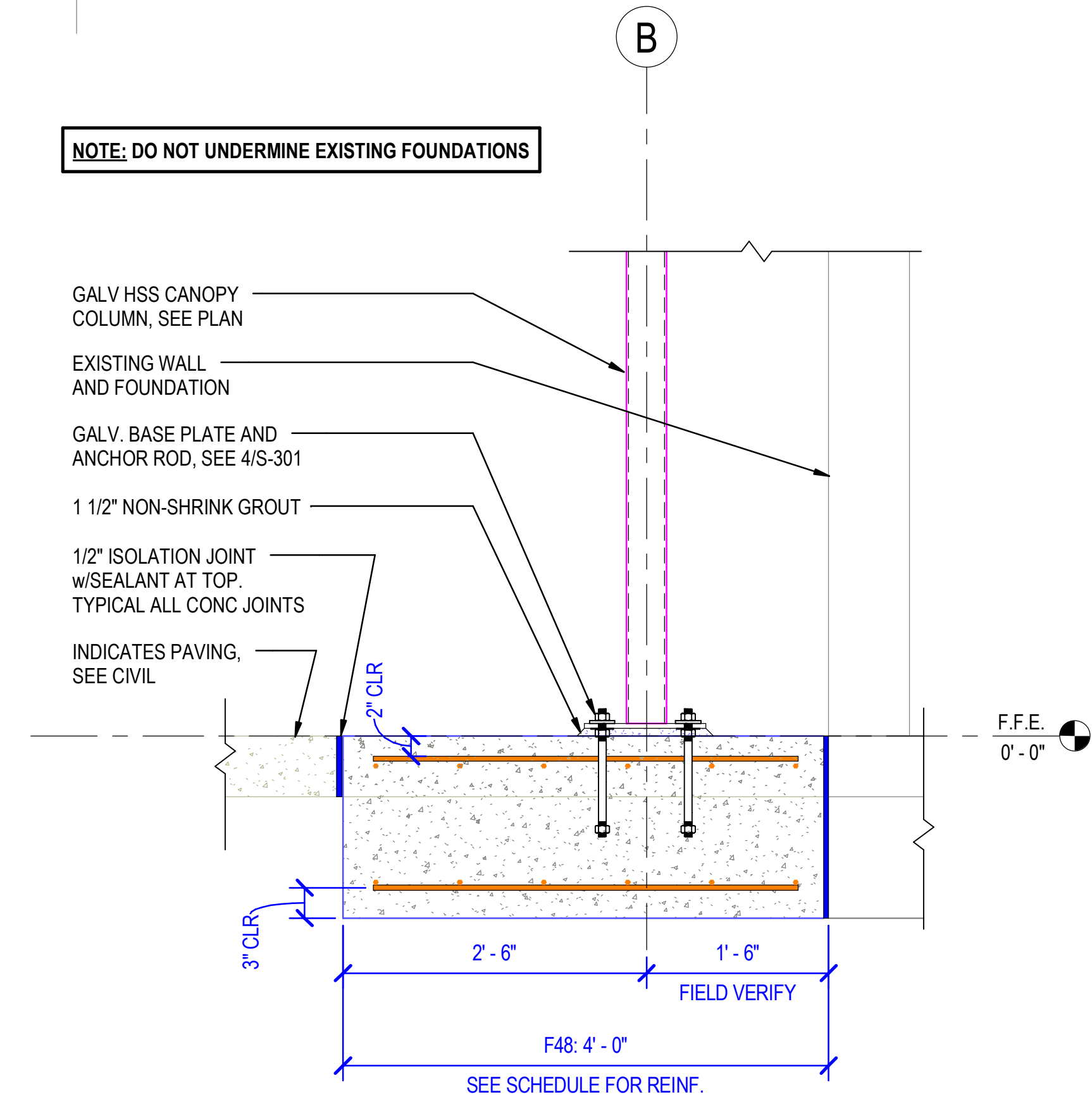
2 SECTION AT DOCK LEVELER



3 TYPICAL INTERIOR COLUMN FOOTING NEAR EXISTING WALL



4 TYPICAL INTERIOR COLUMN



5 TYPICAL CANOPY COLUMN FOOTING

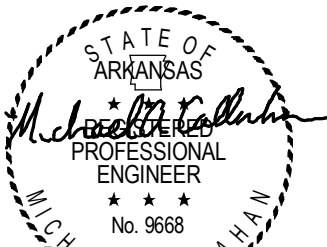
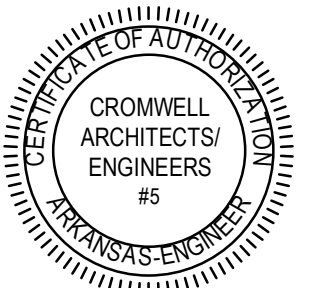
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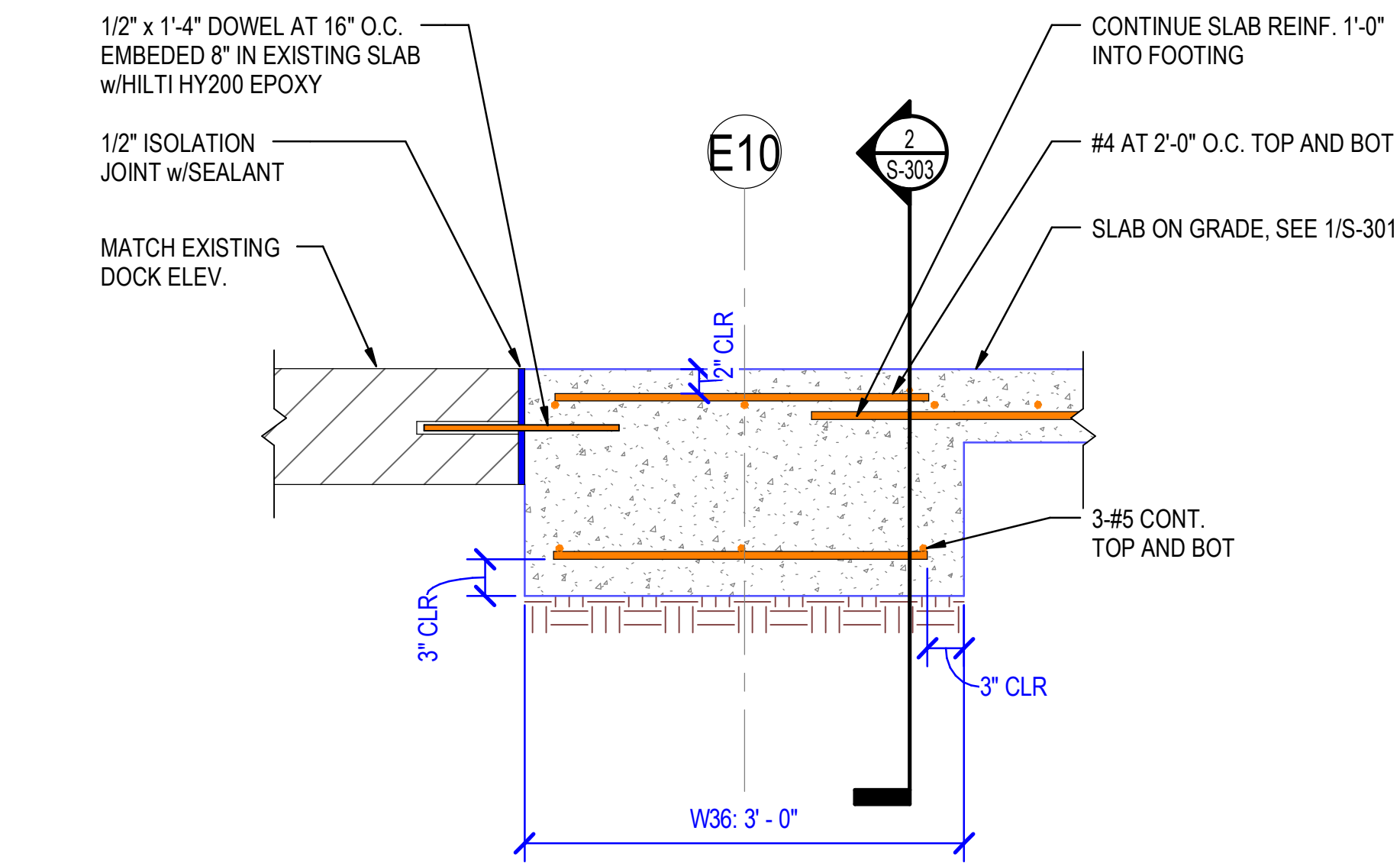
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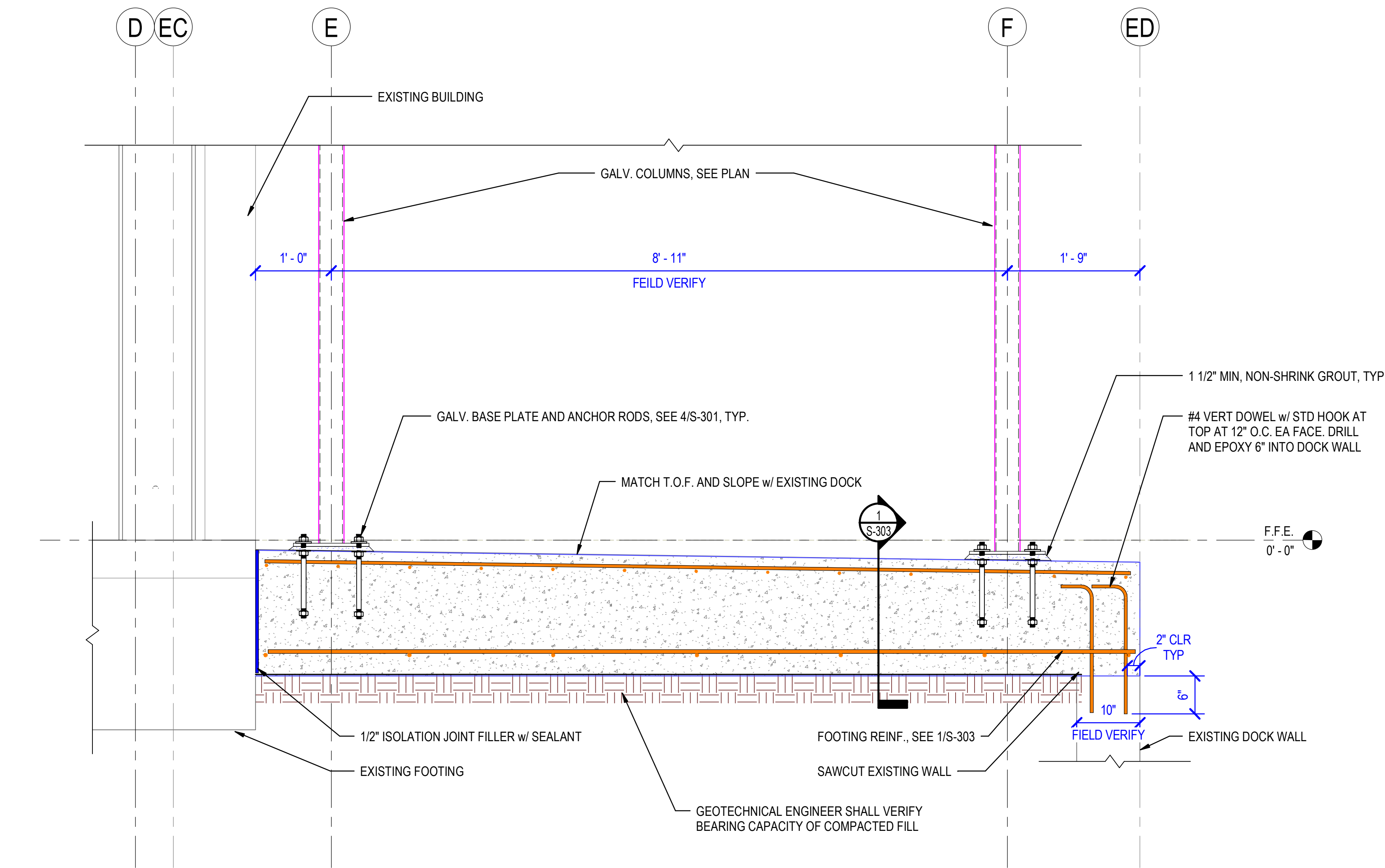
FOUNDATION AND
SLAB DETAILS

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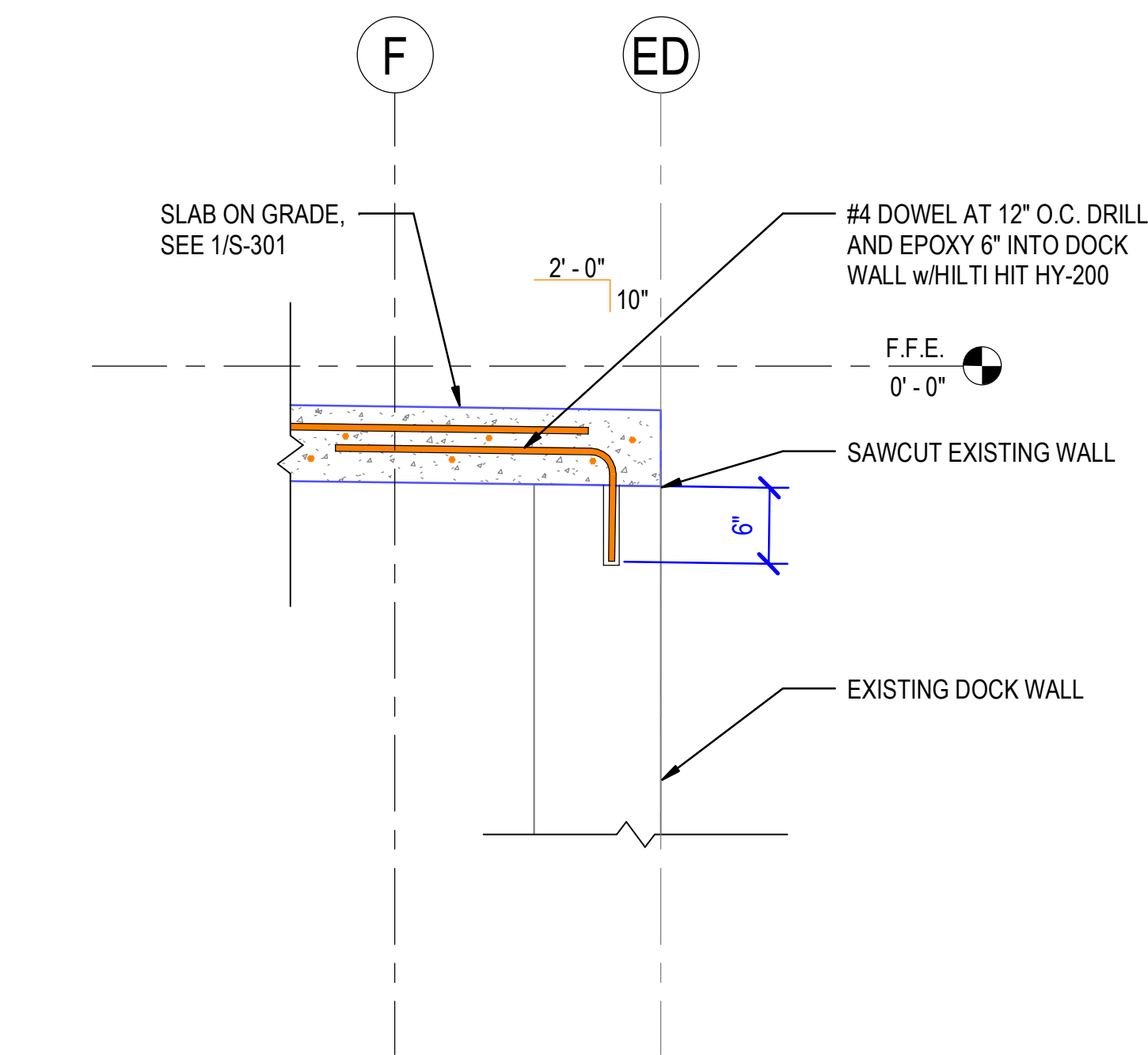
S-302



1 TYPICAL FOOTING AT DOCK
1" = 1'-0"



2 TYPICAL CANOPY FOUNDATION AT DOCK
1" = 1'-0"



3 TYPICAL SLAB TO EXISTING DOCK CONNECTION
1" = 1'-0"

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CROMWELL ARCHITECTS/ENGINEERS #5

STATE OF ARKANSAS PROFESSIONAL ENGINEER No. 9668

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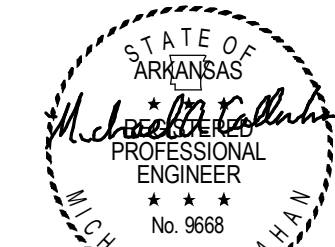
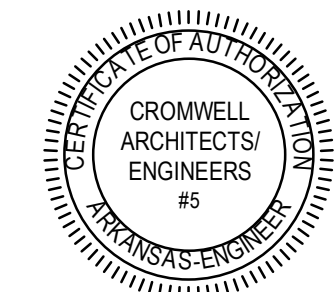
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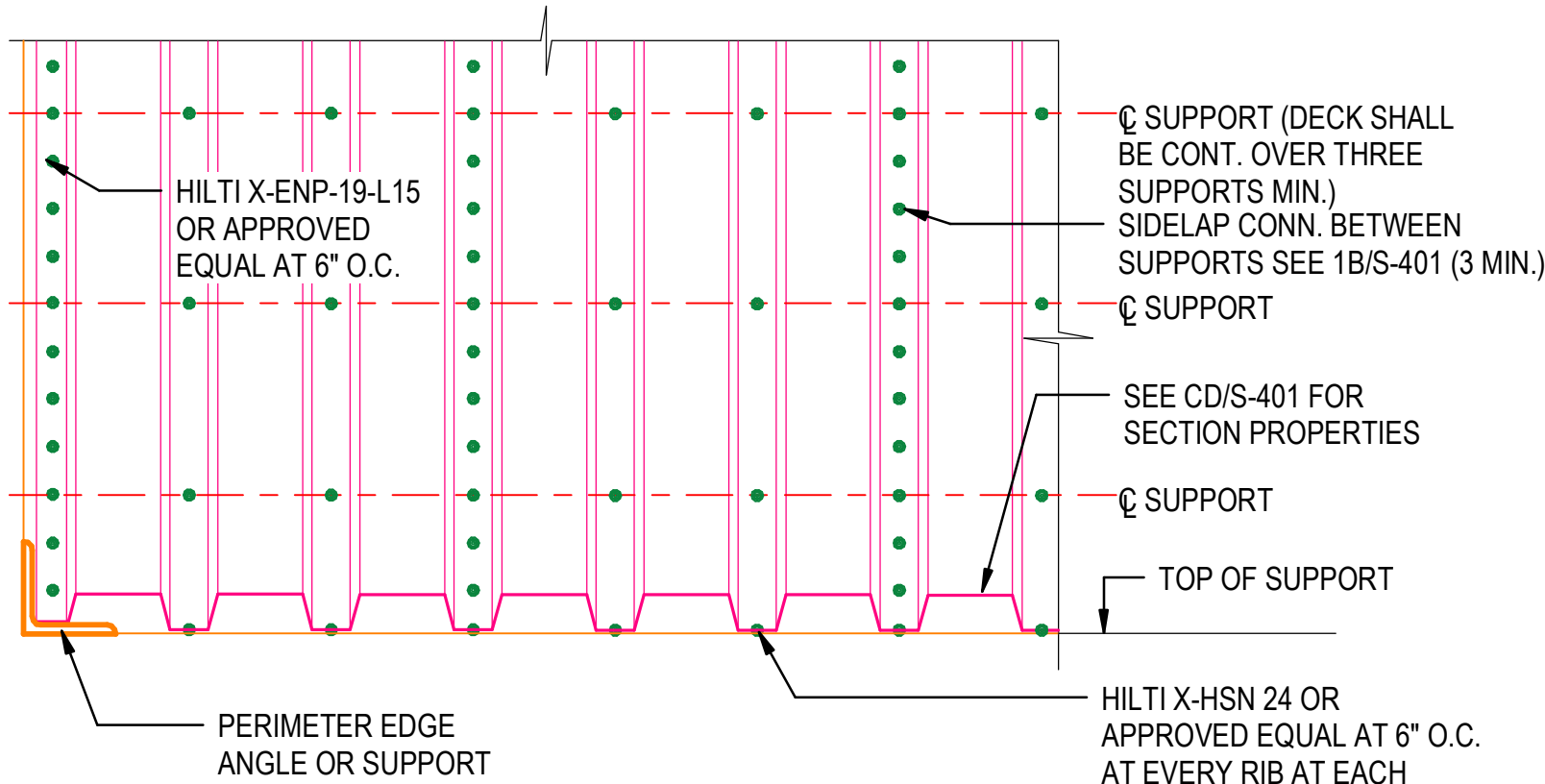
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FRAMING DETAILS

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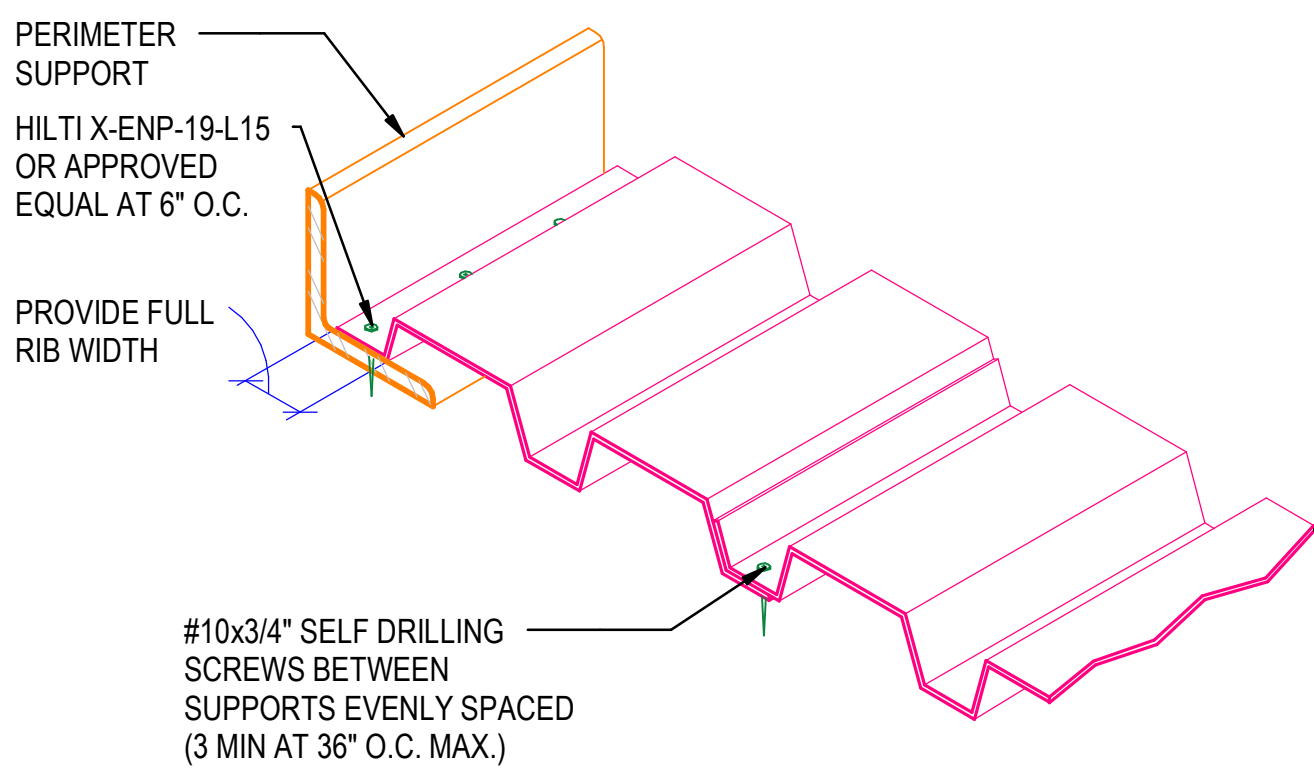
S-401



A TYPICAL DECK FASTENING PATTERN PLAN

- EDGES OF SHEET SHALL BE COMPLETELY ENGAGED.
- DECK GALVANIZING SHALL CONFORM TO ASTM a653 G60

C DECK NOTES

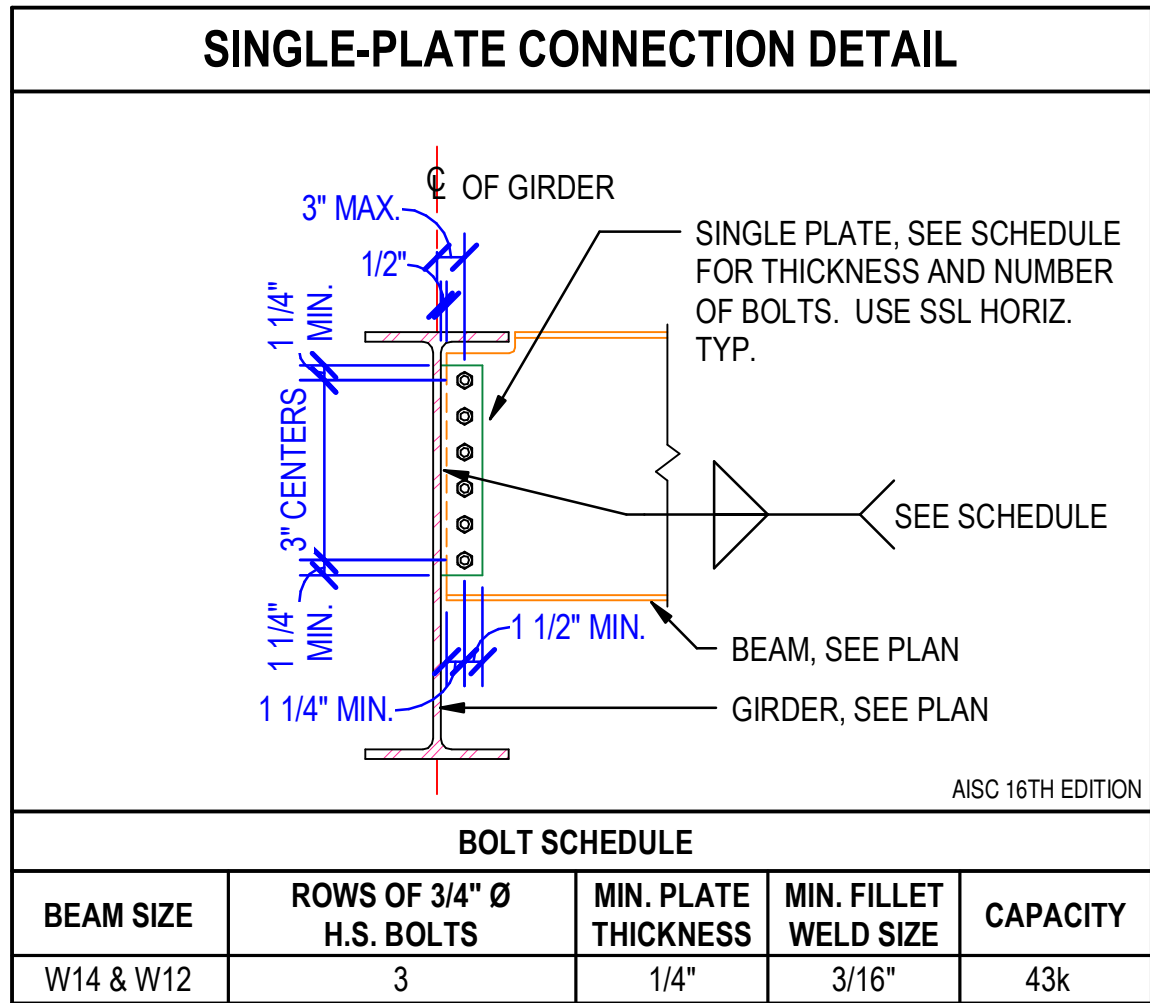


B ISOMETRIC AT TYPICAL SIDE LAP

LOCATION	DEPTH	GAGE	ALLOWABLE DIAPHRAGM SHEAR VALUE	WEIGHT	I	Sp	Sn
				LBS./SQ. FT. GALVANIZED			
CANOPY DECK	1.5"	20	430 PLF AT 6'-0" SPAN	2.14	0.211	0.234	0.247

- NOTES:
- SECTION PROPERTIES SHOWN MAY VARY 10%.
 - DECK Fy = 33 ksi MIN.

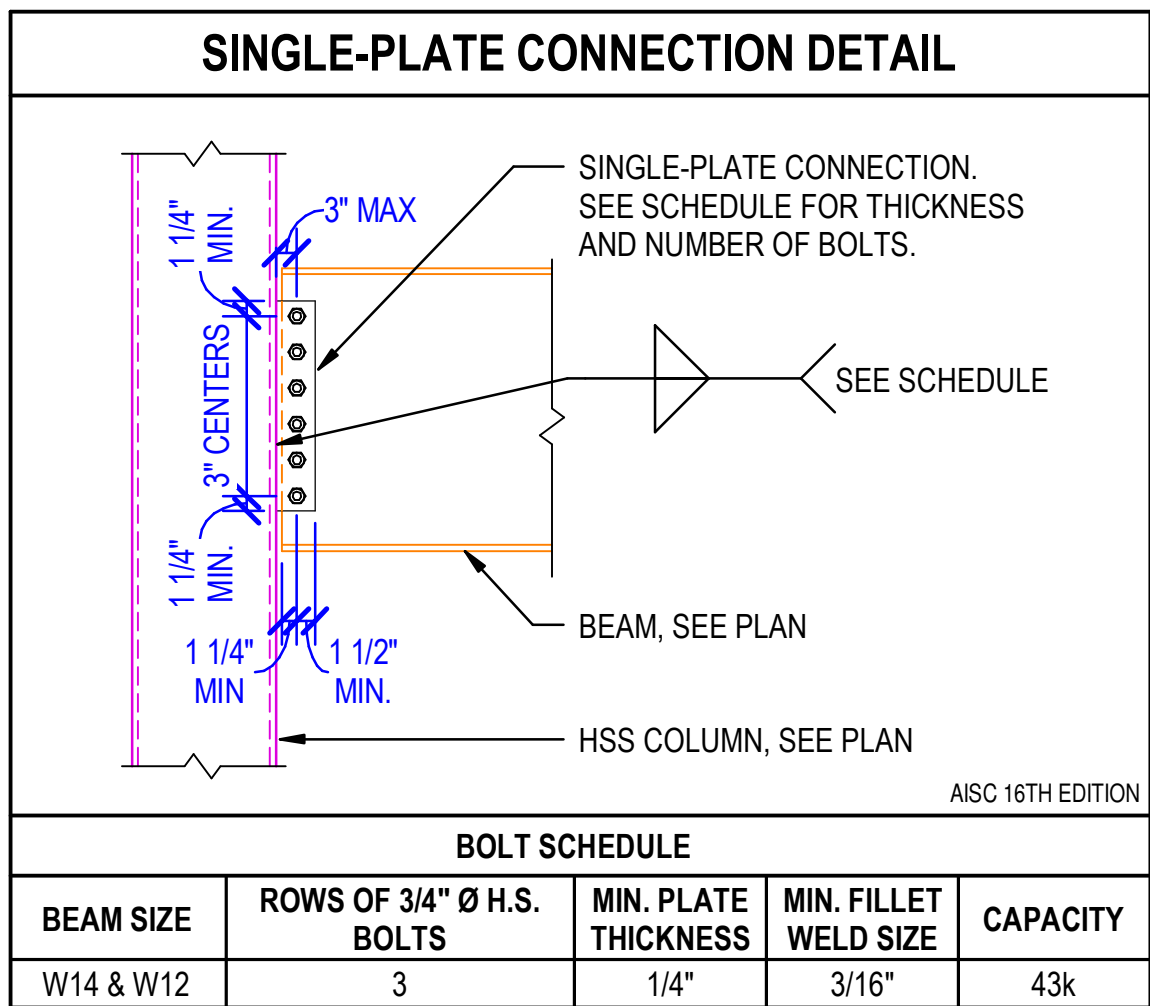
D SECTION PROPERTIES



- NOTES:
- CAPACITY IS BASED ON LRFD DESIGN WITH STANDARD OR SHORT HORIZONTAL SLOTTED HOLES. OVERSIZE OR VERTICAL SLOTTED HOLES ARE NOT ALLOWED. FOR STANDARD HOLES MINIMUM PLATE THICKNESS SHALL BE USED.
 - NUMBER OF BOLTS SHOWN IS THE MINIMUM REQUIRED FOR CONNECTIONS
 - PREHEAT WELDS AS REQUIRED BY AISC.
 - BEAMS WITH LARGE COPES MAY REQUIRE WEB STIFFENER.

2 TYPICAL SINGLE PLATE BEAM TO GIRDER CONNECTION

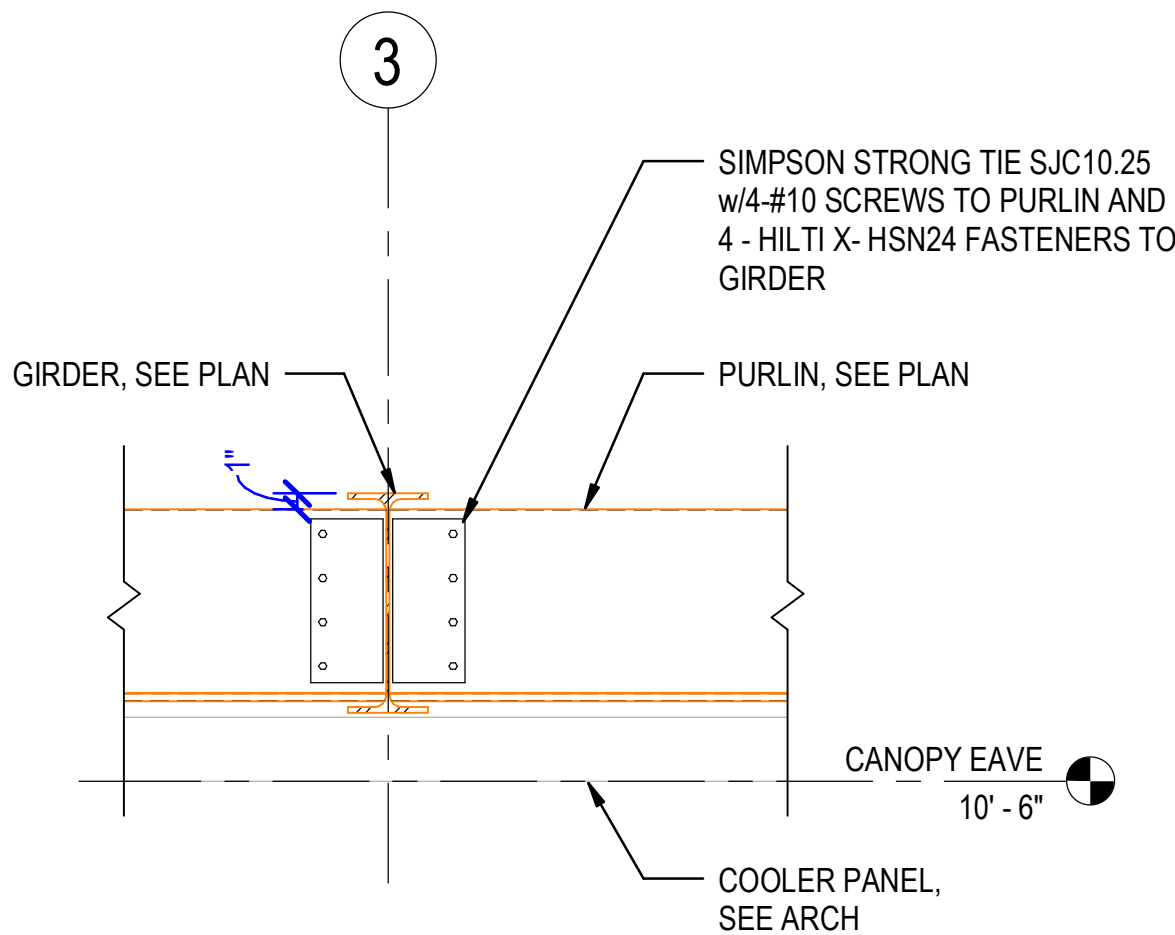
NOT TO SCALE



- NOTES:
- CAPACITY IS BASED ON LRFD DESIGN WITH STANDARD OR SHORT HORIZONTAL SLOTTED HOLES AND A MAXIMUM HSS WIDTH TO THICKNESS RATIO OF 33.7. OVERSIZE OR VERTICAL SLOTTED HOLES ARE NOT ALLOWED. FOR STANDARD HOLES MINIMUM PLATE THICKNESS SHALL BE USED.
 - NUMBER OF BOLTS SHOWN IS THE MINIMUM REQUIRED FOR CONNECTIONS
 - PREHEAT WELDS AS REQUIRED BY AISC.
 - USE STANDARD HOLES ALONG BRACED FRAME OR MOMENT FRAME GRID LINES.

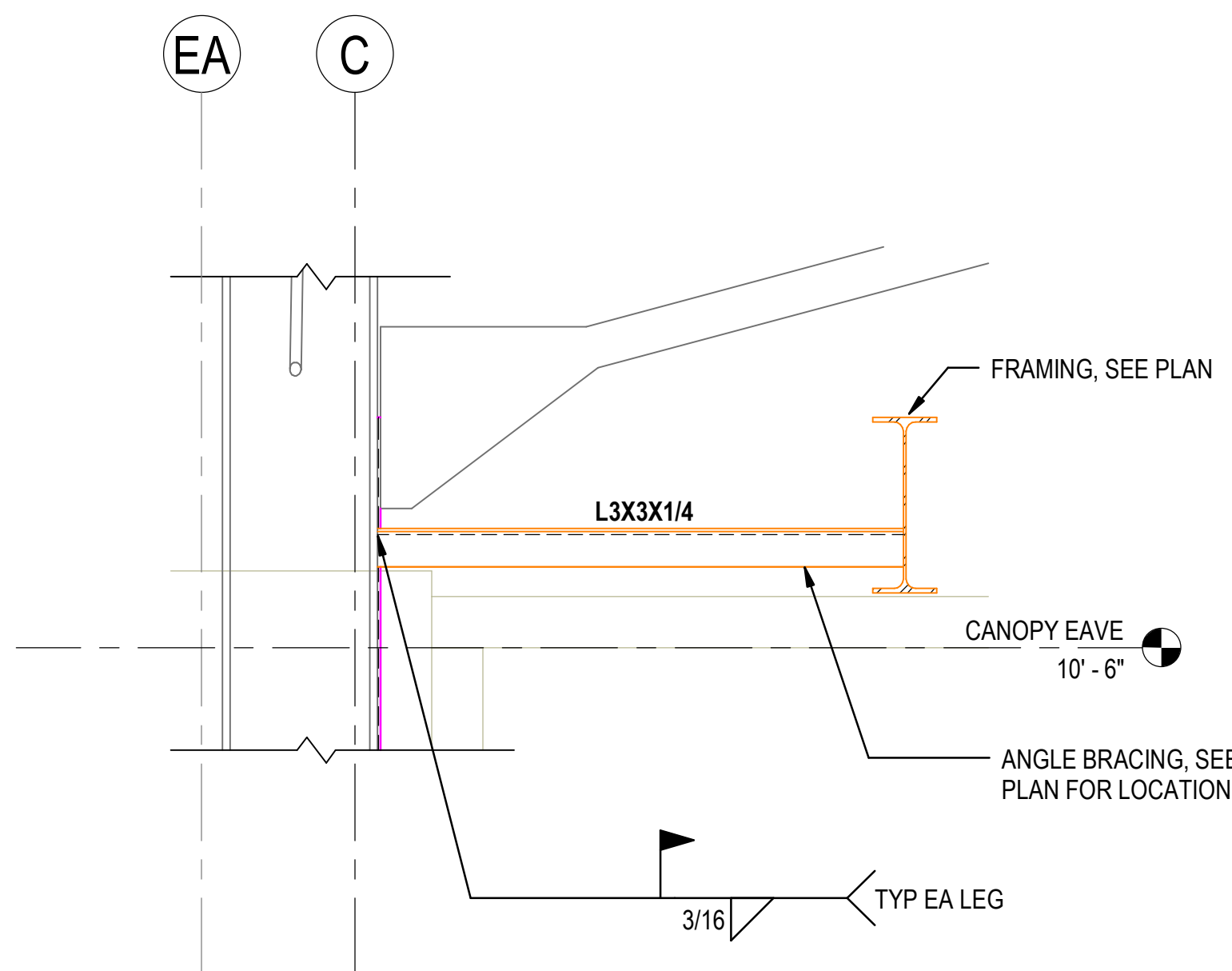
3 TYPICAL BEAM TO HSS COLUMN CONN. SCHEDULE

NOT TO SCALE



4 TYPICAL PURLIN TO GIRDER CONNECTION

1" = 1'-0"



5 TYPICAL ANGLE BRACE CONNECTION

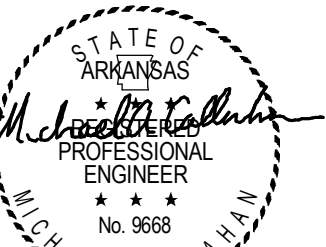
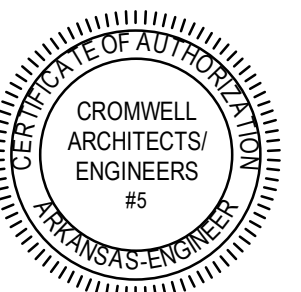
1" = 1'-0"

AEROJET BUILDING 2SH8
COLD BOX
CONVERSION
EAST CAMDEN,
ARKANSAS

CONSTRUCTION
DOCUMENTS

Revisions		
No.	Date	Description

Stamp



10-09-2024

Notes

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Project Number

2024-079

Issue Date

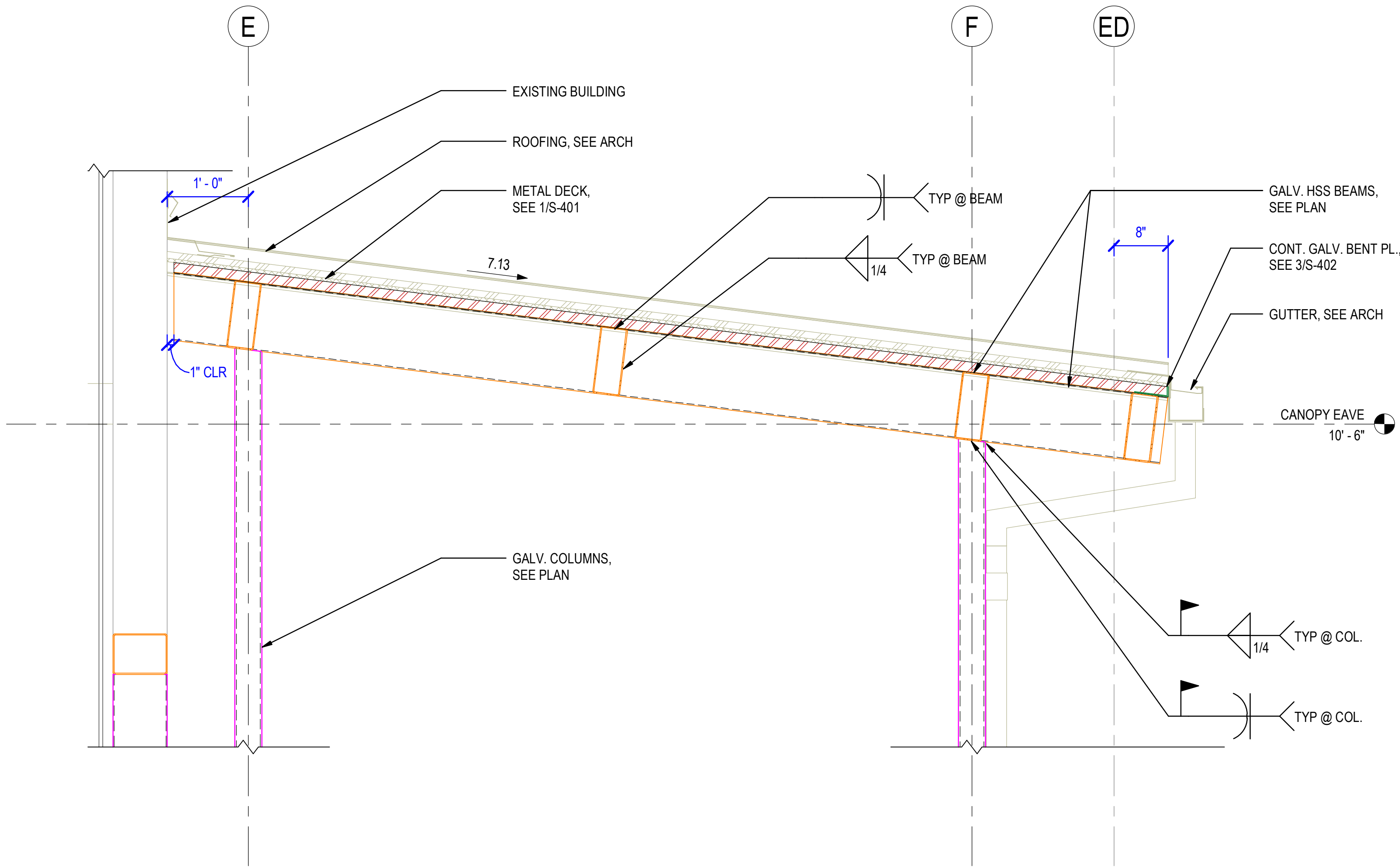
10-09-2024

Sheet Title

FRAMING DETAILS

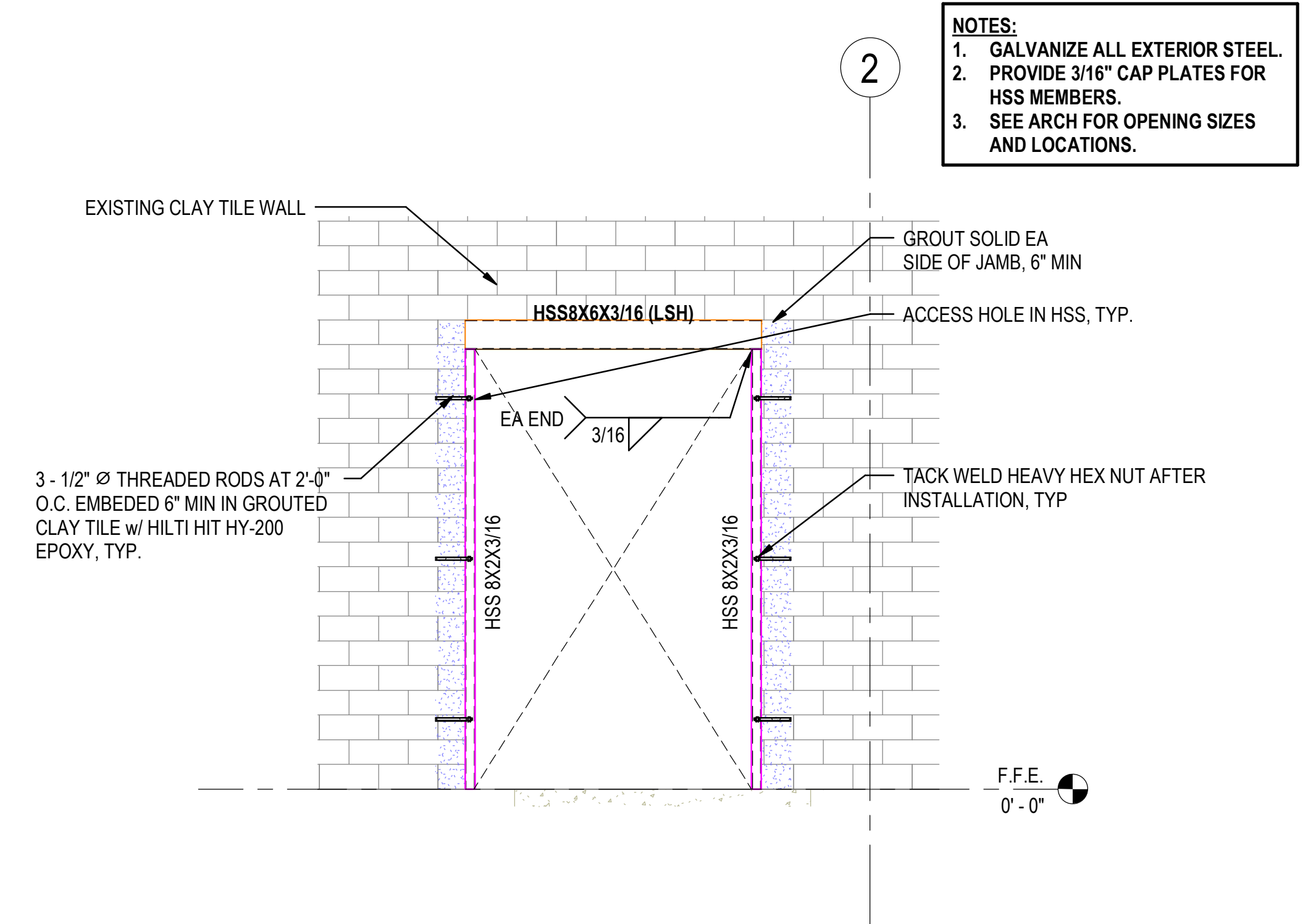
Sheet Number

S-402



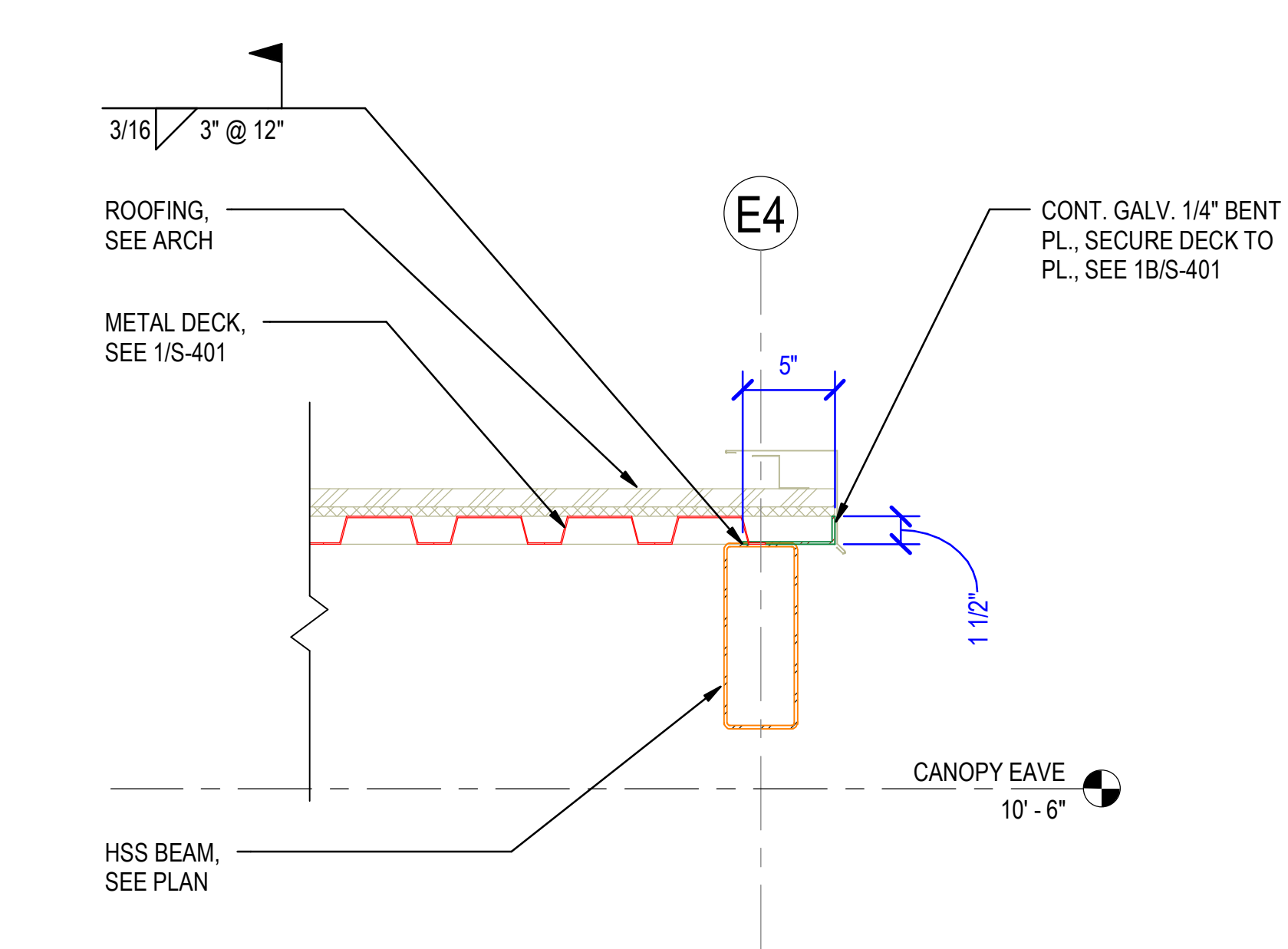
1 TYPICAL CANOPY CONNECTIONS

1" = 1'-0"



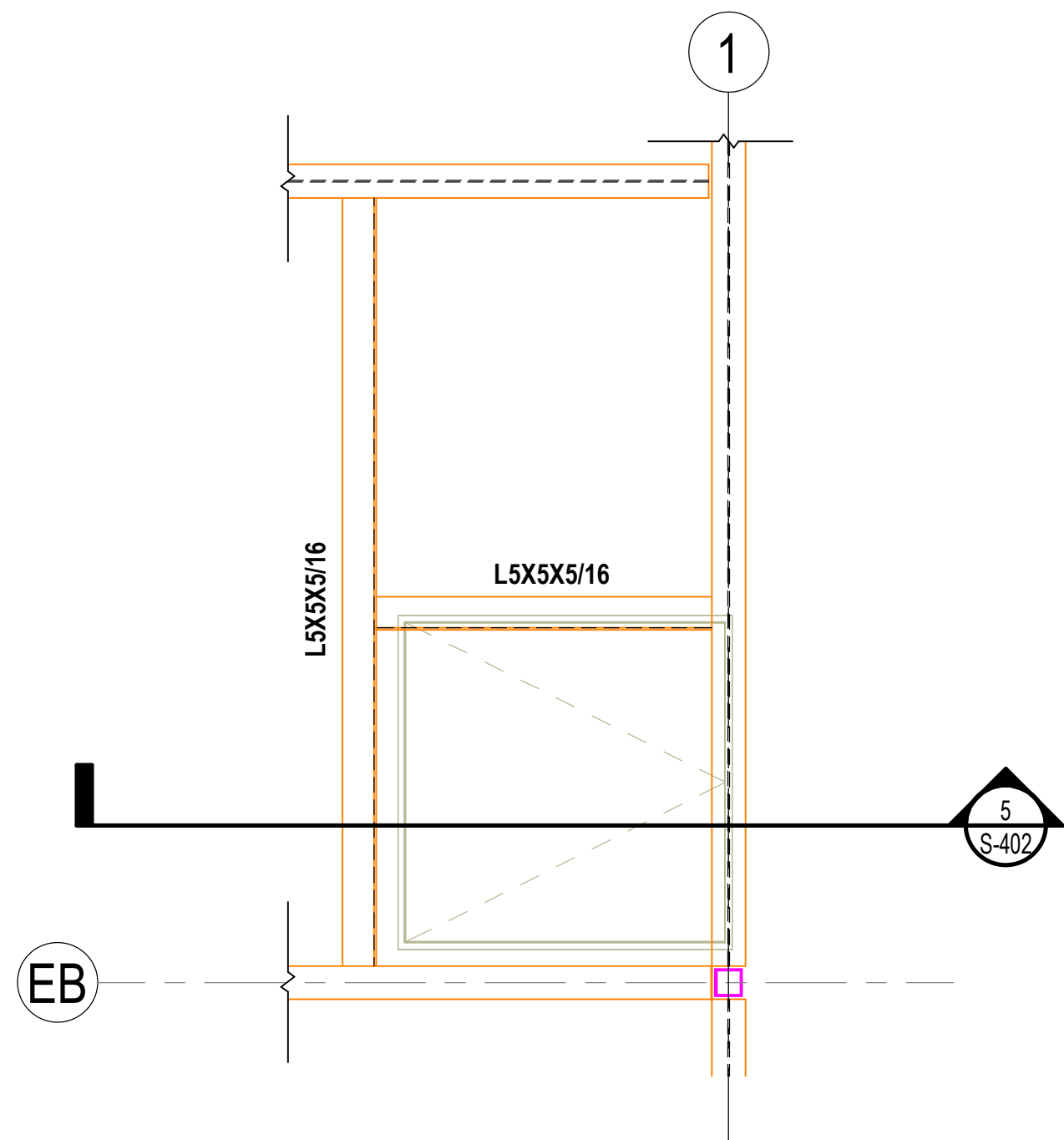
2 TYPICAL OPENING FRAMING IN EXISTING WALL

1/2" = 1'-0"



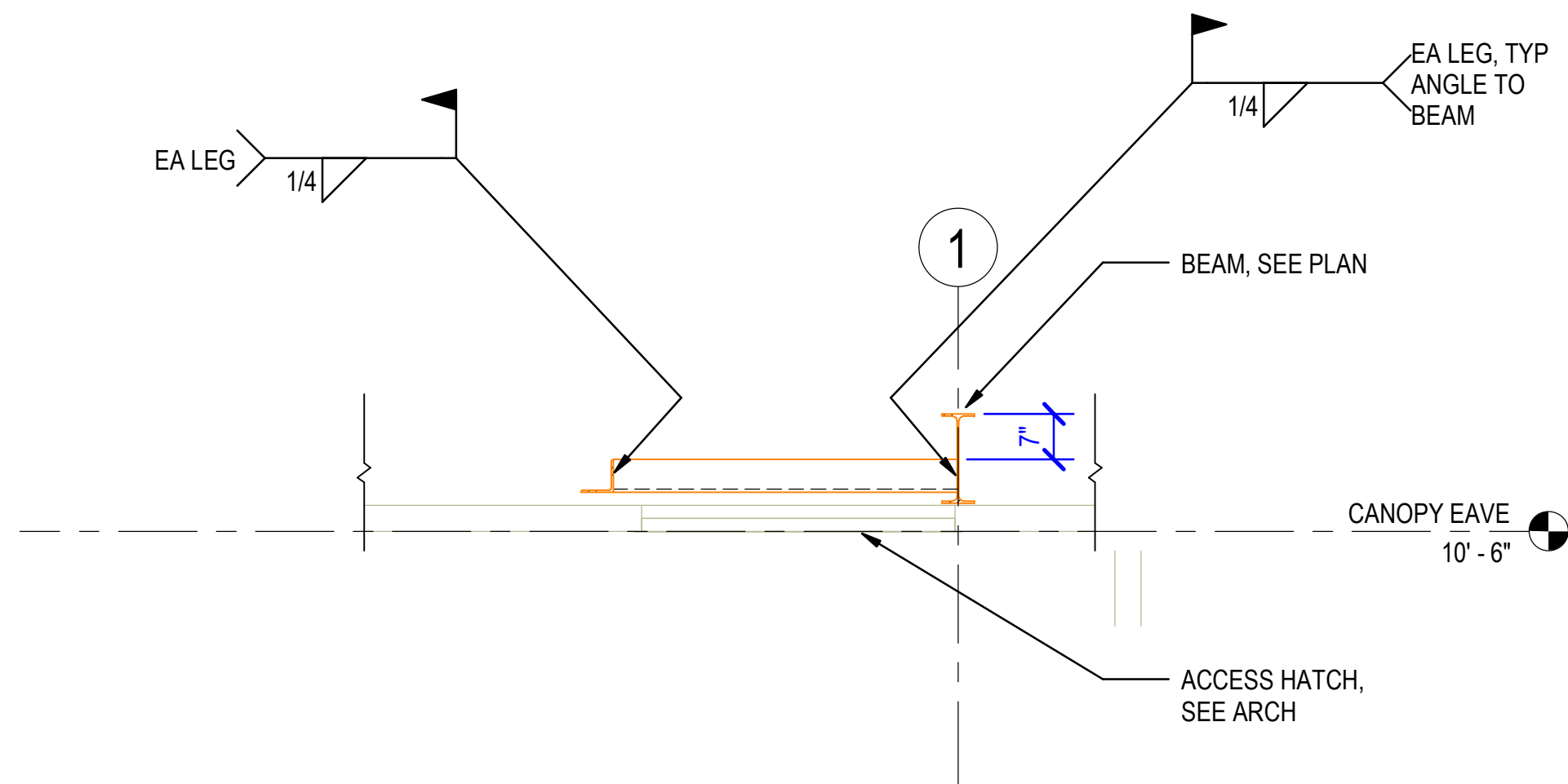
3 TYPICAL CANOPY EDGE

1 1/2" = 1'-0"



4 FRAMING AT ACCESS HATCH

1/2" = 1'-0"



5 SECTION AT ACCESS HATCH

1/2" = 1'-0"